

PART 1 - GENERAL

- 1.1 Related Sections
- .1 Refer to other Specification Sections for related information.
 - .2 Refer to Section 01 33 00 for Shop Drawings/Submission requirements.
- 1.2 References
- .1 Do concrete formwork and falsework in accordance with CSA standard A23.1 94 (or latest edition). Concrete Materials and Methods of Concrete Construction, except where stricter standards specify otherwise.
 - .2 CSA S269.1 1975 (or latest edition), Falsework for Construction Purposes.
- 1.3 Submissions
- .1 Shop Drawings:
 - .1 Upon receipt, submit to Departmental Representative for review four (4) sets of formwork and falsework shop drawings, in accordance with Section 01 33 00, at least four(4) weeks prior to erection. All such drawings to be stamped and signed by a professional Engineer registered in the Province of Nova Scotia.
 - .2 Clearly indicate method and schedule of construction, materials, arrangement of joints, ties, shores, liners, and locations of temporary embedded parts. Comply with CSA S269.1 for falsework drawings.
 - .2 Product Data/Samples:
 - .1 Provide product data and samples for form ties.
 - .3 Provide submissions in accordance with Section 01 33 00
- 1.4 Measurement for Payment
- .1 This item will not be measured separately.

PART 2 - PRODUCTS

- 2.1 MATERIALS
- .1 Formwork lumber: plywood and wood framework
 - .2 Falsework materials: to CSA S269.1.
 - .3 Form stripping agent: colourless mineral oil, free of kerosene, with viscosity between 70 and 110 Saybolt Universal, 15 to 14 mm²/s at 40DC, flash point minimum 150DC, open cup.

2.1 MATERIALS
(Cont'd)

- .4 Form ties: removable or snap off metal ties, fixed or adjustable length, free of devices leaving holes larger than 1" dia in concrete surface. When forms are removed, no metal will be less than 1" from the surface of the concrete.

PART 3 - EXECUTION

3.1 Erection

- .1 Verify lines, levels before proceeding with formwork and ensure dimensions agree with drawings.
- .2 Construct forms to produce finished concrete conforming to shape, dimensions, locations and levels indicated within tolerances required by CSA A23.1
- .3 Line forms with material only as approved by Departmental Representative.
- .4 Construct falsework in accordance with CSA S269.1.
- .5 Align form joints and make watertight. Keep form joints to minimum.
- .6 Use 1" chamfer strips on external corners.
- .7 Clean formwork in accordance with CSA A23.1, before placing concrete.
- .8 Leave formwork in place for at least seven(7) days, exclusive for days when temperature falls below 5°C, unless otherwise directed by Departmental Representative.
- .9 Reuse of formwork and falsework subject to requirements of CSA A23.1
- .10 All holes form ties and rods to be plugged with mortar to requirements of CSA A23.1. When forms are removed, no metal will be less than 1" from the surface of the concrete.

PART 1 - GENERAL

1.1 PRICE AND
PAYMENT PROCEDURES

- .1 Measurement and Payment:
 - .1 Measure reinforcing steel in tonnes of steel incorporated into Work, computed from theoretical unit mass specified in CSA-G30.18 for lengths and sizes of bars as indicated or authorized in writing by Departmental Representative.
 - .2 No measurement will be made under this Section.
 - .1 Include reinforcement costs in items of concrete work in Section 03 30 00 - Cast-In-Place Concrete.

