

GENERAL

- THIS SET OF DRAWINGS SHALL BE READ IN CONJUNCTION WITH THE STRUCTURAL SPECIFICATIONS AND WITH THE DRAWINGS AND SPECIFICATIONS FROM ALL OTHER CONSULTANTS. ANY DISCREPANCIES NOTED SHALL BE REPORTED IMMEDIATELY FOR CLARIFICATION.
- THIS SET OF DRAWINGS SHOWS THE COMPLETED STRUCTURE AND DOES NOT SHOW WORK WHICH MAY BE REQUIRED FOR SAFETY DURING CONSTRUCTION. THE CONTRACTOR IS RESPONSIBLE FOR GENERAL SAFETY ON AND ABOUT THE JOB SITE DURING THE CONSTRUCTION PERIOD AND FOR DESIGN AND ERECTION OF ALL FALSEWORK, SHORING, BRACING ETC. TO ENSURE THE SAFETY OF ALL CONSTRUCTION TEMPORARY LOADS AND TO COMPLETE THE WORK. ALL TEMPORARY WORKS AND SHORING ETC. SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN BRITISH COLUMBIA. ADHERE STRICTLY TO ALL REQUIREMENTS OF THE WORKSAFE BRITISH COLUMBIA.
- ALL CODE REFERENCES ARE TO LATEST EDITIONS AS REFERENCED IN THE NBC 2010.

FIELD REVIEW:

- DEPARTMENTAL REPRESENTATIVE PROVIDES FIELD REVIEW FOR THE WORK SHOWN ON THE STRUCTURAL DRAWINGS PREPARED BY DEPARTMENTAL REPRESENTATIVE. THIS REVIEW IS A PERIODIC REVIEW AT THE PROFESSIONAL JUDGMENT OF DEPARTMENTAL REPRESENTATIVE. THE PURPOSE IS TO ASCERTAIN THAT THE WORK IS IN GENERAL CONFORMANCE WITH THE PLANS AND SUPPORTING DOCUMENTS PREPARED BY DEPARTMENTAL REPRESENTATIVE. AND TO FULFILL THE REQUIREMENTS FOR THE COMPLETION OF LETTERS OF ASSURANCE REQUIRED BY THE APPLICABLE BUILDING CODE.
- ALL NON-CONFORMING WORKS THAT REQUIRE REMEDIAL ACTION SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. ANY EXTRA TIME OR COST INCURRED TO DEPARTMENTAL REPRESENTATIVE TO ASSIST OR ADVISE THE CONTRACTOR IN RECTIFYING THE WORK SHALL BE BORNE BY THE CONTRACTOR.
- ENSURE THAT WORK TO BE INSPECTED IS COMPLETE AT THE TIME OF INSPECTION AND IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. ADDITIONAL INSPECTIONS REQUIRED DUE TO THE INCOMPLETE WORK OR POORLY EXECUTED WORK, AS JUDGED BY DEPARTMENTAL REPRESENTATIVE, AS WELL AS ADDITIONAL DESIGN OR REMEDIAL WORK CAUSED BY DEVIATIONS FROM THESE DRAWINGS MAY BE CHARGED TO THE GENERAL CONTRACTOR AT THE DISCRETION OF DEPARTMENTAL REPRESENTATIVE.
- A MINIMUM 72 HOURS NOTICE SHALL BE GIVEN BY THE CONTRACTOR FOR ANY INSPECTION TO BE CARRIED OUT BY DEPARTMENTAL REPRESENTATIVE.

SHOP DRAWINGS:

- DESIGNERS & MANUFACTURERS OF ALL STRUCTURAL ELEMENTS/COMPONENTS/CONNECTIONS SHALL SUBMIT COMPLETE SHOP DRAWINGS SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF BRITISH COLUMBIA TO THE DEPARTMENTAL REPRESENTATIVE FOR REVIEW PRIOR TO FABRICATION. SUBMIT SHOP DRAWINGS IN ACCORDANCE WITH SPECIFICATIONS AND TO ALLOW MINIMUM TWO WEEKS FOR REVIEW. THIS SUBMISSION OR ITS REVIEW SHALL NOT RELIEVE THE CONTRACTOR OF HIS RESPONSIBILITY FOR PROVIDING PROPER ENGINEERING DESIGN, METHODS, EQUIPMENT, WORKMANSHIP, SAFETY PRECAUTION AND PRIOR REVIEW OF THESE ELEMENTS. THE PROFESSIONAL ENGINEER SEALING THE SHOP DRAWINGS SHALL BE RESPONSIBLE FOR INSPECTION OF HIS DESIGN COMPONENTS FOR CONFORMANCE WITH HIS DESIGN AND SHOP DRAWINGS.
- THE CONTRACTOR AND ITS SUBCONTRACTORS SHALL CONFIRM AND COORDINATE DIMENSIONS, LOCATIONS AND NUMBER OF THE STRUCTURAL ELEMENTS FOR WHICH SHOP DRAWINGS ARE TO BE PRODUCED.

NON-STRUCTURAL COMPONENTS:

- NON-STRUCTURAL COMPONENTS ARE NOT THE RESPONSIBILITY OF DEPARTMENTAL REPRESENTATIVE SUCH COMPONENTS OF THE PROJECT ARE DESIGNED, DETAILED, SPECIFIED AND REVIEWED IN THE FIELD BY OTHERS. LETTERS OF CERTIFICATION OF ADEQUACY, INSTALLATION ETC. OF SUCH COMPONENTS ARE BY OTHERS.
- MANUFACTURERS OF NON-STRUCTURAL COMPONENTS WHICH AFFECT THE STRUCTURAL FRAMING SHALL SUBMIT SHOP DRAWINGS TO THE ARCHITECT AND DEPARTMENTAL REPRESENTATIVE FOR REVIEW. THE SHOP DRAWINGS SHALL CLEARLY INDICATE LOADS IMPOSED ON THE STRUCTURE. REVIEW WILL BE LIMITED TO THE EFFECT OF THE COMPONENTS ON THE STRUCTURAL FRAMING.
- EXAMPLES OF NON-STRUCTURAL COMPONENTS INCLUDE, BUT ARE NOT LIMITED TO:
 - ARCHITECTURAL COMPONENTS SUCH AS HANDRAILS, GUARDRAILS, RAILINGS, FLAG POST, REMOVABLE CANOPIES, CEILINGS, VEHICLE PROTECTION SYSTEMS, ORNAMENTAL COMPONENTS, ETC.
 - ARCHITECTURAL PRECAST CONCRETE AND ITS ATTACHMENTS.
 - ARCHITECTURAL GLASS BLOCKS AND THEIR ATTACHMENTS.
 - BRICK AND BLOCK VENEERS, THEIR REINFORCING IF ANY AND TIES
 - LANDSCAPING COMPONENTS SUCH AS BENCHES, LIGHT POSTS, PLANTERS, ETC.
 - CURTAIN WALL SYSTEMS, CLADDING, SKYLIGHT, WINDOW MULLIONS, ETC.
 - INTERIOR AND EXTERIOR NON-LOAD BEARING STEEL STUD WALLS
 - SUPPORT AND BRACING OF MECHANICAL AND ELECTRICAL SYSTEMS AND EQUIPMENTS FOR NON-GRAVITY AND SEISMIC LOADS.
 - WINDOW WASHING EQUIPMENTS AND ITS ATTACHMENT.
 - ELEVATORS, ESCALATORS AND OTHER CONVEYING SYSTEMS, INCLUDING PROPRIETARY SUPPORT BEAMS AND THEIR ATTACHMENTS.
 - NON-STRUCTURAL MASONRY.
- NON-STRUCTURAL STEEL STUD FRAMING
 - INTERIOR AND EXTERIOR STEEL STUD WALLS AND OTHER ARCHITECTURAL FRAMING SHALL BE DESIGNED BY THE FABRICATOR. DESIGN SHALL BE BY A STRUCTURAL ENGINEER REGISTERED IN THE PROVINCE OF BRITISH COLUMBIA AND SHALL BE IN ACCORDANCE WITH PART 4 OF THE GOVERNING BUILDING CODE USING THE DESIGN LOADS REFERENCED ELSEWHERE ON THIS DRAWING. SEE ALSO ITEMS 1 AND 2 ABOVE.
 - UNLESS NOTED OTHERWISE, EXTERIOR STEEL STUDS FRAMING TO THE UNDERSIDE OF STRUCTURAL STEEL BEAMS OR TO STEEL BRACING MEMBERS SHALL BE DETAILED AND DESIGNED SO AS NOT TO IMPART LATERAL WIND AND SEISMIC LOADS TO THESE MEMBERS. WHERE WIND BEARING STUDS ATTACH TO STEEL BEAM BOTTOM FLANGES PROVIDE STEEL STUD BRACING IN GENERAL CONFORMANCE WITH DEPARTMENTAL REPRESENTATIVE'S TYPICAL DETAILS. DETAIL TOP TRACK TO ALLOW FOR ROOF/FLOOR DEFLECTIONS DUE TO GRAVITY LOADS.

