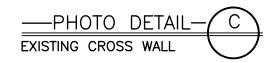


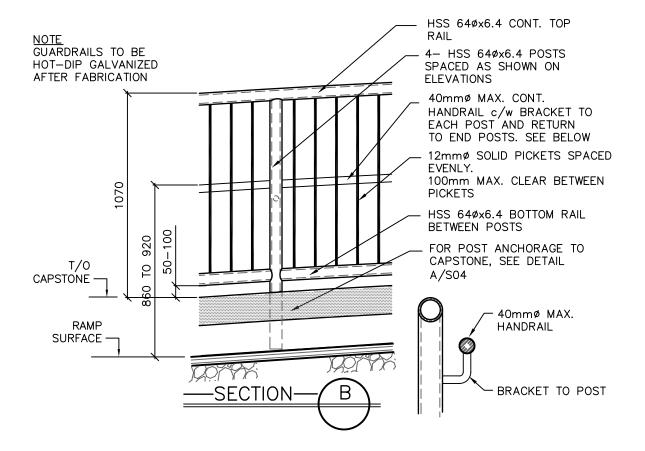
-SECTION-

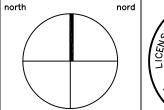
ALL CONDUIT, WIRING,
DUCTWORK ETC. WILL BE
REMOVED FROM THE
CONSTRUCTION AREA BY
THE OWNER PRIOR TO
START OF CONSTRUCTION

SALVAGE STONE FROM
INTERIOR WALL FOR USE
DURING REBUILD. RETURN
ALL UNUSED STONE TO

OWNER











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Ottawa, Ontario

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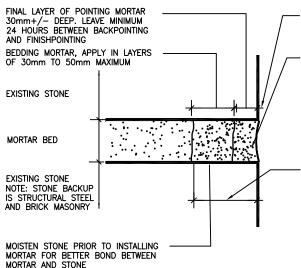
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FOR PRICING, ASSUME THE FOLLOWING:

.1 AVERAGE DEPTH OF STONE: 300mm

REPOINTING DETAIL

DETAIL

RAKING OUT DETAIL

DETAIL

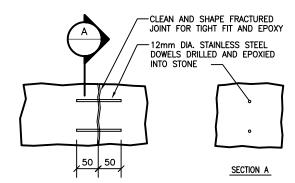
CLEAN STONE FACE AND REMOVE MORTAR DROPPINGS

COMPACT FINAL LAYER OF MORTAR, TOOL JOINT FLUSH AND LIGHTLY BRUSH. DO NOT FEATHER MORTAR OVER STONE FACE. KEEP MORTAR MOIST FOR SEVERAL DAYS
FOLLOWING INSTALLATION. PROTECT MORTAR FROM FLASH SETTING DUE TO EXPOSURE TO DIRECT

RAKE OUT EXISTING MORTAR AS PER DETAIL 1/A-07. SEE ELEVATIONS FOR ASSUMED EXTENT.

NOTE: FOR PRICING, ASSUME:

- .1 40% OF REMOVALS FOR VERTICAL JOINTS ARE IN THE ORDER OF 30 TO 40mm DEEP AND THE REMAINDER ARE FULL DEPTH REMOVALS, UNLESS NOTED OTHERWISE.
- .2 60% OF HORIZONTAL JOINTS ARE IN THE ORDER OF 30 TO 40mm DEEP AND THE REMAINDER ARE FULL DEPTH REMOVALS, UNLESS NOTED OTHERWISE.
- .3 WHERE FULL DEPTH REMOVALS IN HORIZONTAL JOINTS ARE NECESSARY, THE CONTRACTOR MUST EXERCISE CAUTION SO AS NOT TO DESTABILIZE THE WALLS. IN THIS CASE, REMOVE MORTAR IN ONLY SMALL PORTIONS OF THE WALL AND REPOINT EXCEPT FOR FINAL LAYER BEFORE PROCEEDING WITH



FRACTURED STONE REPAIR PROCEDURE:

- REMOVE FRACTURED STONE WITHOUT LOSING PIECES OR WORSENING DAMAGE OR DAMAGING ADJACENT UNITS
- DRILL 13mm Ø HOLES, 60mm LONG IN EACH SECTION, SPACED AT 300 O.C. MAXIMUM. HOLES MUST BE CENTRED IN STONE THICKNESS.
- NISERT 12mm Ø STAINLESS STEEL DOWELS, 100mm LONG AND APPLY EPOXY ADHESIVE. GLUE STONE FACES TOGETHER USING EPOXY ADHESIVE, OR APPROVED GROUT. LET SET FOR 24 HOURS MINIMUM.
- REINSTATE REPAIRED STONE INTO WORK AND REPOINT WITH SPECIFIED MORTAR TO WITHIN 50mm OF POINTING SURFACE. ALLOW MORTAR TO SET 24 HOURS. POINT TO SURFACE IN TWO LAYERS.
- · GROUT SOLID ALL VOIDS BEHIND STONE USING SPECIFIED GROUT.
- · REPAIR SURFACE OF FRACTURE WITH CEMENT BASED RESTORATION MORTAR TO MATCH THE SURROUNDING STONE. AND TOOL FOR TEXTURE