1.2 REFERENCES

- .1 American Concrete Institute (ACI)
 - .1 SP-66-04, ACI Detailing Manual 2004.
 - .1 ACI 315-99, Details and Detailing of Concrete Reinforcement.
 - .2 ACI 315R-04, Manual of Engineering and Placing Drawings for Reinforced Concrete Structures.
- .2 ASTM International
 - .1 ASTM A 82/A 82M-07, Standard Specification for Steel Wire, Plain, for Concrete Reinforcement.
 - .2 ASTM A 143/A 143M-07, Standard Practice for Safeguarding Against Embrittlement of Hot-Dip Galvanized Structural Steel Products and Procedure for Detecting Embrittlement.
 - .3 ASTM A 185/A 185M-07, Standard Specification for Steel Welded Wire Reinforcement, Plain, for Concrete.
 - .4 ASTM A 775/A 775M-07b, Standard Specification for Epoxy-Coated Reinforcing Steel Bars.
- .3 CSA International
 - .1 CSA-A23.1-09/A23.2-09, Concrete Materials and Methods of Concrete Construction/Test Methods and Standard Practices for Concrete.
 - .2 CSA-A23.3-04, Design of Concrete Structures.
 - .3 CSA-G30.18-09, Carbon Steel Bars for Concrete Reinforcement.
 - .4 CSA-G40.20/G40.21-04(R2009), General Requirements for Rolled or Welded Structural Quality Steel/Structural Quality Steel.
 - .5 CAN/CSA-G164-M92(R2003), Hot Dip Galvanizing of Irregularly Shaped Articles.
 - .6 CSA W186-M1990(R2007), Welding of Reinforcing Bars in Reinforced Concrete Construction.
- .4 Reinforcing Steel Institute of Canada (RSIC)
 - .1 RSIC-2004, Reinforcing Steel Manual of Standard Practice.

1.3 ACTION AND
INFORMATIONAL
SUBMITTALS

- .1 Submit in accordance with Section 01 33 00 - Submittal Procedures.

1.3 ACTION AND
INFORMATIONAL
SUBMITTALS
(Cont'd)

- .2 Prepare reinforcement drawings in accordance with RSIC Manual of Standard Practice and ACI 315.
- .3 Shop Drawings:
 - .1 Submit drawings stamped and signed by professional engineer registered or licensed in Nova Scotia of Canada.
 - .1 Indicate placing of reinforcement and:
 - .1 Bar bending details.
 - .2 Lists.
 - .3 Quantities of reinforcement.
 - .4 Sizes, spacings, locations of reinforcement and mechanical splices if approved by Departmental Representative, with identifying code marks to permit correct placement without reference to structural drawings.
 - .5 Indicate sizes, spacings and locations of chairs, spacers and hangers.
 - .2 Detail lap lengths and bar development lengths to CSA-A23.3, unless otherwise indicated.
 - .1 Provide type A tension lap splices where indicated unless otherwise indicated.
 - .4 When Chromate solution is used as replacement for galvanizing non-prestressed reinforcement, provide product description for review by Departmental Representative prior to its use.

1.4 QUALITY
ASSURANCE

- .1 Submit in accordance with Section 01 45 00 - Quality Control and as described in PART 2 - SOURCE QUALITY CONTROL.
 - .1 Mill Test Report: upon request, provide Departmental Representative with certified copy of mill test report of reinforcing steel, minimum 4 weeks prior to beginning reinforcing work.
 - .2 Upon request submit in writing to Departmental Representative proposed source of reinforcement material to be supplied.

1.5 DELIVERY,
STORAGE AND
HANDLING

- .1 Deliver, store and handle materials in accordance with manufacturer's written instructions.
- .2 Delivery and Acceptance Requirements: deliver materials to site in original factory packaging, labelled with manufacturer's name and address.
- .3 Storage and Handling Requirements:
 - .1 Store materials off ground and in accordance with manufacturer's recommendations in clean, dry, well-ventilated area.
 - .2 Replace defective or damaged materials with new.
- .4 Develop Waste Reduction Workplan related to Work of this Section.