EXISTING STRUCTURES:

- PRIOR TO CONSTRUCTION THE CONTRACTOR SHALL VERIFY ALL RELEVANT DIMENSIONS TO AND OF EXISTING STRUCTURES. NOTIFY DEPARTMENTAL REPRESENTATIVE IMMEDIATELY IF DISCREPANCIES ARE NOTED.
- THE CONTRACTOR SHALL AT HIS OWN EXPENSE REPAIR AND MAKE GOOD ANY DAMAGE TO THE EXISTING STRUCTURE, EQUIPMENT AND FINISHES CAUSED BY THE CONSTRUCTION ACTIVITIES. REPAIRS SHALL BE TO THE SATISFACTION OF THE DEPARTMENTAL REPRESENTATIVE.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE TEMPORARY SUPPORT OF ANY ADJACENT EXISTING STRUCTURES DURING CONSTRUCTION. UNDERPINNING OR BRACING SHALL BE DESIGNED BY A QUALIFIED PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF BRITISH COLUMBIA. 4 COPIES OF SIGNED AND SEALED DESIGN DRAWINGS TO DEPARTMENTAL REPRESENTATIVE. FOR REVIEW OF CONFORMANCE WITH GENERAL DESIGN CRITERIA.

DESIGN LOADS:

- THIS STRUCTURE HAS BEEN DESIGNED FOR SNOW, WIND AND SEISMIC FORCES IN SUBSTANTIAL COMPLIANCE WITH THE PROVISIONS SET FORTH IN THE NATIONAL BUILDING CODE 2010. IMPORTANT CATEGORY=NORMAL (AS PER OWNER'S REQUIREMENT).

GROUND SNOW:	S _s = 1.1 kPa								
RAIN LOAD:	S _r = 0.2 kPa								
IMPORTANCE FACTORS FOR SNOW	I _s = 1.0 FOR STRENGTH I _s = 0.9 FOR SERVICEABILITY								
WIND LOAD:	PROBABILITY 1/10 = 0.44 kPa PROBABILITY 1/50 = 0.57 kPa								
IMPORTANCE FACTORS FOR WIND	I _w = 1.0 FOR STRENGTH I _w = 0.75 FOR SERVICEABILITY								
EARTHQUAKE FACTORS:	<table border="1"> <tr> <td>S_a(0.2)</td> <td>S_a(0.5)</td> <td>S_a(1.0)</td> <td>S_a(2.0)</td> </tr> <tr> <td>1.20</td> <td>0.82</td> <td>0.38</td> <td>0.19</td> </tr> </table> <p>I_e = 1.0 FOR STRENGTH I_e = 1.0 FOR SERVICEABILITY (CLAUSE 4.1.8.5 FOR SERVICEABILITY) F_a = 0.9 F_v = 1.9 R_d = 1.5 R_o = 1.5</p>	S _a (0.2)	S _a (0.5)	S _a (1.0)	S _a (2.0)	1.20	0.82	0.38	0.19
S _a (0.2)	S _a (0.5)	S _a (1.0)	S _a (2.0)						
1.20	0.82	0.38	0.19						

- SITE CLASS F
- SPECIFIED UNIFORM SUPERIMPOSED DEAD LOADS ON ROOF AND FLOORS:

ROOF	1.0 kPa
MAIN FLOOR	1.0 kPa
MECHANICAL & ELECTRICAL ROOMS (GENERAL)	1.0 kPa
EXTERIOR WALLS	ACTUAL WEIGHT

 - UPPER FLOORS AND MAIN FLOOR LOADS INCLUDE GENERAL PARTITION LOAD OF 1.0kPa AND NON-STRUCTURAL CONCRETE TOPPING. FOR MASONRY PARTITIONS, ACTUAL WEIGHTS SHALL BE USED.
 - THESE LOADS DO NOT INCLUDE SELFWEIGHT OF STRUCTURE, WEIGHT OF MASONRY PARTITIONS, WEIGHTS OF MECHANICAL EQUIPMENT AND CONCRETE EQUIPMENT PADS.
- SPECIFIED UNIFORM LIVE LOADS ON FLOORS:

MAIN FLOOR / SERVICE PIT	4.8 kPa
SECOND FLOOR	7.2 kPa
DRIVE WAY	12 kPa
- DESIGN SPECIFIED CONCENTRATED LIVE LOADS ON ROOF AND FLOORS:

ROOF	1.3 kN
MAIN FLOOR / SERVICE PIT / SECOND FLOOR	9.0 kN
DRIVE WAY	54 kN
- WORST CASE OF UNIFORM OR CONCENTRATED LIVE LOADS WILL BE USED FOR DESIGN OF STRUCTURAL MEMBERS.

CONSTRUCTION LOADS:

- CONSTRUCTION LOADS ON COMPLETED FLOORS MUST NOT EXCEED THE LOAD CARRYING CAPACITY OF FLOOR AT THE TIME OF THE LOADING UNLESS IT IS PROPERLY SHORED TO SUPPORT THE INTENDED LOAD. MOVING OF HEAVY EQUIPMENT AND PILING UP OF MATERIAL SHALL NOT BE PERMITTED UNLESS DESIGNED SHORING IS IN PLACE.
- SHORING DESIGN BY CONTRACTOR. INFORM CWMM CONSULTING ENGINEERS LTD. PRIOR TO LOAD APPLICATION.

