

GENERAL

1. THIS SET OF DRAWINGS SHALL BE READ IN CONJUNCTION WITH THE STRUCTURAL SPECIFICATIONS AND WITH THE DRAWINGS AND SPECIFICATIONS FROM ALL OTHER CONSULTANTS. ANY DISCREPANCIES NOTED SHALL BE REPORTED IMMEDIATELY FOR CLARIFICATION.
2. THIS SET OF DRAWINGS SHOWS THE COMPLETED STRUCTURE AND DOES NOT SHOW WORK WHICH MAY BE REQUIRED FOR SAFETY DURING CONSTRUCTION. THE CONTRACTOR IS RESPONSIBLE FOR GENERAL SAFETY ON AND ABOUT THE JOB SITE DURING THE CONSTRUCTION PERIOD AND FOR DESIGN AND ERECTION OF ALL FALSEWORK, SHORING, BRACING ETC. TO ENSURE THE SAFETY OF ALL CONSTRUCTION TEMPORARY LOADS AND TO COMPLETE THE WORK. ALL TEMPORARY WORKS AND SHORING ETC. SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN BRITISH COLUMBIA. ADHERE STRICTLY TO ALL REQUIREMENTS OF THE WORKSAFE BRITISH COLUMBIA.
3. ALL CODE REFERENCES ARE TO LATEST EDITIONS AS REFERENCED IN THE NBC 2010.

FIELD REVIEW:

1. DEPARTMENTAL REPRESENTATIVE PROVIDES FIELD REVIEW FOR THE WORK SHOWN ON THE STRUCTURAL DRAWINGS PREPARED BY DEPARTMENTAL REPRESENTATIVE. THIS REVIEW IS A PERIODIC REVIEW AT THE PROFESSIONAL JUDGMENT OF DEPARTMENTAL REPRESENTATIVE. THE PURPOSE IS TO ASCERTAIN THAT THE WORK IS IN GENERAL CONFORMANCE WITH THE PLANS AND SUPPORTING DOCUMENTS PREPARED BY DEPARTMENTAL REPRESENTATIVE, AND TO FULFILL THE REQUIREMENTS FOR THE COMPLETION OF LETTERS OF ASSURANCE REQUIRED BY THE APPLICABLE BUILDING CODE.
2. ALL NON-CONFORMING WORKS THAT REQUIRE REMEDIAL ACTION SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. ANY EXTRA TIME OR COST INCURRED TO DEPARTMENTAL REPRESENTATIVE TO ASSIST OR ADVISE THE CONTRACTOR IN RECTIFYING THE WORK SHALL BE BORNE BY THE CONTRACTOR.
3. ENSURE THAT WORK TO BE INSPECTED IS COMPLETE AT THE TIME OF INSPECTION AND IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. ADDITIONAL INSPECTIONS REQUIRED DUE TO THE INCOMPLETE WORK OR POORLY EXECUTED WORK, AS JUDGED BY DEPARTMENTAL REPRESENTATIVE, AS WELL AS ADDITIONAL DESIGN OR REMEDIAL WORK CAUSED BY DEVIATIONS FROM THESE DRAWINGS MAY BE CHARGED TO THE GENERAL CONTRACTOR AT THE DISCRETION OF DEPARTMENTAL REPRESENTATIVE.
4. A MINIMUM 72 HOURS NOTICE SHALL BE GIVEN BY THE CONTRACTOR FOR ANY INSPECTION TO BE CARRIED OUT BY DEPARTMENTAL REPRESENTATIVE.

SHOP DRAWINGS:

1. DESIGNERS & MANUFACTURERS OF ALL STRUCTURAL ELEMENTS/COMPONENTS/CONNECTIONS SHALL SUBMIT COMPLETE SHOP DRAWINGS SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF BRITISH COLUMBIA TO THE DEPARTMENTAL REPRESENTATIVE FOR REVIEW PRIOR TO FABRICATION. SUBMIT SHOP DRAWINGS IN ACCORDANCE WITH SPECIFICATIONS AND TO ALLOW MINIMUM TWO WEEKS FOR REVIEW. THIS SUBMISSION OR ITS REVIEW SHALL NOT RELIEVE THE CONTRACTOR OF HIS RESPONSIBILITY FOR PROVIDING PROPER ENGINEERING DESIGN, METHODS, EQUIPMENT, WORKMANSHIP, SAFETY PRECAUTION AND PRIOR REVIEW OF THESE ELEMENTS. THE PROFESSIONAL ENGINEER SEALING THE SHOP DRAWINGS SHALL BE RESPONSIBLE FOR INSPECTION OF HIS DESIGN COMPONENTS FOR CONFORMANCE WITH HIS DESIGN AND SHOP DRAWINGS.
2. THE CONTRACTOR AND ITS SUBCONTRACTORS SHALL CONFIRM AND COORDINATE DIMENSIONS, LOCATIONS AND NUMBER OF THE STRUCTURAL ELEMENTS FOR WHICH SHOP DRAWINGS ARE TO BE PRODUCED.

NON-STRUCTURAL COMPONENTS:

1. NON-STRUCTURAL COMPONENTS ARE NOT THE RESPONSIBILITY OF DEPARTMENTAL REPRESENTATIVE SUCH COMPONENTS OF THE PROJECT ARE DESIGNED, DETAILED, SPECIFIED AND REVIEWED IN THE FIELD BY OTHERS. LETTERS OF CERTIFICATION OF ADEQUACY, INSTALLATION ETC. OF SUCH COMPONENTS ARE BY OTHERS.
2. MANUFACTURERS OF NON-STRUCTURAL COMPONENTS WHICH AFFECT THE STRUCTURAL FRAMING SHALL SUBMIT SHOP DRAWINGS TO THE ARCHITECT AND DEPARTMENTAL REPRESENTATIVE FOR REVIEW. THE SHOP DRAWINGS SHALL CLEARLY INDICATE LOADS IMPOSED ON THE STRUCTURE. REVIEW WILL BE LIMITED TO THE EFFECT OF THE COMPONENTS ON THE STRUCTURAL FRAMING.
3. EXAMPLES OF NON-STRUCTURAL COMPONENTS INCLUDE, BUT ARE NOT LIMITED TO:
- ARCHITECTURAL COMPONENTS SUCH AS HANDRAILS, GUARDRAILS, RAILINGS, FLAG POST, REMOVABLE CANOPIES, CEILINGS, VEHICLE PROTECTION SYSTEMS, ORNAMENTAL COMPONENTS, ETC.
 - ARCHITECTURAL PRECAST CONCRETE AND ITS ATTACHMENTS.
 - ARCHITECTURAL GLASS BLOCKS AND THEIR ATTACHMENTS.
 - BRICK AND BLOCK VENEERS, THEIR REINFORCING IF ANY AND TIES
 - LANDSCAPING COMPONENTS SUCH AS BENCHES, LIGHT POSTS, PLANTERS, ETC.
 - CURTAIN WALL SYSTEMS, CLADDING, SKYLIGHT, WINDOW MULLIONS, ETC.
 - INTERIOR AND EXTERIOR NON-LOAD BEARING STEEL STUD WALLS
 - SUPPORT AND BRACING OF MECHANICAL AND ELECTRICAL SYSTEMS AND EQUIPMENTS FOR NON-GRAVITY AND SEISMIC LOADS.
 - WINDOW WASHING EQUIPMENTS AND ITS ATTACHMENT.
 - ELEVATORS, ESCALATORS AND OTHER CONVEYING SYSTEMS, INCLUDING PROPRIETARY SUPPORT BEAMS AND THEIR ATTACHMENTS.
 - NON-STRUCTURAL MASONRY.
4. NON-STRUCTURAL STEEL STUD FRAMING
- INTERIOR AND EXTERIOR STEEL STUD WALLS AND OTHER ARCHITECTURAL FRAMING SHALL BE DESIGNED BY THE FABRICATOR. DESIGN SHALL BE BY A STRUCTURAL ENGINEER REGISTERED IN THE PROVINCE OF BRITISH COLUMBIA AND SHALL BE IN ACCORDANCE WITH PART 4 OF THE GOVERNING BUILDING CODE USING THE DESIGN LOADS REFERENCED ELSEWHERE ON THIS DRAWING. SEE ALSO ITEMS 1 AND 2 ABOVE.
 - UNLESS NOTED OTHERWISE, EXTERIOR STEEL STUDS FRAMING TO THE UNDERSIDE OF STRUCTURAL STEEL BEAMS OR TO STEEL BRACING MEMBERS SHALL BE DETAILED AND DESIGNED SO AS NOT TO IMPART LATERAL WIND AND SEISMIC LOADS TO THESE MEMBERS. WHERE WIND BEARING STUDS ATTACH TO STEEL BEAM BOTTOM FLANGES PROVIDE STEEL STUD BRACING IN GENERAL CONFORMANCE WITH DEPARTMENTAL REPRESENTATIVE'S TYPICAL DETAILS. DETAIL TOP TRACK TO ALLOW FOR ROOF/FLOOR DEFLECTIONS DUE TO GRAVITY LOADS.

EXISTING STRUCTURES:

1. PRIOR TO CONSTRUCTION THE CONTRACTOR SHALL VERIFY ALL RELEVANT DIMENSIONS TO AND OF EXISTING STRUCTURES. NOTIFY DEPARTMENTAL REPRESENTATIVE IMMEDIATELY IF DISCREPANCIES ARE NOTED.
2. THE CONTRACTOR SHALL AT HIS OWN EXPENSE REPAIR AND MAKE GOOD ANY DAMAGE TO THE EXISTING STRUCTURE, EQUIPMENT AND FINISHES CAUSED BY THE CONSTRUCTION ACTIVITIES. REPAIRS SHALL BE TO THE SATISFACTION OF THE DEPARTMENTAL REPRESENTATIVE.
3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE TEMPORARY SUPPORT OF ANY ADJACENT EXISTING STRUCTURES DURING CONSTRUCTION. UNDERPINNING OR BRACING SHALL BE DESIGNED BY A QUALIFIED PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF BRITISH COLUMBIA. 4 COPIES OF SIGNED AND SEALED DESIGN DRAWINGS TO DEPARTMENTAL REPRESENTATIVE, FOR REVIEW OF CONFORMANCE WITH GENERAL DESIGN CRITERIA.

DESIGN LOADS:

1. THIS STRUCTURE HAS BEEN DESIGNED FOR SNOW, WIND AND SEISMIC FORCES IN SUBSTANTIAL COMPLIANCE WITH THE PROVISIONS SET FORTH IN THE NATIONAL BUILDING CODE 2010. IMPORTANT CATEGORY=NORMAL (AS PER OWNER'S REQUIREMENT).
- GROUND SNOW: S_s = 1.1 kPa
RAIN LOAD: S_r = 0.2 kPa
- IMPORTANCE FACTORS FOR SNOW
- I_s = 1.0 FOR STRENGTH
I_w = 0.9 FOR SERVICEABILITY
- WIND LOAD: PROBABILITY 1/10 = 0.44 kPa
PROBABILITY 1/50 = 0.57 kPa
- IMPORTANCE FACTORS FOR WIND
- I_w = 1.0 FOR STRENGTH
I_w = 0.75 FOR SERVICEABILITY
- EARTHQUAKE FACTORS:
- | Sa(0.2) | Sa(0.5) | Sa(1.0) | Sa(2.0) |
|---------|---------|---------|---------|
| 1.20 | 0.82 | 0.38 | 0.19 |
- I_e 1.0 FOR STRENGTH
I_e 1.0 FOR SERVICEABILITY
(CLAUSE 4.1.8.5 FOR SERVICEABILITY)
F_a = 0.9 F_v = 1.9
R_d = 1.5 R_o = 1.5

- SITE CLASS F
2. SPECIFIED UNIFORM SUPERIMPOSED DEAD LOADS ON ROOF AND FLOORS:
- ROOF 1.0 kPa
MAIN FLOOR 1.0 kPa
MECHANICAL & ELECTRICAL ROOMS (GENERAL) 1.0 kPa
EXTERIOR WALLS ACTUAL WEIGHT
- UPPER FLOORS AND MAIN FLOOR LOADS INCLUDE GENERAL PARTITION LOAD OF 1.0kPa AND NON-STRUCTURAL CONCRETE TOPPING. FOR MASONRY PARTITIONS, ACTUAL WEIGHTS SHALL BE USED.
 - THESE LOADS DO NOT INCLUDE SELFWEIGHT OF STRUCTURE, WEIGHT OF MASONRY PARTITIONS, WEIGHTS OF MECHANICAL EQUIPMENT AND CONCRETE EQUIPMENT PADS.
3. SPECIFIED UNIFORM LIVE LOADS ON FLOORS:
- MAIN FLOOR / SERVICE PIT 4.8 kPa
SECOND FLOOR 7.2 kPa
DRIVE WAY 12 kPa
4. DESIGN SPECIFIED CONCENTRATED LIVE LOADS ON ROOF AND FLOORS:
- ROOF 1.3 kN
MAIN FLOOR / SERVICE PIT / SECOND FLOOR 9.0 kN
DRIVE WAY 54 kN
5. WORST CASE OF UNIFORM OR CONCENTRATED LIVE LOADS WILL BE USED FOR DESIGN OF STRUCTURAL MEMBERS.

CONSTRUCTION LOADS:

1. CONSTRUCTION LOADS ON COMPLETED FLOORS MUST NOT EXCEED THE LOAD CARRYING CAPACITY OF FLOOR AT THE TIME OF THE LOADING UNLESS IT IS PROPERLY SHORED TO SUPPORT THE INTENDED LOAD. MOVING OF HEAVY EQUIPMENT AND PILING UP OF MATERIAL SHALL NOT BE PERMITTED UNLESS DESIGNED SHORING IS IN PLACE.
2. SHORING DESIGN BY CONTRACTOR. INFORM CWMM CONSULTING ENGINEERS LTD. PRIOR TO LOAD APPLICATION.

