GENERAL NOTES:

- 1. ALL WORK AND MATERIALS SHALL CONFORM TO THE REQUIREMENTS SET OUT IN THE 2010 NATIONAL BUILDING CODE OF CANADA, ALL APPLICABLE CSA STANDARDS AND SAFETY STANDARDS, LATEST EDITIONS. IN THE EVENT THAT DETAIL INFORMATION ON DRAWINGS AND SPECIFICATIONS EXCEEDS THE MINIMUM STANDARDS, INFORMATION ON THE DRAWINGS AND SPECIFICATIONS IS TO BE FOLLOWED.
- 2. ALL WORK TO BE CARRIED OUT IN ACCORDANCE WITH THE NOVA SCOTIA OCCUPATIONAL HEALTH AND SAFETY CODE AND ALL APPLICABLE SAFETY REGULATIONS
- 3. THE STRUCTURE HAS BEEN DESIGNED FOR LOADS IN ACCORDANCE WITH THE 2010 NATIONAL BUILDING CODE OF CANADA.
- 4. CONTRACTOR WILL ASSUME FULL RESPONSIBILITY FOR THE INTEGRITY OF THE STRUCTURES DURING ERECTION. CONTRACTOR IS TO PROVIDE ADEQUATE TEMPORARY BRACING AND SHORING TO MAINTAIN STRUCTURAL SAFETY, PLUMB AND TRUE ALIGNMENT UNTIL COMPLETION OF WORK.
- 5. THE USE OF THESE DRAWINGS IS LIMITED TO THAT IDENTIFIED IN THE REVISION COLUMN. DO NOT CONSTRUCT FROM THESE DRAWINGS UNLESS MARKED "ISSUED FOR CONSTRUCTION" IN THE REVISION COLUMN.
- 6. SECTION MARK SHOWN THUS A MEANS SECTION 'A' SHOWN ON DRAWING 202.
- 7. CONTRACTOR TO REVIEW ALL DRAWINGS AND CHECK DIMENSIONS PRIOR TO CONSTRUCTION FOR PROPER FIT AND REPORT ANY DISCREPANCIES BEFORE PROCEEDING WITH WORK.
- 8. DO NOT CUT OR DRILL ANY OPENINGS IN STRUCTURAL MEMBERS WITHOUT WRITTEN APPROVAL FROM DEPARTMENTAL REPRESENTATIVE.
- 9. ALL TRADES SHALL SUBMIT SHOP DRAWINGS STAMPED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF NOVA SCOTIA, PRIOR TO COMMENCEMENT OF FABRICATION.
- 10. DO NOT SCALE DRAWINGS.
- 11. DIMENSIONS SHOWN ON DRAWINGS ARE IN MILLIMETERS. ELEVATIONS ARE IN METERS. ELEVATIONS INDICATED ARE REFERENCED TO GEODETIC DATUM.
- 12. DISSIMILAR METALS SHALL BE PROTECTED FROM CORROSION BY SEPARATION WITH HDPE SPACERS.

FOUNDATION NOTES:

- 1. SITE PREPARATION AND FOUNDATION DESIGN IS BASED ON THE GEOTECHNICAL INVESTIGATION REPORT No. 1389-001 DATED JUNE 2, 2014. REFER TO THESE DOCUMENTS FOR ANY PARTICULARS AS TO SOILS CONDITIONS AND FOUNDATION BEARING RECOMMENDATIONS.
- 2. ENSURE THAT THE REQUIREMENTS OUTLINED IN THE GEOTECHNICAL INVESTIGATION REPORT AND DESIGN DRAWINGS ARE READ AND UNDERSTOOD PRIOR TO COMMENCING WITH FOUNDATION WORK.
- 3. CAST-IN-PLACE FOOTINGS TO BE PLACED ON UNDISTURBED SOIL OR ENGINEERED FILL HAVING A MINIMUM DESIGN SOIL BEARING CAPACITY AS STATED IN GEOTECHNICAL PARAMETERS BELOW. BEARING CAPACITY MUST BE CONFIRMED IN WRITING BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF CONCRETE.
- 4. ALL DOWELS SHALL BE PLACED BEFORE CONCRETE FOOTINGS ARE POURED.
- 5. DOWELS FROM FOOTINGS SHALL MATCH SIZE AND SPACING OF VERTICAL REINFORCING IN WALL OR PEDESTAL ABOVE UNLESS NOTED ON DRAWINGS.
- 6. ALL BACKFILL MATERIALS AND BACKFILL INSTALLATION SHALL BE REVIEWED BY A GEOTECHNICAL ENGINEER TO ENSURE COMPLIANCE TO THE RECOMMENDATIONS AS NOTED IN THE GEOTECHNICAL REPORT.
- 7. REFER TO SITE PLAN FOR FINISH GRADE ELEVATIONS.
- 8. THE UNDERSIDE OF FOOTINGS SHALL BE FOUNDED AT LEAST THE DEPTH OF FROST PENETRATION (NOTED HEREIN) BELOW FINISHED EXTERIOR GRADE UNLESS NOTED OTHERWISE.
- 9. 9ENGINEERED BACKFILL AND SUBGRADE AS PER CIVIL.
- 10. ENGINEERED FILL BENEATH FOOTINGS SHALL BE AS RECOMMENDED IN THE GEOTECHNICAL INVESTIGATION REPORT AND AS SHOWN ON DRAWINGS.
- 11. FROST PROTECTION TO <u>ALL FOOTINGS</u> MUST BE MAINTAINED EQUIVALENT TO THE DEPTH OF FROST PENETRATION. CONTRACTOR SHALL PROVIDE AND MAINTAIN THIS PROTECTION USING A COMBINATION OF SOIL, RIGID INSULATION AND STRAW AS REQUIRED.
- 12. GEOTECHNICAL PARAMETERS:

-DEPTH OF FROST PENETRATION -ALLOWABLE DIFFERENTIAL SETTLEMENT -SUBGRADE WATER TABLE -DESIGN SOIL BEARING CAPACITY -MINIMUM FOOTING WIDTH

- 1.8 METRES 20mm VARIES (SEE GEOTECHNICAL REPORT) 150 kPa (SLS) OR 450 kPa (ULS FACTORED NET RESISTANCE) 600mm FOR STRIP UNLESS NOTED 1000mm FOR PAD UNLESS NOTED
- 13. PRIOR TO EXCAVATION FOR FOUNDATION LOCATE EXISTING IN GROUND SERVICES. CONTACT DEPARTMENTAL REPRESENTATIVE WHEN EXISTING SERVICES INTERFERE WITH NEW CONSTRUCTION.
- 14. EXCAVATE FOR FOUNDATION TO MINIMUM DEPTH INDICATED. VERIFY WITH GEOTECHNICAL ENGINEER THAT FOUNDATION SURFACE EXCEEDS THE DESIGN BEARING PRESSURE. SHORE EXCAVATION OR PROVIDE SAFE SLOPES.

15. MUD SLABS TO BE MIN. 150 THK. 25 MPa CONCRETE.

NON-STRUCTURAL COMPONENTS NOTES:

- 1. THE GENERAL CONTRACTOR IS RESPONSIBLE TO DESIGN, DETAIL, SPECIFY AND REVIEW ALL NON-STRUCTURAL COMPONENTS WHICH AFFECT THE STRUCTURAL FRAMING WITHIN THE BUILDING, INCLUDING ANCHORAGE AND SEISMIC RESTRAINTS, IF REQUIRED. THE ANALYSIS AND DESIGN SHALL BE PERFORMED BY A STRUCTURAL ENGINEER WITH RELEVANT EXPERIENCE IN SEISMIC RESTRAINTS OF NON-STRUCTURAL COMPONENTS AND MUST BE A PROFESSIONAL ENGINEER REGISTERED OR LICENSED TO PRACTICE IN THE PROVINCE OF NOVA SCOTIA. ALL DESIGNS SHALL CONFORM TO THE NATIONAL BUILDING CODE OF CANADA 2010 EDITION FOR A CATEGORY 2, NORMAL IMPORTANCE FACILITY. THE ABOVE NOTED ENGINEER SHALL ALSO BE RESPONSIBLE FOR FIELD REVIEW OF THE INSTALLATIONS.
- 2. SHOP DRAWINGS, SIGNED AND SEALED BY THE ENGINEER RESPONSIBLE FOR THE DESIGN OF NON-STRUCTURAL COMPONENTS SHALL BE SUBMITTED FOR REVIEW AND SHALL CLEARLY INDICATE LOADS IMPOSED ON THE PRIMARY STRUCTURE. REVIEW WILL BE LIMITED TO THE EFFECT OF THE COMPONENTS ON THE STRUCTURAL FRAMING.

3. EXAMPLES OF NON-STRUCTURAL COMPONENTS INCLUDE, BUT ARE NOT LIMITED TO:

- ARCHITECTURAL COMPONENTS SUCH AS HANDRAILS, GUARDRAILS, RAILINGS, FLAG POST, REMOVABLE CANOPIES, CEILINGS, VEHICLE PROTECTION SYSTEMS, ORNAMENTAL COMPONENTS, ETC.
- BRICK AND BLOCK VENEERS, INCLUDING REINFORCING AND TIES.
- LANDSCAPING COMPONENTS SUCH AS BENCHES, LIGHT POSTS, PLANTERS, ETC.
 CURTAIN WALL SYSTEMS, CLADDING, SKYLIGHTS, WINDOW MULLIONS, ETC.
- INTERIOR AND EXTERIOR NON-LOAD BEARING STEEL STUD WALLS.
- SUPPORT AND BRACING OF PLUMBING, FIRE SUPPRESSION, MECHANICAL AND ELECTRICAL SYSTEMS AND EQUIPMENT FOR NON-GRAVITY AND SEISMIC LOADS.
 WINDOW WASHING EQUIPMENT, INCLUDING ALL ATTACHMENTS AND ANCHORAGE.
- ELEVATORS, ESCALATORS AND OTHER CONVEYING SYSTEMS, INCLUDING PROPRIETARY SUPPORT BEAMS AND THEIR ATTACHMENTS.

FORMWORK NOTES:

- 1. DESIGN, CONSTRUCT AND REMOVE FORMWORK, FRAMING SUPPORTS AND BRACING TO CONFORM TO REQUIREMENTS SPECIFIED IN CSA A23.1 AND CSA S269.1, LATEST EDITIONS TO PROVIDE FINISHED POURED CONCRETE SURFACES WITHIN SPECIFIED TOLERANCES.
- 2. ALLOWABLE TOLERANCES TO REQUIREMENTS OF CSA-A23.1, LATEST EDITION.
- 3. INSTALL ITEMS SUPPLIED BY OTHERS SUCH AS INSERTS, ANCHOR BOLTS, MISCELLANEOUS FRAMES, METAL FLASHING REGLETS, HOLES SLEEVES, LADDER RUNGS AND ANCHOR BOLTS.