PART 2 - PRODUCTS

2.1 MATERIALS

- .1 Substitute different size bars only if permitted in writing by Departmental Representative.
- .2 Reinforcing steel: billet steel, grade 400, deformed bars to CSA-G30.18, unless indicated otherwise.
- .3 Reinforcing steel: weldable low alloy steel deformed bars to CSA-G30.18.
- .4 Cold-drawn annealed steel wire ties: to ASTM A 82/A 82M.
- .5 Deformed steel wire for concrete reinforcement: to ASTM A 82/A 82M.
- .6 Welded steel wire fabric: to ASTM A 185/A 185M.
 - .1 Provide in flat sheets only.
- .7 Welded deformed steel wire fabric: to ASTM A 82/A 82M.
 - .1 Provide in flat sheets only.
- .8 Epoxy Coating of non-prestressed reinforcement: to ASTM A 775/A 775M.
- .9 Galvanizing of non-prestressed reinforcement: to CAN/CSA-G164, minimum zinc coating 610 g/m².
 - .1 Protect galvanized reinforcing steel with chromate treatment to prevent reaction with Portland cement paste.
 - .2 If chromate treatment is carried out immediately after galvanizing, soak steel in aqueous solution containing minimum 0.2% by weight sodium dichromate or 0.2% chromic acid.
 - .1 Temperature of solution equal to or greater than 32 degrees and galvanized steels immersed for minimum 20 seconds.
 - .3 If galvanized steels are at ambient temperature, add sulphuric acid as bonding agent at concentration of 0.5% to 1%.
 - .1 In this case, no restriction applies to temperature of solution.
 - .4 Chromate solution sold for this purpose may replace solution described above, provided it is of equivalent effectiveness.
 - .1 Provide product description as described in PART 1 - ACTION AND INFORMATIONAL SUBMITTALS.
 - .10 Chairs, bolsters, bar supports, spacers: to CSA-A23.1/A23.2.
 - .11 Mechanical splices: subject to approval of Departmental Representative.
 - .12 Plain round bars: to CSA-G40.20/G40.21.

2.2 FABRICATION

- .1 Fabricate reinforcing steel in accordance with CSA-A23.1/A23.2 ACI 315 and Reinforcing Steel Manual of Standard Practice by the Reinforcing Steel Institute of Canada.
 - .1 ACI 315R unless indicated otherwise.
- .2 Obtain Departmental Representative's written approval for locations of reinforcement splices other than those shown on placing drawings.
- .3 Upon approval of Departmental Representative, weld reinforcement in accordance with CSA W186.
- .4 Ship bundles of bar reinforcement, clearly identified in accordance with bar bending details and lists.
 - .1 Ship epoxy coated bars in accordance with ASTM A 775A/A 775M.

2.3 SOURCE QUALITY CONTROL

- .1 Upon request, provide Departmental Representative with certified copy of mill test report of reinforcing steel, showing physical and chemical analysis, minimum 4 weeks prior to beginning reinforcing work.
- .2 Upon request inform Departmental Representative of proposed source of material to be supplied.

PART 3 - EXECUTION

3.1 PREPARATION

- .1 Galvanizing to include chromate treatment.
 - .1 Duration of treatment to be 1 hour per 25 mm of bar diameter.
- .2 Conduct bending tests to verify galvanized bar fragility in accordance with ASTM A 143/A 143M.

3.2 FIELD BENDING

- .1 Do not field bend or field weld reinforcement except where indicated or authorized by Departmental Representative.
- .2 When field bending is authorized, bend without heat, applying slow and steady pressure.
- .3 Replace bars, which develop cracks or splits.

3.3 PLACING REINFORCEMENT

- .1 Place reinforcing steel as indicated on placing drawings and in accordance with CSA-A23.1/A23.2.
- .2 Use plain round bars as slip dowels in concrete.
 - .1 Paint portion of dowel intended to move within hardened concrete with one coat of asphalt paint.
 - .2 When paint is dry, apply thick even film of mineral lubricating grease.

3.3 PLACING
REINFORCEMENT
(Cont'd)

- .3 Prior to placing concrete, obtain Departmental Representative's approval of reinforcing material and placement.
- .4 Ensure cover to reinforcement is maintained during concrete pour.
- .5 Protect epoxy and paint coated portions of bars with covering during transportation and handling.

3.4 FIELD TOUCH-UP

- .1 Touch up damaged and cut ends of epoxy coated or galvanized reinforcing steel with compatible finish to provide continuous coating.