FOUNDATION AND SITE WORK

1. REFER TO GEOTECHNICAL REPORT PREPARED BY AMEC FOSTER WHEELER ENVIRONMENT & INFRASTRUCTURE, DATED DECEMBER 04, 2016 AND ALL ITS SUPPLEMENTS AND AMENDMENTS FOR EXCAVATION, BACKFILLING, FILL MATERIALS, COMPACTION, FROST PROTECTION AND OTHER SITE PREPARATION REQUIREMENTS NOT SHOWN ON THESE DRAWINGS.
2. DESIGN ALLOWABLE SOIL BEARING CAPACITIES:
- RAFT FOOTINGS SLS=60 kPa
ULS=80 kPa
ULS=105 kPa (SEISMIC)
3. ANY FOOTING ELEVATIONS INDICATED ON THE DRAWINGS ARE GENERAL AND SHALL BE USED FOR ESTIMATING AND BIDDING PURPOSES. FOOTINGS MAY HAVE TO BE PLACED AT DIFFERENT ELEVATIONS AS A RESULT OF LOCAL SOILS CONDITIONS, UNDERGROUND SERVICES AND TO ACCOMMODATE OTHER MECHANICAL AND ELECTRICAL SERVICES. FOLLOW TYPICAL DETAILS SHOWN ON THESE DRAWINGS FOR FOOTING PLACEMENT RELATIVE TO ADJACENT FOOTINGS, SLUMP AND OTHER EXCAVATED STRUCTURES AND LOCATE AS DIRECTED BY GEOTECHNICAL ENGINEER.
4. THE BASES OF FOUNDATIONS SHALL BE PROTECTED FROM RAIN, SNOW AND ANY WATER INFILTRATION.
5. NO FOUNDATIONS MAY BE POURED BEFORE THE BEARING MATERIAL HAS BEEN INSPECTED BY THE GEOTECHNICAL ENGINEER. NOTIFY THE GEOTECHNICAL ENGINEER MINIMUM 48 HOURS BEFORE INSTALLATION OF FOOTING REINFORCEMENT.
6. IMMEDIATELY AFTER INSPECTION AND APPROVAL BY THE GEOTECHNICAL ENGINEER, OR OTHERWISE AS DIRECTED BY THE DEPARTMENTAL REPRESENTATIVE, THE BEARING SURFACE SHALL BE COVERED BY A 50mm THICK CONCRETE GROUND SEAL OF 10MPa STRENGTH.
7. COORDINATE CONSTRUCTION WITH UNDERSLAB SERVICES AS SHOWN ON MECHANICAL, ELECTRICAL, ARCHITECTURAL AND LANDSCAPING DRAWINGS.
8. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SITE DRAINAGE, GROUND ELEVATIONS AND DRAINAGE SLOPES.
9. CENTRE ALL FOOTINGS UNDER COLUMNS OR WALLS UNLESS NOTED OTHERWISE.
10. DO NOT BACKFILL RETAINING WALLS INCLUDING PERIMETER BASEMENT WALLS BEFORE THEY ARE ADEQUATELY SUPPORTED BY THE SUPPORTING FLOOR(S). ALL CONCRETE SUPPORTING FLOORS MUST HAVE CURED FOR MINIMUM 7 DAYS OR ATTAINED MINIMUM 75% OF THEIR 28-DAYS STRENGTH. ALL BACKFILLING TO COMPLY WITH THE REQUIREMENTS PROVIDED BY THE GEOTECHNICAL ENGINEER.
11. REFER TO ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR WATERPROOFING AND SEALING REQUIREMENTS.
12. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE TEMPORARY SUPPORT OF THE ADJACENT STRUCTURE DURING CONSTRUCTION. UNDERPINNING OR BRACING SHALL BE DESIGNED BY A QUALIFIED PROFESSIONAL ENGINEER, REGISTERED IN BRITISH COLUMBIA. SUBMIT 4 COPIES OF DESIGN DRAWINGS, SEALED BY A PROFESSIONAL ENGINEER, TO DEPARTMENTAL REPRESENTATIVE FOR REVIEW OF CONFORMANCE WITH GENERAL DESIGN CRITERIA. CRITERIA, (REFER TO DWG 5603, 5604 & 5607 FOR PERMANENT UNDERPINNING.)

REINFORCED CONCRETE

1. REFER TO SPECIFICATIONS FOR CONCRETE STRENGTH, EXPOSURE CLASS & OTHER REQUIREMENTS.
2. REINFORCING BARS f =400 mPa. ALL DOWELS ANCHOR BOLTS AND INSERTS SHALL BE PLACED BEFORE THE CONCRETE IS POURED.
3. PROVIDE MINIMUM CONCRETE COVER TO REINFORCEMENT AS FOLLOWS:
- CAST AGAINST EARTH 75mm
- EXPOSED TO EARTH OR WEATHER: 50mm
- ELSEWHERE: 20mm
4. UNLESS NOTED OTHERWISE, PROVIDE MINIMUM SPLICE LENGTHS TO REINFORCEMENT AS FOLLOWS:
- 10M 450mm
- 15M 600mm
- 20M 750mm

MASONRY

1. REFER TO SPECIFICATIONS FOR CONCRETE MASONRY, CODE REFERENCES AND OTHER REQUIREMENTS.
2. REINFORCING BARS f_y =400 MPa
3. UNLESS NOTED OTHERWISE REINFORCE 200 & 250 LOADBearing WALLS AS FOLLOWS:
- VERTICAL: 1-20M @ 1200 CENTERED IN GROUTED CORE
HORIZONTAL: 3.8mm DIA. LADDER JOINT REINF. @ 400
- 2-15M IN CONTINUOUS BOND BEAMS AS SHOWN BELOW:
H<2400: 1 BOND BEAM AT TOP OF WALL
2400<H<4800: 1 BOND BEAM AT TOP OF WALL PLUS 1 BOND BEAM MIDHEIGHT (OVER WALL OPENINGS) (H = CLEAR HEIGHT OF WALLS)
- ADDITIONAL: 1-20M VERT. AT UNSUPPORTED ENDS OF WALLS
1-20M VERT. AT ALL CORNERS AND INTERSECTIONS
1-20M VERT. AT EACH SIDE OF OPENINGS
1-20M VERT. IN EACH CELL OF PIERS AND PILASTERS
2-15M ABOVE AND BELOW ALL OPENINGS, 800 PAST EDGE.
(MAY BE PART OF CONTINUOUS BOND BEAMS)

FOR NON-LOADBEARING WALLS SUBSTITUTE 15M VERTICAL BARS FOR 20M VERTICALS (INCLUDING ADDITIONAL BARS).

4. UNLESS NOTED OTHERWISE SPLICE REINFORCING AND EMBED DOWELS AS FOLLOWS:
- DOWEL EMBEDMENT: 25M BARS: 800 SPLICES: 25M BARS: 1500
(INCL. LENGTH 20M BARS: 500 20M BARS: 900
OF HOOK) 15M BARS: 400 15M BARS: 650
10M BARS: 300 10M BARS: 450
WIRE REINF: 200
5. ALL VERTICAL REINFORCING SHALL RUN CONTINUOUS THROUGH BOND BEAMS AND LINTELS OR BE SPLICED AS SPECIFIED.
6. PROVIDE CORNER BARS FOR ALL HORIZONTAL REINFORCING. SPLICE LENGTH AS SPECIFIED.
7. STRAIGHT OR HOOKED DOWELS SHALL BE PROVIDED IN FOUNDATIONS OR GRADE BEAMS TO MATCH ALL VERTICAL REINFORCING BARS. SPLICE LENGTH AS SPECIFIED.
8. CELLS TO BE REINFORCED SHALL BE KEPT CLEAR OF MORTAR.
9. FILL CELLS CONTAINING REINFORCING STEEL OR ANCHOR BOLTS WITH 20MPa GROUT, 10mm AGGREGATE, 200-250 SLUMP. PUDDLE OR VIBRATE TO COMPLETELY FILL CELLS. REVIBRATE AFTER 10 TO 40 MINUTES, WHEN EXCESS WATER HAS BEEN ABSORBED BY MASONRY UNITS. TOP OFF FILLED CELLS WITH FRESH GROUT AFTER REVIBRATION.
10. PROVIDE CLEAN-OUTS AT BOTTOM OF ALL GROUTED CORES FOR LIFTS OVER 1500.
11. CHECK STRUCTURAL, ARCHITECTURAL, MECHANICAL, ELECTRICAL, CIVIL, LANDSCAPE AND ALL OTHER RELEVANT DRAWINGS FOR LOCATIONS AND SIZES OF BOLTS, SLEEVES AND OPENINGS. SUPPLY AND SET ANCHOR BOLTS, SLEEVES, PIPE HANGERS, JOINTS AND OTHER INSERTS AND OPENINGS AS INDICATED OR SPECIFIED ELSEWHERE.
12. VERTICAL CONTROL JOINTS SHALL BE PROVIDED AT A MAXIMUM SPACING OF 8000 FOR STRAIGHT WALLS AND 4000 FROM WALL CORNERS. TERMINATE BOND BEAM REINFORCING AT CONTROL JOINTS, LADDER REINFORCING TO RUN THROUGH JOINTS.
13. UNLESS NOTED OTHERWISE PROVIDE LINTELS OVER OPENINGS AS FOLLOWS:
- CLEAR SPAN DEPTH OF LINTEL REINFORCING
LESS THAN 1500 400 2-15M 800 PAST EDGES
1500 - 2400 600 2-20M 800 PAST EDGES
2500 - 3000 800 2-25M 800 PAST EDGES
14. 90mm BRICK VENEER: UNLESS NOTED OTHERWISE ON THE DRAWINGS PROVIDE HOT DIP GALVANIZED STEEL ANGLE LINTELS OVER OPENINGS AS FOLLOW:

| MAX. CLEAR OPENING WIDTH | MINIMUM ANGLE SIZE (LONG LEG VERTICAL) |
|--------------------------|--|
| <2.7m | L90x90x6 |
| 3.3m | L125x90x8 |
| 3.5m | L125x90x10 |
| >3.5m | SEE DRAWINGS |

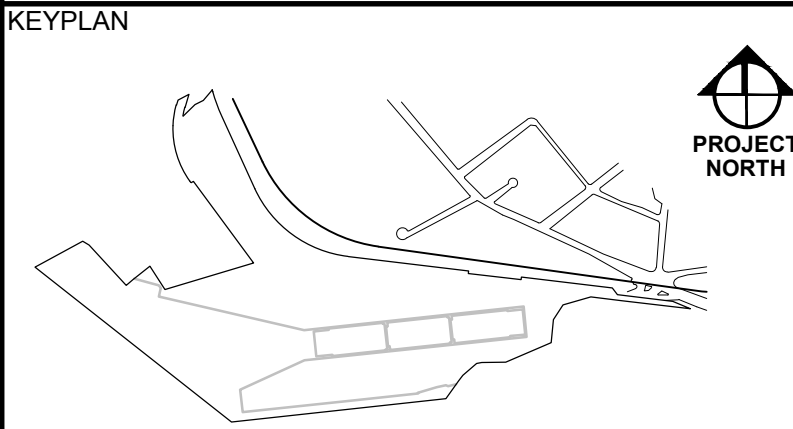
90mm MINIMUM BEARING EACH END.

STRUCTURAL STEEL

1. REFER TO SPECIFICATIONS FOR STEEL WORK, OPEN WEB STEEL JOISTS, STEEL DECK, DESIGN CODE REFERENCES AND OTHER REQUIREMENTS.
2. DRAWINGS FROM ALL CONSULTANTS SHALL BE EXAMINED FOR EXACT LOCATIONS, DIMENSIONS AND ELEVATIONS.
3. STEEL FABRICATORS AND CONTRACTOR SHALL CONFIRM ALL LOCATIONS, DIMENSIONS AND ELEVATIONS WITH ACTUAL SITE MEASUREMENTS BEFORE FABRICATION. THE CONTRACTOR SHALL ASSUME FULL RESPONSIBILITY FOR ANY FABRICATION AND WORK DONE PRIOR TO REVIEW AND APPROVAL OF THE SHOP DRAWINGS.

ABBREVIATIONS

A.BOLT
ALT. ALTERNATE
ARCH. ARCHITECTURAL
BLDG. BUILDING
BOT. BOTTOM
BTW. BETWEEN
C/C CENTER TO CENTER
C/W COMPLETE WITH
C.I.P. CAST IN PLACE
CANT. CANTILEVER
CJ CONSTRUCTION JOINT
CL. CLEAR
COL. COLUMN
CONC. CONCRETE
CONT. CONTINUOUS
DL. DEAD LOAD
DN. DOWN
DO. DITTO
DP. DEEP
DWG. DRAWING
E.W. EACH WAY
LV. LENGTH VARIES
LG. LONG
LL. LOW LEVEL
LLV. LONG LEG VERTICAL
LLH. LONG LEG HORIZONTAL
LONG. LONGITUDINAL
MAX. MAXIMUM
MECH. MECHANICAL
MIN. MINIMUM
N/A. NOT AVAILABLE
N.S. NEAR SIDE
N.STUD NELSON STUD
N.T.S. NOT TO SCALE
O/C ON CENTRES
OPP. OPPOSITE HAND
OWSJ. OPEN WEB STEEL JOIST
P.C. PRECAST CONCRETE
PL. PLATE
PLY. PLYWOOD
PROJ. PROJECTION



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| - | - | - |
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| 3 | ISSUED FOR TENDER | 2016-01-28 |
| 2 | ISSUED FOR 99% SUBMISSION | 2016-01-07 |
| 1 | ISSUED FOR 60% CD REVIEW | 2015-11-25 |
| 0 | ISSUED FOR 33% CD REVIEW | 2015-10-29 |

| Revision / Revision | Description / Description | Date / Date |
|---------------------|---------------------------|-------------|
| Client / client | | |

ESQUIMALT
GRAVING DOCK

825 ADMIRALS ROAD
VICTORIA, BC, V9A 2P1

Project title/Titre du projet
825 ADMIRALS ROAD VICTORIA BC
ESQUIMALT GRAVING DOCK
ELECTRICAL SAFETY UPGRADE