FOUNDATION AND SITE WORK

- REFER TO GEOTECHNICAL REPORT PREPARED BY AMEC FOSTER WHEELER ENVIRONMENT & INFRASTRUCTURE, DATED DECEMBER 04, 2015 AND ALL ITS SUPPLEMENTS AND AMENDMENTS FOR EXCAVATION, BACKFILLING, FILL MATERIALS, COMPACTION, FROST PROTECTION AND OTHER SITE PREPARATION REQUIREMENTS NOT SHOWN ON THESE DRAWINGS.
- DESIGN ALLOWABLE SOIL BEARING CAPACITIES:

RAFT FOOTINGS	SLS=60 kPa ULS=80 kPa ULS=105 kPa (SEISMIC)
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- ANY FOOTING ELEVATIONS INDICATED ON THE DRAWINGS ARE GENERAL AND SHALL BE USED FOR ESTIMATING AND BIDDING PURPOSES. FOOTINGS MAY HAVE TO BE PLACED AT DIFFERENT ELEVATIONS AS A RESULT OF LOCAL SOILS CONDITIONS, UNDERGROUND SERVICES AND TO ACCOMMODATE OTHER MECHANICAL AND ELECTRICAL SERVICES. FOLLOW TYPICAL DETAILS SHOWN ON THESE DRAWINGS FOR FOOTING PLACEMENT RELATIVE TO ADJACENT FOOTINGS, SLUMP AND OTHER EXCAVATED STRUCTURES AND LOCATE AS DIRECTED BY GEOTECHNICAL ENGINEER.
- THE BASES OF FOUNDATIONS SHALL BE PROTECTED FROM RAIN, SNOW AND ANY WATER INFILTRATION.
- NO FOUNDATIONS MAY BE POURED BEFORE THE BEARING MATERIAL HAS BEEN INSPECTED BY THE GEOTECHNICAL ENGINEER. NOTIFY THE GEOTECHNICAL ENGINEER MINIMUM 48 HOURS BEFORE INSTALLATION OF FOOTING REINFORCEMENT.
- IMMEDIATELY AFTER INSPECTION AND APPROVAL BY THE GEOTECHNICAL ENGINEER, OR OTHERWISE AS DIRECTED BY THE DEPARTMENTAL REPRESENTATIVE, THE BEARING SURFACE SHALL BE COVERED BY A 50mm THICK CONCRETE GROUND SEAL OF 10MPa STRENGTH.
- COORDINATE CONSTRUCTION WITH UNDERLAB SERVICES AS SHOWN ON MECHANICAL, ELECTRICAL, ARCHITECTURAL AND LANDSCAPING DRAWINGS.
- REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SITE DRAINAGE, GROUND ELEVATIONS AND DRAINAGE SLOPES.
- CENTRE ALL FOOTINGS UNDER COLUMNS OR WALLS UNLESS NOTED OTHERWISE.
- DO NOT BACKFILL RETAINING WALLS INCLUDING PERIMETER BASEMENT WALLS BEFORE THEY ARE ADEQUATELY SUPPORTED BY THE SUPPORTING FLOOR(S). ALL CONCRETE SUPPORTING FLOORS MUST HAVE CURED FOR MINIMUM 7 DAYS OR ATTAINED MINIMUM 75% OF THEIR 28-DAYS STRENGTH. ALL BACKFILLING TO COMPLY WITH THE REQUIREMENTS PROVIDED BY THE GEOTECHNICAL ENGINEER.
- REFER TO ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR WATERPROOFING AND SEALING REQUIREMENTS.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE TEMPORARY SUPPORT OF THE ADJACENT STRUCTURE DURING CONSTRUCTION. UNDERPINNING OR BRACING SHALL BE DESIGNED BY A QUALIFIED PROFESSIONAL ENGINEER, REGISTERED IN BRITISH COLUMBIA. SUBMIT 4 COPIES OF DESIGN DRAWINGS, SEALED BY A PROFESSIONAL ENGINEER, TO DEPARTMENTAL REPRESENTATIVE FOR REVIEW OF CONFORMANCE WITH GENERAL DESIGN CRITERIA. (REFER TO DWG 5603,5604 & 5607 FOR PERMANENT UNDERPINNING.)

REINFORCED CONCRETE

- REFER TO SPECIFICATIONS FOR CONCRETE STRENGTH, EXPOSURE CLASS & OTHER REQUIREMENTS.
- REINFORCING BARS f =400 mPa. ALL DOWELS ANCHOR BOLTS AND INSERTS SHALL BE PLACED BEFORE THE CONCRETE IS POURED.
- PROVIDE MINIMUM CONCRETE COVER TO REINFORCEMENT AS FOLLOWS:

CAST AGAINST EARTH	75mm
EXPOSED TO EARTH OR WEATHER:	50mm
ELSEWHERE:	20mm
- UNLESS NOTED OTHERWISE, PROVIDE MINIMUM SPLICE LENGTHS TO REINFORCEMENT AS FOLLOWS:

10M	450mm
15M	600mm
20M	750mm

MASONRY

- REFER TO SPECIFICATIONS FOR CONCRETE MASONRY, CODE REFERENCES AND OTHER REQUIREMENTS.
- REINFORCING BARS f_y =400 MPa.
- UNLESS NOTED OTHERWISE REINFORCE 200 & 250 LOADBEARING WALLS AS FOLLOWS:

VERTICAL:	1-20M @ 1200 CENTERED IN GROUTED CORE
HORIZONTAL:	1.0 kPa 3.8mm DIA. LADDER JOINT REINF. @ 400 2-15M IN CONTINUOUS BOND BEAMS AS SHOWN BELOW: H<2400: 1 BOND BEAM AT TOP OF WALL 2400<H<4800: 1 BOND BEAM AT TOP OF WALL PLUS 1 BOND BEAM MIDEHEIGHT (OVER WALL OPENINGS) (H = CLEAR HEIGHT OF WALL)
ADDITIONAL:	1-20M VERT. AT UNSUPPORTED ENDS OF WALLS 1-20M VERT. AT ALL CORNERS AND INTERSECTIONS 1-20M VERT. AT EACH SIDE OF OPENINGS 1-20M VERT. IN EACH CELL OF PIERS AND PILASTERS 2-15M ABOVE AND BELOW ALL OPENINGS, 800 PAST EDGE. (MAY BE PART OF CONTINUOUS BOND BEAMS)

FOR NON-LOADBEARING WALLS SUBSTITUTE 15M VERTICAL BARS FOR 20M VERTICALS (INCLUDING ADDITIONAL BARS).

- UNLESS NOTED OTHERWISE SPLICE REINFORCING AND EMBED DOWELS AS FOLLOWS:

DOWEL EMBEDMENT: (INCL. LENGTH OF HOOK)	25M BARS: 800 20M BARS: 500 15M BARS: 400 10M BARS: 300	SPLICES:	25M BARS: 1500 20M BARS: 900 15M BARS: 650 10M BARS: 450 WIRE REINF: 200
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- ALL VERTICAL REINFORCING SHALL RUN CONTINUOUS THROUGH BOND BEAMS AND LINTELS OR BE SPLICED AS SPECIFIED.
- PROVIDE CORNER BARS FOR ALL HORIZONTAL REINFORCING. SPLICE LENGTH AS SPECIFIED.
- STRAIGHT OR HOOKED DOWELS SHALL BE PROVIDED IN FOUNDATIONS OR GRADE BEAMS TO MATCH ALL VERTICAL REINFORCING BARS. SPLICE LENGTH AS SPECIFIED.
- CELLS TO BE REINFORCED SHALL BE KEPT CLEAR OF MORTAR.
- FILL CELLS CONTAINING REINFORCING STEEL OR ANCHOR BOLTS WITH 20MPa GROUT, 10mm AGGREGATE, 200-250 SLUMP. PUDDLE OR VIBRATE TO COMPLETELY FILL CELLS. REVIBRATE AFTER 10 TO 40 MINUTES, WHEN EXCESS WATER HAS BEEN ABSORBED BY MASONRY UNITS. TOP OFF FILLED CORES WITH FRESH GROUT AFTER REVIBRATION.
- PROVIDE CLEAN-OUTS AT BOTTOM OF ALL GROUTED CORES FOR LIFTS OVER 1500.
- CHECK STRUCTURAL, ARCHITECTURAL, MECHANICAL, ELECTRICAL, CIVIL, LANDSCAPE AND ALL OTHER RELEVANT DRAWINGS FOR LOCATIONS AND SIZES OF BOLTS, SLEEVES AND OPENINGS. SUPPLY AND SET ANCHOR BOLTS, SLEEVES, PIPE HANGERS, JOINTS AND OTHER INSERTS AND OPENINGS AS INDICATED OR SPECIFIED ELSEWHERE.
- VERTICAL CONTROL JOINTS SHALL BE PROVIDED AT A MAXIMUM SPACING OF 8000 FOR STRAIGHT WALLS AND 4000 FROM WALL CORNERS. TERMINATE BOND BEAM REINFORCING AT CONTROL JOINTS. LADDER REINFORCING TO RUN THROUGH JOINTS.
- UNLESS NOTED OTHERWISE PROVIDE LINTELS OVER OPENINGS AS FOLLOWS:

CLEAR SPAN	DEPTH OF LINTEL	REINFORCING
LESS THAN 1500	400	2-15M 800 PAST EDGES
1500 - 2400	600	2-20M 800 PAST EDGES
2500 - 3000	800	2-25M 800 PAST EDGES
- 90mm BRICK VENEER: UNLESS NOTED OTHERWISE ON THE DRAWINGS PROVIDE HOT DIP GALVANIZED STEEL ANGLE LINTELS OVER OPENINGS AS FOLLOWS:

MAX. CLEAR OPENING WIDTH	MINIMUM ANGLE SIZE (LONG LEG VERTICAL)
<2.7m	L90x90x8
3.3m	L125x90x8
3.5m	L125x90x10
>3.5m	SEE DRAWINGS

MAX. CLEAR OPENING WIDTH	MINIMUM ANGLE SIZE (LONG LEG VERTICAL)
<2.7m	L90x90x8
3.3m	L125x90x8
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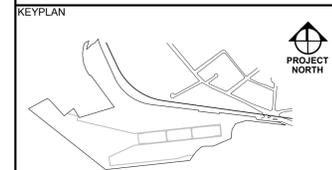
90mm MINIMUM BEARING EACH END.

STRUCTURAL STEEL

- REFER TO SPECIFICATIONS FOR STEEL WORK, OPEN WEB STEEL JOISTS, STEEL DECK, DESIGN CODE REFERENCES AND OTHER REQUIREMENTS.
- DRAWINGS FROM ALL CONSULTANTS SHALL BE EXAMINED FOR EXACT LOCATIONS, DIMENSIONS AND ELEVATIONS.
- STEEL FABRICATORS AND CONTRACTOR SHALL CONFIRM ALL LOCATIONS, DIMENSIONS AND ELEVATIONS WITH ACTUAL SITE MEASUREMENTS BEFORE FABRICATION. THE CONTRACTOR SHALL ASSUME FULL RESPONSIBILITY FOR ANY FABRICATION AND WORK DONE PRIOR TO REVIEW AND APPROVAL OF THE SHOP DRAWINGS.

ABBREVIATIONS

A	BOLT	ANCHOR BOLT
ALT.	ALTERNATE	
ARCH. BLDG.	ARCHITECTURAL BUILDING	
BOT.	BOTTOM	
BTW.	BETWEEN	
C/C	CENTER TO CENTER	
C/W	COMPLETE WITH	
C.I.P.	CAST IN PLACE	
CANT.	CANTILEVER	
CJ	CONSTRUCTION JOINT	
CL	CLEAR	
COL.	COLUMN	
CONC.	CONCRETE	
CONT.	CONTINUOUS	
DL	DEAD LOAD	
DN	DOWN	
DO.	DITTO	
DP.	DEEP	
DWG.	DRAWING	
E.W.	EACH WAY	
L.V.	LENGTH VARIES	
L.G.	LONG	
LL	LOW LEVEL	
LLV	LONG LEG VERTICAL	
LLH	LONG LEG HORIZONTAL	
LONG.	LONGITUDINAL	
MAX.	MAXIMUM	
MECH.	MECHANICAL	
MIN.	MINIMUM	
N/A	NOT AVAILABLE	
N.S.	NEAR SIDE	
N.STUD	NELSON STUD	
N.T.S.	NOT TO SCALE	
O/C	ON CENTRES	
OPP.	OPPOSITE HAND	
OWSJ	OPEN WEB STEEL JOIST	
P.C.	PRECAST CONCRETE	
PL	PLATE	
PLY.	PLYWOOD	
PROJ.	PROJECTION	



Revision/Revision	Description/Description	Date/Date
-	-	-
-	-	-
3	ISSUED FOR TENDER	2016-01-28
2	ISSUED FOR 99% SUBMISSION	2016-01-07
1	ISSUED FOR 65% CD REVIEW	2015-11-25
0	ISSUED FOR 33% CD REVIEW	2015-10-29

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Client/client

ESQUIMALT GRAVING DOCK

825 ADMIRALS ROAD VICTORIA, BC, V9A 2P1

Project title/Titre du projet
825 ADMIRALS ROAD VICTORIA BC ESQUIMALT GRAVING DOCK ELECTRICAL SAFETY UPGRADE

SOUTH SUBSTATION SWITCHGEAR REPLACEMENT PROJECT (SSSR)

Consultant Signature Box Only

Designed by/Concept par
P.L. / L.L.

Drawn by/Dessiné par
M.C.

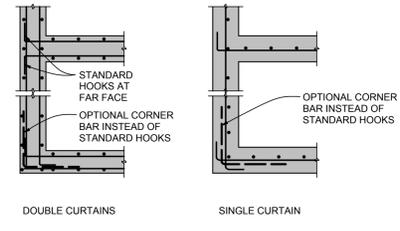
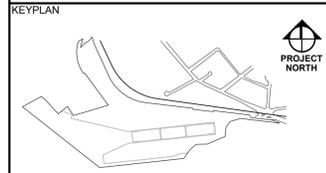
PWSC Project Manager/Administrateur de Projets TPSGC
France Veillette

PWSC, Regional Manager, Architectural and Engineering Services/
Gestionnaire Régionale, Services d'architecture et de génie, TPSGC
Preetipal Paul

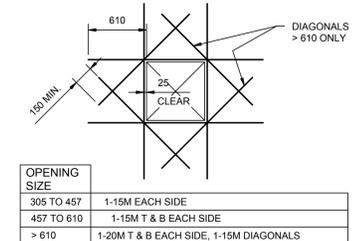
Drawing title/Titre du dessin
GENERAL NOTES

Project No./No. du projet	Sheet/Feuille	Revision no./Loi Révision
12355	R.062548.002	5601
12320		3