REINFORCED CONCRETE NOTES:

 ALL CONCRETE, CONCRETE MATERIALS, FORMS, PRACTICES, ETC., SHALL CONF LATEST EDITION. TESTING SHALL BE REQUIRED FOR EACH 50 m³ OR FRACTION TO COMPRESSIVE STRENGTH, SLUMP AND AIR ENTRAINMENT (USE NON AAR AGGRE

50mm

60mm

50mm

50mm

60mm

60mm

- 2. CONCRETE PROTECTIVE COVERING FOR REINFORCING STEEL SHALL BE: A - SURFACES PLACED AGAINST SOIL 75mm
- B PILASTERS (TO VERTICAL BARS) 50mm C - WALLS AND GRADE BEAMS 40mm
- D S.O.G. BOTTOM
- S.O.G. TOP
- E SUSPENDED SLAB TOP SUSPENDED SLAB BOTTOM
- F TANK WALLS TANK BASE TOP
- CONTRACTOR MUST PROVIDE SUPERVISION DURING CONSTRUCTION TO ENSUR MAINTAINED IN CORRECT POSITION AND OVERALL QUALITY.
- ALL REINFORCING STEEL SHALL HAVE MINIMUM YEILD STRENGTH OF 400 MPa AN CONFORM TO CSA G30.18, LATEST EDITION.
- ALL REINFORCING STEEL SHALL BE DETAILED, FABRICATED, PLACED AND SUPPO ACCORDANCE WITH REINFORCING STEEL MANUAL OF STANDARD PRACTICE BY STEEL INSTITUTE OF CANADA LATEST EDITION, AND CSA-A 23.3, LATEST EDITION
- 6. DESIGN OF CONCRETE STRUCTURES SHALL CONFORM TO CSA-A 23.3 LATEST EL
- AT LEAST ONE SLUMP TEST AND ONE AIR ENTRAINMENT TEST SHALL BE TAKEN \ COMPRESSIVE STRENGTH TEST.
- 8. NO ADMIXTURES SHALL BE USED WITHOUT PRIOR APPROVAL FROM THE DEPART REPRESENTATIVE.
- 9. INSTALL CONTINUOUS VERTICAL DOVETAIL ANCHOR SLOT TO FORMS WHERE MA CONCRETE WALL OR COLUMN OR STEEL COLUMN FOR ANCHORAGE OF MASONR
- 10. CONCRETE SLUMP FOR WALLS TO BE 75mm.
- 11. AIR CONTENT TO BE AS PER TABLE 4 OF CSA A23.1 FOR CLASSIFICATION OF CON IN SPECIFICATION. AIR CONTENT TO BE 3% MAX. FOR CONCRETE REQUIRING FLC
- 12. FILL WALL AND SLAB CRACKS WITH SEALANT AS SPECIFIED.
- 13. PROVIDE CONTROL AND EXPANSION JOINT SEALANT AS SPECIFIED.
- 14. UNLESS NOTED OTHERWISE, REINFORCING STEEL TO BE SPLICED AS FOLLOWS:A. ALL SPLICES TO BE TENSION LAP SPLICES, CLASS "B".B. NO MORE THAN 50% OF THE REINFORCING TO BE SPLICED AT ANY GIVEN LCCC.
- 15. USE STANDARD BENDS UNLESS NOTED OTHERWISE.
- 16. REINFORCING STEEL, EMBEDDED PARTS, ANCHOR BOLTS AND DOWELS, ETC. TO POSITION PRIOR TO PLACING CONCRETE, AND HELD RIGIDLY DURING PLACEMEN
- 17. REINFORCING STEEL TO BE INSPECTED BY DEPARTMENTAL REPRESENTATIVE P FORMS.
- 18. ALL CORNERS AND INTERSECTIONS TO HAVE CORNER BARS, SAME SIZE AND SP. BARS. PROVIDE TENSION LAP WITH MAIN BARS.
- 19. SUBMIT REINFORCING STEEL SHOP DRAWINGS FOR REVIEW.
- 20. CONCRETE SHALL HAVE A UNIT WEIGHT OF 2350 kg/cu.m.
- 21. CONCRETE MIX DESIGN SHALL BE SUBMITTED TO THE DEPARTMENTAL REPRESE REVIEW.
- 22. CURING, PROTECTION AND FINISHING OF CONCRETE SHALL CONFORM TO CSA-A LATEST EDITION.
- 23. ALL CONCRETE SHALL BE TESTED IN ACCORDANCE WITH CSA-A23.2, LATEST EDI
- 24. EXCEPT WHEN APPROVED BY THE DEPARTMENTAL REPRESENTATIVE, PIPES, CC SLEEVES EMBEDDED IN CONCRETE SHALL BE INSTALLED IN ACCORDANCE WITH CSA-A23.1, LATEST EDITION. AND AS SHOWN ON THE ARCHITECTURAL, CIVIL, MEC AND ELECTRICAL DRAWINGS.
- 25. FORMS SHALL NOT BE REMOVED BEFORE THE CONCRETE HAS SET AND REACHE ITS DESIGN STRENGTH, AND AFTER THE FOLLOWING MINIMUM WAIT PERIODS: A - FOOTINGS 3 DAYS

B - FROST WALLS	3 DAYS
C - SUSPENDED SLABS, BEAMS	28 DAYS
D - STRUCTURAL WALLS (TANK, RETAINING WALLS)	7 DAYS
E - PEDESTALS	7 DAYS

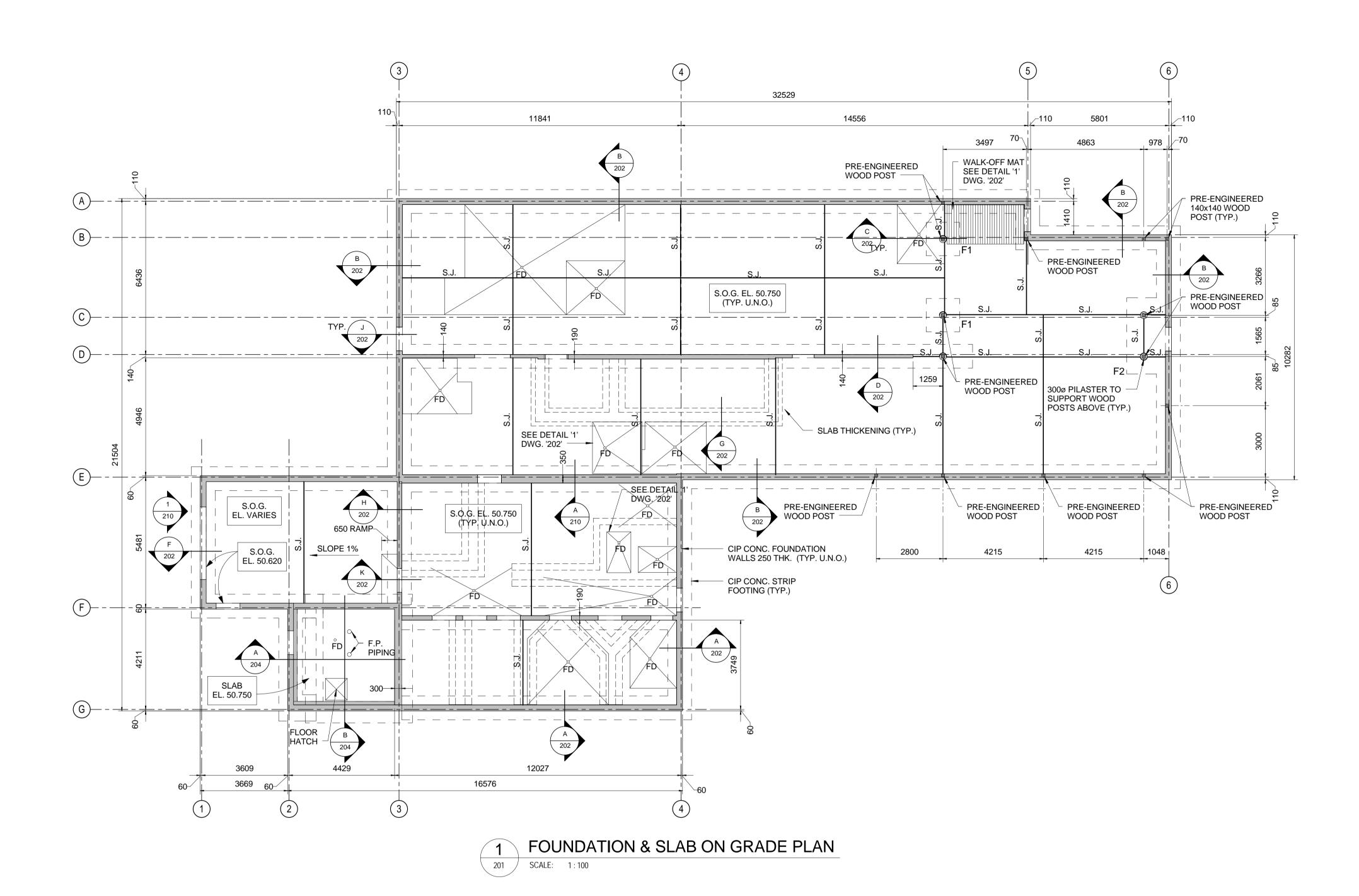
- 26. CONSTRUCTION JOINTS IN FOOTINGS AND WALLS SHALL BE LOCATED SO AS TO I IMPAIR THE STRENGTH OF THE STRUCTURE AND TO THE DEPARTMENTAL REPRE APPROVAL. CONSTRUCTION JOINTS SHALL BE KEYED AND ADD DOWELS MINIMUM MATCH WALL/FOOTING REINFORCEMENT.
- 27. ALL EXPOSED CORNERS TO HAVE A MINIMUM CHAMFER OF 25 mm UNLESS NOTE

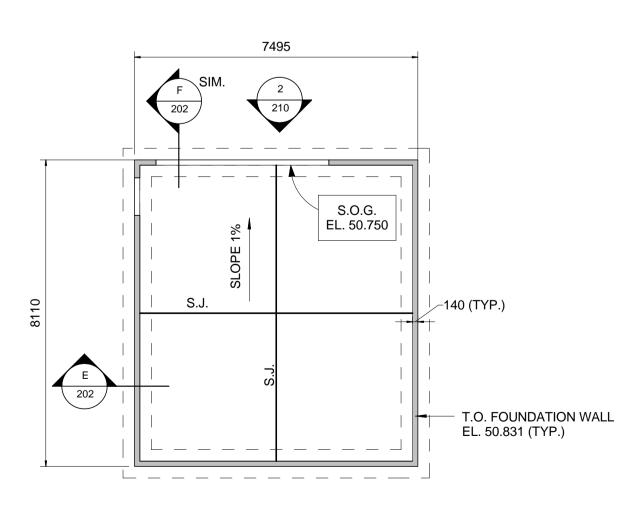
MASONRY NOTES:

- ALL MASONRY WORK TO BE IN ACCORDANCE WITH CSA-A371, LATEST EDITION EX SPECIFIED OTHERWISE.
- ALL CONCRETE BLOCK WALLS SHALL BE OF STANDARD CONCRETE MASONRY UN CAN/CSA-A 165.1, LATEST EDITION CLASS H/15/A/S.
- 3. ALL MASONRY MORTAR FOR CORE FILLED CONCRETE BLOCK WALLS SHALL BE T LATEST EDITION. UNLESS NOTED ON DRAWINGS/SPECIFICATIONS.
- ALL CONCRETE GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AT 28 MAXIMUM AGGREGATE SIZE 10mm. SLUMP 150mm.
- 5. PROVIDE MINIMUM "CLASS B" TENSION LAP SPLICE IN ACCORDANCE WITH CSA-S
- 6. ALL MORTAR JOINTS SHALL BE 10mm.
- HOLLOW MASONRY UNITS SHALL BE LAID WITH FACE SHELL BED, AND HEAD JOIN THE WEBS SHALL BE LAID IN A FULL BED IN ALL COURSES, IN THE STARTING COU WEBS SHALL ALSO BE LAID IN FULL BED IN SOLID FOUNDATION WALLS AND WHEF CELLS OR CAVITIES THAT ARE TO BE CORE FILLED.
- ALL CONCRETE BLOCK WALLS SHALL BE REINFORCED WITH HEAVY DUTY LADDEL HORIZONTAL BLOCK REINFORCEMENT EVERY SECOND COURSE TO CSA S304.1, L 4.8mm LONGITUDINAL AND 3.6mm TRANSVERSE WIRES, GALVANIZED AFTER FABF A153, 456 G/SQ.M. LAP REINFORCEMENT 150mm AT EACH SPLICE U.N.O.
- ALL NON LOAD-BEARING MASONRY PARTITIONS SHALL BE SUPPORTED LATERALI AT 1800 O.C MAX. AT JOIST/BEAM LEVEL AS DETAILED. PARTITION WALLS TO HAV BOND BEAM AT TOP. BOND BEAM TO BE 'KNOCKOUT BLOCK' w/ 2-15M TOP AND BC SOLID WITH 20 MPa CONCRETE.
- 10. PROVIDE CONTROL JOINTS IN THE MASONRY BLOCK WALL, SPACED AT 6000mm C THE DRAWINGS AND SPECIFICATIONS.
- 11. PROVIDE BLOCK WALL EXPANSION JOINT AT TRANSITION BETWEEN FIRE WATER SLAB TO SLAB ON GRADE.
- 12. MASONRY BLOCK WALLS THAT TRANSITION FROM SUPPORT ON SLAB ON GRADE SUSPENDED CONCRETE SLAB ARE TO HAVE CONTROL JOINTS AT ALL SUPPORT T LOCATIONS.
- 13. CONTRACTOR TO COORDINATE SLIDING DOOR REQUIREMENTS AND MASONRY WALL TOLERANCES WITH DOOR SUPPLIER.