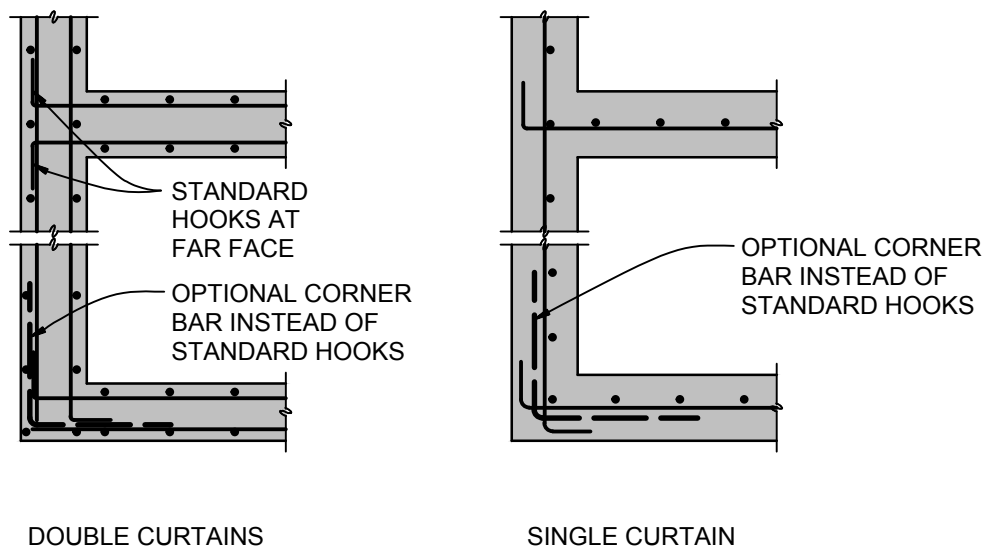
SOUTH SUBSTATION
SWITCHGEAR
REPLACEMENT PROJECT
(SSSR)

| | | |
|---|--|--|
| Consultant Signature Box Only | | |
| Designed by/Concept par P.L./L.L. | | |
| Drawn by/Dessiné par M.C. | | |
| PWGSC Project Manager/Administrateur de Projets TPSGC France Veillette | | |
| PWGSC, Regional Manager, Architectural and Engineering Services/ Gestionnaire régionale, Services d'architectural et de génie, TPSGC Preetipal Paul | | |
| Drawing title/Titre du dessin | | |

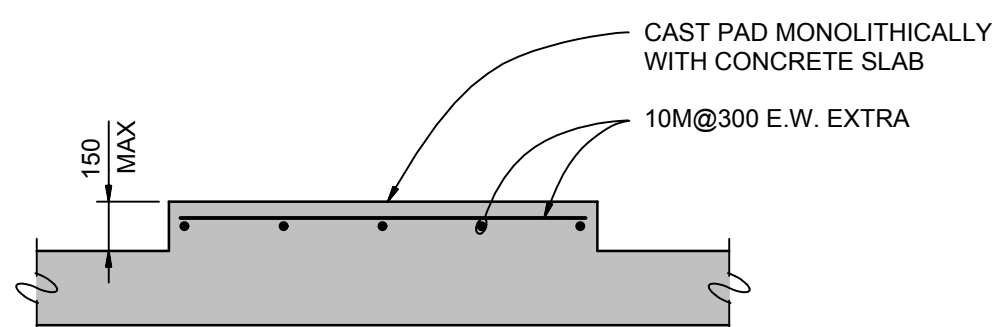
GENERAL NOTES

| Project No./No. du projet | Sheet/feuille | Revision no./ La Révision no. |
|---------------------------|---------------|-------------------------------------|
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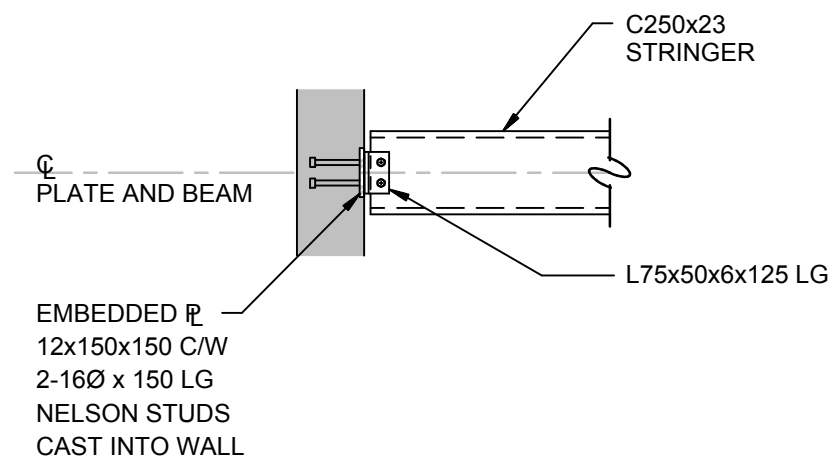


TYPICAL WALL INTERSECTION PLAN DETAILS

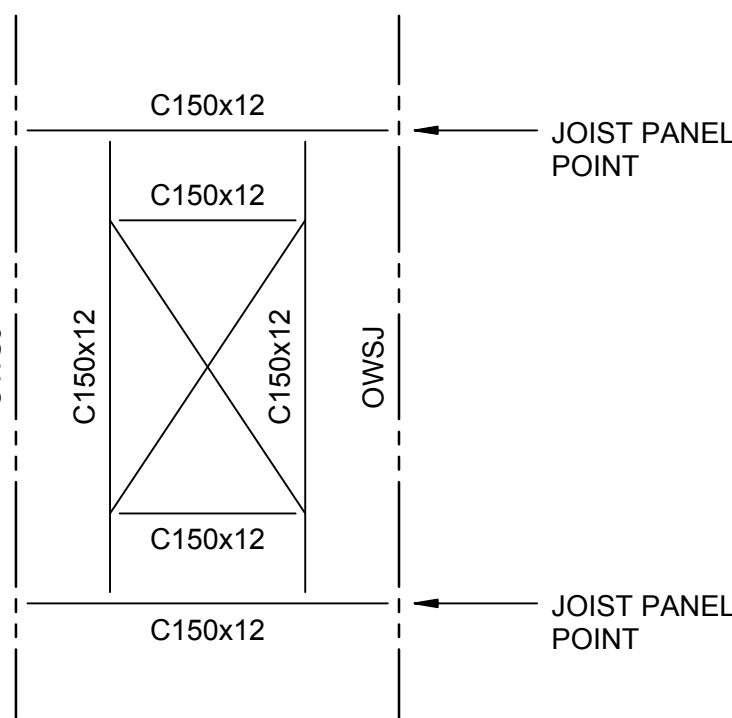


(FOR LOCATION AND SIZE OF PADS AND ANCHOR BOLTS FOR EQUIPMENT SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS)

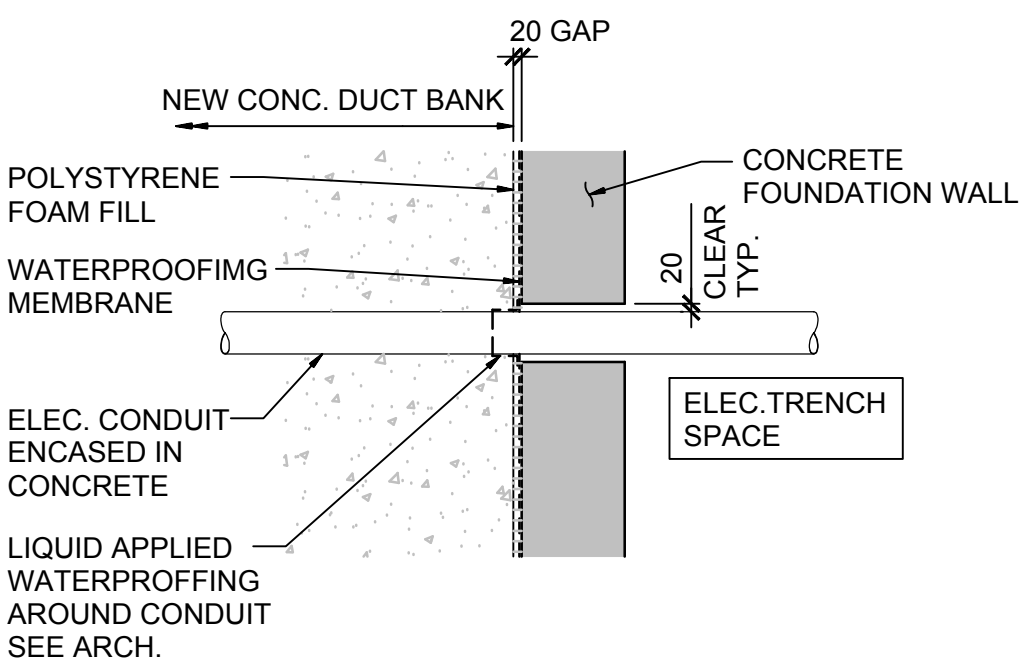
TYPICAL EQUIPMENT PADS ON CONCRETE SLABS



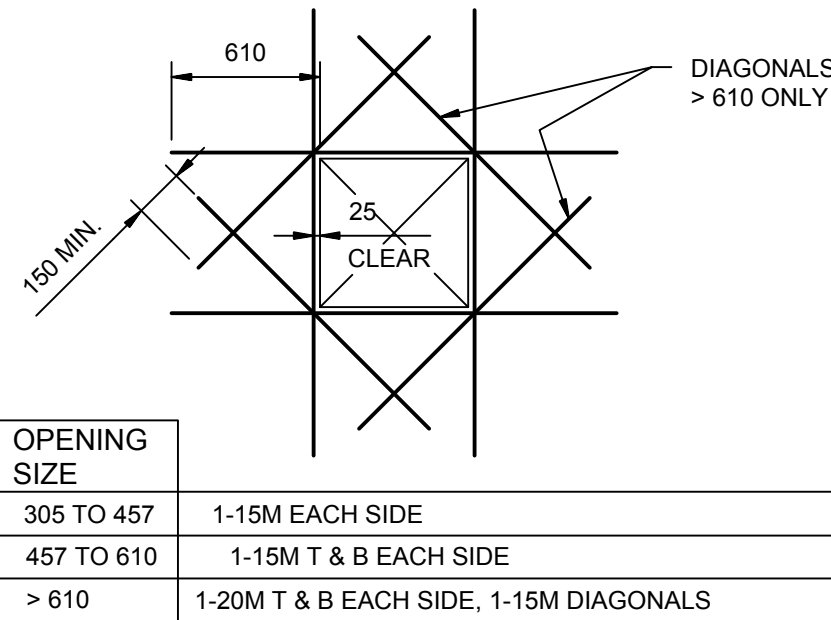
STEEL STRINGER/CONCRETE WALL DETAIL



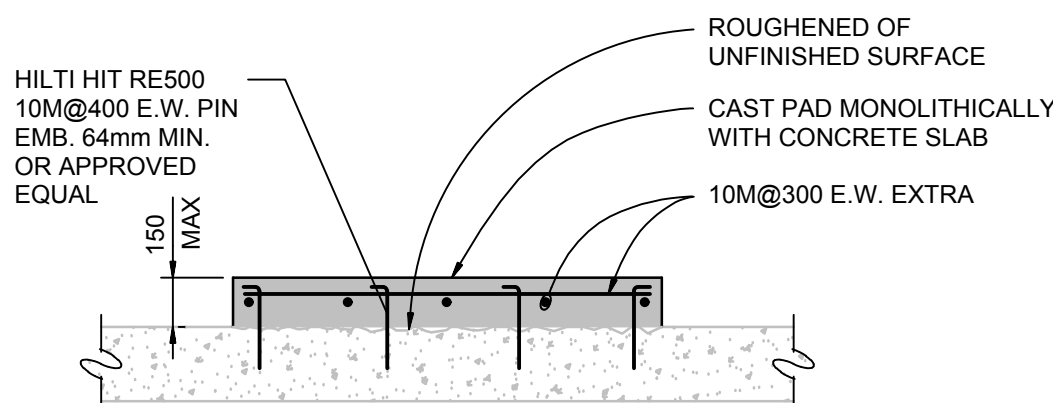
DECK OPENINGS > 450 SQUARE
SIZE OF OPENING TO BE CONFIRMED BY TRADE REQUIRING SAME PRIOR TO FABRICATION



TYPICAL CONDUIT PENETRATION / DUCK BANK INTERFACE DETAIL

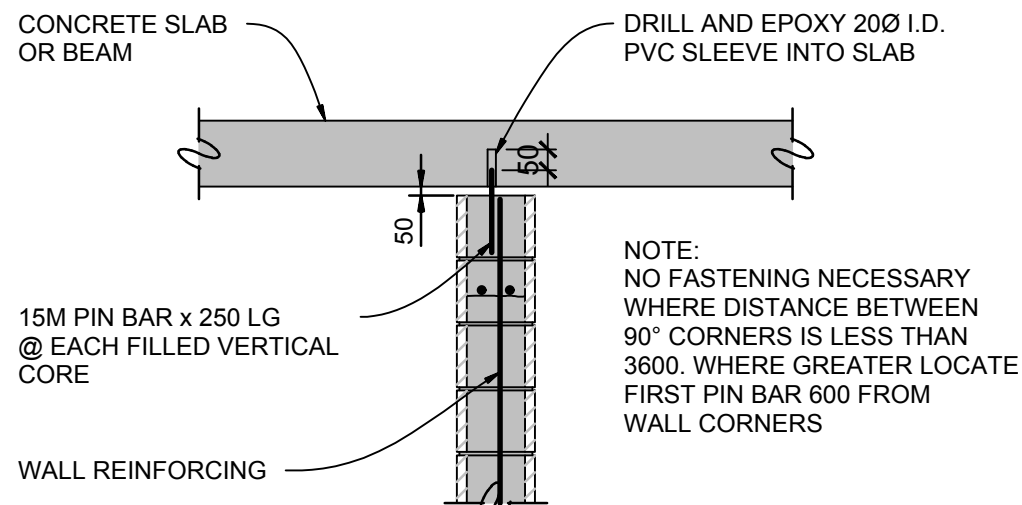


CONCRETE WALL AND SLAB OPENINGS

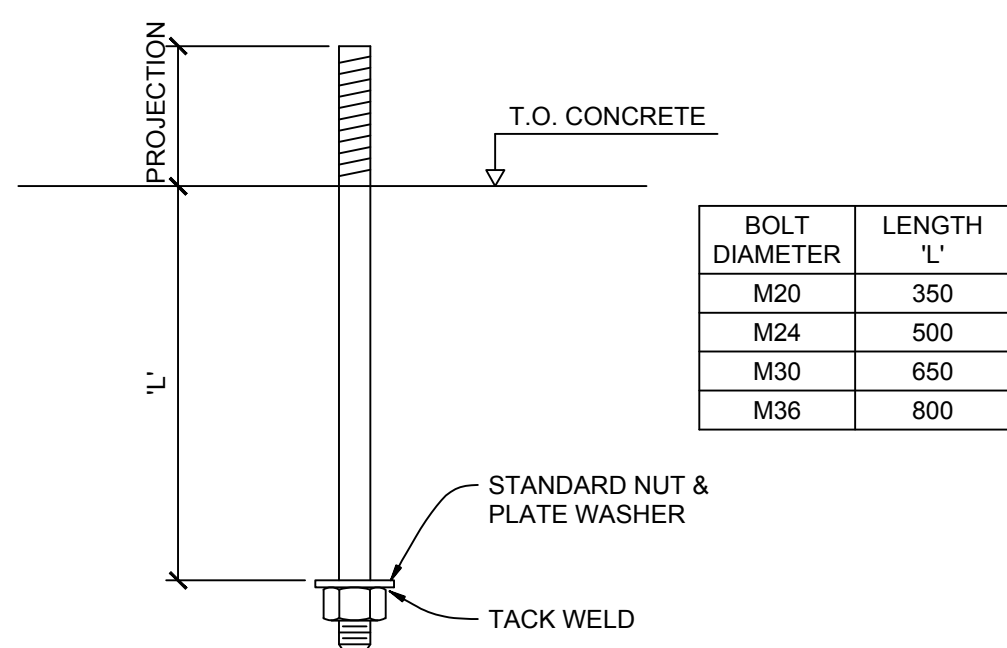


(FOR LOCATION AND SIZE OF PADS AND ANCHOR BOLTS FOR EQUIPMENT SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS)