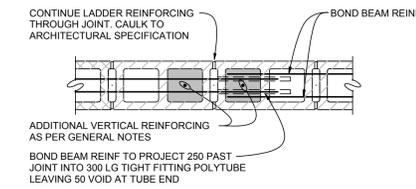


TYPICAL WALL INTERSECTION PLAN DETAILS

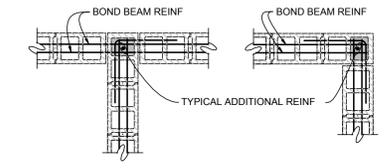


CONCRETE WALL AND SLAB OPENINGS

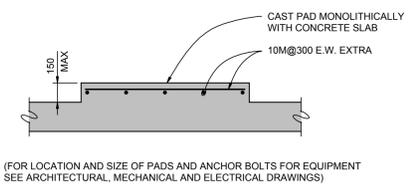
OPENING SIZE	REINFORCEMENT
305 TO 457	1-15M EACH SIDE
457 TO 610	1-15M T & B EACH SIDE
> 610	1-20M T & B EACH SIDE, 1-15M DIAGONALS



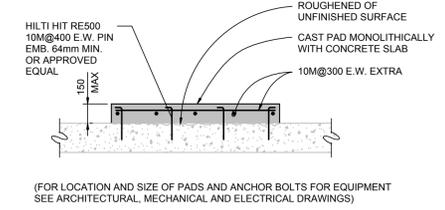
TYPICAL BLOCKWALL JOINTS



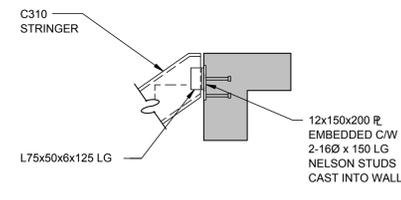
TYPICAL BLOCKWALL INTERSECTIONS



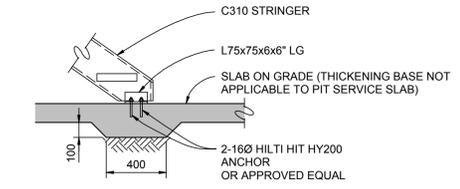
TYPICAL EQUIPMENT PADS ON CONCRETE SLABS



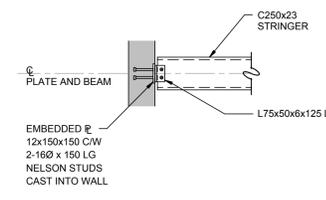
TYPICAL EQUIPMENT PADS ON CONCRETE SLABS



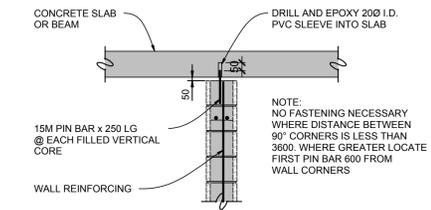
STEEL STRINGER TOP DETAIL



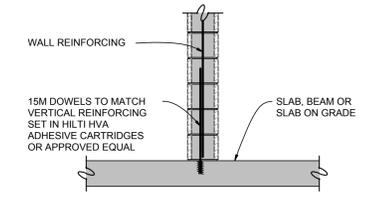
STEEL STRINGER BOTTOM DETAIL



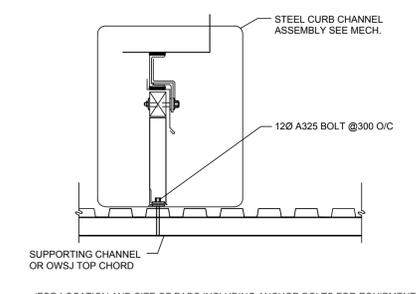
STEEL STRINGER/CONCRETE WALL DETAIL



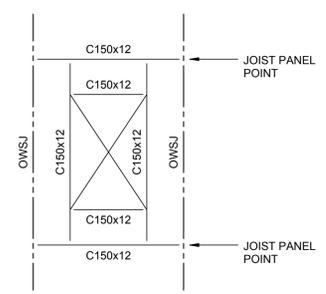
TYPICAL NON-LOADBEARING BLOCK WALL TOP SUPPORT



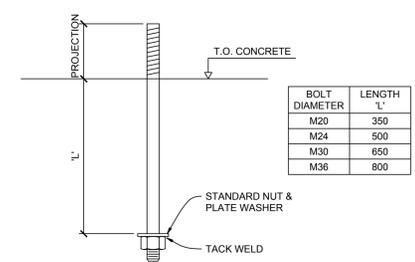
TYPICAL NON-LOAD BEARING BLOCK WALL BASE



TYPICAL MECHANICAL CURB ON ROOF DECK

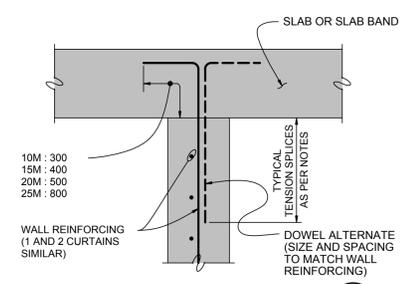


DECK OPENINGS > 450 SQUARE

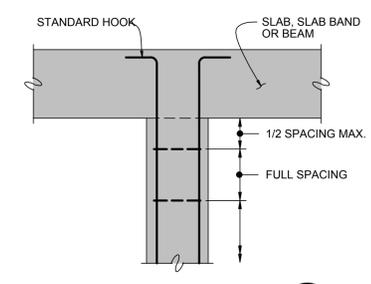


TYPICAL ANCHOR BOLT DETAIL

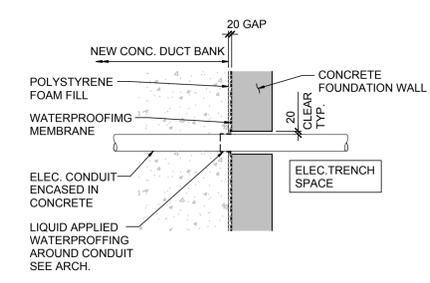
BOLT DIAMETER	LENGTH 'L'
M20	350
M24	500
M30	650
M36	800



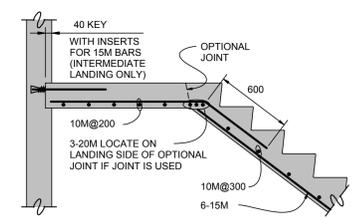
TYPICAL INTERIOR WALL TERMINATION AT SLAB OR SLAB BAND



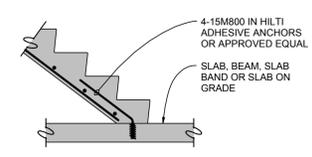
TYPICAL COLUMN TERMINATION



TYPICAL CONDUIT PENETRATION / DUCK BANK INTERFACE DETAIL



TYPICAL CAST-IN-PLACE STAIR AND LANDING SECTIONS



TYPICAL STAIR SUPPORT ON SLAB

Revision/Revisions	Description/Description	Date/Date
-	-	-
3	ISSUED FOR TENDER	2016-01-28
2	ISSUED FOR 99% SUBMISSION	2016-01-07
1	ISSUED FOR 65% CD REVIEW	2015-11-25
0	ISSUED FOR 33% CD REVIEW	2015-10-29

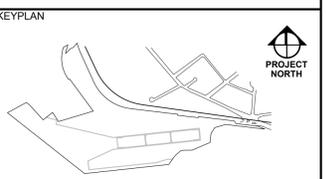
Client/client
ESQUIMALT GRAVING DOCK
825 ADMIRALS ROAD
VICTORIA, BC, V9A 2P1
Project title/Titre du projet
825 ADMIRALS ROAD VICTORIA BC
ESQUIMALT GRAVING DOCK
ELECTRICAL SAFETY UPGRADE
SOUTH SUBSTATION SWITCHGEAR REPLACEMENT PROJECT (SSSR)