CONFORM TO CSA-A23.1, ON THEREOF FOR GGREGATE)	 BUILDING DESIGN LOADS: 1. THE BUILDING STRUCTURE TO BE DESIGNED FOR LOADS AND LOAD COMBINATIONS IN ACCORDANCE WITH THE NATIONAL BUILDING CODE, 2010 EDITION. 2. CONTRACTORS CONSTRUCTION LOADS MUST NOT EXCEED THE DESIGN LOADS SPECIFIED HEREIN. 3. <u>SLAB ON GRADE:</u> ADMINISTRATION: 4.8 kPa (PLUS PARTITION WEIGHT). SHOP AREAS: 4.8 kPa OPERATIONAL AREAS: 4.8 kPa MINIMUM AREA LOAD. VEHICLES: 4.8 kPa MINIMUM AREA LOAD 	 WOOD NOTES CONTINUED: 20. ALL TIMBER ELEMENTS ARE DESIGNED FOR DRY-SERVICE CONDITIONS, SEE PROJECT DESIGN DRAWINGS FOR WATERPROOFING AND VENTILATION DETAILS. 21. ALL WOOD FRAME CONSTRUCTION SHALL SATISFY THE FOLLOWING CONSTRUCTION TOLERANCES AS A MINIMUM. REFER TO ARCHITECTURAL AND WARRANTY REQUIREMENTS FOR ADDITIONAL TOLERANCE SPECIFICATIONS. A. FLOORS - NOT MORE THAN 6mm IN 3000mm OUT OF LEVEL B. WALLS - NOT MORE THAN 6mm IN 2500mm OUT OF PLUMB 	 PRE-ENGINEERED WOOD TRUSSES: THE STRUCTURAL DRAWINGS SHOW CONCEPTUAL WOOD TRUSS FRAMING ONLY. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS, ROOF SLOPES, ROOF OVERHANGS, ELEVATIONS, OPENINGS ETC. WOOD TRUSSES ARE BOTTOM CHORD BEARING UNLESS NOTED OTHERWISE. THE DESIGN, PREPARATION OF SHOP DRAWINGS, REVIEW OF FABRICATION AND FIELD REVIEW OF INSTALLATION SHALL BE CARRIED OUT BY A SPECIALTY STRUCTURAL ENGINEER. THE WOOD TRUSSES SHALL BE DESIGNED FOR THE LOADS SPECIFIED IN THE GENERAL NOTES, OR AN OLIVARIA ON FLAME THEY SHOLL BALSO FOR THE LOADS SPECIFIED IN THE GENERAL NOTES, OR 	KEY PLAN
NSURE REBAR IS Pa AND SHALL	 MECHANICAL ROOM: 7.2 kPa <u>SUPERIMPOSED DEAD LOADS (SDL) AT CEILING(S)</u>: SDL AT CEILING ARE NON-STRUCTURAL LOADS DUE TO MECHANICAL AND ELECTRICAL SERVICES AT CEILING. (ON BOTTOM CHORD OF JOISTS) - UNIFORMLY DISTRIBUTED SERVICE LOAD OF 0.50 kPa <u>OR</u> POINT LOADS DUE TO MECHANICAL SERVICES / ELECTRICAL FIXTURES HANGER LOADS, WHICHEVER IS GREATER. SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATION AND SIZE OF SERVICES HANGING FROM CEILING. JOISTS SUPPLIER TO CO-ORDINATE HANGER LOCATIONS WITH THE CONTRACTOR. 	 - NOT MORE THAN 6mm IN 3000mm FOR ANY BOWING C. OVERALL - BUILDING WALLS AND FLOORS SHALL NOT BE MORE THAN 10mm DIFFERENCE IN MEASUREMENT FROM DIMENSIONS SHOWN ON CONTRACT DOCUMENTS. 22. ALL DIMENSION LUMBER TO BE SURFACED FOUR SIDES ('S4S') 23. MISCELLANEOUS STEEL TO BE CAN/CSA-G40.21 OR APPROVED EQUAL. 24. BOLTS SHALL BE ASTM A307 OR APPROVED EQUAL, USED WITH STANDARD CUT STEEL WASHERS 	 AS SHOWN ON PLAN. THEY SHOULD ALSO BE DESIGNED FOR A VERTICAL POINT LOAD OF 0.90 kN (UNFACTORED) APPLIED ANYWHERE ON BOTTOM CHORD (ONE POINT LOAD PER TRUSS). 5. TRUSS SUPPLIER MUST DESIGN AND SUPPLY THE ENTIRE TRUSS SYSTEM WHICH INCLUDES THE FOLLOWING ELEMENTS: LATERAL BRACING AND BRIDGING. CONNECTING HARDWARE. BEARING PLATES, HOLD DOWNS, AND TENSION TIES SHOWN ON TRUSS SHOP DRAWINGS. 	SNC • LAVALIN Inc. Halifax, Nova Scotia, Canada
UPPORTED IN E BY THE REINFORCING TION. ST EDITION. KEN WITH EACH PARTMENTAL RE MASONRY ABUTS SONRY WALL.	 6. <u>WIND LOAD DESIGN:</u> DESIGN OF ALL STRUCTURAL ELEMENTS ARE BASED ON THE FOLLOWING: - AN HOURLY WIND PRESSURE FOR RETURN PERIOD OF 50 YEARS, q(1/50) = 1.05 kPa - IMPORTANCE CATEGORY FOR WIND: NORMAL, Iw = 1.0 7. <u>SEISMIC LOAD DESIGN:</u> DESIGN OF ALL STRUCTURAL ELEMENTS ARE BASED - Sa(0.2) = 0.66, Sa(0.5) = 0.30, Sa(1.0) = 0.13, Sa(2.0) = 0.039 - PGA = 0.35 - SITE CLASSIFICATION FOR SEISMIC RESPONSE: BEDROCK SITE CLASS - C NATIVE TILL SITE CLASS - C - SFRS DUCTILITY RELATED FORCE MODIFICATION FACTOR FOR CONVENTIONAL WOOD CONSTRUCTION AS PER TABLE 4.1.8.9 IN NBCC, 2010 - IMPORTANCE CATEGORY FOR SEISMIC: NORMAL, Ie = 1.0 	 UNLESS NOTED OTHERWISE ON DRAWINGS. 25. ANCHOR RODS TO BE ASTM 11554, GRADE 36, U.N.O. 26. MOISTURE CONTENT OF ALL TIMBER ELEMENTS SHALL NOT EXCEED 19% AT THE TIME OF CONSTRUCTION OR FABRICATION. 27. ALL FASTENERS AND CONNECTION HARDWARE THROUGH PRESERVATIVE TREATED MATERIALS OR OUTSIDE OF THE MOISTURE BARRIER TO BE HOT DIPPED GALVANIZED OR STAINLESS STEEL AS SPECIFIED. 28. USE 2-2x20 BEAMS OVER ALL OPENINGS IN BEARING WALLS UNLESS NOTED OTHERWISE. BEAMS SHALL BE SUPPORTED AT EACH END UNLESS NOTED OTHERWISE. WALLS: 	 6. THE SHOP DRAWINGS SHALL INCLUDE THE FOLLOWING ELEMENTS: A. PROJECT NAME, LOCATION AND SHOP DRAWING REVISION DATES. B. TRUSS LAYOUT, INDICATION ALL TRUSSES WITH CLEAR REFERENCE BETWEEN THE LAYOUT AND INDIVIDUAL TRUSS DESIGN DRAWINGS. C. SECTIONS, CONNECTION DETAILS, DESIGN LOADS, MATERIAL SIZE, GRADE AND SPECIES. D. CONNECTOR PLATE SIZE, GAUGE AND LOCATION. E. COMPLETE DIMENSIONS. F. LAMINATING INSTRUCTIONS FOR MULTIPLE PLY TRUSSES G. ALL BRACING AND BRIDGING NECESSARY FOR THE STABILITY OF THE TRUSSES DURING ERECTION AND IN THE COMPLETED STRUCTURE. H. HOLD DOWN ANCHORS TO RESIST WIND UPLIFT, CONNECTING ROOF TRUSSES TO THE SUPPORTING STRUCTURE. I. LIVE LOAD AND TOTAL LOAD DEFLECTION LIMITS. J. END REACTIONS OF GIRDER TRUSSES ON THE SUPPORTING STRUCTURE IN KN 	Member of the SNC-LAVALIN Group NOTES
CONCRETE AS INDICATED G FLOOR HARDENER.	 8. <u>SNOW LOAD DESIGN:</u> DESIGN OF ALL STRUCTURAL ELEMENTS ARE BASED ON THE FOLLOWING: Ss = 5.3 kPa, Sr = 0.4 kPa ROOF SNOW LOAD: 5.8 kPa + DRIFT LOADS AS SHOWN ON PLAN IMPORTANCE CATEGORY FOR SNOW: NORMAL, Is = 1.0 9. <u>CONCENTRATED LOADS:</u> UNLESS NOTED OTHERWISE, SPECIFIED CONCENTRATED LOADS AS FOLLOWS: ROOF SURFACES = 1.3 kN OVER AN AREA OF 750mm x 750mm 	 NAILING BUILT-UP STUD POSTS SHALL CONFORM TO CAN/CSA-086. EACH STUD OF BUILT-UP POST SHALL BE NAILED. ALL POSTS AND BUILT-UP STUD POSTS SHOWN ON ANY LEVEL SHALL BE CARRIED DOWN TO THE CONCRETE UNLESS NOTED OTHERWISE. ALL LOAD BEARING WALLS SHALL HAVE 2 CONTINUOUS TOP PLATES AND 1 CONTINUOUS BOTTOM PLATE. BEAMS OR HEADERS OVER OPENINGS IN WALLS SHALL BE DROPPED TO ALLOW THE TOP 	 (UNFACTORED). NOTE THAT IT IS THE TRUSS ENGINEER'S RESPONSIBILITY TO CHECK BEARING CONDITIONS AT THE SUPPORT POINTS OF ALL TRUSSES BUT ESPECIALLY GIRDERS AND PROVIDE STEEL HARDWARE AS REQUIRED TO PREVENT OVER STRESS IN BOTH THE TOP PLATES AND THE GIRDERS. 7. WHERE BOTTOM CHORD BRACING IS REQUIRED AT SPACING OTHER THAN 3000 mm CENTRES, SUCH AS CANTILEVERS, INTERIOR BEARINGS OR WIND UPLIFT CONDITIONS, SUCH AS BRACING IS TO BE SHOWN GRAPHICALLY ON THE TRUSS SHOP DRAWINGS. 8. ALL MARK NUMBERS TO BE CLEARLY DISPLAYED ON THE UNDERSIDE OF TRUSS BOTTOM CHORD. 	
DWS: EN LOCATION. C. TO BE SECURED IN EMENT OF CONCRETE.	 10. <u>DEAD LOADS:</u> MIN. ROOF DEAD LOADS: SUPERIMPOSED ROOF DEAD LOADS: 1.05 kPa TOTAL ROOF LOADS: 1.70 kPa <u>NOTE:</u> LOADS DO NOT INCLUDE SELF WEIGHT OF JOISTS AND OTHER BUILDING DESIGN LOADS SPECIFIED IN NOTE #5. 11. CAMBER FOR DEAD LOAD FOR ROOF WOOD TRUSSES: 	 PLATES TO BE CONTINUOUS. 4. FASTEN WOOD-FRAME STRUCTURE AT BASE BY BOLTING THE BOTTOM PLATE (SILL PLATE) TO THE CONCRETE WITH 1/2" Ø ANCHOR BOLTS @ 4'-0" O.C. UNLESS NOTED OTHERWISE. ANCHOR BOLTS SHALL HAVE A MINIMUM 300 EMBEDMENT AND A MINIMUM 3" PROJECTION ABOVE THE CONCRETE. THE ANCHOR BOLTS MAY BE CAST IN PLACE. FULL WIDTH OF WALLS SHALL BEAR ON CONCRETE UNLESS NOTED OTHERWISE. 5. SILL PLATES SHALL BEAR ON A LEVEL SURFACE; PROVIDE A LEVELING BED OF MORTAR IF 	 FIELD DRILLING, CUTTING, NOTCHING OR ANY OTHER MODIFICATION TO TRUSSES IS NOT PERMITTED WITHOUT PRIOR WRITTEN APPROVAL OF THE TRUSS FABRICATOR'S SPECIALTY ENGINEER. COPIES OF ANY MODIFICATION APPROVALS TO BE SUBMITTED TO THE DEPARTMENTAL REPRESENTATIVE. PROVIDE HURRICANE CLIPS H10 BY SIMPSON STRONG TIE OR APPROVED EQUAL AT SUPPORTS. THE WOOD TRUSS LAYOUT SHOWN ON THESE DRAWINGS HAS BEEN COORDINATED WITH THE 	
IVE PRIOR TO CLOSING OF	 ALL ROOF LEVELS: CAMBER TRUSS FOR 1.05 kPa + SELF WEIGHT OF TRUSS. 12. UNLESS NOTED OTHERWISE, ALL CONNECTIONS TO BE DESIGNED FOR 50% OF THE MEMBER TENSILE CAPACITY. 13. UNLESS NOTES OTHERWISE, ALL CONNECTIONS TO BE DESIGNED FOR 50% OF THE MEMBER SHEAR CAPACITY. 	 REQUIRED. PROVIDE A SILL GASKET UNDER ALL SILL PLATES BEARING ON CONCRETE. SEE NOTES ON MOISTURE BARRIERS FOR GASKET REQUIREMENTS. 6. WHERE THE SPACING OF JOISTS OR ROOF TRUSSES MATCHES THE SPACING OF THE STUDS IN THE SUPPORTING WALL (OR MULTIPLE THEREOF), EACH JOIST OR TRUSS SHALL BEAR DIRECTLY OVER A STUD. SHEATHING: 	 SUPPORTING STRUCTURE BELOW. THE WOOD TRUSS LAYOUT SHALL NOT BE CHANGED WITHOUT PRIOR WRITTEN APPROVAL BY THE DEPARTMENTAL REPRESENTATIVE, THE COST OF REDESIGNING THE SUPPORTING STRUCTURE TO SUIT CHANGES TO THE TRUSS LAYOUT WILL BE CHARGED TO THE CONTRACTOR. 12. TRUSS SUPPLIER SHALL SUBMIT A LETTER ATTESTING TO THE SUCCESSFUL COMPLETION AND INSTALLATION OF ALL ELEMENTS IN COMPLIANCE WITH THE ENGINEERED TRUSS SHOP DRAWINGS TO THE ENGINEER OF RECORD AND DEPARTMENTAL REPRESENTATIVE. THIS LETTER SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF NOVA 	
