

TO ALL PROPONENTS:

The tender specifications and drawings shall be amended and new clauses and sections added and/or revised to become part of the contract documents as follows:

1.0 TO THE SPECIFICATIONS:

Section 01 10 10 – General Instructions

1.2 Description

Revise 1.2.1.2 to state the following:

- .2 All excavation, dewatering, and backfilling as required. De-watering methodology to be submitted by the Contractor to the Departmental Representative for review 1 month prior to construction. Sediment laden water **will not** be permitted to discharge into harbor.
- .6 Installation of stormwater system including **de-watering**, catch basins, piping, stormwater treatment unit and discharge piping. **Construction methodology** for the stormwater infrastructure beneath the existing concrete wharf deck shall be submitted by the Contractor to the Departmental Representative for review 1 month prior to construction. The storm water pipe from the storm water treatment chamber to the outfall the pipe material may be changed to HDPE. Construction methodology shall limit disturbance to the existing concrete wharf structure. Existing wharf drawings are available for review.
- .8 Non-disturbance of the 1800mm dia. City Sanitary Discharge Piping that is located within the project limits.
- .18 Decommissioning and demolition of existing electrical building, select storm system, existing test well, select power system. This demolition work will occur after installation of the new systems are in operation or as permitted by the Departmental Representative.
- .24 Precast Concrete Engineered Retaining Wall Alternative
 - .1 Precast Concrete Engineered Retaining Walls are an acceptable alternative to the cast in place reinforced concrete retaining walls detailed on the project drawings. Walls shall be designed and constructed in accordance with Section 32 32 34 and main structural component will be concrete.
 - .2 Precast retaining wall shall be installed to alignments and extents indicated on drawings. Precast wall alignment shall be curved as required to match roadway alignment.
 - .3 Precast retaining wall alternative shall include installation of a Guide Rail system meeting the requirements of The City of St. John’s Department of Engineering Specifications Book, latest edition. Guide rail to be located between precast concrete wall and concrete curb, aligned along the west side of the driveway, to begin and end respective of the curb.
 - .4 Precast retaining wall supplier shall design and furnish all details required for attachment of handrail system and fencing at top of precast retaining wall.

.5 The precast supplier shall coordinate his precast blocks with the light standard locations. Slight location adjustments will be considered by Departmental Representative.

.6 All precast shop drawings shall be designed and stamped by Professional Engineer Licensed to practice in the Province of Newfoundland Labrador.

Section 01 22 00 – Measurement for Payment

1.2 Lump Sum Item

Revise 1.2.1.9 to state the following:

.9 Removal and Disposal of all contaminated soil displaced during the work from trenching and removal activities and all other surplus soil material. Contaminated soil cannot be reused as fill. Refer to Section 02 61 00.01 - Contaminated Soil Managements.

1.3 Unit Price Items

General: Delete this section entirely including .1 and .2.

Additional Items: Delete this section entirely including .1, .2 and .3; and all associated sub-section items.

Section 01 35 29 – Health and Safety Requirements

1.20 Blasting

Revise 1.20.1 to state the following:

.1 Notify Departmental Representative prior to any blasting.

Section 02 41 13 – Selective Site Demolition

1.6 Environmental Protection

Add section 1.6.3 to state the following:

.3 Prior to site demolition activities, existing monitoring wells and boreholes are to be decommissioned and removed. Monitoring wells and boreholes shall be filled to the top of the water table with an acceptable sealant such as bentonite grout (pellets or chips) sized no more than 1/4 of the well diameter. They shall be poured in the wells at a rate to prevent bridging.

Section 02 41 16 – Structure Demolition

3.3 Safety

Remove section 3.3.2.

