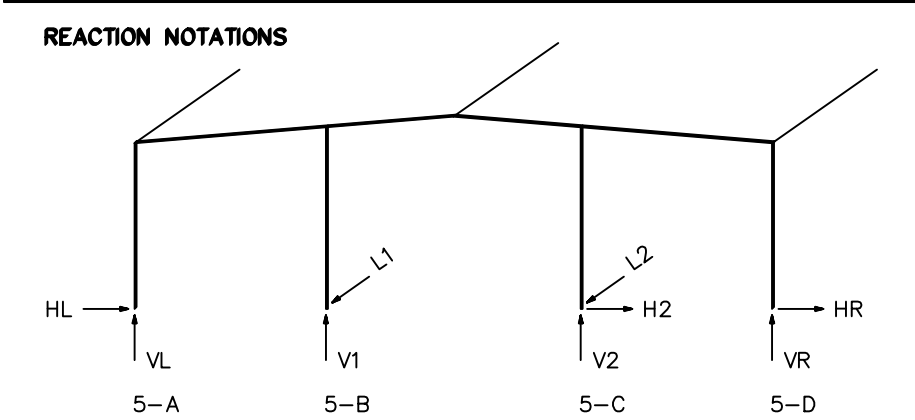


**LOAD GROUP REACTION TABLE**

COLUMN	1-C			1-B		
LOAD GROUP	H1	V1	L1	H2	V2	L2
D	0.	0.6	0.	0.	0.6	0.
W+	0.	0.	9.6	0.	0.	9.6
W-	0.	0.	-9.0	0.	0.	-9.0

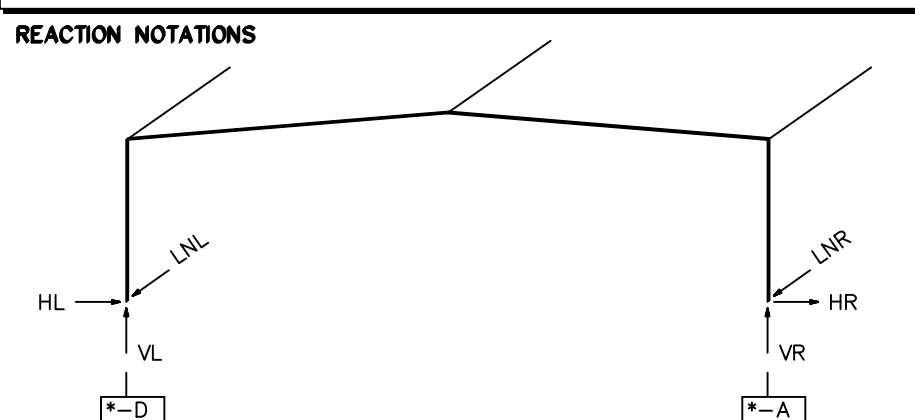
**LOAD GROUP DESCRIPTION**  
 D : DEAD LOAD  
 W+ : WIND LOAD AS AN INWARD ACTING PRESSURE  
 W- : WIND LOAD AS AN OUTWARD ACTING SUCTION



**LOAD GROUP REACTION TABLE**

COLUMN	5-A			5-D			5-B			5-C		
LOAD GROUP	HL	VL	LL	HR	VR	LR	H1	V1	L1	H2	V2	L2
D	0.0	1.4	0.	0.0	1.4	0.	0.	3.5	0.0	0.	3.5	0.0
C	0.0	0.3	0.	0.0	0.3	0.	0.	0.9	0.0	0.	0.9	0.0
L	0.2	5.8	0.	-0.2	5.8	0.	0.	18.8	-0.2	0.	18.8	-0.2
S	0.2	9.4	0.	-0.2	9.4	0.	0.	30.4	-0.3	0.	30.4	-0.3
W+	-0.2	-6.3	0.	0.2	-6.3	0.	0.	-16.7	9.8	0.	-16.7	9.8
W-	-0.2	-6.3	0.	0.2	-6.3	0.	0.	-16.7	-9.8	0.	-16.7	-9.8
WR	-0.2	-6.3	0.	4.1	-10.1	0.	0.	-16.7	0.2	0.	-13.0	0.2
WL	-0.2	-6.3	0.	0.2	-2.9	0.	0.	-16.7	0.2	-3.9	-20.1	0.2
ER	0.	0.	0.	1.6	-1.5	0.	0.	0.	0.	0.	1.5	0.
EL	0.	0.	0.	0.	1.4	0.	0.	0.	0.	-1.6	-1.4	0.