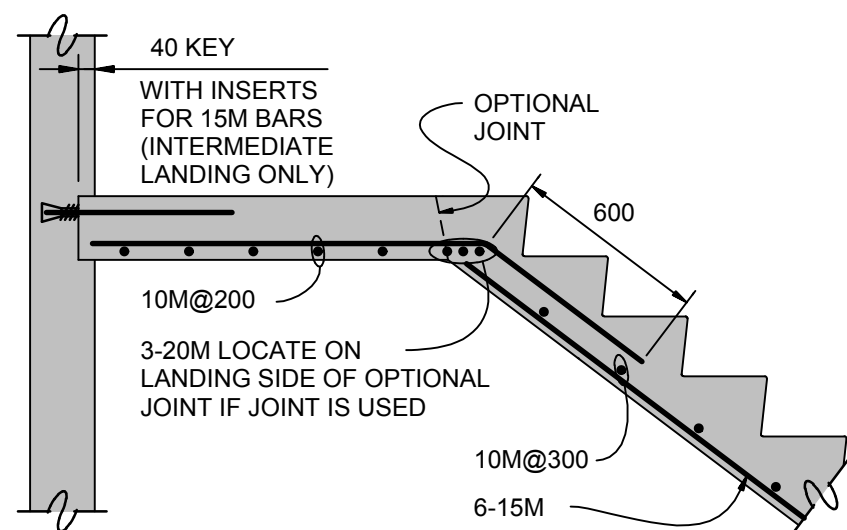
TYPICAL EQUIPMENT PADS ON CONCRETE SLABS



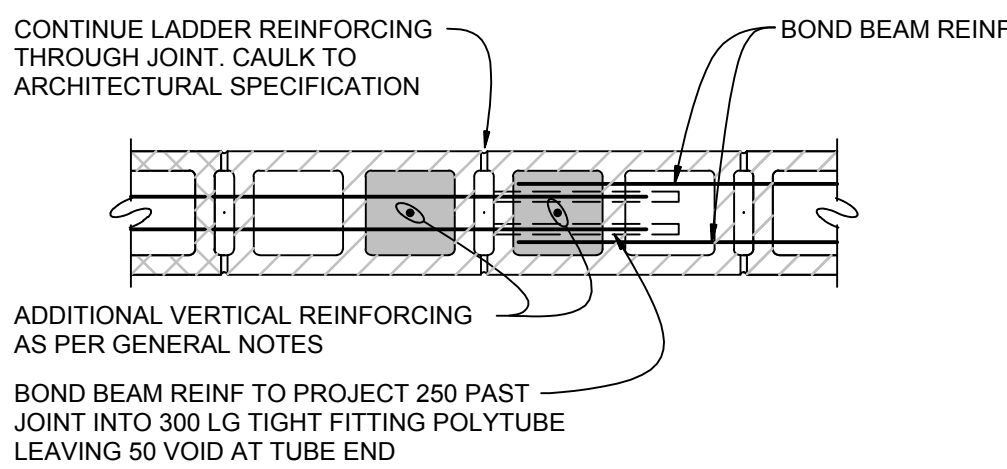
TYPICAL NON-LOADBEARING BLOCK WALL TOP SUPPORT
(MILD REINFORCING ONLY IN SLABS)



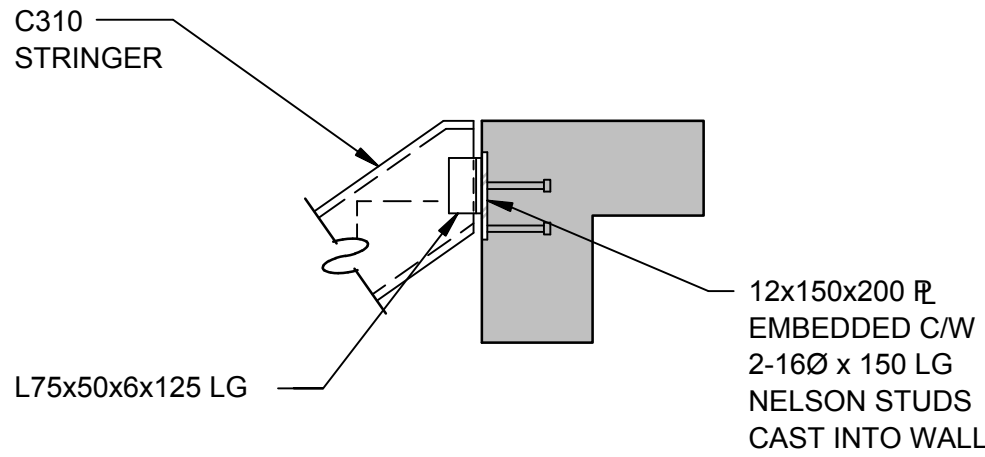
TYPICAL ANCHOR BOLT DETAIL



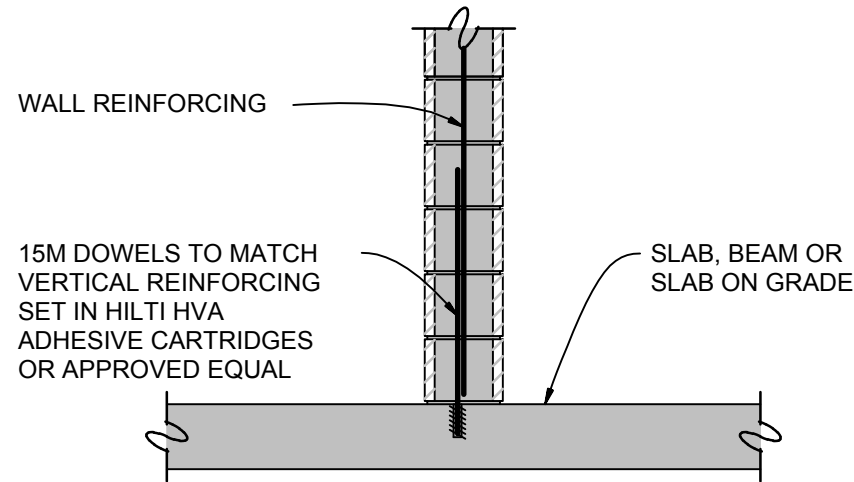
TYPICAL CAST-IN-PLACE STAIR AND LANDING SECTIONS



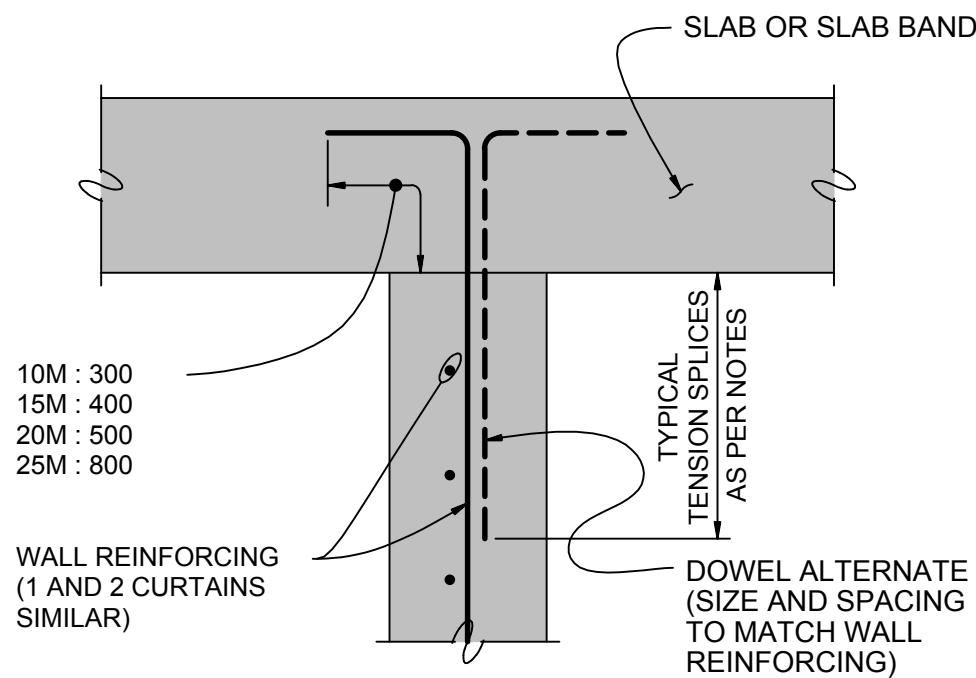
TYPICAL BLOCKWALL JOINTS
(LOCATIONS TO BE APPROVED BY ARCHITECT BEFORE POURING WALL FOUNDATIONS)



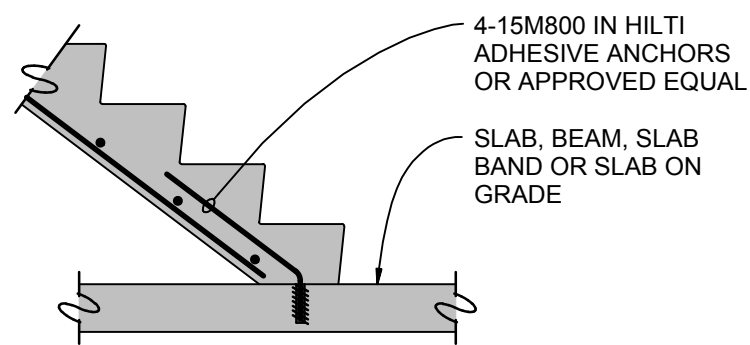
STEEL STRINGER TOP DETAIL



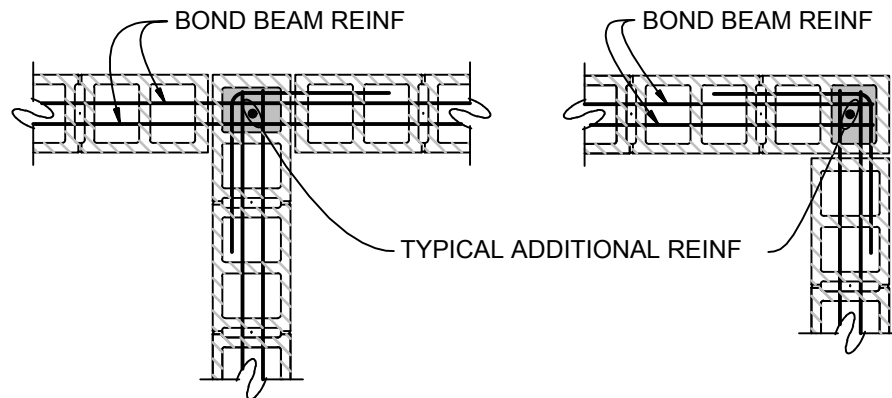
TYPICAL NON LOAD BEARING BLOCK WALL BASE
(MILD REINFORCING ONLY IN SLAB)