Section 02 61 00.01 – Contaminated Soils

1.1 Summary

Revise 1.1.2 to state the following:

- .2 It is the intent that work performed as per this section will result in the disposal to a landfill or treatment facility licensed to receive polycyclic aromatic hydrocarbons (PAH), petroleum hydrocarbons (TPH), and metals impacted soil excavated at the Site. All soil less than 1 meter below ground surface (mbgs) will be stockpiled on site prior to disposal at a certified landfill facility to permit Departmental Representative to retest/confirm that these soils are able to go to the landfill. All soil greater than 1 mbgs must go to a treatment facility capable of treating soils that contain metals, PAH and hydrocarbons. Prior to removal of the impacted soil refer to Civil Drawings and the Remediation Action Plan provided in Appendix C.

3.3 Application

Add 3.3.3.6 as follows:

- .3 Soil excavation and management.
 - .6 Soils excavated less than 1 mbgs shall be temporarily stockpiled separately such that excavated soils from different areas on the site are not mixed together. The contractor must limit the stockpile sizes to no more than 50 cubic meters.

3.5 Method of Remediation

Add 3.5.2 and .3 as follows:

- .2 All soils less than 1 mbgs will be stockpiled on site to permit the Departmental Representative to collect samples for analysis to determine disposal requirements.
- .3 All soils excavated greater than the 1 mbgs must be sent to a certified treatment facility that can treat soils contaminated with TPH, PAH & metals.

Section 31 22 13 - Rough Grading

3.4 Stockpiling

Revise 3.4.1 to state the following:

- .1 Pile excavated fill separately, as approved by Departmental Representative, such that excavated soils from the different areas outlined in the Remediation Action Plan are not mixed in an effort to minimize treatment requirements.

Section 31 23 16 – Rock Removal

Replace existing section 31 23 16 in its entirety with Section 31 2316 – Rock Removal, attached to the addendum.

Section 31 23 33.01 – Excavating, Trenching and Backfilling

1.6 Existing Conditions

Revise 1.6.1 as follows:

- .1 No soil report has been completed for this site. Contractor to engage the services of a Geotechnical Engineer licensed to practice in the Province of Newfoundland Labrador to inspect all excavation bottoms to ensure they are suitable for all the structures and infrastructure intended, and to confirm that the subgrade meets or exceeds the requirements stated in Section 03 30 00 prior to placing granular or concrete materials.

2.1 Materials

Revise 2.1.2 as follows:

- .2 Bedding and Surround Material: Granular “A” Material in accordance with Item 323.02 of City of St. John’s Department of Engineering Specifications Book (latest edition).

2.2 Bedding and Surround Material

Delete all of Section 2.2.

3.4 Excavation

Revise 3.4.1 as follows:

- .1 Excavate to lines, grades, elevations and dimensions as indicated and as follows:
 - .1 All soils excavated less than 1 mbgs will be stockpiled on site to permit the Departmental Representative to collect samples for analysis to determine disposal requirements.

- .2 Soils excavated less than 1 mbgs shall be stockpiled separately such that excavated soils from different areas on the site are not mixed together. The contractor must limit the stockpile sizes to no more than 50 cubic meters.
- .3 All soils excavated greater than 1 mbgs must be sent to a certified treatment facility that can treat soils contaminated with TPH, PAH & metals.

Revise 3.4.13 as follows:

- .13 Correct unauthorized over-excavation as follows:
 - .1 Fill under bearing surfaces and footings with **Granular A** material fill compacted to a minimum of 100% of maximum dry density, to ASTM D 698.
 - .2 Fill under other areas with **Granular B** material fill compacted to a minimum **100%** of maximum dry density, to ASTM D 698.

3.5 Fill Types and Compaction

Revise 3.5.1.1 and .2 as follows:

- .1 Exterior side of perimeter walls: use Granular B material fill to subgrade level. Compact to 100% of corrected maximum dry density.
- .2 Within building area: use Granular A material to underside of floor slabs. Compact to 100 % of corrected maximum dry density.

Section 31 24 13 – Roadway Embankments

1.3 Definitions

Revise 1.3.1.2 as follows:

- .2 Boulder or rock fragments measuring in volume one half (0.5) cubic metre or more.