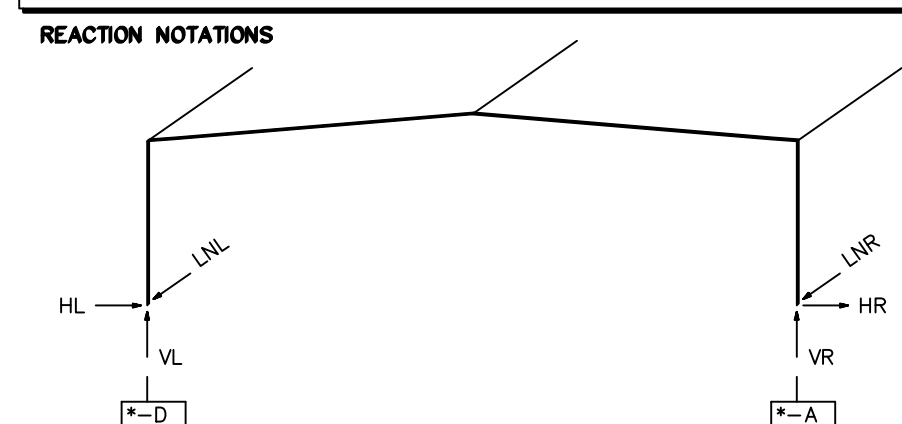
**LOAD GROUP DESCRIPTION**  
 D : DEAD LOAD  
 C : COLLATERAL LOAD  
 L : LIVE LOAD  
 S : DESIGN SNOW LOAD  
 W+ : WIND LOAD AS AN INWARD ACTING PRESSURE  
 W- : WIND LOAD AS AN OUTWARD ACTING SUCTION  
 WR : WIND FORCE FROM THE RIGHT  
 WL : WIND FORCE FROM THE LEFT  
 ER : EARTHQUAKE FORCE FROM RIGHT  
 EL : EARTHQUAKE FORCE FROM LEFT



**LOAD GROUP REACTION TABLE \* = 1 2 3 4**

COLUMN	*-D			*-A		
LOAD GROUP	HL	VL	LNL	HR	VR	LNR
DL	5.0	8.2	0.0	-5.0	8.2	0.0
COLL	1.5	2.2	0.0	-1.5	2.2	0.0
PAR1	25.8	57.4	0.0	-25.8	57.4	0.0
PAR2	25.8	17.9	0.0	-25.8	57.4	0.0
SNOW	51.6	75.2	0.0	-51.6	75.2	0.0
LL	31.9	46.5	0.0	-31.9	46.5	0.0
RBUPEQ	0.1	-4.4	-6.8	-0.1	-4.4	-6.8
RBDWEQ	-0.1	4.4	0.0	0.1	4.4	0.0
EQ	-1.7	-0.8	0.0	-1.7	0.8	0.0
WL1	-22.5	-27.5	0.0	8.8	-16.7	0.0
WL2	-27.0	-38.1	0.0	13.3	-27.2	0.0
WL3	-15.7	-11.6	0.0	2.0	-0.8	0.0
WL4	-8.8	-16.7	0.0	22.5	-27.5	0.0
WL5	-13.3	-27.2	0.0	27.0	-38.1	0.0
WL6	-2.0	-0.8	0.0	15.7	-11.6	0.0
LWL1	-10.5	-25.5	0.0	11.7	-18.7	0.0
RBUPLW	0.1	-5.6	-8.7	-0.1	-5.6	-8.7
LWL2	-15.0	-36.0	0.0	16.2	-29.3	0.0
LWL3	-3.7	-9.6	0.0	4.9	-2.8	0.0
LWL4	-11.7	-18.7	0.0	10.5	-25.5	0.0

**LOAD GROUP DESCRIPTION**  
 DL : Roof Dead Load  
 COLL : Roof Collateral Load  
 PAR1 : Partial Load [PARxx]  
 PAR2 : Partial Load [PARxx]  
 SNOW : Roof Snow Load  
 LL : Roof Live Load  
 RBUPEQ : Upward Acting Rod Brace Load from Longit. Seismic  
 RBDWEQ : Downward Acting Rod Brace Load from Long. Seismic  
 EQ : Lateral Seismic Load [parallel to plane of frame]  
 WL1 : Lateral Primary Wind Load  
 WL2 : Lateral Primary Wind Load  
 WL3 : Lateral Primary Wind Load  
 WL4 : Lateral Primary Wind Load  
 WL5 : Lateral Primary Wind Load  
 WL6 : Lateral Primary Wind Load  
 LWL1 : Longitudinal Primary Wind Load  
 RBUPLW : Upward Acting Rod Brace Load from Longitud. Wind  
 LWL2 : Longitudinal Primary Wind Load  
 LWL3 : Longitudinal Primary Wind Load  
 LWL4 : Longitudinal Primary Wind Load



**LOAD GROUP REACTION TABLE \* = 1 2 3 4**

COLUMN	*-D			*-A		
LOAD GROUP	HL	VL	LNL	HR	VR	LNR
LWL5	-16.2	-29.3	0.0	15.0	-36.0	0.0
LWL6	-4.9	-2.8	0.0	3.7	-9.6	0.0
RBDWLW	-0.1	5.6	0.0	0.1	5.6	0.0