Consultant Signature Box Only
Designed by/Concept par
P.L./L.L.
Drawn by/Dessiné par
M.C.
PWSC Project Manager/Administrateur de Projets TPSGC
France Veillette
PWSC, Regional Manager, Architectural and Engineering Services/
Gestionnaire régionale, Services d'architecture et de génie, TPSGC
Prestitipal Paul

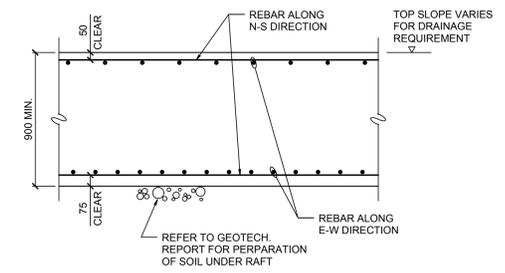
Drawing title/Titre du dessin
TYPICAL DETAILS

Project No./No. du projet	Sheet/Feuille	Revision no./No. de révision
12355	R.062548.002	5602
12320		3

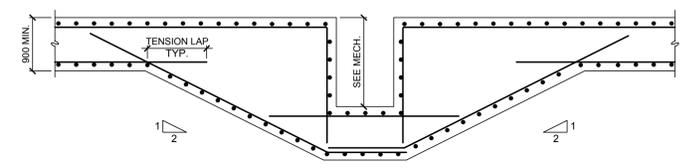




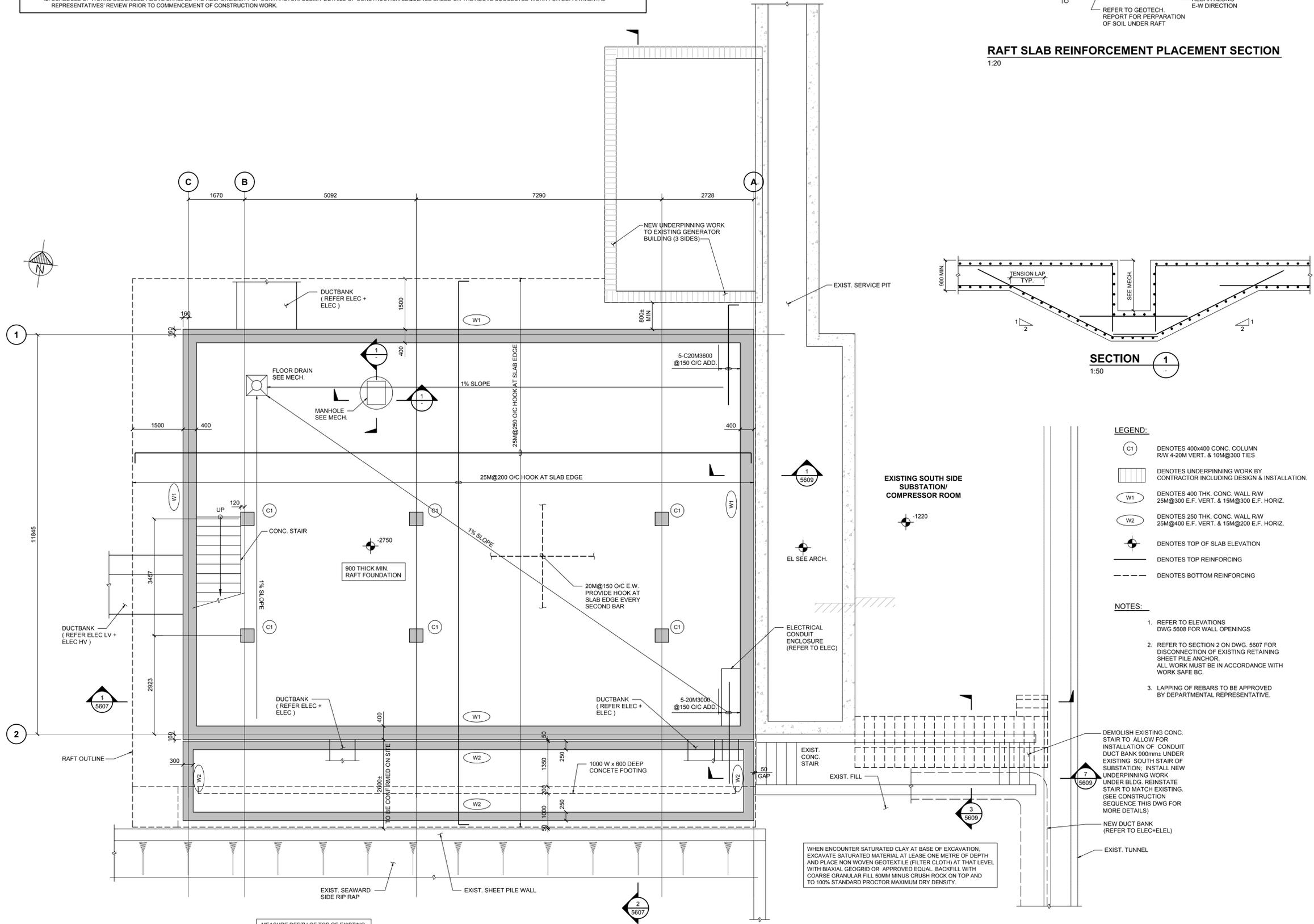
- SUGGESTED CONSTRUCTION SEQUENCE FOR DISCONNECTION OF THE RODS/EXCAVATION WORK/RAFT SLAB & NEW SOUTH SIDE PAVEMENT/DUCT BANK INSTALLATION ALONG SOUTH SIDE STAIR OF EXISTING SUBSTATION.
1. THE SUGGESTED CONSTRUCTION SEQUENCE IS FOP GUIDE LINE ONLY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL TEMPORARY WORK DESIGN TO MEET WORK SAFE B.C. REQUIREMENTS OF ALL SITE SAFETY BUT NOT LIMIT TO SAFETY OF OTHER USES IN GRAVING DOCK. EXISTING STRUCTURES REQUIREMENTS AS DESCRIBED IN GENERAL NOTES SHALL ALSO BE OBSERVED.
 2. MARK LOCATIONS OF EXISTING TIE RODS ACROSS LENGTH OF NEW SOUTH SUBSTATION SWITCHGEAR REPLACEMENT SITE AND SOUTH SIDE OF EXISTING STAIRS OF SUBSTATION.
 3. AVOID ANY DISTURBANCE TO THE TIE RODS. EXCAVATE NORTH SIDE OF SHEETPILE WALL WITH CARE TO FOOTPRINT OF BASE OF RAFT SLAB OF NEW SOUTH SUBSTATION SWITCHGEAR REPLACEMENT AND DEMOLISH EXISTING SOUTH SIDE STAIR OF EXISTING SUBSTATION FOR INSTALLATION OF DUCT BANKS.
 4. INSTALL UNDERPINNING WORK IN RAFT SLAB OF EXISTING GENERATOR BUILDING, FOOTPRINT OF NEW SOUTH SIDE SWITCHGEAR REPLACEMENT BUILDING AND WALL/FOOTING ALONG SOUTH SIDE OF EXISTING SUBSTATION AS PER PLANS AND SECTIONS. EXERCISE WITH CARE TO PROTECT THE EXISTING TIE RODS WITHIN THE SAID FOOTPRINT AND INSTALL SEQUENTIALLY IN SECTIONS OF UNDERPINNING WORK. CONTRACTOR SHALL BE RESPONSIBLE TO KEEP TEMPORARY BACK SLOPE AS STEEP AS POSSIBLE. PLACE CONCRETE UNDER A BIT OF PRESSURE FROM TOP OF SLAB LEVEL TO AVOID ANY VOID BEHIND THE WALL. DISCONNECT EXISTING TIE RODS IN THE VICINITY OF NEW SOUTH SUBSTATION SWITCHGEAR REPLACEMENT FOOT PRINT.
 5. FIX REBARS AND PLACE CONCRETE OF RAFT SLAB; UPON COMPLETION OF SERVICE PIT WALLS/COLUMNS/MAIN FLOOR SLAB. WATERPROOF THE NEW SOUTH SUBSTATION SWITCHGEAR REPLACEMENT BUILDING TO ARCHITECT'S SPECIFICATION.
 6. CONSTRUCT SOUTH SIDE NEW WALLS/ ROAD PAVEMENT BESIDE EXISTING SHEET PILE SEAWALL UPON COMPLETION OF THE MENTIONED WATERPROOF WORK.
 7. INSTALL DUCT BANKS WITH CONCRETE CASING (BY OTHERS) THAT RUNS SOUTH OF EXISTING STAIR OF SUBSTATION ONTO VOID BOX UNDER NEW ROADWAY PAVEMENT ROUTING INTO THE NEW SOUTH SUBSTATION SWITCHGEAR REPLACEMENT.
 8. POUR MASS CONCRETE PRIOR TO REINSTATE PREVIOUSLY DEMOLISHED SOUTH SIDE STAIR OF EXISTING SUBSTATION (SEE DRAWING 5603.5604 & 5608 FOR DETAILS).
 9. BACKFILL THE VICINITY OF EXCAVATED AREAS PER GEOTECHNICAL RECOMMENDATIONS.
 10. COMPLETE CONSTRUCTION JOINTS EAST AND WEST SIDE OF EXISTING CONCRETE PAVEMENT TO NEW ROAD PAVEMENT IN SOUTH SIDE AND OF NEW SOUTH SIDE SWITCHGEAR REPLACEMENT AND WITHIN THE VICINITY BETWEEN NEW AND EXISTING CONCRETE PAVEMENT OF EXISTING SUBSTATION OF NEW DUCT BANKS & SOUTH SIDE STAIR OF EXISTING SUBSTATION.
 11. SCHEDULE OF WORKS DESCRIBED ABOVE SHALL BE THE RESPONSIBILITY OF CONTRACTOR. SUBMIT DETAILS OF CONSTRUCTION SEQUENCE BASED ON THE ABOVE SUGGESTED WORK FOR DEPARTMENTAL REPRESENTATIVES' REVIEW PRIOR TO COMMENCEMENT OF CONSTRUCTION WORK.