1.4 Regulatory Requirements

Revise 1.4.1.1 to state the following:

- .1 Adhere to regulations of authority having jurisdiction.

Section 31 33 26 – Rock Slope Stabilization

New section – reference attached section.

Section 32 12 16 – Asphalt Paving

1.3 References

Add 1.3.3 as follows:

.3 Government of Newfoundland and Labrador Department of Transportation and Work's Specifications Book.

3.1 Preparation

Delete 3.1.3 including 3.1.3.1 and 3.1.3.2 and renumber 3.1.4 to 3.1.3.

Section 32 32 13.13 – Packaged Sewage Lift Wet Well Type

.1 Reference item 2.1 Pump Station

.1 Add the following paragraph: “.2 Wet well and interior of conduits from wet well are classified as hazardous location Class I, Zone 2, Gr. IIA & IIB, T3 and Category 2 area (corrosive, hydrogen sulfide) with continuous ventilation. All installation shall be suitable for the area classification.

.2 Reference item 2.7 Submersible Motors

.1 Change motor and integral cable hazardous location rating from Class I, Zone 2, Gr. IIA & IIB, T3 to Class I, Zone 1, Gr. IIA & IIB, T3.

.3 Reference item 2.8 Pump Control System

.1 Change liquid level switches hazardous location rating from Class I, Zone 2, Gr. IIA & IIB, T3 to Class I, Zone 1, Gr. IIA & IIB, T3.

.2 Add the following paragraph: “.4 Provide interlock to activate an alarm and isolate from power fan when ventilation fails.”

.3 Reference item 2.10 Electrical Control Panel and Wiring

.1 Change equipment hazardous location rating from Class I, Zone 2, Gr. IIA & IIB, T3 to Class I, Zone 1, Gr. IIA & IIB, T3.

.2 Item 2.10.4.7: power outlet shall be 15 A, 120 V, GFCI duplex receptacle.

.2 Item 2.10.4.12: Revise clause to read: “Provide PLC for local control of lift station operation and, as required, RTU with communication module with serial, IP & BacNet ports. The installed equipment shall allow for local control/monitoring and future control/monitoring from Building Automation System.”

.3 Add the following paragraph: “.4.13 Interlock to activate an alarm and isolate from power fan when ventilation fails.”

.4 Reference item 2.11 Ventilation

- .1 Add the following paragraphs:
 - .3 Provide pressure switch in fan outlet to monitor air flow. Pressure switch shall send signal to station control system to initiate action outlined in item 2.8.4.
 - .4 Heater to be capable of maintaining a minimum temperature of 6° C at the outlet.
 - .5 Provide calculations confirming the 12 air changes per hour and the minimum temperature.

Section 32 32 34 – Segmental Retaining Walls

Replace this section with the attached section - **32 32 34 – Precast Concrete Engineered Retaining Wall**.

Section 33 05 13 – Manholes, Catch Basin, and Utility Structures

2.1 Materials

Revise 2.1.5 to state the following:

- .5 Joints: to be made watertight using NITRILE rubber rings.

2.2 Storm Water Treatment Units

Delete 2.2 and 2.2.1 entirely

3.3.14 Add this section. Section to include the following:

The catch basin units and storm water treatment unit are required to be emptied of water periodically for inspection and maintenance. The storm water treatment and all upstream manholes and catch basin units therefore must be anchored appropriately to address the risk of buoyancy (floatation) and to ensure that when empty, all units remains static. For design purposes the high water level shall be assumed to be at the storm water treatment chamber finish grade. Contractor shall engage and pay for the services of an Engineer licensed to practice in Newfoundland and Labrador to design the anti-floatation pad and associated tie-down method for the approved storm water treatment chamber, manholes, and catchbasins. Stamped Engineering Drawings c/w buoyancy calculations prepared by this Engineer shall be submitted for review.

Section 33 06 01 – Oil/Sediment Interceptor

2.1.2 General

Revise 2.1.2.1 as follows:

- .1 Maximum treatable flow is 0.0754 cu m/s from a combined area of 8,600 sq. m.