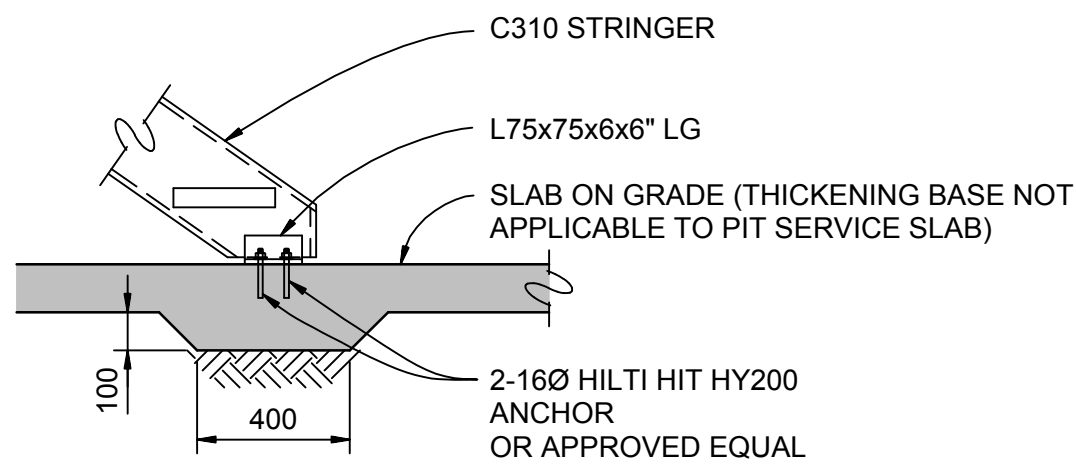
TYPICAL INTERIOR WALL TERMINATION AT SLAB OR SLAB BAND



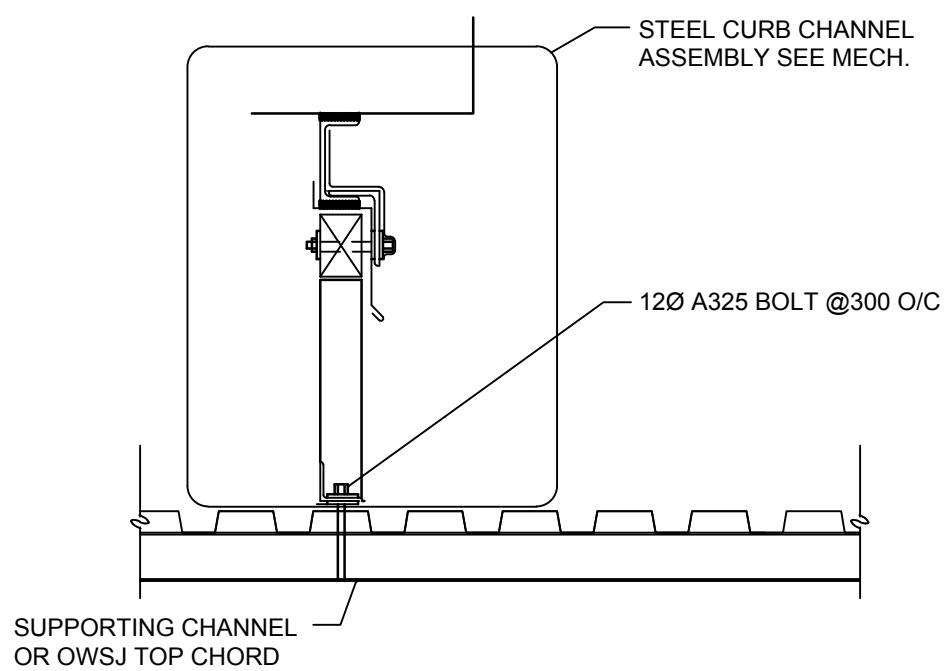
TYPICAL STAIR SUPPORT ON SLAB



TYPICAL BLOCKWALL INTERSECTIONS

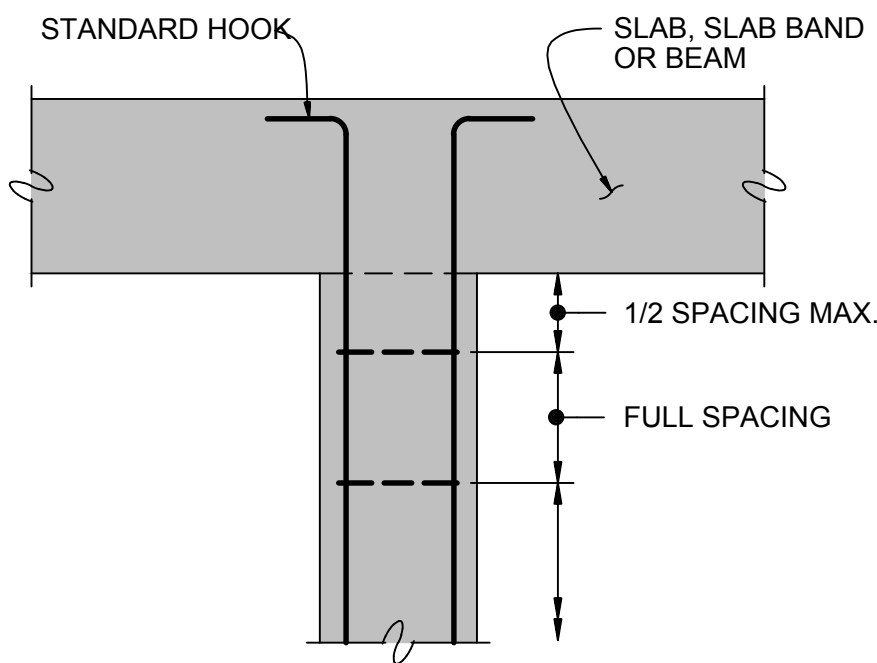


STEEL STRINGER BOTTOM DETAIL

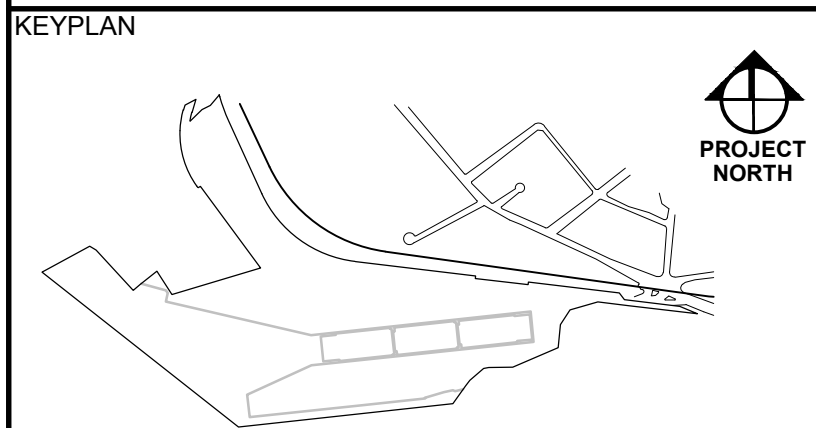


(FOR LOCATION AND SIZE OF PADS INCLUDING ANCHOR BOLTS FOR EQUIPMENT SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS)

TYPICAL MECHANICAL CURB ON ROOF DECK



TYPICAL COLUMN TERMINATION



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Client/client

ESQUIMALT GRAVING DOCK

825 ADMIRALS ROAD
VICTORIA, BC, V9A 2P1

Project title/Titre du projet
825 ADMIRALS ROAD VICTORIA BC
ESQUIMALT GRAVING DOCK
ELECTRICAL SAFETY UPGRADE

SOUTH SUBSTATION
SWITCHGEAR
REPLACEMENT PROJECT
(SSSR)

Consultant Signature Box Only

Designed by/Concept par
P.L./L.L.

Drawn by/Dessiné par
M.C.

PWGSC Project Manager/Administrateur de Projets TPSGC
France Veillette

PWGSC, Regional Manager, Architectural and Engineering Services/
Gestionnaire régionale, Services d'architecture et de génie, TPSGC
Preetipal Paul

Drawing title/Titre du dessin

TYPICAL DETAILS

| | | |
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| Project No./No. du projet | Sheet/Feuille | Revision no./ La Révision no. |
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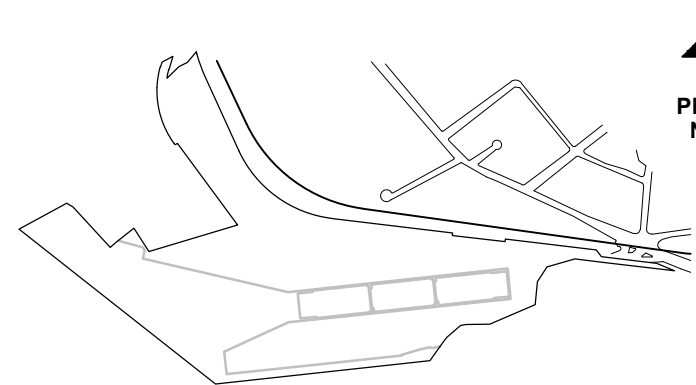
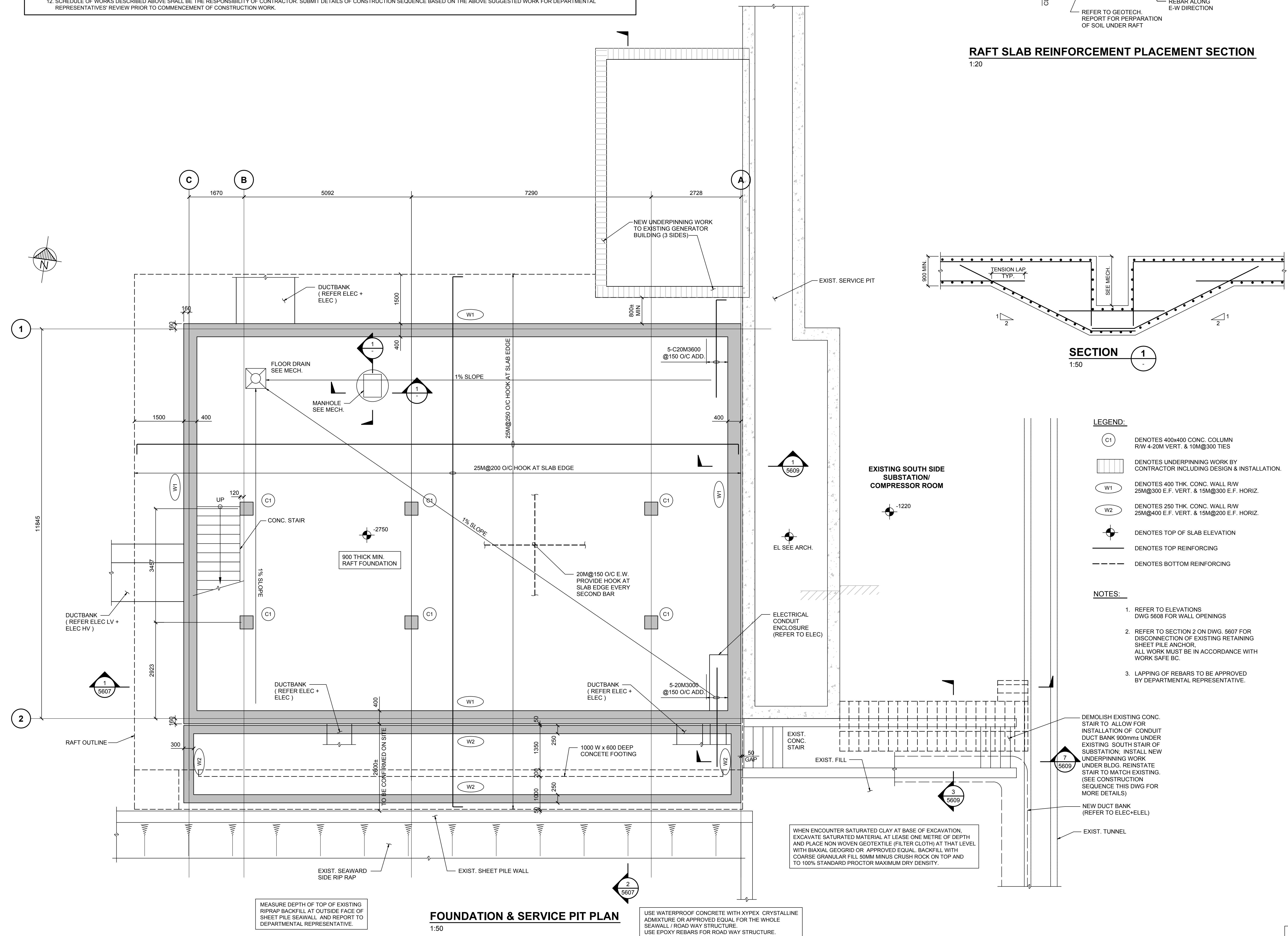
12355
12320