RAFT SLAB REINFORCEMENT PLACEMENT SECTION
1:20



SECTION 1
1:50



FOUNDATION & SERVICE PIT PLAN
1:50

- LEGEND:**
- (C1) DENOTES 400x400 CONC. COLUMN R/W 4-20M VERT. & 10M@300 TIES
 - [Hatched Area] DENOTES UNDERPINNING WORK BY CONTRACTOR INCLUDING DESIGN & INSTALLATION.
 - (W1) DENOTES 400 THK. CONC. WALL R/W 25M@300 E.F. VERT. & 15M@300 E.F. HORIZ.
 - (W2) DENOTES 250 THK. CONC. WALL R/W 25M@400 E.F. VERT. & 15M@200 E.F. HORIZ.
 - (Elev. Symbol) DENOTES TOP OF SLAB ELEVATION
 - (---) DENOTES TOP REINFORCING
 - (---) DENOTES BOTTOM REINFORCING
- NOTES:**
1. REFER TO ELEVATIONS DWG 5608 FOR WALL OPENINGS
 2. REFER TO SECTION 2 ON DWG. 5607 FOR DISCONNECTION OF EXISTING RETAINING SHEET PILE ANCHOR. ALL WORK MUST BE IN ACCORDANCE WITH WORK SAFE BC.
 3. LAPPING OF REBARS TO BE APPROVED BY DEPARTMENTAL REPRESENTATIVE.

WHEN ENCOUNTER SATURATED CLAY AT BASE OF EXCAVATION, EXCAVATE SATURATED MATERIAL AT LEAST ONE METRE OF DEPTH AND PLACE NON WOVEN GEOTEXTILE (FILTER CLOTH) AT THAT LEVEL WITH BIAIXIAL GEOGRID OR APPROVED EQUAL. BACKFILL WITH COARSE GRANULAR FILL 50MM MINUS CRUSH ROCK ON TOP AND TO 100% STANDARD PROCTOR MAXIMUM DRY DENSITY.

MEASURE DEPTH OF TOP OF EXISTING RIPRAP BACKFILL AT OUTSIDE FACE OF SHEET PILE SEAWALL. AND REPORT TO DEPARTMENTAL REPRESENTATIVE.

USE WATERPROOF CONCRETE WITH XYPEX CRYSTALLINE ADMIXTURE OR APPROVED EQUAL FOR THE WHOLE SEAWALL / ROAD WAY STRUCTURE. USE EPOXY REBARS FOR ROAD WAY STRUCTURE.

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-	-	-
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0	ISSUED FOR 33% CD REVIEW	2015-10-29

Client/client

ESQUIMALT GRAVING DOCK

825 ADMIRALS ROAD
VICTORIA, BC, V9A 2P1

Project title/Titre du projet
**825 ADMIRALS ROAD VICTORIA BC
ESQUIMALT GRAVING DOCK
ELECTRICAL SAFETY UPGRADE**

**SOUTH SUBSTATION
REPLACEMENT PROJECT
(SSSR)**

Consultant Signature Box Only

Designed by/Concept par
P.L. / L.L.

Drawn by/Dessiné par
M.C.