3.5 CLEANING

- .1 Progress Cleaning: clean in accordance with Section 01 74 11 - Cleaning.
 - .1 Leave Work area clean at end of each day.
- .2 Final Cleaning: upon completion remove surplus materials, rubbish, tools and equipment in accordance with Section 01 74 11 - Cleaning.
- .3 Waste Management: separate waste materials for reuse and recycling.

PART 1 - GENERAL

- 1.1 Related Work
- .1 Refer to other Specification Sections for related information on aggregates, form work and false work, concrete reinforcement, paint, miscellaneous items.
 - .2 Refer to Section 01 33 00 for Shop Drawings/Submissions requirements.
- 1.2 Reference Standards
- .1 Do structural concrete work in accordance with CSA A23.1-94 (or latest edition), Concrete Materials and Methods of Concrete Construction, except where stricter standards specify otherwise.
 - .2 CAN/CSA-A5/A8/A362-93 (or latest edition), Portland Cement/Masonry Cement/Blended Hydraulic Cement.
 - .3 CSA A23.5-M86 (R1992) (or latest edition), Supplementary Cementing Materials.
 - .4 ASTM C494-92 (or latest edition), Chemical Admixtures for Concrete.
- 1.3 Submissions
- .1 Shop Drawings:
 - .1 Upon request, submit shop drawings and erection drawings for formwork and falsework. All such drawings to be stamped and signed by a Professional Engineer registered in the Province of Nova Scotia.
 - .2 Upon request, submit placement drawings for reinforcing steel.
 - .3 Upon request, submit placement drawings for miscellaneous items.
 - .2 Product Data/Sample:
 - .1 Provide technical data and/or samples for curing compounds (winter/summer green/white/red), evaporation retardant and finishing aids, expansion joint materials/sealants, grouts.
 - .3 Certificates:
 - .1 Minimum four weeks prior to starting concrete work submit to Departmental Representative manufacturer's test data and certification by qualified independent inspection and testing laboratory that the following materials will meet specified requirements:
 - .1 Portland cement
 - .2 Admixtures
 - .2 Provide certification that plant, equipment, and materials to be used in concrete work comply with requirements of CSA A23.1
 - .3 Provide certification that mix proportions selected will produce concrete of specified quality and yield and that strength will comply with CSA A23.1

-
- 1.3 Submissions (Cont'd)
- .3 (Cont'd)
 - .4 Provide certification that concrete will not include alkali reactivity aggregates.
 - .4 Methodology:
 - .1 Submit methodology for cold weather concreting.
 - .2 Submit methodology for hot weather concreting.
 - .3 Submit methodology for concrete placement operations.
 - .4 Submit methodology for concrete deck finishing operations.
 - .5 Submit methodology for supporting reinforcing steel.
 - .5 Test Results:
 - .1 Provide design mix tests results.
 - .2 Provide mill test certificates for reinforcing steel.
- 1.4 Storage of Materials
- .1 Store all materials to prevent contamination or deterioration, whether at the plant or at the job site.
 - .2 Store cement in watertight bins or silos that provide protection from dampness and easy access for inspection and identification of each shipment whether at the plant or at the job site.
 - .3 Prevent stored liquid admixtures and compounds from freezing and powdered admixtures and compounds from absorbing moisture.
- 1.5 Source Sampling
- .1 At least 4 weeks prior to commencing work, inform Departmental Representative of proposed source of aggregates and provide access for sampling.
- 1.6 Ready-Mix Concrete Supply
- .1 Provide, with each load of concrete delivered to site, duplicate delivery slips containing following:
 - .1 Name of ready-mix batch plant
 - .2 Serial number of ticket
 - .3 Date and truck number
 - .4 Project identification
 - .5 Class of concrete or mix
 - .6 Amount of concrete in cubic metres
 - .7 Time of loading or first mixing of aggregate, cement and water.
 - .8 Time of discharge of concrete
 - .9 Admixtures added at plant
 - .10 Amount of water added at plant
- 1.7 Measurement for Payment
- .1 Heating of water and aggregates and providing cold weather protection will not be measured but considered incidental to work.
-