R.062548.002

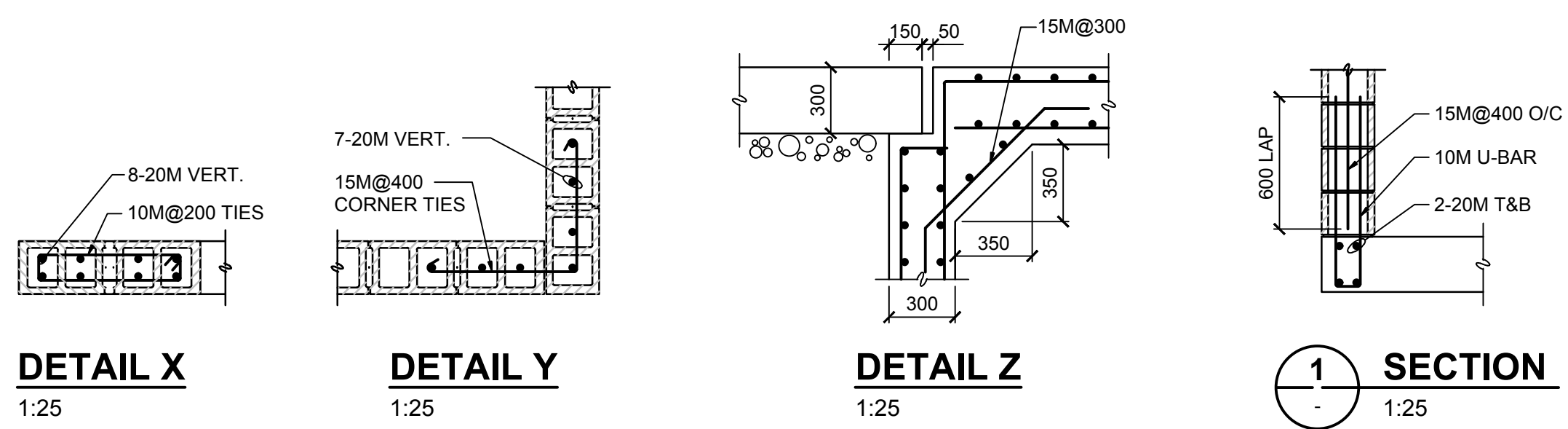
5602

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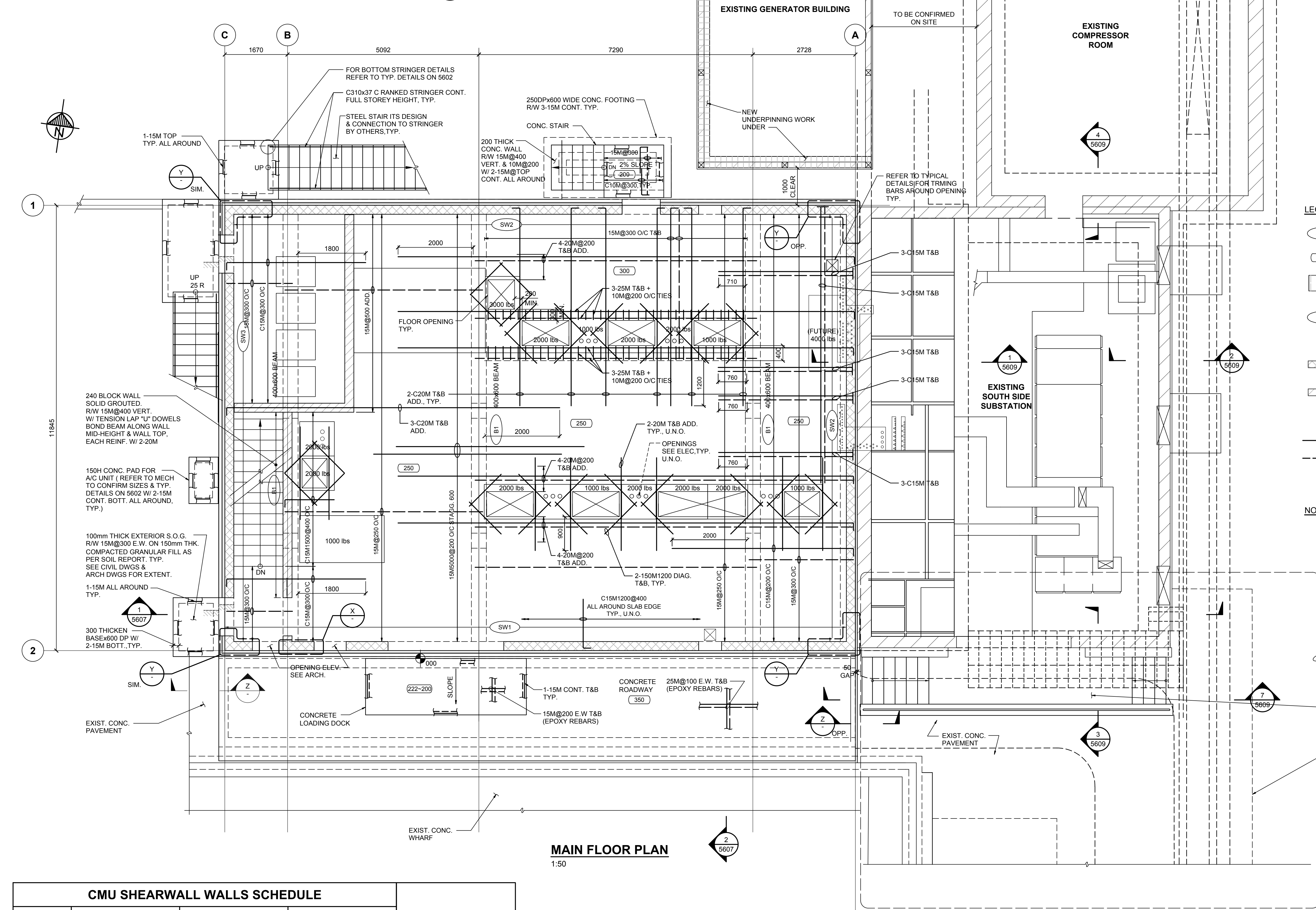
- SUGGESTED CONSTRUCTION SEQUENCE FOR DISCONNECTION OF THE RODS/EXCAVATION WORK/RAFT SLAB & NEW SOUTH SIDE PAVEMENT/DUCT BANK INSTALLATION ALONG SOUTH SIDE STAIR OF EXISTING SUBSTATION:
1. THE SUGGESTED CONSTRUCTION SEQUENCE IS FOP GUIDE LINE ONLY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL TEMPORARY WORK DESIGN TO MEET WORK SAFE B.C. REQUIREMENTS OF ALL SITE SAFETY BUT NOT LIMIT TO SAFETY OF OTHER USES IN GRAVING DOCK. EXISTING STRUCTURES REQUIREMENTS AS DESCRIBED IN GENERAL NOTES SHALL ALSO BE OBSERVED.
 2. MARK LOCATIONS OF EXISTING TIE RODS ACROSS LENGTH OF NEW SOUTH SUBSTATION SWITCHGEAR REPLACEMENT SITE AND SOUTH SIDE OF EXISTING STAIRS OF SUBSTATION.
 3. AVOID ANY DISTURBANCE TO THE TIE RODS. EXCAVATE NORTH SIDE OF SHEET PILE WALL WITH CARE TO FOOTPRINT OF BASE OF RAFT SLAB OF NEW SOUTH SUBSTATION SWITCHGEAR REPLACEMENT AND DEMOLISH EXISTING SOUTH SIDE STAIR OF EXISTING SUBSTATION FOR INSTALLATION OF DUCT BANKS.
 4. INSTALL UNDERPINNING WORK IN RAFT SLAB OF EXISTING GENERATOR BUILDING, FOOTPRINT OF NEW SOUTH SIDE SWITCHGEAR REPLACEMENT BUILDING AND WALL/FOOTING ALONG SOUTH SIDE OF EXISTING SUBSTATION AS PER PLANS AND SECTIONS. EXERCISE WITH CARE TO PROTECT THE EXISTING TIE RODS WITHIN THE SAID FOOTPRINT AND INSTALL SEQUENTIALLY IN SECTIONS OF UNDERPINNING WORK. CONTRACTOR SHALL BE RESPONSIBLE TO KEEP TEMPORARY BACK SLOPE AS STEEP AS POSSIBLE. PLACE CONCRETE UNDER A BIT OF PRESSURE FROM TOP OF SLAB LEVEL TO AVOID ANY VOID BEHIND THE WALL. DISCONNECT EXISTING TIE RODS IN THE VICINITY OF NEW SOUTH SUBSTATION SWITCHGEAR REPLACEMENT FOOT PRINT.
 5. FIX REBARS AND PLACE CONCRETE OF RAFT SLAB; UPON COMPLETION OF SERVICE PIT WALLS/COLUMNS/MAIN FLOOR SLAB. WATERPROOF THE NEW SOUTH SUBSTATION SWITCHGEAR REPLACEMENT BUILDING TO ARCHITECT'S SPECIFICATION.
 6. CONSTRUCT SOUTH SIDE NEW WALLS/ ROAD PAVEMENT BESIDE EXISTING SHEET PILE SEAWALL UPON COMPLETION OF THE MENTIONED WATERPROOF WORK.
 7. INSTALL DUCT BANKS WITH CONCRETE CASING (BY OTHERS) THAT RUNS SOUTH OF EXISTING STAIR OF SUBSTATION ONTO VOID BOX UNDER NEW ROADWAY PAVEMENT ROUTING INTO THE NEW SOUTH SUBSTATION SWITCHGEAR REPLACEMENT.
 8. POUR MASS CONCRETE PRIOR TO REINSTATE PREVIOUSLY DEMOLISHED SOUTH SIDE STAIR OF EXISTING SUBSTATION (SEE DRAWING 5603.5604 & 5608 FOR DETAILS).
 9. BACKFILL THE VICINITY OF EXCAVATED AREA AS PER GEOTECHNICAL RECOMMENDATIONS.
 10. COMPLETE CONSTRUCTION JOINTS EAST AND WEST SIDE OF EXISTING CONCRETE PAVEMENT TO NEW ROAD PAVEMENT IN SOUTH SIDE AND OF NEW SOUTH SIDE SWITCHGEAR REPLACEMENT AND WITHIN THE VICINITY BETWEEN NEW AND EXISTING CONCRETE PAVEMENT OF EXISTING SUBSTATION OF NEW DUCT BANKS & SOUTH SIDE STAIR OF EXISTING SUBSTATION.
 11. SCHEDULE OF WORKS DESCRIBED ABOVE SHALL BE THE RESPONSIBILITY OF CONTRACTOR. SUBMIT DETAILS OF CONSTRUCTION SEQUENCE BASED ON THE ABOVE SUGGESTED WORK FOR DEPARTMENTAL REPRESENTATIVES' REVIEW PRIOR TO COMMENCEMENT OF CONSTRUCTION WORK.
 - 12.



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| Client/client | | |
| ESQUIMALT GRAVING DOCK | | |
| 825 ADMIRALS ROAD VICTORIA, BC, V9A 2P1 | | |
| Project title/Titre du projet 825 ADMIRALS ROAD VICTORIA BC ESQUIMALT GRAVING DOCK ELECTRICAL SAFETY UPGRADE | | |
| SOUTH SUBSTATION SWITCHGEAR REPLACEMENT PROJECT (SSSR) | | |
| Consultant Signature Box Only | | |
| Designed by/Concept par P.L. / L.L. | | |
| Drawn by/Dessiné par M.C. | | |
| PWGSC Project Manager/Administrateur de Projets TPSGC France Veillette | | |
| PWGSC, Regional Manager, Architectural and Engineering Services/ Gestionnaire régionale, Services d'architecture et de génie, TPSGC Preetipal Paul | | |
| Drawing title/Titre du dessin FOUNDATION & SERVICE PIT PLAN | | |
| Project No./No. du projet | Sheet/Feuille | Revision no./ La Révision no. |
| 12355 | R.062548.002 | 5603 |
| 12320 | | 3 |



DETAIL X 1:25
DETAIL Y 1:25
DETAIL Z 1:25
SECTION 1 1:25



- LEGEND:**
- SW1 DENOTES SHEARWALL TYPE. SEE SCHEDULE ON THIS DWG.
 - 250 DENOTES CONCRETE SLAB THICKNESS.
 - [Hatched Box] DENOTES UNDERPINNING WORK BY CONTRACTOR INCLUDING DESIGN & INSTALLATION.
 - B1 DENOTES 400x600 DP. CONC. BEAM R/W 4-25M T&B CONT. HOOKED TOP BARS AT EACH END, WITH 7-10M@150 O/C TIES EACH SIDE OF COLUMNS & END WALL + 10M@300 TIES IN BETWEEN.
 - [Cross-hatched Box] DENOTES 240 CMU LOAD-BEARING WALLS CLASSIFICATION TO BE H/20/A/M.
 - [Diagonal-hatched Box] DENOTES 240 CMU PARTITION WALLS CLASSIFICATION TO BE H/15/A/M. SEE SPECIFICATION FOR REINF. DETAILS SEE TYPICAL DETAILS FOR TOP & BOTTOM CONNECTION.
 - DENOTES TOP REINFORCING
 - - - DENOTES BOTTOM REINFORCING
- NOTES:**
- REFER TO ELEVATIONS ON DWG 5608 FOR WALL OPENINGS.
 - PROVIDE 15M@400 T&B SHRINKAGE BARS U.N.O.
 - LAPPING OF REBARS TO BE APPROVED BY DEPARTMENTAL REPRESENTATIVE.
 - ADHESIVE ANCHORS CONNECTING SUPPORTS OF HANDRAIL ASSEMBLY TO CONCRETE STAIRS TO BE APPROVED BY DEPARTMENTAL REPRESENTATIVE.
- REFER TO 5603 FOR SCOPE OF NEW WORK DETAILS IN THIS AREA
- EXIST. CONC. STAIR
- SUGGESTED APPROX. JOINT FOR EXTENT OF MATCHING CONC. PAVEMENT

| CMU SHEARWALL WALLS SCHEDULE | | | | NOTE: REFER TO GENERAL NOTES (MASONRY) FOR JOINT LADDER REINFORCING & OTHER TYPICAL REQUIREMENTS. ALL WALL VERTICAL REINFORCEMENT EXTEND TO TOP OF PARAPET. SOLID GROUT BOND BEAM AT TOP. REINFORCED WITH 2-15M CONT. |
|------------------------------|---|---|---|--|
| | SW1 | SW2 | SW3 | |
| ROOF TO 2ND FLOOR | PARTIAL GROUTED W/ 20M@600 O/C VERT. & BOND BEAM @2000 MAX. O/C EACH BOND BEAM R/W 2-20M CONT. SOLID GROUT CELL W/ REINF., U.N.O. | PARTIAL GROUTED W/ 20M@600 O/C VERT. & BOND BEAM @2000 MAX. O/C EACH BOND BEAM R/W 2-20M CONT. SOLID GROUT CELL W/ REINF., U.N.O. | SOLID GROUTED W/ 20M@600 O/C VERT. & BOND BEAM @2000 MAX. O/C EACH BOND BEAM R/W 2-20M CONT. GROUT CELL W/ REINF., U.N.O. | |
| 2ND FLOOR TO MAIN FLOOR | SOLID GROUTED W/ 20M@400 O/C VERT. & BOND BEAM @1200 MAX. O/C EACH BOND BEAM R/W 2-20M CONT. | PARTIAL GROUTED W/ 20M@600 O/C VERT. & BOND BEAM @1200 MAX. O/C EACH BOND BEAM R/W 2-20M CONT. | SOLID GROUTED W/ 20M@400 O/C VERT. & BOND BEAM @1200 MAX. O/C EACH BOND BEAM R/W 2-20M CONT. | |

MAIN FLOOR PLAN 1:50

NEW SUBSTATION SWITCHGEAR REPLACEMENT

EXISTING SUBSTATION

Public Works and Government Services Canada
Travaux publics et Services gouvernementaux Canada

REAL PROPERTY SERVICES
Pacific Region
SERVICES IMMOBILIERS
Region de Pacifique

CWMM CONSULTING ENGINEERS LTD.

KEYPLAN

ESQUIMALT GRAVING DOCK

825 ADMIRALS ROAD
VICTORIA, BC, V9A 2P1

Project title/Titre du projet
825 ADMIRALS ROAD VICTORIA BC
ESQUIMALT GRAVING DOCK
ELECTRICAL SAFETY UPGRADE

SOUTH SUBSTATION
SWITCHGEAR
REPLACEMENT PROJECT
(SSSR)

Consultant Signature Box Only

Designed by/Concept par
P.L./L.L.

Drawn by/Dessiné par
M.C.