RESENTATIVE FOR CSA-A23.1, T EDITION. S, CONDUITS, AND WITH ., MECHANICAL ACHED 70% OF DS:	 WOOD NOTES: 1. WOOD CONSTRUCTION, MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE LATEST EDITIONS OF THE FOLLOWING: A. NATIONAL BUILDING CODE OF CANADA 2010 B. CAN/CSA-086 - ENGINEERING DESIGN IN WOOD C. CSA 0121 - DOUGLAS FIR PLYWOOD D. CAN/CSA - LO 4000 - PARALLAMS AND MICROLLAMS E. CAN/CSA-0122 - STRUCTURAL GLUED-LAMINATED TIMBER F. CAN/CSA-0177 - QUALIFICATION CODE FOR MANUFACTURERS OF STRUCTURAL GLUED-LAMINATED TIMBER G. CSA 0437 SERIES - STANDARDS FOR OSB AND WAFERBOARD H. CSA B111 - WIRE NAILS, SPIKES AND STAPLES J. CANADIAN WOOD-FRAME HOUSE CONSTRUCTION - CMHC K. "WOOD DESIGN MANUAL" - CANADIAN WOOD COUNCIL L. "WOOD BUILDING TECHNOLOGY" - CANADIAN WOOD COUNCIL 	 A. <u>ROOF SHEATHING:</u> 5/8" TONGUE AND GROOVE PLYWOOD. B. <u>EXTERIOR WALL SHEATHING:</u> 3/8" PLYWOOD ON EXTERIOR SIDE 1/2" PLYWOOD SHEATHING IF WALLS CLAD WITH VERTICAL STRAPPING OR BRICK VENEER. SEE ALSO ARCHITECTURAL FOR ADDITIONAL SHEATHING REQUIREMENTS. C. <u>SHEAR WALL SHEATHING</u>: SEE SHEAR WALL SCHEDULE FOR SHEATHING REQUIREMENTS AT SHEAR WALL LOCATIONS. LAY FLOOR AND ROOF SHEATHING WITH THE SURFACE GRAIN AT RIGHT ANGLES TO THE JOISTS. STAGGER THE JOINTS PARALLEL TO THE JOISTS. DRYWALL OR SHEATHING ON LOAD BEARING WALLS OR SHEAR WALLS SHALL BE FASTENED DIRECTLY TO THE STUDS, WITHOUT THE USE OF RESILIENT METAL CHANNELS. SHRINKAGE: 	SCOTIA. 13. THE WOOD TRUSSES SHALL BE DESIGNED FOR A MAXIMUM LIVE LOAD DEFLECTION OF 1/ 360 OF THE SPAN. 14. THE WOOD TRUSSES SHALL BE KEPT DRY AND PROTECTED FROM THE ENVIRONMENT DURING STORAGE ON OR OFF THE PROJECT SITE AS PER THE MANUFACTURERS REQUIREMENTS.	
S TO LEAST EPRESENTATIVE'S NIMUM 900mm LONG TO NOTED OTHERWISE.	 M. OSB TO CONFORM TO CSA 0325. 2. ALL LUMBER USED FOR THE BUILDING FRAME SHALL BE S-P-F NO.1/NO. 2 GRADE OR BETTER. LUMBER SHALL BE SEASONED AND THE MOISTURE CONTENT SHALL NOT BE MORE THAN 19% UNLESS NOTED OTHERWISE. 3. ALL METAL CONNECTORS EXPOSED TO EXTERIOR SHALL BE HOT-DIP GALVANIZED (CLASS G90, ZINC-COATED THICKNESS 0.05mm). 4. PLYWOOD USED AS SHEATHING FOR WALL AND ROOF CONSTRUCTION SHALL BE EXTERIOR TYPE. 	 FINISHES SHALL BE DETAILED TO ACCOMMODATE SHRINKAGE OF THE TIMBER OVER TIME FRAMING DETAILS SHALL ENSURE UNIFORM VERTICAL SHRINKAGE. ADJACENT PORTIONS OF STRUCTURE SHALL BE SUPPORTED ON ROUGHLY EQUIVALENT AMOUNTS OF HORIZONTAL TIMBER (JOISTS AND SILL PLATES). DO NOT MIX KILN-DRIED AND NON-KILN DRIED JOISTS IN ANY GIVEN FLOOR. 		
	 5. ANY TIMBER NOT GRADE MARKED WILL BE REJECTED. 6. NAILS SHALL BE COMMON STEEL WIRE OR COMMON SPIRAL NAILS CONFORMING TO CSA STANDARD B111, "WIRE NAILS, SPIKES AND STAPLES". 7. BOLTS SHALL CONFORM TO CSA STANDARD, "MISCELLANEOUS BOLTS AND SCREWS". 	 ENGINEERED WOOD PRODUCTS: ENGINEERED WOOD PRODUCTS INCLUDE ALL PRE-MANUFACTURED BEAM, AND COLUMNS. BEAMS EXPOSED TO VIEW IN FINISHED BUILDING SHALL BE SANDED APPEARANCE GRADE WITH STAMPS IN COVERED LOCATIONS. SIZES OF BEAMS AND POSTS SHALL BE SPECIFIED ON PLAN. 	ABBREVIATIONS: A.F.F. ABOVE FINISH FLOOR A.C. ABOVE FINISH FLOOR ALT. ALTERNATE HORIZ. HORIZONTAL ARCH. ARCHITECTURAL JP. JACK POST	C01 ISSUED FOR TENDER 18 JUL 20 NO. REVISION DATE STAMP
RY UNITS, TYPE A TO BE TYPE 'S' TO CSA A179, AT 28 DAYS OF 20 MPa	 ALL LUMBER USED IN WOOD-FRAME CONSTRUCTION SHALL HAVE THE GRADE MARK STAMPED BY AN APPROVED AGENCY AS PER NATIONAL BUILDING CODE OF CANADA. THE CONTRACTOR MUST ENSURE THAT PROPER GRADES OF LUMBER ARE USED AS INDICATED ON THE DRAWINGS AND SHALL CHECK THAT THE WORK IS DONE PROPERLY IN ACCORDANCE WITH THE DRAWINGS AND THE BUILDING STANDARDS AND CODES. CONTRACTOR SHALL ENSURE THAT TEMPORARY BRACING FOR WIND IS PROVIDED AND MAINTAINED 	3. SIZES OF BEAMS AND FOSTS SHALL BE SPECIFIED ON PLAN. 4. <u>BEAMS</u> : MINIMUM STRENGTHS OF BEAMS AS SPECIFIED ON PLAN: WEYERHAEUSER/ DESIGNATION MODULAS OF ELASTICITY SHEAR RESISTANCE (Fv) BENDING RESISTANCE (Fb) BEARING RESISTANCE (Fcp) PSL 2.0E (2000 KSI) 540 PSI 5360 PSI 1365 PSI	BLLBOTTOM LOWER LAYER(L)LOWERBULBOTTOM UPPER LAYERLBMBLOAD BEARING MASONRY BLOCKBOT.BOTTOMLBWSLOAD BEARING WOOD STUDCCENTRE LINELLLIVE LOADCIPCAST-IN-PLACELLH.LONG LEG HORIZONTALC.O.CLEANOUTLLV.LONG LEG VERTICALCONC.CONCRETEMAX.MAXIMUMCONT.CONTINUOUSMECH-ANICAL	DATE DS BULGEF * 7210 CO PROFESSIO.V42 2014/07/18 \$2 DATE * TE TO SO PROFESSIO.V42 2014/07/18 \$2 DATE * TE * 7 2014/07/18 \$2 C TE * TE * 7 2014/07/18 \$2 C TE * TE * 7 2014/07/18 \$2 C TE * * * * * * * * * * * * *
SA-S304.1, LATEST EDITION. JOINTS, IN ADDITION, COURSE ON FOOTINGS.	UNTIL ALL THE STRUCTURAL FRAMING AND DIAPHRAGMS ARE COMPLETED. 11. ALL NAILING REQUIREMENTS FOR ROOF AND EXTERIOR WALL SHEATHING SHALL CONFORM TO PART 9 OF THE NATIONAL BUILDING CODE OF CANADA (LATEST EDITION) UNLESS NOTED. 12. ALL WALL AND ROOF SHEATHING SHALL BE NAILED AT 100mm o/c AT EDGES AND 300mm o/c ON	PSL 2.0E (2000 KSI) 540 PSI 5500 PSI 1305 PSI LSL 1.5E (1500 KSI) 745 PSI 4200 PSI 1450 PSI BEAM DEFLECTIONS ARE TO BE LIMITED TO LIVE LOAD SPAN/360 AND TOTAL SPAN/240 TOTAL SPAN/240 TOTAL SPAN/240	CL.CLEARMIN.MINIMUMC.J.CONTROL JOINTMLMASONRY LINTELc/wCOMPLETE WITHNOM.NOMINALØDIAMETERN.T.S.NOT TO SCALEDIA.DIAMETERO.C.ON CENTRESD.L.DEAD LOAD.O.H.OVERHEADDNDOWN-ARROW POINTSPROP.PROPOSED	INGONISH CIVIC
WHERE ADJACENT TO ADDER TYPE 04.1, LATEST EDITION; FABRICATION TO ASTM	 INTERMEDIATE SUPPORT MEMBERS. 13. ALL ROOF RAFTERS AND TRUSSES SHALL BE ANCHORED DOWN BY HURRICANE UPLIFT CONNECTORS ON BOTH FACES OF THE RAFTERS AND TRUSSES. 14. ALL LUMBER EXPOSED TO THE EXTERIOR AS FINAL CONSTRUCTION SHALL BE PRESSURE TREATED. 15. ALL LOAD BEARING STUD WALLS SHALL BE FULLY BLOCKED AT 1/3 HEIGHT. (2 ROWS OF HORIZONTAL BLOCKING MIN.) 	 PSL - PARALLAM BEAM LSL - TIMBERSTRAND BEAM 5. COLUMNS SHALL BE PSL 2.0E BY WEYERHAEUSER OR PRE-APPROVED EQUIVALENT. 6. U.N.O. ON PLAN STEEL CONNECTING HARDWARE FOR PSL AND LSL BEAMS SHALL BE CAPABLE OF DEVELOPING 100% OF THE BEAM SHEAR CAPACITY. 7. PRODUCT SUBSTITUTIONS MUST BE PRE-APPROVED. 	TO LOWER ELEVATIONREF.REFER TODP.DEEPREINF.REINFORCMENTE.E.EACH ENDRDROOF DRAINE.F.EACH FACES.D.L.SUPERIMPOSED DEAD LOADELEC.ELECTRICALSIM.SIMILAREL.ELEVATIONS.J.SAWCUT JOINTE.S.EACH SIDES.O.G.SLAB ON GRADEEXIST.EXISTINGSPMDDSTANDARD PROCTOR	BUILDING INGONISH, NS DSRA JOB #: 12169 DRAWN BY: J.O.
ERALLY ALONG THE TOP HAVE MIN. 2- COURSE ND BOTTOM, GROUTED Imm O.C. (TYP.) U.N.O. ON	 16. ANY CHANGES TO THE FRAMING SHOWN ON THESE DRAWINGS SHALL HAVE PRIOR WRITTEN APPROVAL OF THE DEPARTMENTAL REPRESENTATIVE. FRAMING CHANGES WHICH HAVE NOT BEEN APPROVED WILL BE REJECTED. 17. CONFIRM ALL DIMENSIONS AND OUTLINES WITH THE ARCHITECTURAL DRAWINGS. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL DIMENSIONS, ELEVATIONS, AND DETAILS. 	 PRODUCT SUBSTITUTIONS MUST BE PRE-APPROVED. DO NOT SUBSTITUTE BUILT-UP MEMBERS OF SAWN TIMBER FOR ENGINEERED WOOD PRODUCTS. PARALLAMS USED IN EXTERIOR APPLICATIONS SHALL MEET THE EXPOSURE REQUIREMENTS SPECIFIED BY THE MANUFACTURER. DO NOT USE MICROLAMS. ALL E.W.P. SHALL BE KEPT DRY AND PROTECTED FROM THE ENVIRONMENT DURING STORAGE ON OR OFF THE PROJECT SITE AS PER THE MANUFACTURERS REQUIREMENTS. STORE MATERIAL 	EXT.EXTERIORMAXIMUM DRY DENSITYE.W.EACH WAYSSTSIMPSON STRONG TIEE.W.B.EACH WAY BOTTOMT & BTOP AND BOTTOME.W.T.EACH WAY TOPTHK.THICKFDFLOOR DRAINTYP.TYPICALFOUND.FOUNDATIONU/NUNLESS NOTED.FTG.FOOTINGU.N.OUNLESS NOTED OTHERWISEF.P.FIRE PROTECTIONV.B.VAPOUR BARRIER	CHECKED BY: D.B. SCALE: 1:100
ATER TANK STRUCTURAL RADE TO SUPPORT ON ORT TRANSITION IRY WALL TOLERANCES	 18. DO NOT COVER WOOD FRAMING WITH FINISHES UNTIL THE DEPARTMENTAL REPRESENTATIVE HAS COMPLETED A REVIEW OF THE FRAMING. PROVIDE 48 HOURS ADVANCE NOTIFICATION WHEN FRAMING REVIEWS ARE REQUIRED. 19. NOTCHING AND DRILLING OF THE STRUCTURAL ELEMENTS SHALL FOLLOW THE GUIDELINES SET FORTH IN THE NATIONAL BUILDING CODE PART 9, UNLESS OTHERWISE APPROVED IN WRITING BY THE DEPARTMENTAL REPRESENTATIVE. 	ELEVATED FROM GROUND AND WRAPPED TO SHED MOISTURE.	GALV. GALVANIZED VERT. VERTICAL (H) HIGHER w/o WITHOUT H & V HORIZONTAL AND VERTICAL WL WOOD LINTEL	STRUCTURAL NOTES