1.7 Measurement for
Payment
(Cont'd)

- .2 Cooling of concrete and providing hot weather protection will not be measured but considered incidental to work.
- .3 Supply of anchor bolts, washers and nuts will not be measured but considered incidental to work. Bolt grouting will be considered incidental to the work.
- .4 Supply and installation of rigid PVC sleeves, expansion joints/sealants and curing compounds, or other compounds will be considered incidental to the work.
- .5 Concrete work will be measured in accordance with Section 01 29 00

PART 2 - PRODUCTS

2.1 Materials

- .1 Aggregates: to CSA-A23.1, for Class "C-1" exposure.
- .2 Portland Cement: to CSA A5, normal type 10
- .3 Water: to CSA A23.1
- .4 Admixtures:
 - .1 Air entraining admixture: to CSA A23.5
 - .2 Chemical admixture: to CSA A23.5 and ASTM C494
 - .3 Pozzolanic mineral admixtures: to CSA A23.5
- .5 Non-Shrink grout: premixed compound consisting of non-metallic aggregate, Portland cement, water reducing and plasticizing agents of pouring consistency, capable of developing compressive strength of 50 MPa at 28 days.
- .6 Curing compound: to ASTM -C390-M81 and CSA A23.1-M94 (or latest edition) type 1, 1D, or 2.
- .7 Premoulded joint fillers:
 - .1 Bituminous impregnated fiber board: to ASTM D1751 (latest edition), non-extruding resilient type.
- .8 Joint sealer: self leveling, two component sealant capable of remaining resilient over temperatures ranging from -25° C to 35° C. Material will be capable of an elongation of 300%, have tensile recovery of 90% ASTM D412-75 (or latest edition), hardness of 25-35 Shore A and have high bond stretch the concrete faces.

- 2.2 Concrete Mixes
- .1 Prior to starting concrete work, submit to the Departmental Representative the proposed mix design(s) for approval. Mix design(s) to be in accordance with Alternative 1 of Table 11 in CSA A23.1-94 (or latest edition). Comply with additional requirements of CSA A23.1-94 I (or latest edition), Section 15 for concrete placed near sea water:
 - .1 Use concrete mix designed to produce air entrained concrete.
 - .2 Cement to be normal Portland cement, Type 10.
 - .3 Minimum compressive strength at 28 days: 35 MPA.
 - .4 Exposure: Class C-1
 - .5 Maximum aggregate size to CSA A23.1-M94 (or latest edition) table 2, Group 1, 20 mm sieve size.
 - .6 Minimum cement content 390 kg/m³
 - .7 Air content: 6 to 8%
 - .8 Maximum water/cement ratio to be 0.40
 - .9 Slump at time and point of discharge 20 to 80 mm. Where the nature of the work requires larger slumps, they are to be obtained by the use of admixtures rather than increasing the water content. The use of such admixtures and the increase in slump to be approved by the Departmental Representative prior to implementation in the work.
 - .10 Modify concrete mix to the approval of the Departmental Representative to accommodate pumping.
 - .11 Admixtures to the approval of the Departmental Representative and the recommendation of the manufacturer. Admixtures must be dispersed separately into mixing water.
 - .2 Do not use calcium chloride or compounds containing calcium chloride.
 - .3 Weigh aggregates, cement, water and admixtures separately when batching. Inspect and test scales for accuracy as directed. Accuracy to be such that successive quantities can be measured to within one percent of desired amounts. Test certificates to be submitted to Departmental Representative upon request.
 - .4 Where seven day strength is less than 70% of specified 28 day strength, provide additional protection curing and make changes to mix proportions to the satisfaction of the Departmental Representative.
 - .5 Provide certification that plant, equipment and all materials to be used in concrete comply with the requirements of CSA A23.1-94 (or latest edition)
 - .6 Provide certification from independent testing and inspection company that mix proportions selected will produce concrete of specified quality and can be effectively placed and finished for all work under this contract.