PWGSC Project Manager/Administrateur de Projets TPSGC
France Veillette

PWGSC, Regional Manager, Architectural and Engineering Services/
Gestionnaire régionale, Services d'architecture et de génie, TPSGC
Preetipal Paul

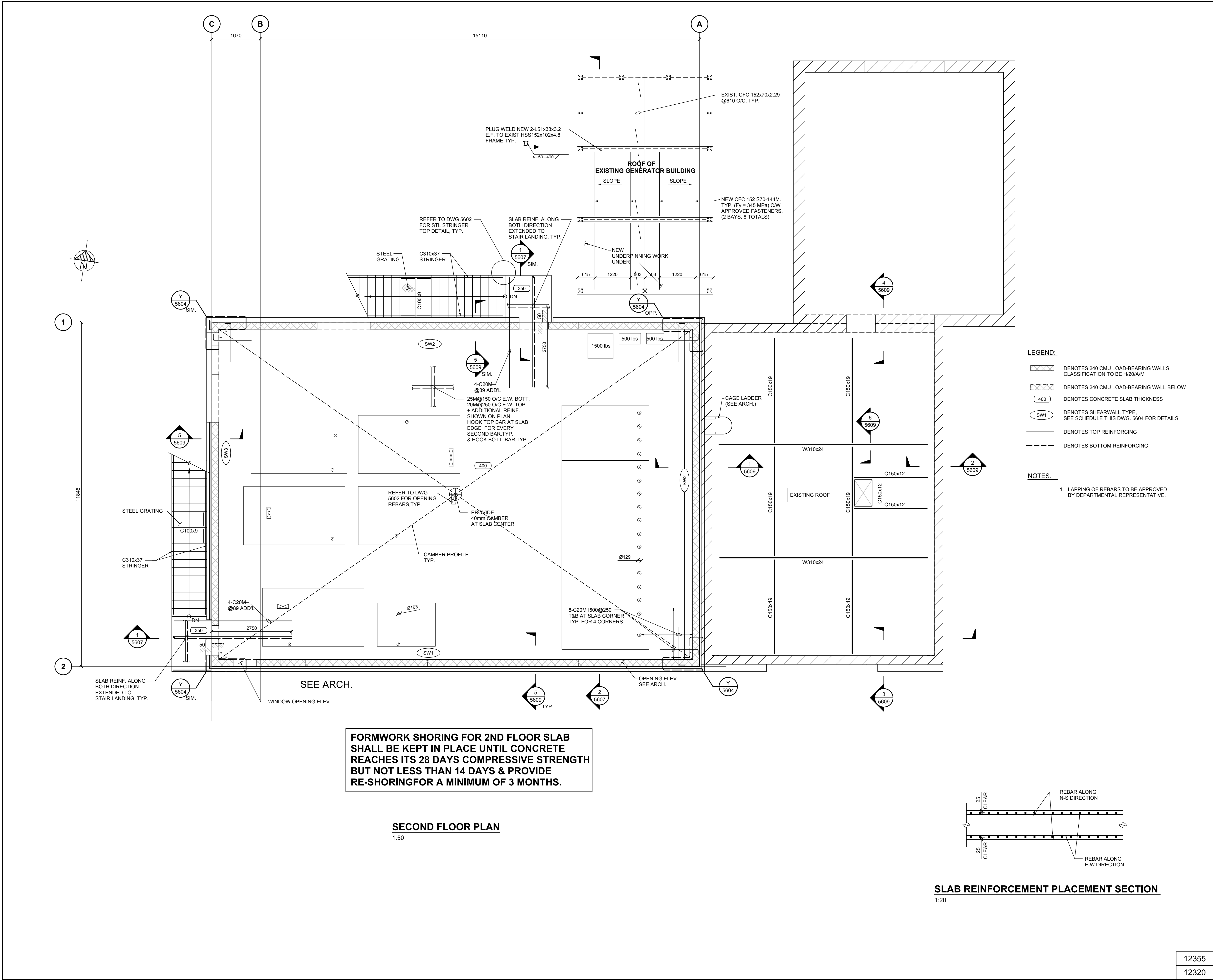
Drawing title/Titre du dessin
MAIN FLOOR PLAN & DETAILS

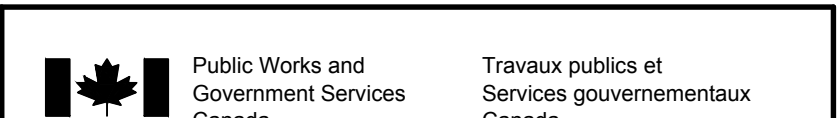
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12355

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5604

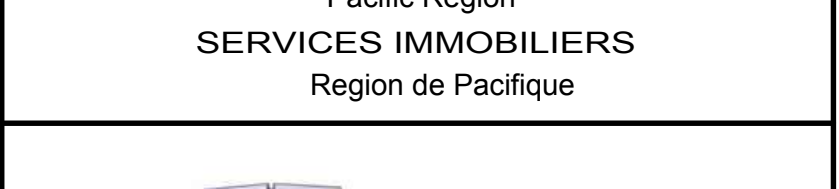
Revision no./
La Révision no.
3





REAL PROPERTY SERVICES
Pacific Region
SERVICES IMMOBILIERS
Region de Pacifique



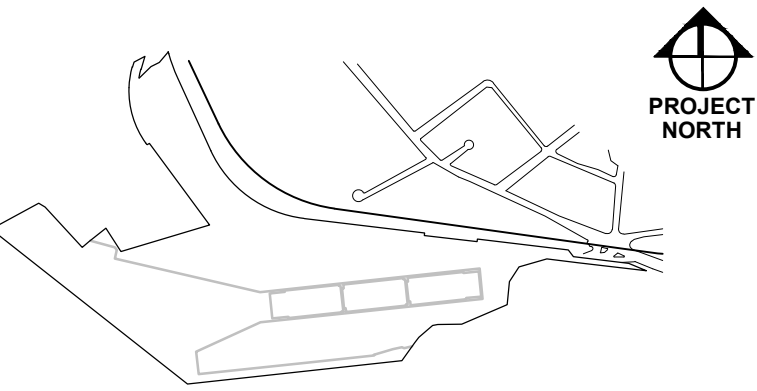
KEYPLAN

PROJECT NORTH

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| 825 ADMIRALS ROAD VICTORIA, BC, V9A 2P1 | | |
| Project title/Titre du projet 825 ADMIRALS ROAD VICTORIA BC ESQUIMALT GRAVING DOCK ELECTRICAL SAFETY UPGRADE | | |
| SOUTH SUBSTATION SWITCHGEAR REPLACEMENT PROJECT (SSSR) | | |
| Consultant Signature Box Only | | |
| Designed by/Concept par P.L./L.L. | | |
| Drawn by/Dessiné par M.C. | | |
| PWGSC Project Manager/Administrateur de Projets TPSGC France Veillette | | |
| PWGSC, Regional Manager, Architectural and Engineering Services/ Gestionnaire régionale, Services d'architecture et de génie, TPSGC Preetipal Paul | | |
| Drawing title/Titre du dessin SECOND FLOOR PLAN | | |
| Project No./No. du projet | Sheet/Feuille | Revision no./ La Révision no. |
| 12355 | R.062548.002 | 5605 |
| 12320 | | 3 |

PWGSC - 81 - 1000x707

KEYPLAN



LEGEND:

■ DENOTES 240 CMU LOAD-BEARING WALLS
CLASSIFICATION TO BE H/20/A/M

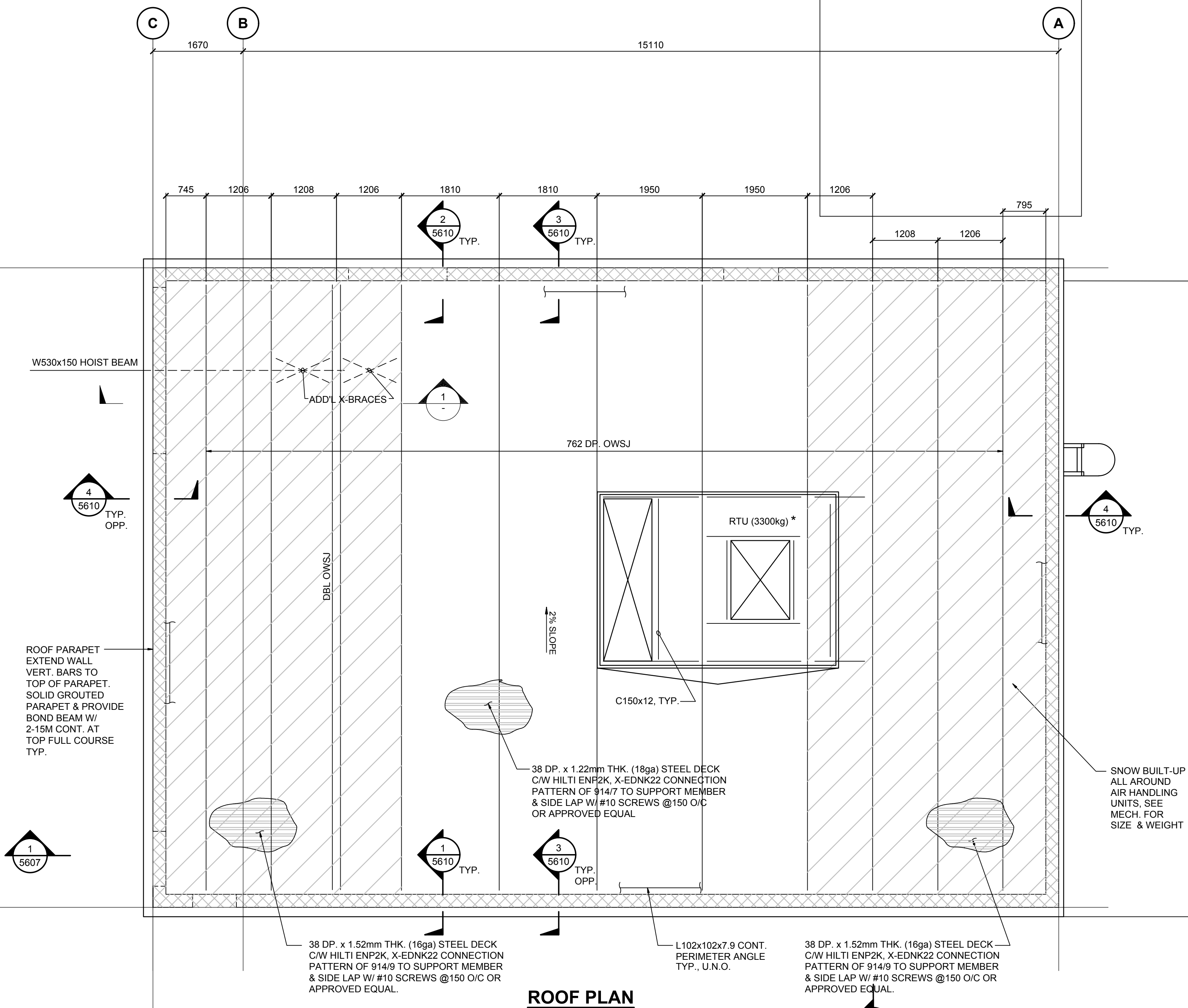
* DENOTES DIMENSIONS TO BE
CONFIRMED WITH MECHANICAL

NOTE:

1. ADHESIVE ANCHORS CONNECTING SUPPORTS
OF CAT LADDER TO EXTERIOR CMU BLOCK WALL
TO BE APPROVED BY DEPARTMENTAL
REPRESENTATIVE.