200

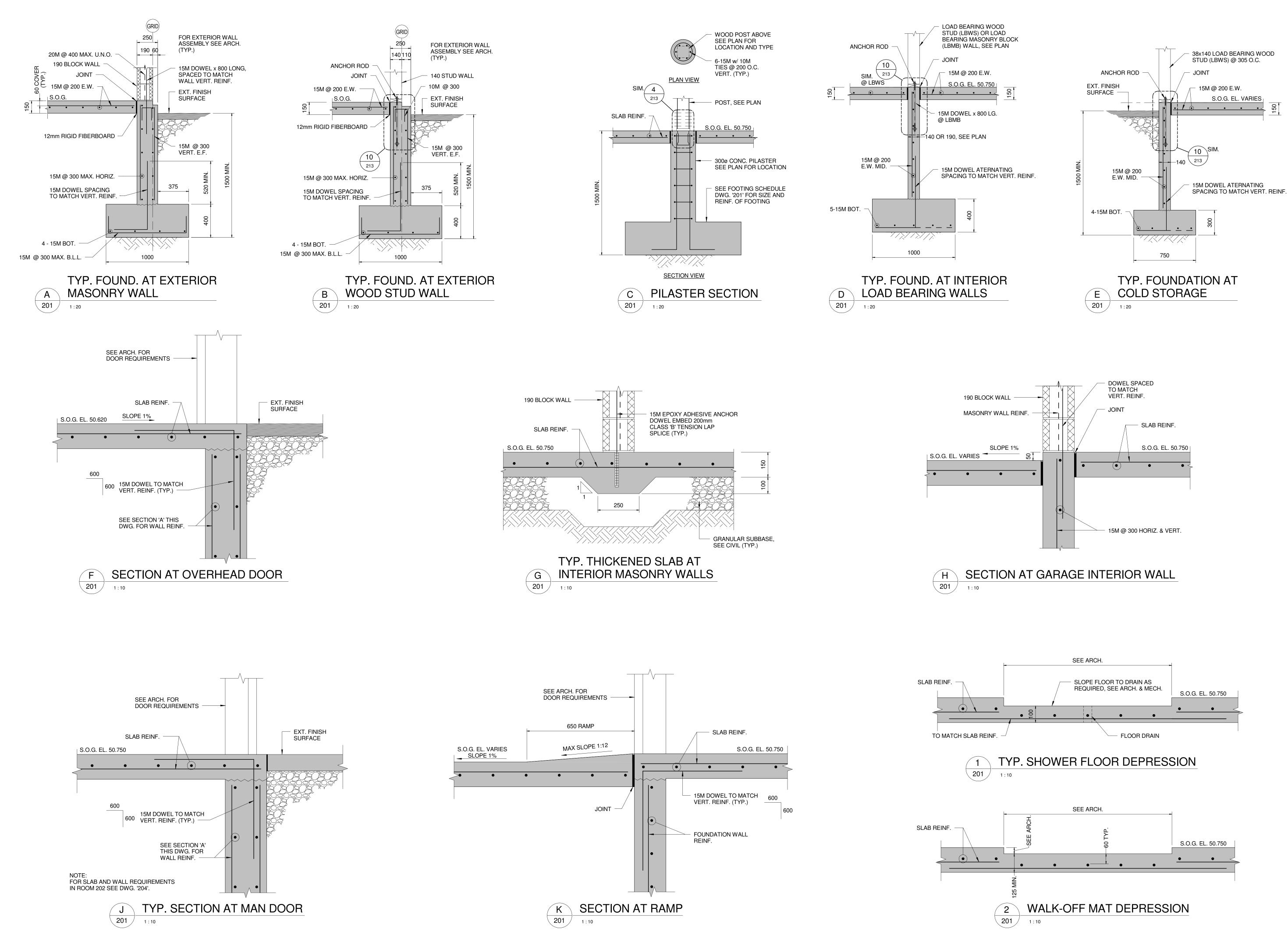




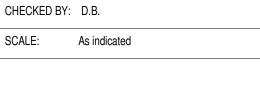


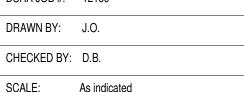
FOOTING SCHEDULE				
TYPE	SIZE	REINF.	REMARKS	
F1	1400x1400x400	20M @ 300 MAX. E.W. BOTTOM		
F2	1250x3200x400	20M @ 300 MAX. E.W. BOTTOM		

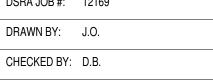
KEY PLAN	CAS	×		
	SNC + LAVALIN In Halifax, Nova Scotia, Member of the SNC-LAVA	IC. Canada		
DWG.	R TO MASONRY NOTES 200 FOR SLIDING DOC REMENTS.	S ON)R		
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FOUNDATION & SLAB ON GRADE PLAN				
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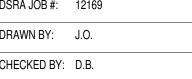