PART 3 - EXECUTION

3.1 General

- .1 Obtain Departmental Representative's approval before placing concrete. Provide 24 hours notice of intended placement.
- .2 Place, consolidate, finish, cure and protect concrete to CSA A23.1-94 (or latest edition) except where specified otherwise.
- .3 Prior to placing of concrete, obtain Departmental Representative's approval of proposed method for protection of concrete during a placing and curing in adverse weather.
- .4 Pumping of concrete is permitted only after approval of equipment and mix.
- .5 Do not commence placing concrete until Departmental Representative had inspected/reviewed forms, inserts, dowels, reinforcing steel, joints; conveying, spreading, consolidation, finishing, curing and protective methods.
- .6 Ensure that reinforcement and anchorage are not disturbed during placing.
- .7 Maintain accurate records of placed concrete items to indicate date, location of pour, quality, air temperature and test samples taken.
- .8 Do not place load(s) upon new concrete until Departmental Representative is satisfied that the Contractor has carried out all calculations and test necessary to confirm that the load(s) will not cause damage or create a safety hazard. Calculations and tests to be stamped by a Professional Engineer registered in the Province of Nova Scotia.
- .9 Comply with additional requirements of CSA A23.1-94 (or latest edition), Clause 15, for concrete exposed to seawater environments during placement and curing.

3.2 Reinforcing
Stee

- .1 Place new reinforcing steel according to Section 03 20 00.
- .2 Provide 75 mm minimum cover for all reinforcing steel unless indicated otherwise on drawings.

3.3 Formwork

- .1 Verify field dimensions to determine applicable sizes of formwork.
- .2 Design and construct form work to allow adequately for proper placement and consolidation while conforming with shape and dimensions shown on plans.

3.3 Formwork
(Cont'd)

- .3 Formwork design will include closures at both top and bottom of form, and all necessary hardware to support the forms.
- .4 Upon request, submit drawings for review by the Departmental Representative, at least 3 weeks before placement of concrete. Drawings, will show formwork details and illustrate dimensions, method of placing of concrete, connections and support.
- .5 Strip formwork after minimum (5) days. This condition might be waived only if an alternative method to curing and preventing alternate wetting and drying is provided, to the satisfaction of the Departmental Representative. This condition will be waived if the forms are left permanently in place, where approved by the Departmental Representative.

3.4 Placement of
Concrete

- .1 Place and consolidate concrete to CSA A23.1-94 (or latest edition).
- .2 If allowed by Departmental Representative, pump concrete to following requirements:
 - .1 Arrange equipment so that no vibrations result which might damage freshly placed
 - .2 Where concrete is conveyed and placed by mechanically applied pressure, provide suitable equipment.
 - .3 Operate pump so that concrete, without air pockets, is produced.
 - .4 When pumping is discontinued and concrete remaining in pipe line is to be used, void pipe line in a manner that prevents contamination of concrete or separation of ingredients.
- .3 Concrete will be deposited in all cases as neatly as practicable, directly in its final position and will not be caused to flow in a manner to permit or cause segregation.
- .4 Each layer of concrete will be vibrated and tamped with an appropriate vibrator as allowed by the Departmental Representative. The concrete must be compacted to the maximum practicable density, free of air pockets, and until it is in complete contact with the reinforcement and formwork.

3.5 Inserts

- .1 Set galvanized sleeves and other inserts and openings as indicated or specified elsewhere. Sleeves and openings greater than 100 x 100 mm not indicated on drawings must be approved by Departmental Representative.
- .2 Do not eliminate or displace reinforcement to accommodate hardware. If inserts cannot be located as specified, obtain approval of all modifications from Departmental Representative before placing of concrete.

3.5 Inserts
(Cont'd)

- .3 Galvanized items embedded in concrete will be completely separated from reinforcing steel.
- .4 Anchor bolts:
 - .1 Set anchor bolts to templates under supervision of appropriate trade prior to placing concrete
 - .2 With Departmental Representative's concurrence, grout anchor bolts in pre-formed holes or holes drilled after concrete has it. Formed holes to be at least 100 mm in diameter. Drilled holes to be minimum 25 mm larger in diameter than bolts used.
 - .3 Protect anchor bolt holes from water accumulations.
 - .4 Set bolts and fill holes with non-shrink grout.
 - .5 Anchor bolts for base plates will be set to allow at least 25 mm of grout under the base plates.