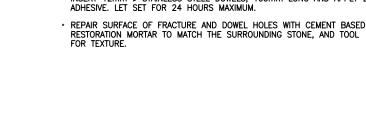
REPAIR OF FRACTURED STONE DETAIL

ESTIMATED WALL WIDTH IS 600mm TO BE CONFIRMED ON SITE ALLOW FOR REPOINTING ALL JOINTS IN BACKUP MASONRY, AND RESTORING COLLAR JOINTS BEHIND FACE **EXTERIOR** WALL FACE INNER WYTHE IS STONE MASONRY SEE DETAIL 1/S-05 8mm STAINLESS STEEL SPIRAL ANCHORS DRILLED THROUGH MORTAR JOINTS IN STONE, INTO STONE BACKUP. SPACING OF ANCHORS TO BE AT 400mm

O.C. WHERE WALLS ARE RUBBLE STONE ANCHORAGE DETAIL

NTS

DETAIL





IN-SITU FRACTURED STONE REPAIR PROCEDURE

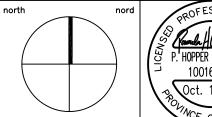
DRILL 13mm Ø HOLES, SPACED AT 300 O.C. MAXIMUM, MINIMUM 2 ANCHORS PER STONE. EXTEND 60mm BEYOND

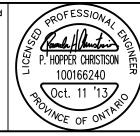
INSERT 12mm Ø STAINLESS STEEL DOWELS, 100mm LONG AND APPLY EPOXY

FRACTURE. MINIMUM LENGTH OF HOLE TO BE 140mm.



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-CLEAN AND SHAPE FRACTURED JOINT FOR TIGHT FIT AND EPOXY

12mm DIA. STAINLESS STEEL DOWELS DRILLED AND EPOXIED INTO STONE

FACE, ONE STARTING ON ONE SIDE OF THE FRACTURE AND THE SECOND

ON THE OTHER SIDE

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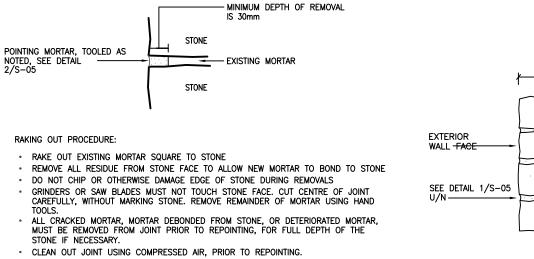
TYPICAL DETAILS