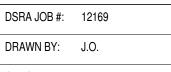












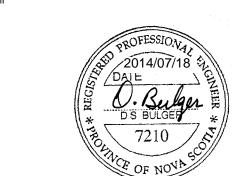


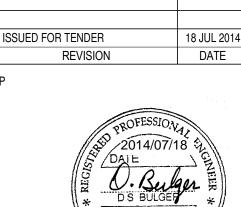


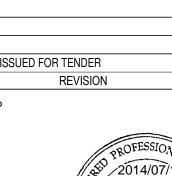


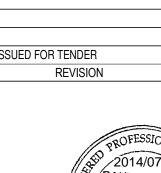


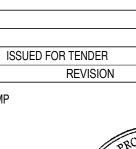


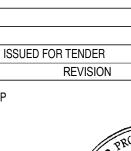


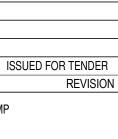


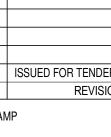


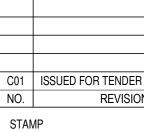


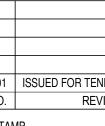


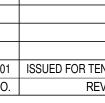


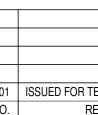


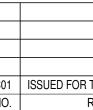


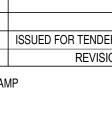


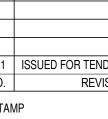


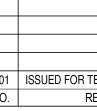


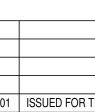


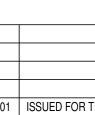


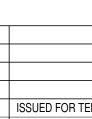


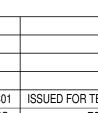


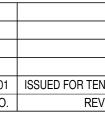


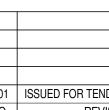


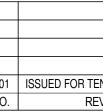


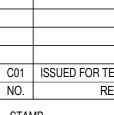


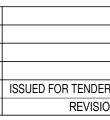


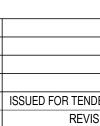


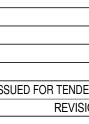


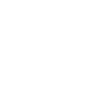












KEY PLAN

ISO 9001

NOTES

SNC · LAVALIN

Halifax, Nova Scotia, Canada

Member of the SNC-LAVALIN Group

SNC + LAVALIN Inc.

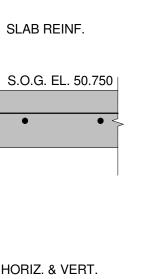
REFER TO ARCH. FOR EXTERIOR

WALL, AND UNDERSLAB INSULATION

AND UNDERSLAB VAPOUR BARRIER (TYP. ALL SECTIONS AND DETAILS)

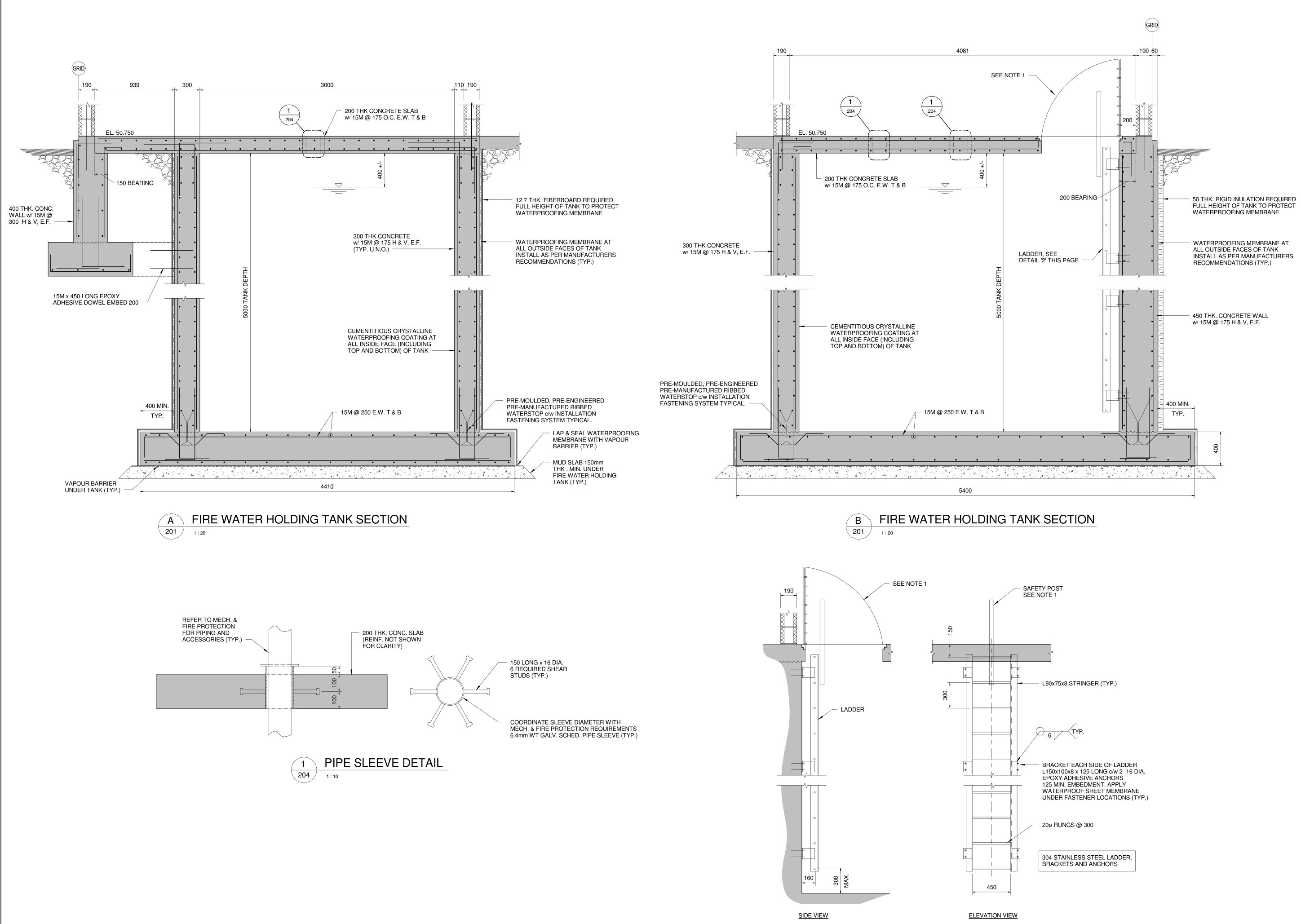
REFER TO PROJECT IFT DOCUMENTS FOR SUBGRADE CONCERNS AND

REQUIREMENTS (TYP.)