3.6 Protection and
Curing

- .1 Provide protection and curing in accordance with CSA A23.1
- .2 Protect concrete with windproof shelter to allow free circulations of inside air around fresh concrete. Do not let walls of shelter touch formwork and provide sufficient space for removal of formwork.
- .3 Supply approved heating equipment to maintain inside air at following temperatures:
 - .1 For an initial three days, at not less than 10° C nor more than 25° C at surfaces.
 - .2 At not less than 10° for an additional 4 consecutive days or for the time necessary to attain 70% of the specified 28-day compressive strength of the concrete.
 - .3 Reduce temperature near end of curing period at rate not exceeding 20° C per day.
 - .4 Do not overheat.
- .4 Keep concrete surfaces continuously moist during protection stage and allow concrete to dry before removal of protection.
- .5 Freshly deposited concrete will be protected from premature drying and excessively hot and cold temperatures, will be maintained without drying at a relatively constant temperature for the period of time necessary for hydration of the cement and proper hardening of the concrete. It will be protected from harmful effects of sunshine, drying winds, cold weather, running or surface watt and mechanical shock.
- .6 Wood floating, broom finishing, placing of burlap and inspection of concrete to be done from transverse bridges of rigid construction free from wobbles and springing under use, unless other methods have been submitted and accepted.

3.7 Finishing

- .1 Finish concrete in accordance with CSA A23.1-94 (or latest edition)
- .2 Grind off fins, nibs and other raised protuberances with an approved hand stone.
- .3 When concrete has hardened sufficiently, give deck surface a uniform finish free from porous spots, irregularities, depressions, small pockets or rough spots using a power float leaving a rough spiral finish with one pass of the float.
- .4 Following use of power float, provide coarse broom finish using steel wire or stiff, coarse, fibre broom. Use broom in a transverse ridges satisfactory to Departmental Representative. Brooming will be delayed until concrete is sufficiently hard to retain ridges.
- .5 Rub exposed sharp edges of concrete with carborundum to produce 3 mm radius edges unless otherwise details.

3.8 Joint Fillers

- .1 Furnish filler for each joint in single piece for depth and width required for joint, unless otherwise authorized by Departmental Representative. When more than one piece is required for a joint, fasten abutting ends and hold securely to shape by stapling or other positive fastening.
- .2 Locate and form separation joint as indicated. Install joint filler.
- .3 Unless indicated otherwise, use 25 mm thick joint filler to separate deck slabs and extend joint filler from bottom of slab to within 25 mm of finished concrete surface.

3.9 Field Quality Control

- .1 Inspection and testing of concrete and concrete materials will be carried out by Testing Laboratory designated by the Departmental Representative in accordance with CSA A23.1-94 (or latest edition)
- .2 Departmental Representative will pay for costs of tests as specified in Section 01 45 00
- .3 Departmental Representative will take additional test cylinders during cold weather concreting. Cure cylinders on job site under same conditions as concrete which they represent.
- .4 If tests do not meet requirements of the Departmental Representative, take such measures as indicated in CAS A23.1-94 and CSA A23.2-94 (or latest editions).

-
- 3.9 Field Quality Control
(Cont'd)
- .5 Arrange and pay for inspection and testing when necessary for production control to meet requirements.
- .6 Inspection and testing by Departmental Representative will not augment Contractor's quality control or relieve him of contractual responsibility.
- 3.10 Defective Work
- .1 Concrete is defective when:
- .1 failing to meet any requirement of this specification
 - .2 concrete contains honeycombing or embedded debris
 - .3 28-day strength in any area is less than 95% of specified minimum
- .2 Repair or remove and replace defective work as directed by the Departmental Representative.
- .3 Take corrective measures as directed by the Departmental Representative to prevent occurrence of further defective concrete.