Add the following:

- .6a Shall be capable of treating storm water in submerged condition.
- .6b Shall be capable of withstanding buoyancy forces with groundwater assumed at finish grade and the storm water treatment unit empty.

Revise 2.2.7 as follows:

- .7 The following SWTU providers have been pre-approved:
 - .1 Stormceptor (OSR)
 - .2 Contech Engineered Solutions
 - .1 Continuous Deflection Separation Technology (CDS)
 - .3 Stormwater 360
 - .1 Vortechs

Add the following 2.2.8:

- .8 The SWTU manufacturer chosen shall submit calculations illustrating their system conforming to the requirements listed, and shall also supply documentation required by LEED v4.

2.3 Joints

Revise 2.3.1 to state the following:

- .1 The concrete joints shall be water-tight using NITRILE rubber rings and shall meet the design criteria according to CSA A257.4 latest edition.

Section 33 11 17 – Water Mains

2.2 Polyvinyl Chloride Pipe and Fittings

Revise 2.2.1 to state the following:

- .1 Pipe: shall conform to AWWA C900 and certified to CSA B137.3 for sizes 100mm to 300mm. Pipe shall be cast iron pipe equivalent OD's and be DR 18 (pressure class 235 psi). Pipe shall be nitrile gasket and bell type joint, be Factory Mutual, ULC and ULI approved and NSF-61 certified. Pipe shall be delivered to site complete with capped ends.

2.3 Hydrants

Revise 2.3.1 and 2.3.2 to state the following in **bold**:

- .1metal caps. Depth of Bury **1.8m** minimum to top of pipe **and suitable for a salt water environment**. Provide two hose
- .2draining type with a **1.8m** depth of bury **and suitable for a salt water**

environment.....

Add section 2.1.3 as follows:

- .3 Supply and install a separate check valve adjacent hydrant to match diameter of hydrant lead, if the hydrants, and post hydrant are not available with an internal check valve.

Add section 2.14 as follows:

2.14 Geo-Exchange Ball Valves

- .1 Buried: up to 100mm diameter, minimum pressure rating of 1035kPa
 - .1 Body: PVC
 - .2 Seals: EPDM
 - .3 Ends: Flanged
 - .4 Shaft Extension to be complete with waterproof PVC housing designed for buried service.
 - .5 Operating Nut: 50mm square
 - .6 Acceptable Material:
 - .1 Chemline Type 21
 - .2 Hayward
 - .3 SAHI / America

Section 33 21 00 – Water Supply Wells

Replace existing section 33 21 00 in its entirety with Section 33 21 00 – Water Supply Wells attached to the addendum.

Section 33 31 13 – Public Sanitary Utility Sewerage Piping

2.1 Plastic Pipe

Revise 2.1 to state the following:

- .1 Locked-in or Separate Nitrile gasket and integral bell system.

Delete the following:

- .2 Acrylonitrile - Butadiene - Styrene (ABS): to CSA B182.2.
- .3 Corrugated High Density Polyethylene (HDPE): to CSA B182.6.
 - .1 320 kPa 180 kPa pipe stiffness.
 - .2 Sewer Storm sewer class.
 - .3 Gasket and bell Mechanical non-gasket coupling system.

Section 33 34 00 – Sanitary Utility Sewerage Force Mains

2.1 Materials

Revise 2.1.1 to state the following:

- .1 Polyvinyl chloride (PVC) pipe: to CSA B137.3 .
 - .1 SDR: 18.
 - .2 Pressure Class: 1 MPa.
 - .3 Gasket bell end.
 - .4 Pipe joints: bell and spigot with Nitrile rubber gaskets.
 - .5 Nitrile Rubber gaskets: to ANSI/AWWA C111/A21.11.