FOUNDATION SECTIONS AND DETAILS

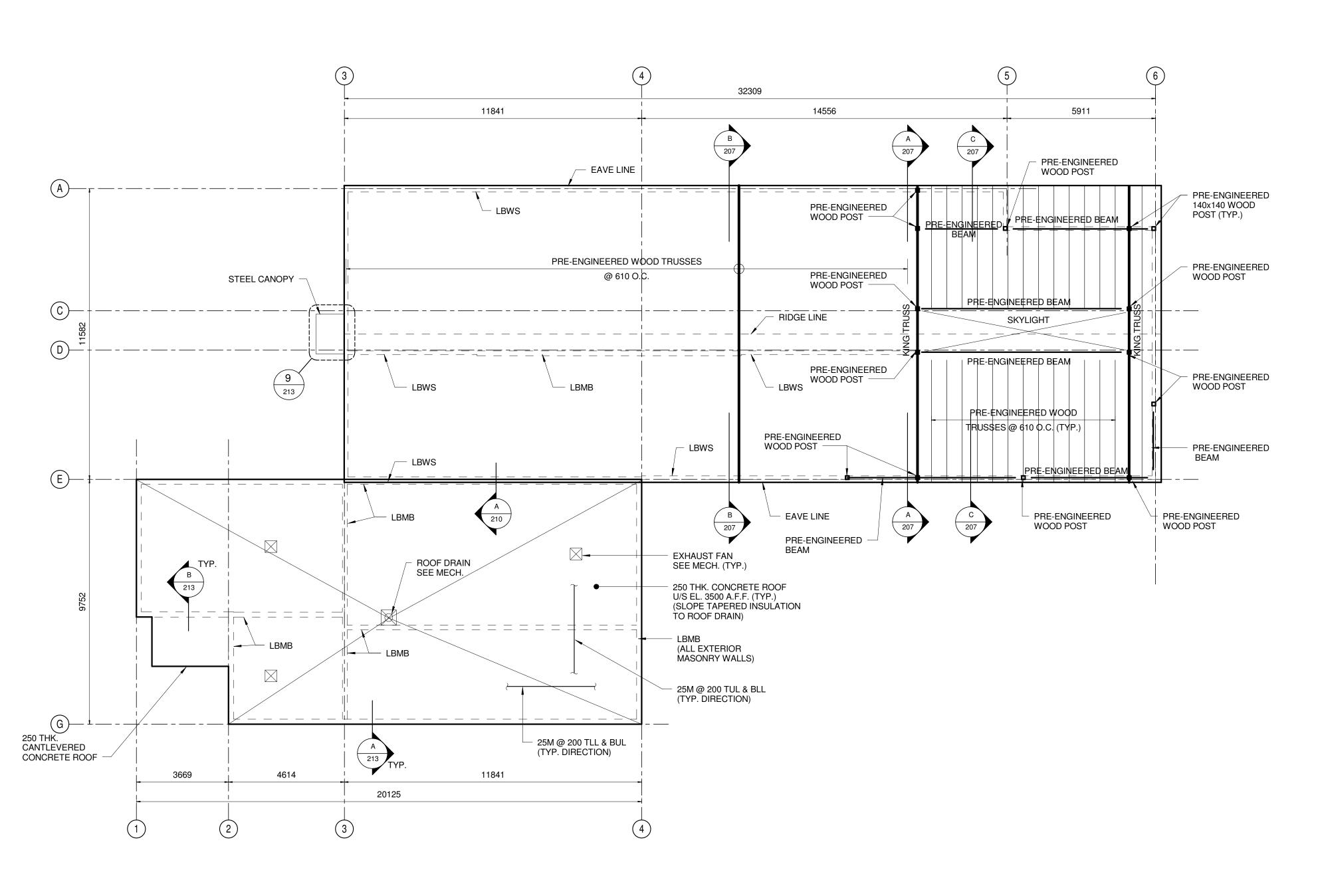


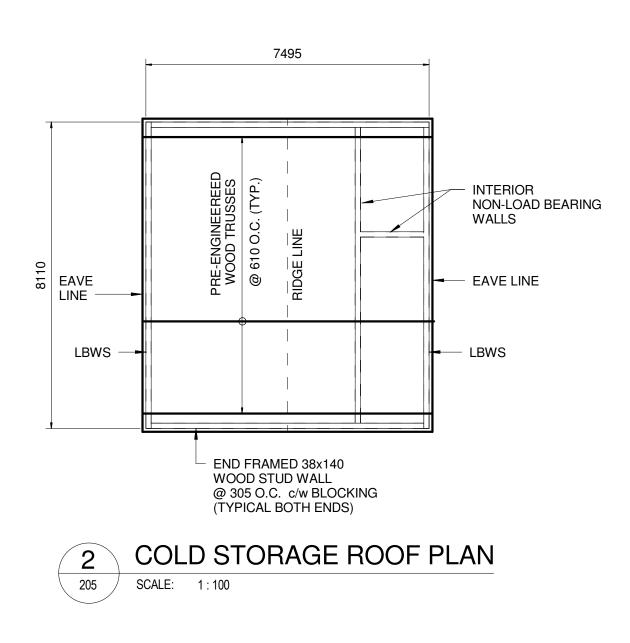
LADDER DETAIL

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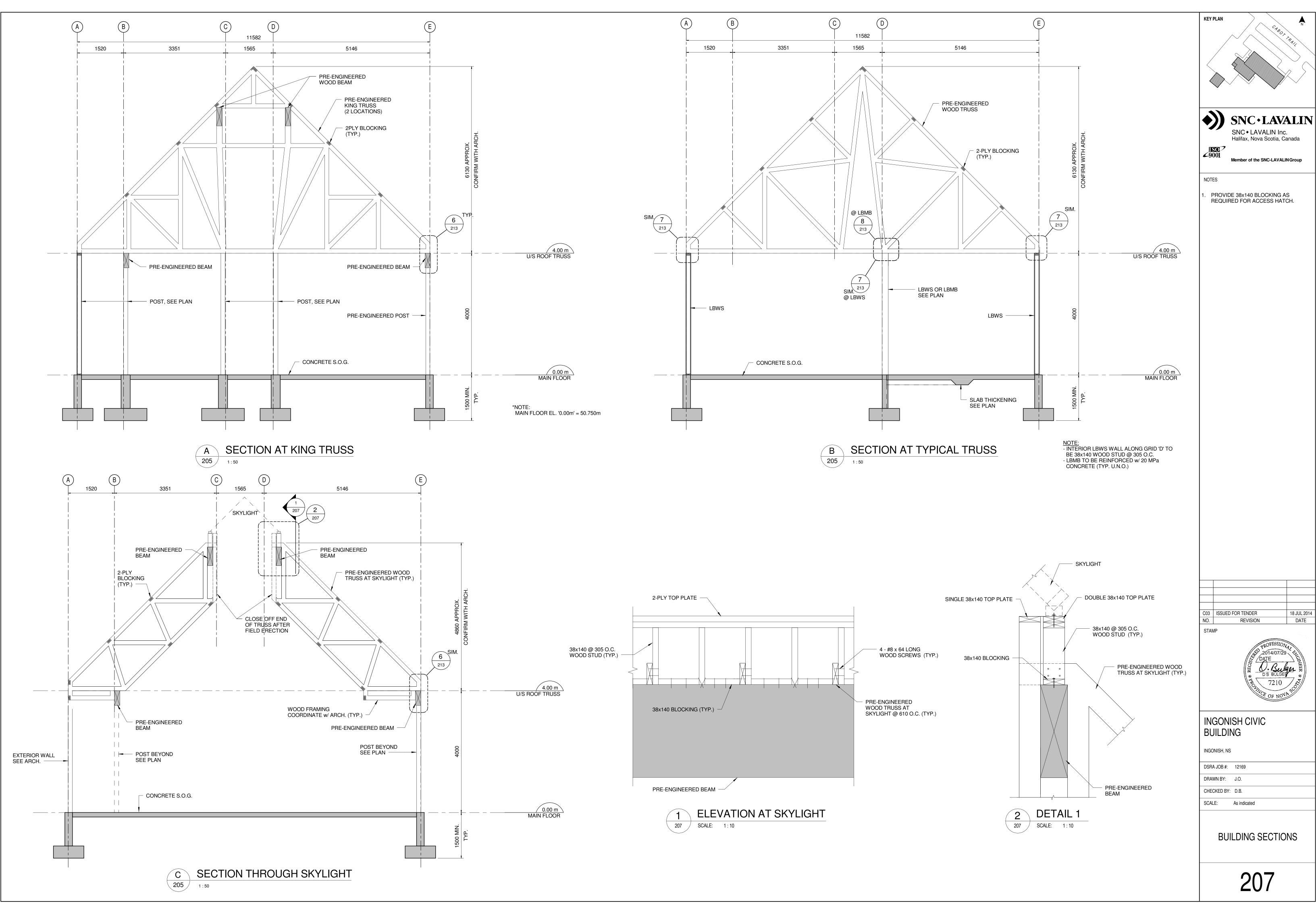
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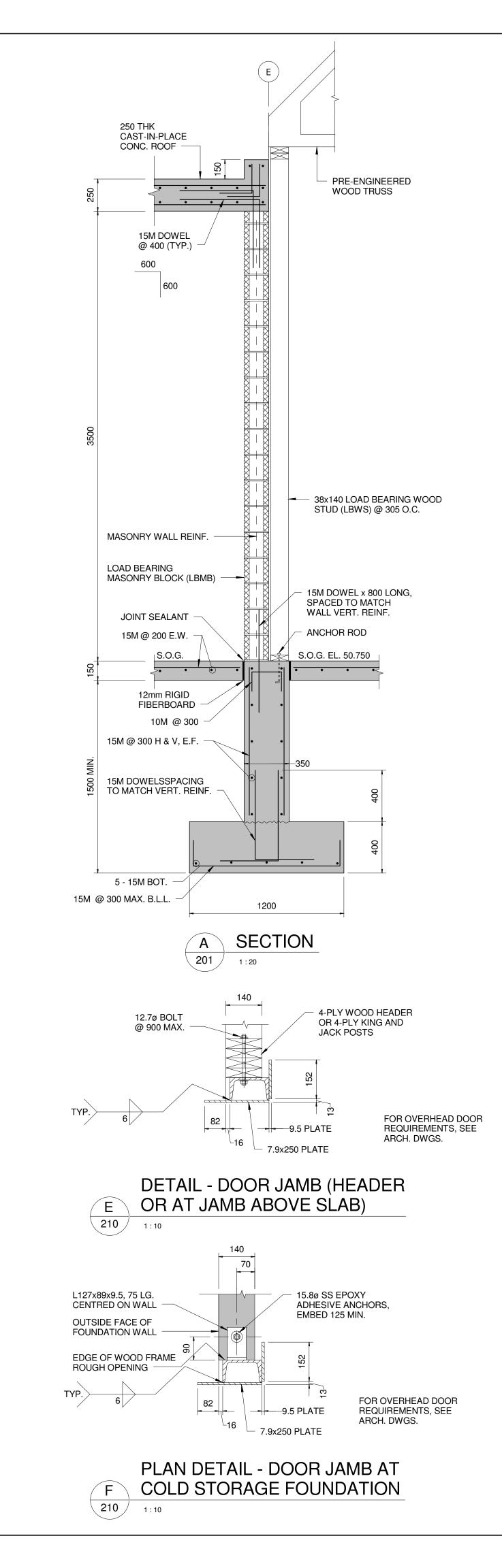