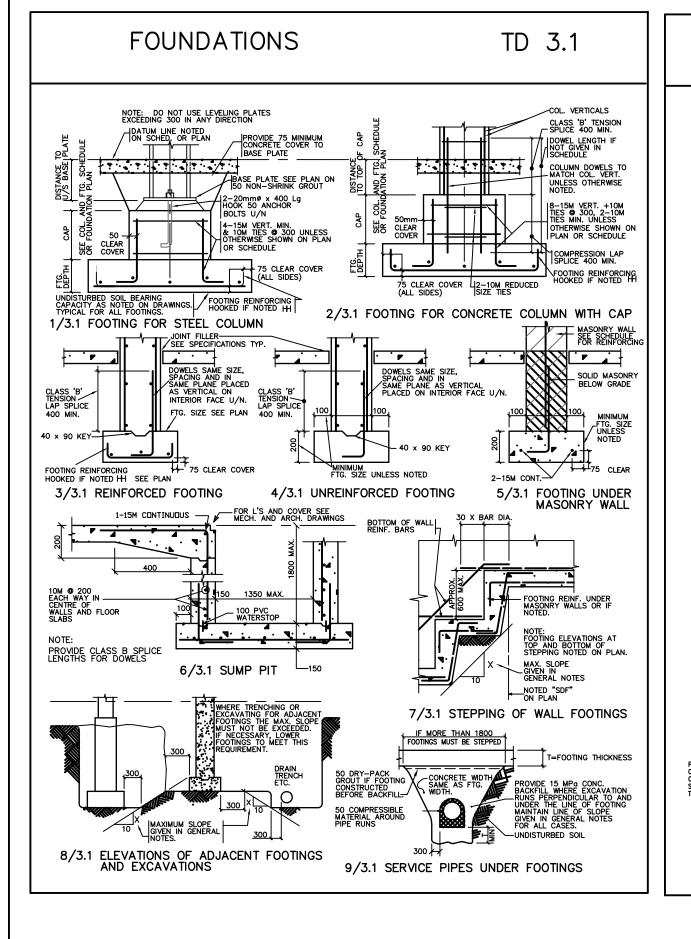
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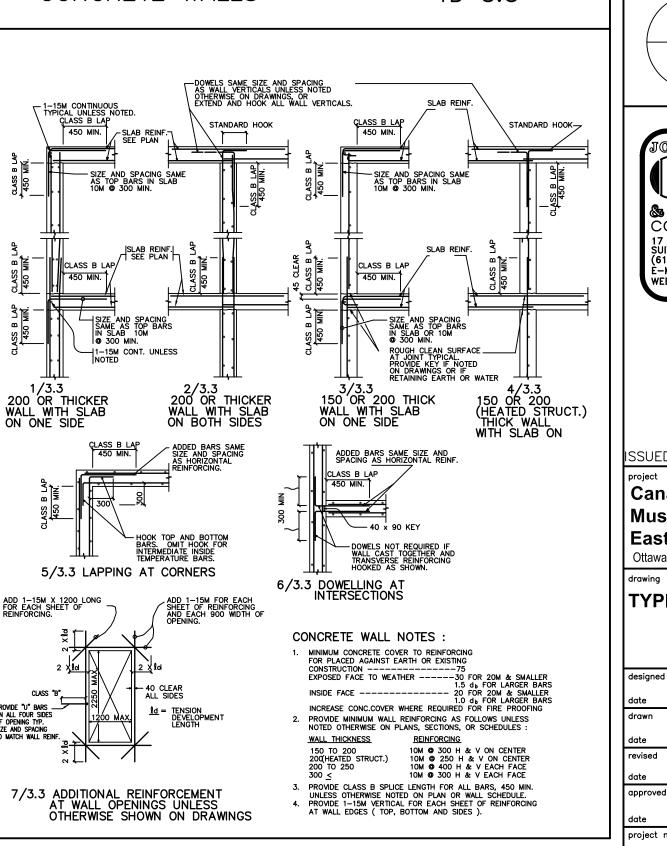
MCE13 A532





CONCRETE WALLS

TD 3.3

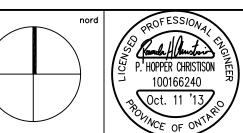




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A. GENERAL NOTES

General

- 1.1 Check all dimensions on Structural Drawings with the site conditions, and the manufacturer's drawings. These drawings are intended to be used in conjunction with the manufacturer's drawings of the unit. Report any inconsistencies before proceeding with the work. DO NOT scale these drawings.
- 1.2 T.D. sections on the drawings refer to Typical Detail Sheets. They show structural intent rather than actual conditions for this project.
- 1.3 All dimensions unless otherwise noted are in millimeters.
- 1.4 Conform to the requirements of the included Typical Details.
- 1.5 Independent Inspection and Testing: The Owner will appoint an independent inspection and testing agency. The cost of inspection shall be paid by the Owner. Work will be inspected as required by the Departmental Representative to determine conformance to the drawings and specifications.
- 1.6 These drawings show the completed structure. The Contractor shall have the sole responsibility for the design, erection, operation, maintenance, and removal of temporary supports, structures, and facilities, and the design and execution of construction methods required in their use.
- 1.7 The use of these drawings shall be strictly limited to the instructions in the revision block. Building from these drawings shall proceed only when "ISSUED FOR CONSTRUCTION".
- 1.8 The scope of the work depends on the site conditions. Notify the Departmental Representative where on site conditions may require modifications to the contract documents.
- 1.9 Before all excavation, the contractor must verify the existence of utilities/services and their elevations. The contractor is responsible for diverting or relocating of conduits, water, sewer or power lines which interfere with the execution of the works and obtaining the authorization of the proper organization.
- 1.10 Design and Construction to be in accordance with the Ontario Building Code 2006.
- 1.11 Repair all damaged sodded or asphalt areas upon completion of work
- 1.12 Contractor to locate all underground services prior to excavation and protect during Construction

2. Foundations

2.1 Bearing pressures to be verified in writing in the field by a Geotechnical Engineer registered in the Province of Ontario prior to placing concrete. Contact owner to arrange for bearing capacity testing upon removal of concrete in area of work. Allow 48 hours from concrete removal for completion of test. Cost of testing to be paid by owner.