Section 33 41 00 – Storm Utility Drainage Piping

2.1 Plastic Pipe

Revise 2.1 to state the following:

- .1 Type PSM Poly Vinyl Chloride (PVC): CAN/CSA-B182.2-M95.
 - .1 Standard Dimensional Ratio (SDR): 35.
 - .2 Locked-in or Separate Nitrile gasket and integral bell system.
 - .3 Nominal lengths: 4.0 m.

Add the following and revise numbering on following sections as required:

2.3 Outfall Pipe from Chambers, Manholes, or Catchbasins to Harbour discharge locations.

- .1 Polyethylene pipe:
 - .1 High Density to ASTM F714, DR 17, (O.D. 610 mm, ID 533 mm for pipe from Storm Water Treatment Unit).
 - .2 Polyethylene to polyethylene joints: to be thermal butt fusion joined, to ASTM D 2657.
 - .3 Cast iron fittings with flanged ends: to ANSI/AWWA C110/A21.10 for pipe size above NPS 4.

3.1 Preparation

Add the following 3.1.2

- .2 Construction methodologies for the stormwater infrastructure beneath the existing concrete wharf deck shall be submitted by the Contractor to the Departmental Representative for review 1 month prior to construction. The storm water pipe from the storm water treatment chamber to the outfall may be changed to HDPE. Construction methodology shall limit disturbance to the existing concrete wharf structure. Existing wharf drawings are available for review.

Section 02 41 13 clause 1.6.3

Revise to state the following: *“Prior to site demolition activities, existing monitoring well is to be decommissioned and removed. Monitoring well shall be fill to the top of the water table with an acceptable sealant such as bentonite grout (pellets or chips) sized no more than 1/4 of the well diameter. They shall be poured in the wells at a rate to prevent bridging.”*

2.0 TO THE DRAWINGS:

2.1 Civil Drawings:

- .1 Reference drawings containing manholes of all types (storm, sanitary, electrical, communication, etc.) or catch basin top elevations within the pavement area:
 - .1 Tops of all manholes and catch basins to be placed 40mm lower than indicated in order to have them flush with the first base lift of asphalt. Asphalt pavement spot elevations reference the top of the base lift of asphalt surface. The structure’s frames and covers will be adjusted vertically in the next tender phase prior to the application of the surface coat of asphalt which is not included in this contract.
 - .2 All subsurface chambers to be designed to address the risk of buoyancy (floatation) and to ensure that when empty, all units remains static. For design purposes the high water level shall be assumed to be at the structure finish grade. Contractor shall engage and pay for the services of a Professional Engineer licensed to practice in Newfoundland Labrador to design the anti-floatation pad and associated tie-down method, or alternate method for the catch basins. Stamped Engineering Drawings c/w buoyancy calculations will be prepared and submitted by designing engineer for review to Departmental Representative as shop drawings.
- .2 Reference drawing C02
 - .1 Existing Water Well #1 (WELL 1) is now indicated (**ref. CSK-13 enclosed**).
- .3 Reference drawing C03
 - .1 Delete from drawing, dashed lines representing future building, electrical building and future gate house.
- .4 Reference drawing C04

- .1 Revise note: “construction fencing and jersey barriers along here to be removed at contract completion”, to say “construction fencing and jersey barriers along here to remain”.
- .2 Add detail for fence and jersey barrier – MOVEABLE FENCE SECTION (**ref. CSK-12 enclosed**).
- .5 Reference drawing C05 (**ref. CSK-09 enclosed**).
 - .1 Clean-outs and 45 degree bends added to geoexchange piping.
 - .2 NCB3 to have 375mm PVC DR35 inlet stub c/w removable water tight cap, opposite outlet:
Invert at CB = -0.239m
Approx. 1m length @ 2.0%
 - .3 Change Storm Pipe Sizes as follows:

NCBMH1 to NSTMH1 - 375mm diameter
NCB3 to NSTMH1 - 375mm diameter
NSTMH1 to NSTMH3 - 525mm diameter
NSTMH3 to NSTMH2 (Treatment Chamber) - 525mm
NSTMH2 (Treatment Chamber) to Outfall - HDPE 610mm O.D. 533mm I.D.
 - .4 Remove all but 1 post hydrants and associated bollards. Move new fire hydrants, bollards, hydrant leads, and tees to previous location of post hydrant tapping sleeves. Extend hydrants, leads, bollards, and valves to 1.5m from the concrete deck (as per existing). Contractor to excavate existing hydrants and new hydrant type to match existing type, i.e. drain hole locations.
 - .5 Weeping tile for retaining wall to be connected [drain] to NCBMH1.
 - .6 SAN line in new driveway is a force main, FM.
 - .7 Blow-up Detail – New Water Configuration at Building
Add the following note:
 - .1 “Note: This figure does not illustrate all of the underground infrastructure required for the pumping station. Reference drawing C10 for additional details.”
 - .2 Water infrastructure that is N.I.C. clarified.
 - .8 At North East corner of the future building delete the requirement for drainage swale, NCB5, NSTMH4 and the associated lengths of 300 PVC pipe. Remove NCB5 and NSTMH4 references from the Structure Table, and the Structure Coordinate Table on C05, and on L01.

- .9 Entirely remove the 300 PVC Collection Pipe for Bldg Storm and Geoexchange Reject Water within the future building footprint to the outfall.

- .6 Reference drawing C07:
 - .1 Low tide (LNT) and high tide (HNT) to be shown on STORM PROFILE at approximately -0.9m and 0.7m respectively.
 - .2 Include 610mm dia. “duck bill” style check valve on end of 610mm OD HDPE outlet pipe from the storm water treatment unit. The check valve shall be capable of transmitting flows with minimal water level head differential.

- .7 Reference drawing C08
 - .1 Typical Driveway Section:
 - .1 Change “50mm Type C Asphalt” annotation to “50mm **Type B Asphalt**”.
 - .2 Change “150mm Class A Gravel” annotation to “150mm **Granular A**”.
 - .3 Change “450mm Class B Gravel” annotation to “450mm **Granular B**”;
 - .4 Add the following annotation: Bedrock surface is to be sloped to allow positive drainage of groundwater along its surface;
 - .2 The cast in place concrete pavement gutter constructed adjacent to the sidewalk shall be constructed to direct water flows in a gradual direction change from near the base of the slope towards the curb leading to the new stormwater catch basin (NCB1).
 - .3 Details provided for stabilization of cut rock, and approximate top of rock cut slope is delineated (**Ref. CSK-15 and CSK-14 enclosed**).
 - .4 Guide rail to City of St. John’s – Department of Engineering – Specifications Book. Guide Rail is required only when pre-cast retaining wall option is chosen. Guide rail to be located between concrete wall extension and concrete curb, aligned along the left side of the driveway, to begin and end respective of the curb.

- .8 Reference drawing C09
 - .1 Geoexchange pipe clean-out detail added (**ref. CSK-10 enclosed**).
 - .2 TYPICAL ASPHALT SECTION
 - .1 Change “150mm Class A Gravel” to “150mm Granular A”
 - .2 Change “450mm Class B Gravel” to “450mm Granular B”
 - .3 Change “50mm Type B Asphalt.....” to “50mm Base Type B Asphalt to

Government of Newfoundland and Labrador Dept. of Transportation and Work's specifications book,

.4 Remove "Type C Asphalt; Second Lift....." to "40mm Surface t"

.5 Add the following note at the bottom of the detail as follows: Where finished grade is >50mm below existing grade, treat additional excavated material as contaminated as per specifications. Where finished grade is above existing grade, place additional structural fill below this asphalt structure to the bottom of the specified 1m (from existing grade) excavation."

.3 Revise Trench Detail –

.1 Remove note 2.

.2 Revise 1600 min. dimension from finish grade to pipe top to **1800mm for Water**.

.4 Revise Geoexchange Well Detail (**Ref. CSK-11 enclosed**).