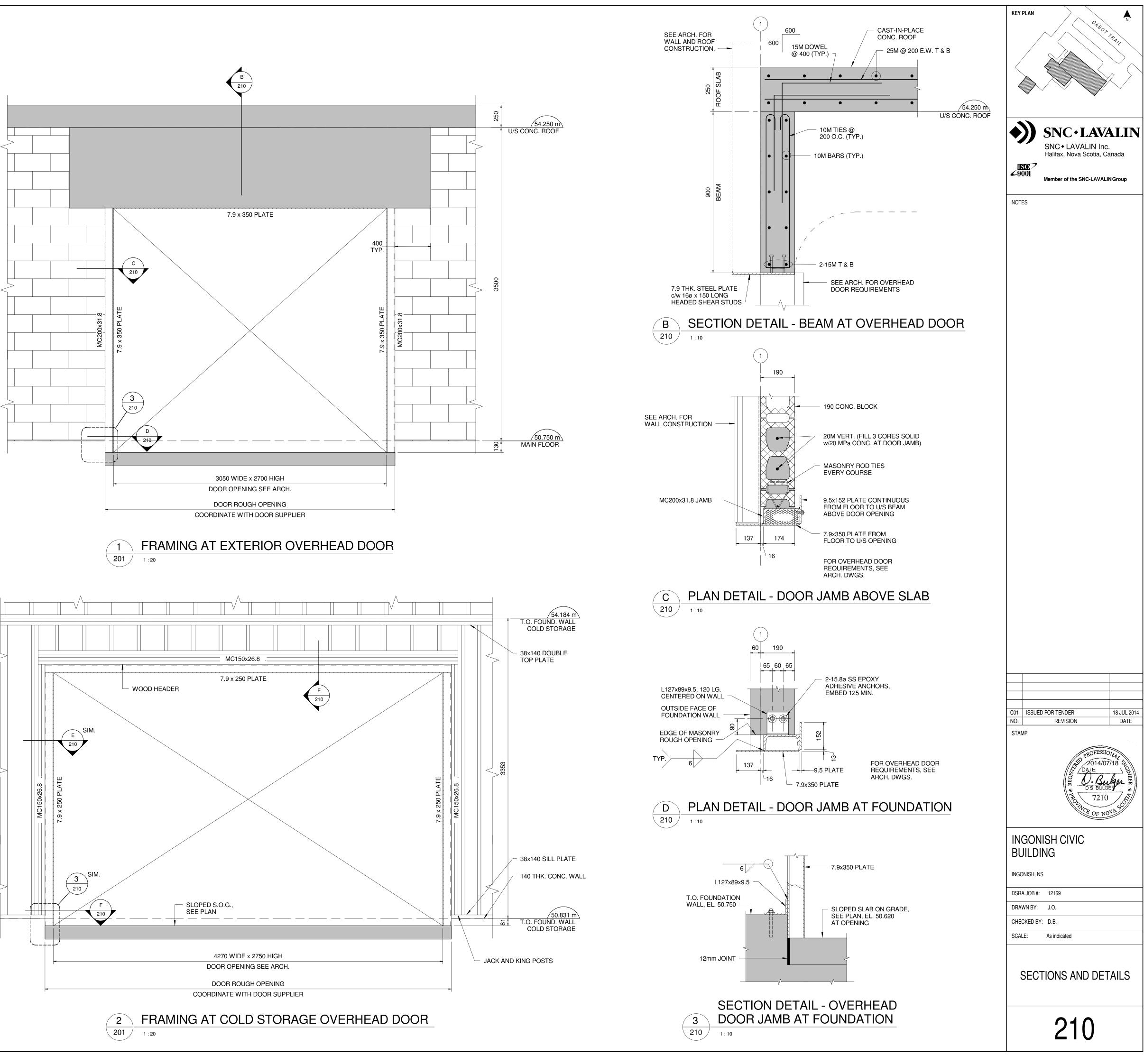


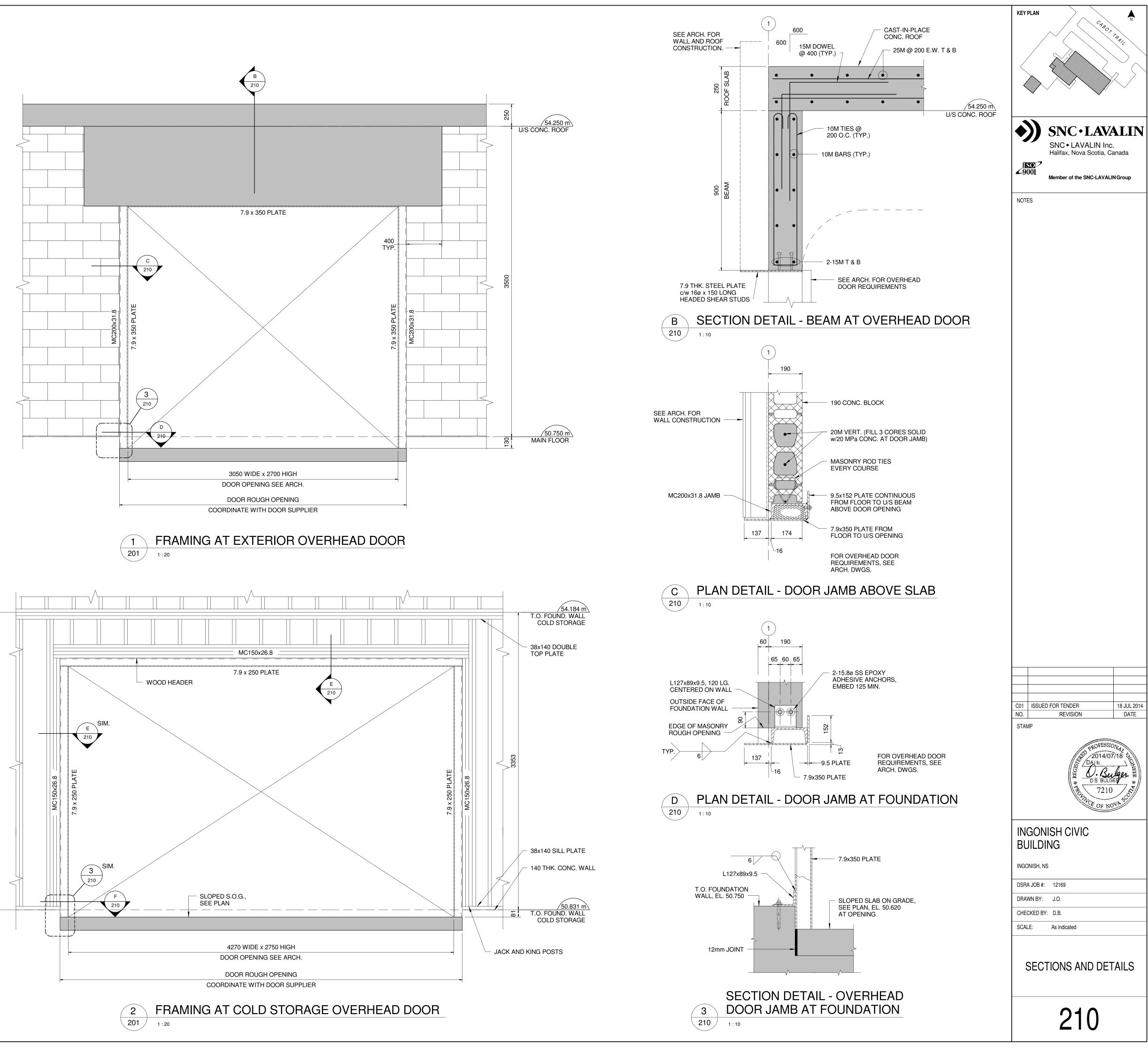


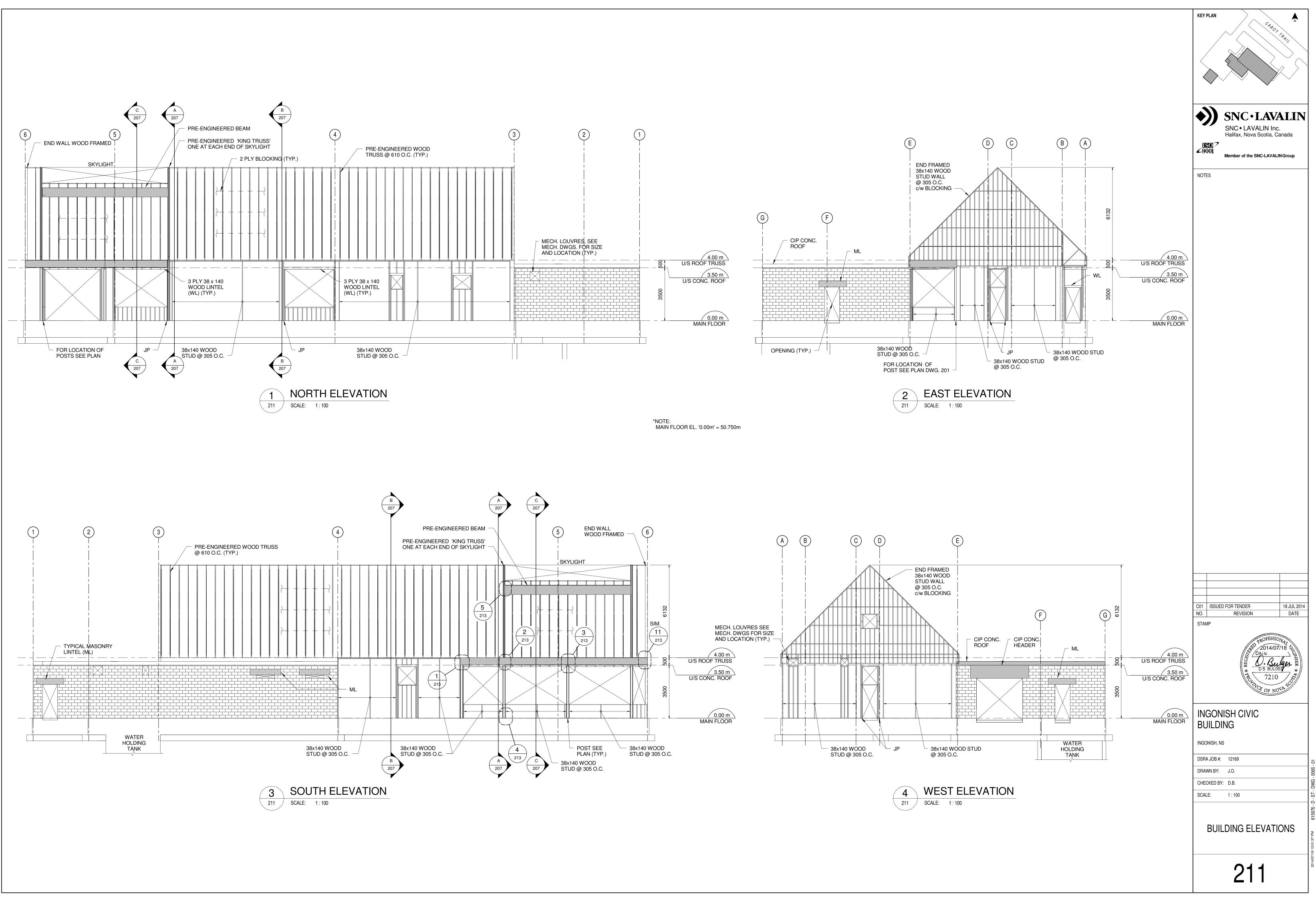
KEY PLAN	CABO,	▲ N
	SNC + LAVALIN Inc Halifax, Nova Scotia, Ca Member of the SNC-LAVALI	anada
DWG. 2	TO MASONRY NOTES 200 FOR SLIDING DOOF REMENTS.	ON
C01 ISSUEI) FOR TENDER REVISION	18 JUL 2014 DATE
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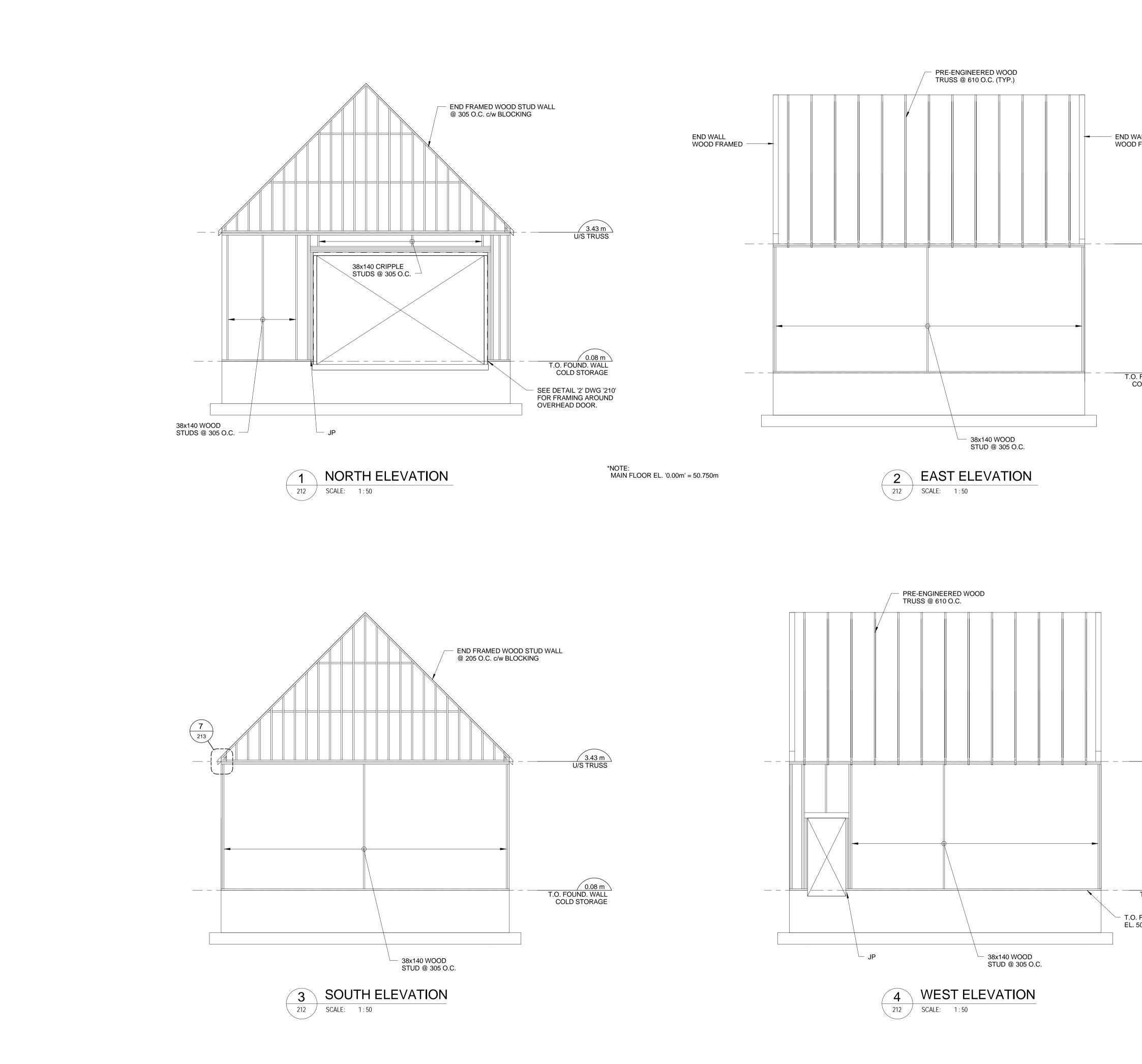












	KEY PLAN	
	KEY PLAN	
	SNC+LAVALIN	
	SNC + LAVALIN Inc.	
	Halifax, Nova Scotia, Canada	
ALL FRAMED	▲9001 Member of the SNC-LAVALIN Group	
	NOTES	
	NOTES	
<u> </u>		
0.08 m . FOUND. WALL OLD STORAGE		
OLDSTORAGE		
	C02ISSUED FOR TENDER18 JUL 2014NO.REVISIONDATE	
	STAMP	
<u>3.43 m</u> U/S TRUSS	D PROFESSIONAL	
0/5 18055	PROFESSION 42 2014/07/18 DATE DATE DS BULGEE ***********************************	
	E OSBULGER #	
	T210 CE NOVA	
	OF NO	
	INGONISH CIVIC	
	BUILDING	
	INGONISH, NS	
T.O. FOUND. WALL COLD STORAGE	DSRA JOB #: 12169	
FOUNDATION WALL 50.831 (TYP.)	DRAWN BY: J.O.	1000
	CHECKED BY: D.B.	0000
	SCALE: 1:50	
		010110
	BUILDING ELEVATIONS	•
	212	100