**LOAD GROUP DESCRIPTION**  
 LWL5 : Longitudinal Primary Wind Load  
 LWL6 : Longitudinal Primary Wind Load  
 RBDWLW : Downward Acting Rod Brace Load from Longit. Wind

NOTES

- THE REACTIONS PROVIDED ARE BASED ON THE ORDER DOCUMENTS AT THE TIME OF MAILING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE MAILING.
- THE REACTIONS PROVIDED HAVE BEEN CREATED WITH THE FOLLOWING LAYOUT (UNLESS NOTED OTHERWISE).
  - A REACTION TABLE IS PROVIDED WITH THE REACTIONS FOR EACH LOAD GROUP.
  - RIGID FRAMES
    - GABLED BUILDINGS
      - LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE LEFT SIDE OF THE BUILDING, AS SHOWN ON THE ANCHOR ROD DRAWING, FROM THE OUTSIDE OF THE BUILDING.
      - INTERIOR COLUMNS ARE SPACED FROM LEFT SIDE TO RIGHT SIDE.
    - SINGLE SLOPE BUILDINGS
      - LEFT COLUMN IS THE LOW SIDE COLUMN.
      - RIGHT COLUMN IS THE HIGH SIDE COLUMN.
      - INTERIOR COLUMNS ARE SPACED FROM LOW SIDE TO HIGH SIDE.
  - ENDWALLS
    - LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE WALL FROM THE OUTSIDE.
    - INTERIOR COLUMNS ARE SPACED FROM LEFT TO RIGHT.
  - ANCHOR ROD SIZE IS DETERMINED BY SHEAR AND TENSION AT THE BOTTOM OF THE BASE PLATE. THE LENGTH OF THE ANCHOR ROD AND METHOD OF LOAD TRANSFER TO THE FOUNDATION ARE TO BE DETERMINED BY THE FOUNDATION ENGINEER.
  - ANCHOR RODS ARE ASTM F1554 Gr. 36 MATERIAL UNLESS NOTED OTHERWISE ON THE ANCHOR ROD LAYOUT DRAWING.
  - X-BRACING
    - ROD BRACING REACTIONS HAVE BEEN INCLUDED IN VALUES SHOWN IN THE REACTION TABLES.
    - FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE SIDEWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RBUPEQ AND RBDWEQ) DO NOT INCLUDE THE AMPLIFICATION FACTOR,  $R_b$ .
    - FOR CANADA BUILDING CODE (NBC), WHEN X-BRACING IS PRESENT IN THE SIDEWALL OR ENDWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RBUPEQ & RBDWEQ) ARE MULTIPLIED BY FORCE REDUCTION FACTOR,  $R_d$ , WHEN SPECIFIED SHORT-PERIOD SPECTRAL ACCELERATION RATIO  $I_p/S_s(0.2)$  IS GREATER THAN 0.45.
- REACTIONS ARE PROVIDED AS UN-FACTORED FOR EACH LOAD GROUP APPLIED TO THE COLUMN. THE FOUNDATION ENGINEER WILL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS TO DETERMINE BEARING PRESSURES AND CONCRETE DESIGN. THE FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN.

THE MANUFACTURER DOES NOT PROVIDE "MAXIMUM" LOAD COMBINATION REACTIONS. HOWEVER, THE INDIVIDUAL LOAD REACTIONS PROVIDED MAY BE USED BY THE FOUNDATION ENGINEER TO DETERMINE THE APPLICABLE LOAD COMBINATIONS FOR HIS/HER DESIGN PROCEDURES AND ALLOW FOR AN ECONOMICAL FOUNDATION DESIGN.

Chk'd	By	Description	Date	Revision

**FERRO BUILDING SYSTEMS**  
 103-19292 60TH AVENUE, 3RD FLOOR  
 SURREY, BRITISH COLUMBIA, V3S 3M2  
 PHONE # 604-530-3224 - FAX # 604-530-9851

**Customer:**  
 FERRO BUILDING SYSTEMS LTD  
 C/O ROYAL CANADIAN MOUNTED POLICE  
 5600 11TH AVE  
 REGINA, SK S4P 3J7, CN

**Project Name & Location:**  
 RCMP STEEL BUILDING, INNISFAIL AB  
 HIGHWAY 2, EXIT 365 EAST  
 INNISFAIL, AB T4G, 1S8, CN

**Drawing Status:**  
 For Approval (Not For Construction)  
 For Construction Permit  
 For Erector Installation

Scale: NOT TO SCALE  
 Drawn by: RJA 3/4/13  
 Checked by: HGF 3/4/13  
 Project Engineer: WWL  
 Job Number: 12-B-93855  
 Sheet Number: F3 of 3

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

Dustin L. Cole, P.ENG  
 Alberta P.ENG M86692