.5 Revise Typical Manhole detail to show hold-down slab and anchors (**Ref. CSK-16 enclosed**).

.6 Revise dimension on Typical Post Hydrant Detail from 1.2m to **1.8m** for dimension from finish grade to top of 50mm Type K Copper Pipe.

.7 Revise dimension on Typical Hydrant Installation Detail from 1520 Cover to **1800** Cover for dimension from finish grade to water main pipe.

.9 Reference drawing C10

.1 Pumping Station Detail -

Section 3 Panel and Vent Pipe Assembly:

.1 Remove the bolted base of the vent pipe (14) at finish grade.

.2 Add the following annotations:

.1 "Vent pipe shall be supported by the precast concrete control panel. Contractor shall coordinate the two (2) support brackets required with the precast concrete control panel manufacturer. Support brackets to be similar to the interior Pipe Support Detail 4."

.2 "Power supply to the Blower/Heater Fan is not indicated. Supply power to the Blower/Heater Fan as required from the Control Panel."

.3 Change Station Parts List Table Item 14 to state "150 mm schedule 40 Galvanized".

- .4 Change Station Parts List Table Item 28 to state “External Blower/Heater Fan”.

2.2 Structural Drawings:

- .1 Reference drawing S01
 - .1 West (outer) Retaining Wall.
 - .1 Contractor to make allowances for modification of configuration of the 7.132 meters West retaining wall at the north end near the bottom of the roadway. The final configuration will be provided during construction by Departmental Representative.
 - .2 Curvature in west wall to be removed and re-aligned in straight segments. Delete note indicated at west wall “CURVATURE OF WALL FOLLOWS ROAD ALIGNMENT. REFER TO CIVIL DRAWINGS FOR ALIGNMENT DETAILS”. This is for cast in-place wall only, precast option shall follow alignment.

2.3 Electrical Drawings:

- .1 Reference drawing E01.
 - .1 Legend: it should be noted that all items (e.g. equipment, wiring, etc.) drawn with faded line are existing to remain.
- .2 Reference drawing E03.
 - .1 Remove word “FUTURE” from note 6 and associated leader’s note to conduits from manhole ‘EP7’ (geothermal wells are provided in this Contract).
 - .2 Geothermal well fed from manhole ‘EP9’ is future. Watertight capped associated conduits at the proposed location.
 - .3 Add the following wording to note 2: “Contractor to pay for all associated utilities (power and communication) costs.”
- .3 Reference drawing E04, Sections ‘N’ and ‘O’
 - .1 Remove word “FUTURE” referring to geothermal wells (they are provided in this Contract).

- .4 Reference drawing E06, Power Distribution Single Line Diagram.
 - .1 Main Switchboard 'MSB1', revise fault duty rating as follows:
 - .1 Bus bracing, main circuit breaker, 400 A circuit breakers – 50 kA,
 - .2 Distribution circuit breakers from 15 A to 60 A – 25 kA.
 - .2 400 A switches and receptacles are existing to remain. This Contractor to provide new wiring with terminations to the existing switches.
- .5 Reference drawing E09.
 - .1 Provide two (2) 21 mm liquid-tight metal conduits from Lift Station Control Panel to Blower assembly: one for power and another for control. All fittings shall be water-tight.
 - .2 Provide wiring in accordance with manufacturer's recommendations and Specification Section 26 05 21. Terminate wiring and verified blower operation including alarm signals.

3.0 GENERAL INFORMATION:

- .1 Utility contacts:
 - .1 NL Power Inc. – Stuart White at swhite@newfoundlandpower.com.
 - .2 Rogers (Cable TV) – Nelson Morgan at Nelson.Morgan@rci.rogers.com.
 - .3 BellAliant (Telephone / Internet) – Tara Power at Tara.Power@bellaliant.ca.
- .2 Attached report for information purposes:
 - .1 Phase II Environmental Site Assessment prepared by Stantec Consulting Ltd, May 06, 2013

END OF ADDENDUM NO. 2