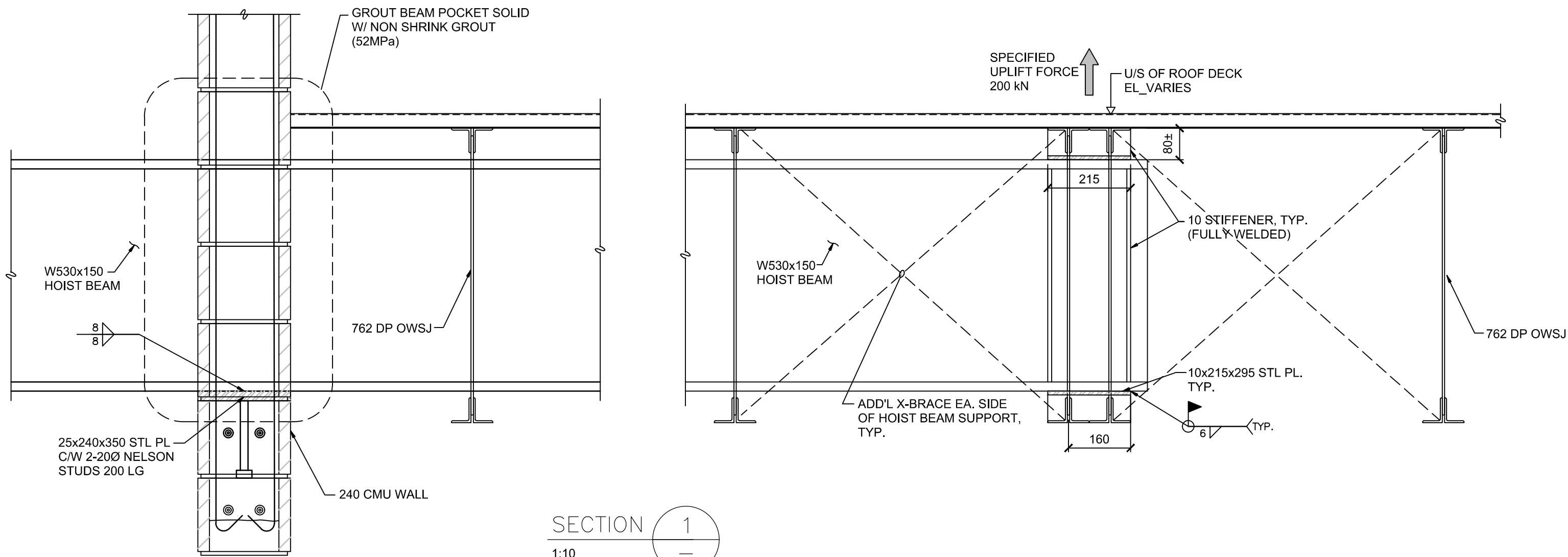
ROOF PLAN

1:50



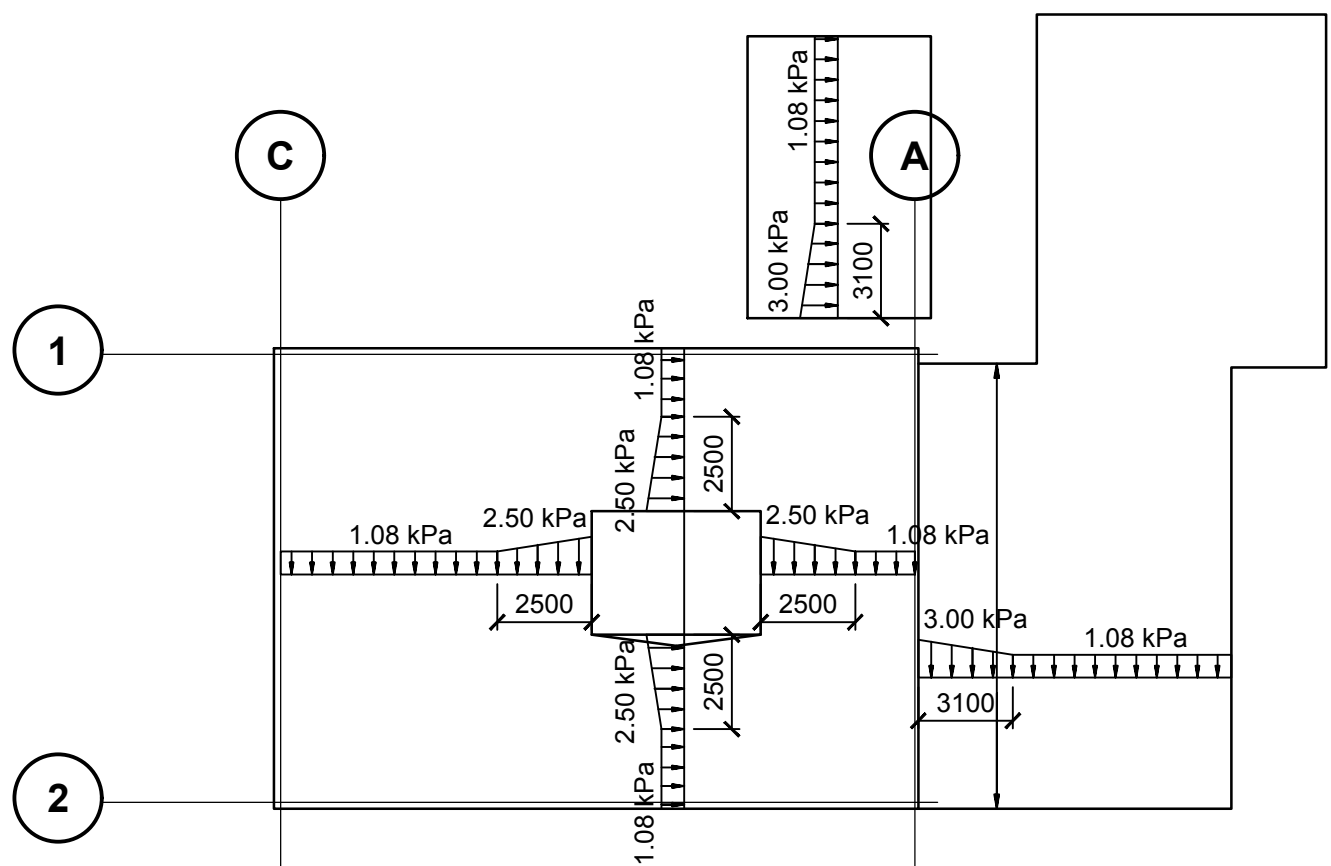
SECTION 1

1:10



ROOF SNOW LOAD DIAPHRAGM

1:200



1. ROOF SNOW LOAD = 1.08 kPa U.N.O.
Is=1.0
ADJUST FACTOR AS REQUIRED
2. DESIGN OWSJ WITH NET UPLIFT
= 1.35 kPa

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Client/client

ESQUIMALT
GRAVING DOCK

825 ADMIRALS ROAD
VICTORIA, BC, V9A 2P1

Project title/Titre du projet
825 ADMIRALS ROAD VICTORIA BC
ESQUIMALT GRAVING DOCK
ELECTRICAL SAFETY UPGRADE

SOUTH SUBSTATION
SWITCHGEAR
REPLACEMENT PROJECT
(SSSR)

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PWSC Project Manager/Administrateur de Projets TPSGC
France Veillette

PWSC, Regional Manager, Architectural and Engineering Services/
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Preetipal Paul

Drawing title/Titre du dessin

ROOF PLAN

Project No./No. du projet

12355

12320

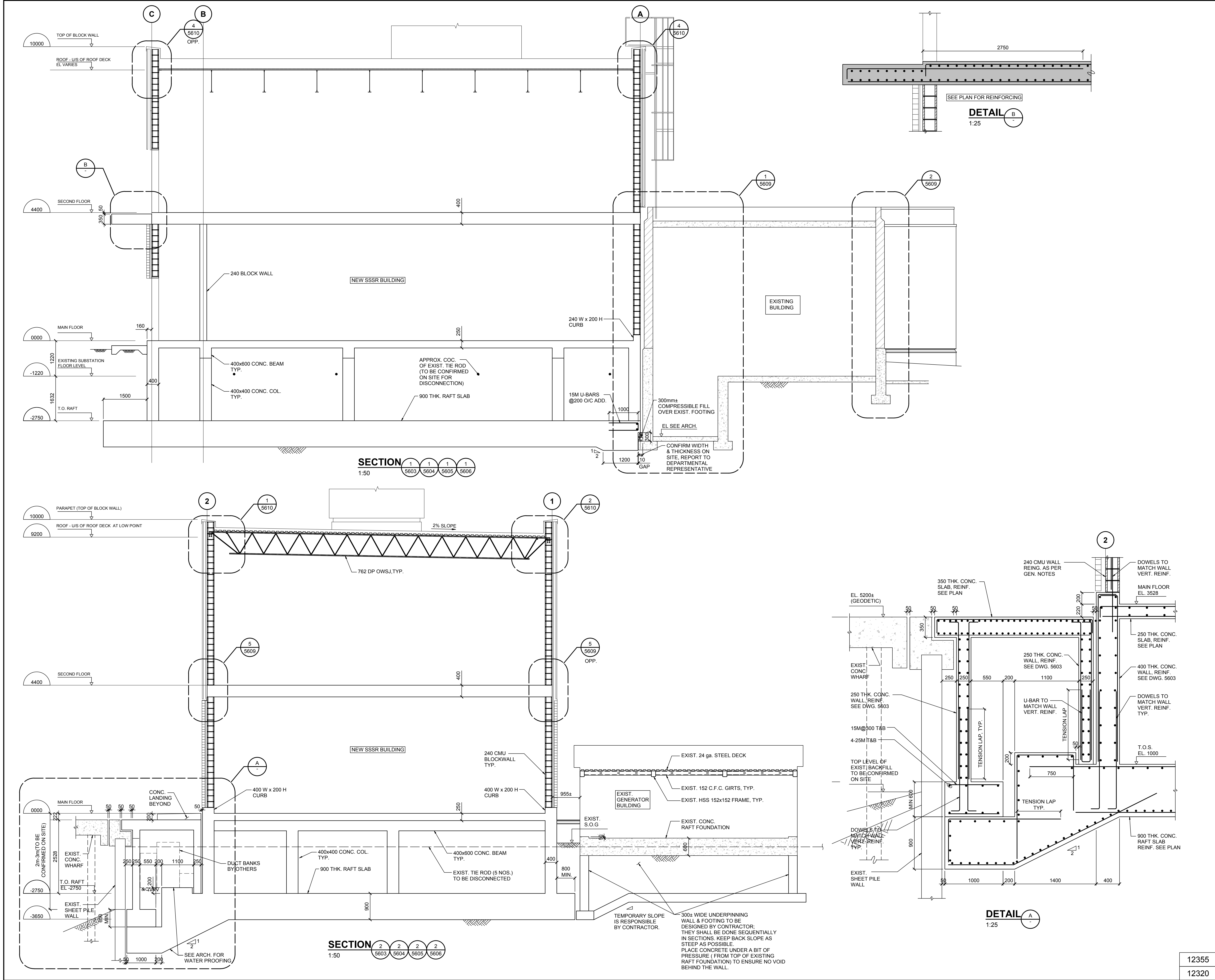
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5606


Revision no./
La Révision
no.

3

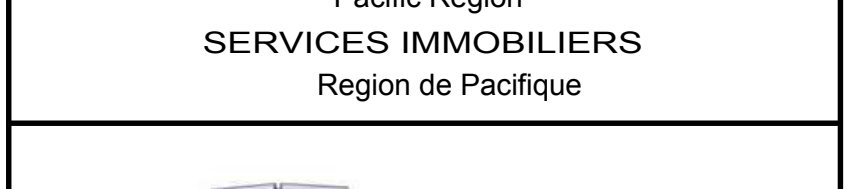




REAL PROPERTY SERVICES
Pacific Region
SERVICES IMMOBILIERS
Région de Pacifique



KEYPLAN



PROJECT NORTH

| Revision/ | Description/Description | Date/Date |
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ELECTRICAL SAFETY UPGRADE**

**SOUTH SUBSTATION
SWITCHGEAR
REPLACEMENT PROJECT
(SSSR)**

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P.L./L.L.

Drawn by/Dessiné par
M.C.

PWGSC Project Manager/Administrateur de Projets TPSGC
France Veillette

PWGSC, Regional Manager, Architectural and Engineering Services/
Gestionnaire régionale, Services d'architecture et de génie, TPSGC
Preetipal Paul

Drawing title/Titre du dessin
BUILDING SECTIONS

| Project No./No. du projet | Sheet/Feuille | Revision no./ La Révision no. |
|---------------------------|---------------|-------------------------------------|
| 12355 | R.062548.002 | 5607 |
| 12320 | | 3 |

Client/client

ESQUIMALT GRAVING DOCK

825 ADMIRALS ROAD
VICTORIA, BC, V9A 2P1

Project title/Titre du projet
825 ADMIRALS ROAD VICTORIA BC
ESQUIMALT GRAVING DOCK
ELECTRICAL SAFETY UPGRADE

**SOUTH SUBSTATION
SWITCHGEAR
REPLACEMENT PROJECT
(SSSR)**

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France Veillette

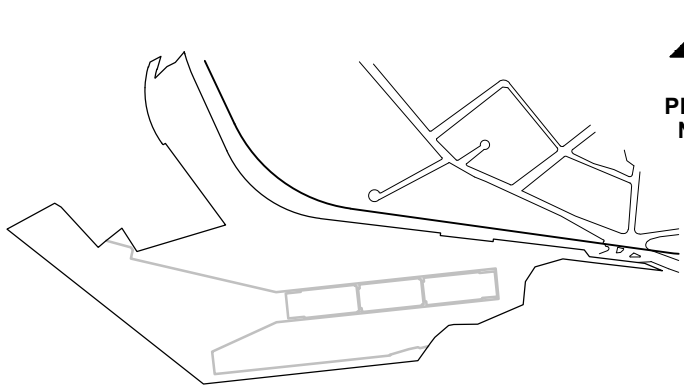
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Preetipal Paul

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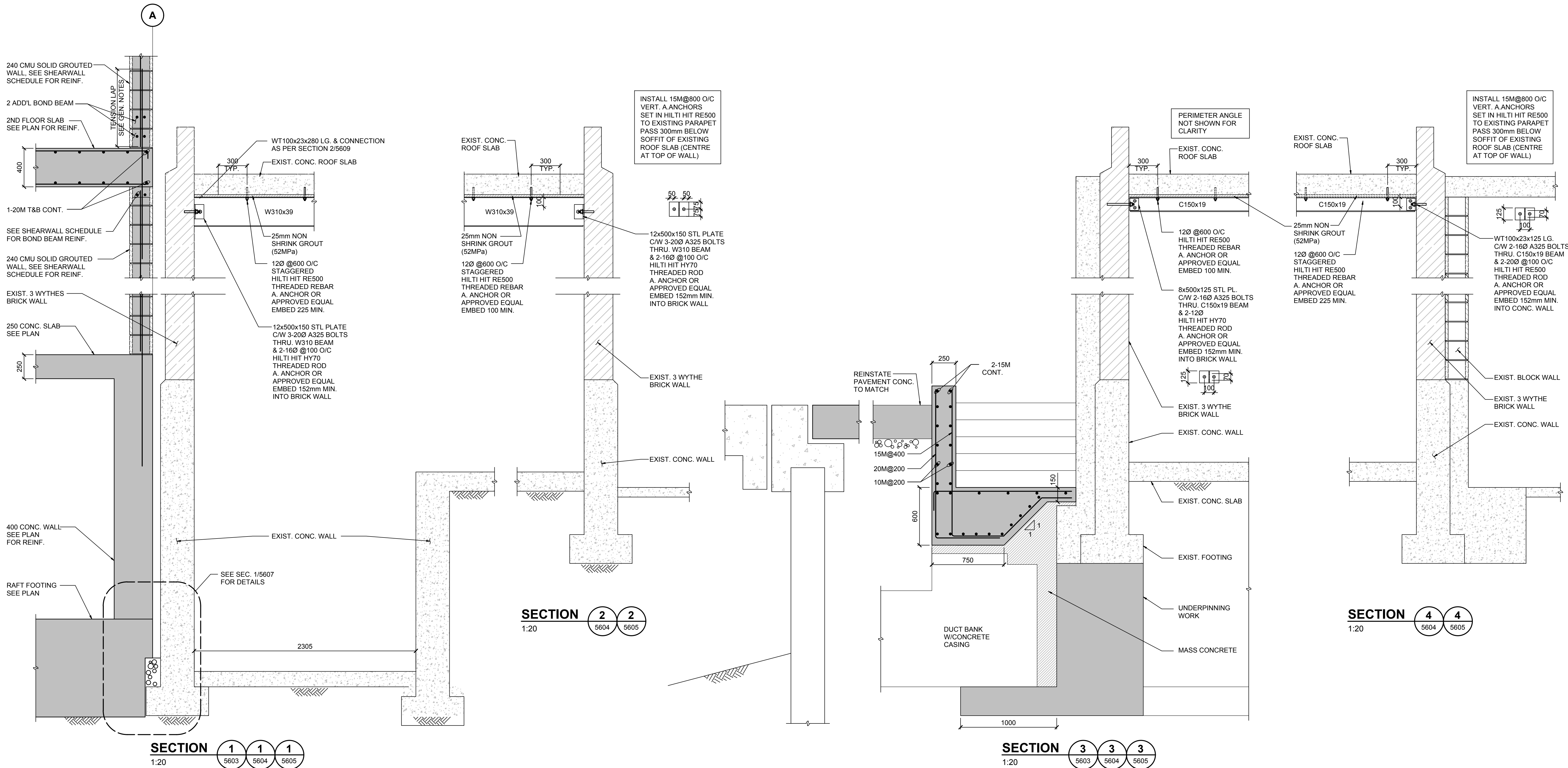
ELEVATIONS

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| R.062548.002 | 5608 | 3 |

KEYPLAN



PROJECT
NORTH



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ELECTRICAL SAFETY UPGRADE

SOUTH SUBSTATION SWITCHGEAR REPLACEMENT PROJECT (SSSR)

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PWGSC Project Manager/Administrateur de Projets TPSGC
France Veillette

PWGSC, Regional Manager, Architectural and Engineering Services/
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Preetipal Paul

Drawing title/Titre du dessin

SECTIONS & DETAILS SHEET 1

Project No./No. du projet

12355

12320

R.062548.002

Sheet/Feuille

5609

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