3. Cast-ın-Place Concrete

- 3.1 All concrete formwork and reinforcing steel work must be done in accordance with CSA A23.1-09 and A23.2-09.
- 3.2 Unless otherwise specified on the plans, provide temperature reinforcing for framed one way slabs in accordance with Typical Detail TD3.6.
- 3.3 Bars marked continuous shall be developed by Class B tension lap where spliced.
- 3.4 The minimum clear concrete cover for steel reinforcement, unless noted on drawings, shall conform to Clause 6.6.6., and Table 17 of CSA-A23.1-09.
- 3.5 The contractor shall notify the Departmental Representative, not less than 24 hours before placing concrete, when the reinforcing steel and formwork are ready for inspection. Do not close forms until the reinforcement has been reviewed. The contractor must give notice to the Testing Laboratory to ensure the presence of their representative for each concrete pour.
- 3.6 Submit I electronic copy of bar lists and placing diagrams to Departmental Representative to review prior to fabrication of reinforcing steel. Draw diagrams to a scale of not less than 1:50. Review of shop drawings is a precaution against oversight or error. It is not a detailed check and shall not be construed as relieving the Contractor of responsibility for making the work accurate and in conformity with the Contract Documents. Maintain a set of reviewed drawings on site.
- 3.7 Independent Inspection and Testing: The Departmental Representative will appoint an independent inspection and testing agency to undertake concrete strength tests. The cost of testing shall be paid by the Departmental Representative. Laboratory curing and testing of samples will be carried out in accordance with CSA A23.1-09 and CSA A23.2-09 except that strength tests, including air entrainment and slump tests, will be required for each 50 m³, but not less than one test, for each class of concrete placed each day. Provide a group of three cylinders for each standard strength test. One specimen will be tested at 7 days and two at 28 days. Provide one additional field cured cylinder for testing at 7 days when concrete is placed under cold weather conditions. Results will be on the form conforming to CSA A23.2-09, Annex B, stating the location of concrete to which tests relate and with comments on abnormal results and conditions, and will be reported to the Departmental Representative with copies to the Contractor.

4.0 Structural Steel

- 4.1 Structural Steel Shapes are shown in the metric designation.
- 4.2 Erect steel in accordance with CSA S16-09 and CSA-S136-07(R2012).
- 4.3 Do not splice material without the written approval of the Departmental Representative. Where granted 100% X-ray inspection will be mandatory and paid for by the contractor.
- 4.4 All railings to be hot-dip galvanized.
- 4.5 Touch up abrasions and imperfections with to zinc-rich primer.
- 4.6 Submit I electronic copy of erection diagrams and shop fabrication details for review prior to fabrication. Review of shop drawings is a precaution against oversight or error. It is not a detailed check and shall not be construed as relieving the Contractor of responsibility for making the work accurate and in conformity with the Contract Documents. Maintain a set of reviewed drawings on site.

B. MATERIAL AND DESIGN DATA

- I. All loads shown on drawings are unfactored service loads in kN and kN/m $^{\rm 2}$ unless otherwise noted.
- 2. Existing footing design bearing pressure: I 00 kN/m² (Assumed), to be confirmed by a Soils Departmental Representative prior to construction.
- 3. Concrete compressive strength at 28 days: Walls and Ramps: 30 MPa, class N exposure. Slab: 35 MPa, class C I exposure
- 4. Reinforcing steel: to CAN/CSA G30.18-09, Grade 400.
- 5. Structural steel: to CSA G40.20/G40.21-04 (R2009) Welding: to CSA W59-03 (R2008).

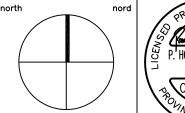
a) Rolled shapes and plates: Grade 300W
b) Welding electrodes: E70XX (E480XX)
c) Fasteners: A325 (A325M)
d) Anchor bolts: Grade 262W

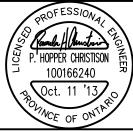
C. CODES AND STANDARDS

- 1. Conform to requirements of the Ontario Building Code 2006 and The Occupational Health and Safety Act and Regulations for Construction Projects. (Latest Edition)
- 2. Concrete Materials and Design: to CSA-A23.1-09 and A23.3-04 (R2008) Respectively.
- 3. Falsework for Construction Purposes: to CSA 5269.1 1975 (R2008).
- 4. Concrete Formwork: to CSA 5269.3 M92 (R2008).
- 5. Concrete Testing: to CSA-A23.2-09.
- 6. Concrete Construction: to CSA-A23.1-09.
- 7. Structural Steel Design, Fabrication and Erection: to CAN/CSA-S16-09.
- 8. Welding: to CSA Standards W59-03 (R2008), CSA-S16-09, and W47.1-09.
- 9. Galvanizing (Hot-Dip): to ASTM A123, minimum zinc coating of 600 g/m³



Agriculture and Agri-Food Canada Agriculture et Agroalimentaire Canada





JOHN G.

ASSOCIATES LTID.

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Canada Agriculture and Food

Museum Building 88
East Ramp Rehabilitation

Ottawa, Ontario

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GENERAL REQUIREMENTS

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