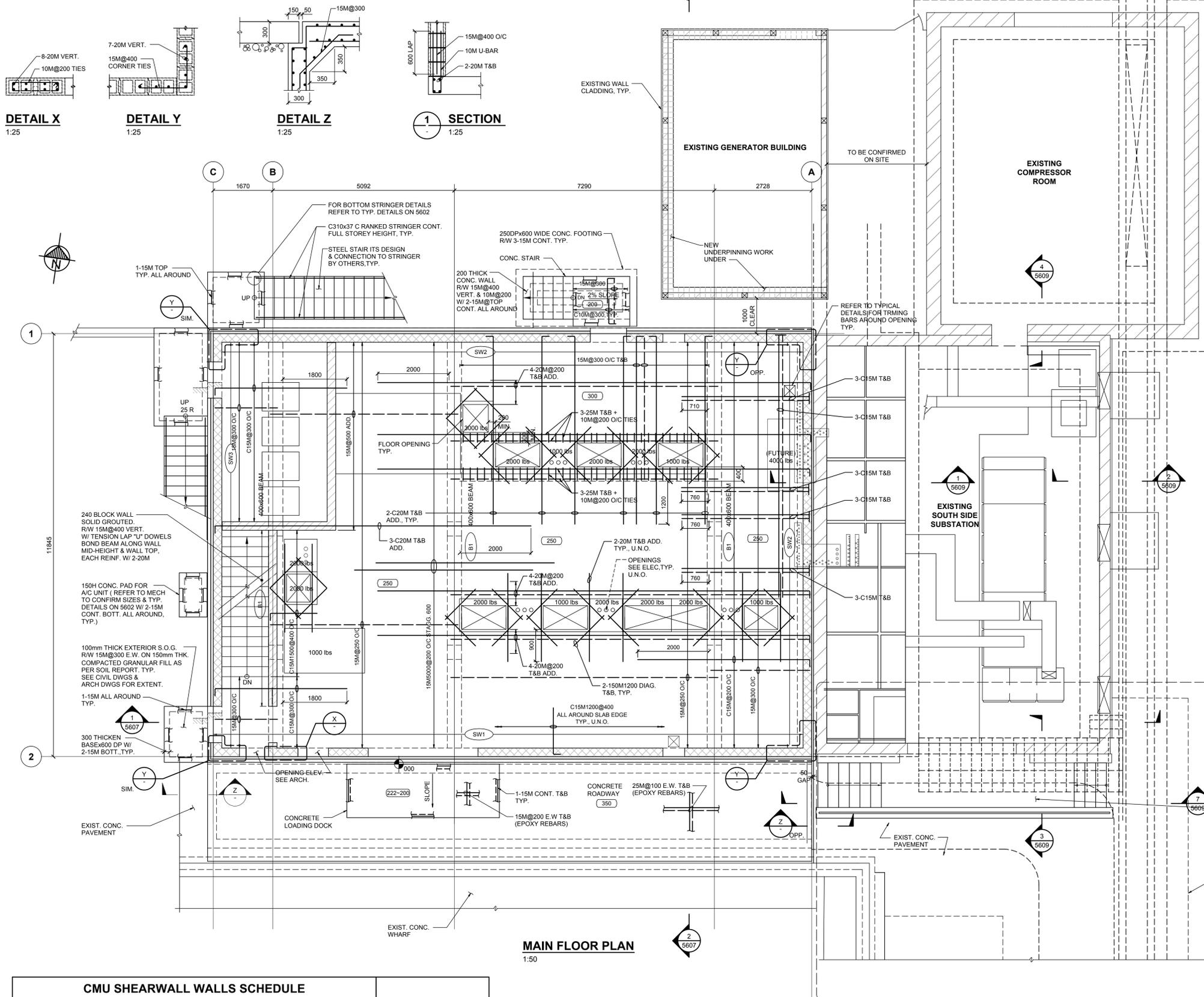
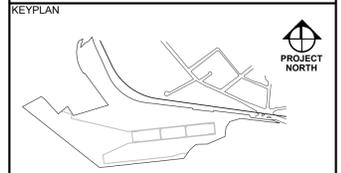
PWSC Project Manager/Administrateur de Projets TPSGC
France Veillette

PWSC, Regional Manager, Architectural and Engineering Services/
Gestionnaire régionale, Services d'architecture et de génie, TPSGC
Preetipal Paul

Drawing title/Titre du dessin
FOUNDATION & SERVICE PIT PLAN

Project No./No. du projet	Sheet/Feuille	Revision no./ No. de révision
12355	R.062548.002	5603
12320		3





- LEGEND:**
- SW1 DENOTES SHEARWALL TYPE. SEE SCHEDULE ON THIS DWG.
 - (250) DENOTES CONCRETE SLAB THICKNESS.
 - ▨ DENOTES UNDERPINNING WORK BY CONTRACTOR INCLUDING DESIGN & INSTALLATION.
 - B1 DENOTES 400x600 DP. CONC. BEAM R/W 4-25M T&B CONT. HOOKED TOP BARS AT EACH END, WITH 7-10M@150 O/C TIES EACH SIDE OF COLUMNS & END WALL + 10M@300 TIES IN BETWEEN.
 - ▨ DENOTES 240 CMU LOAD-BEARING WALLS CLASSIFICATION TO BE H/20/A/M.
 - ▨ DENOTES 240 CMU PARTITION WALLS CLASSIFICATION TO BE H/15/A/M SEE SPECIFICATION FOR REINF. DETAILS SEE TYPICAL DETAILS FOR TOP & BOTTOM CONNECTION.
 - DENOTES TOP REINFORCING
 - - - DENOTES BOTTOM REINFORCING
- NOTES:**
1. REFER TO ELEVATIONS ON DWG 5608 FOR WALL OPENINGS
 2. PROVIDE 15M@400 T&B SHRINKAGE BARS U.N.O.
 3. LAPPING OF REBARS TO BE APPROVED BY DEPARTMENTAL REPRESENTATIVE.
 4. ADHESIVE ANCHORS CONNECTING SUPPORTS OF HANDRAIL ASSEMBLY TO CONCRETE STAIRS TO BE APPROVED BY DEPARTMENTAL REPRESENTATIVE.

- REFER TO 5603 FOR SCOPE OF NEW WORK DETAILS IN THIS AREA
- EXIST. CONC. STAIR
- SUGGESTED APPROX. JOINT FOR EXTENT OF MATCHING CONC. PAVEMENT



CMU SHEARWALL WALLS SCHEDULE			
	SW1	SW2	SW3
ROOF TO 2ND FLOOR	PARTIAL GROUTED W/ 20M@600 O/C VERT. & BOND BEAM @2000 MAX. O/C EACH BOND BEAM R/W 2-20M CONT. SOLID GROUT CELL W/ REINF., U.N.O.	PARTIAL GROUTED W/ 20M@600 O/C VERT. & BOND BEAM @2000 MAX. O/C EACH BOND BEAM R/W 2-20M CONT. SOLID GROUT CELL W/ REINF., U.N.O.	SOLID GROUTED W/ 20M@600 O/C VERT. & BOND BEAM @2000 MAX. O/C EACH BOND BEAM R/W 2-20M CONT. GROUT CELL W/ REINF., U.N.O.
2ND FLOOR TO MAIN FLOOR	SOLID GROUTED W/ 20M@400 O/C VERT. & BOND BEAM @1200 MAX. O/C EACH BOND BEAM R/W 2-20M CONT.	PARTIAL GROUTED W/ 20M@600 O/C VERT. & BOND BEAM @1200 MAX. O/C EACH BOND BEAM R/W 2-20M CONT.	SOLID GROUTED W/ 20M@400 O/C VERT. & BOND BEAM @1200 MAX. O/C EACH BOND BEAM R/W 2-20M CONT.

NOTE:
REFER TO GENERAL NOTES (MASONRY) FOR JOINT LADDER REINFORCING & OTHER TYPICAL REQUIREMENTS. ALL WALL VERTICAL REINFORCEMENT EXTEND TO TOP OF PARAPET. SOLID GROUT BOND BEAM AT TOP REINFORCED WITH 2-15M CONT.

NEW SUBSTATION SWITCHGEAR REPLACEMENT EXISTING SUBSTATION

Revision/	Description/Description	Date/Date
3	ISSUED FOR TENDER	2016-01-28
2	ISSUED FOR 99% SUBMISSION	2016-01-07
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ESQUIMALT GRAVING DOCK

825 ADMIRALS ROAD
VICTORIA, BC, V9A 2P1

Project title/Titre du projet
825 ADMIRALS ROAD VICTORIA BC
ESQUIMALT GRAVING DOCK
ELECTRICAL SAFETY UPGRADE

**SOUTH SUBSTATION
REPLACEMENT
SWITCHGEAR
PROJECT (SSSR)**

Consultant Signature Box Only

Designed by/Concept par
P.L./L.L.

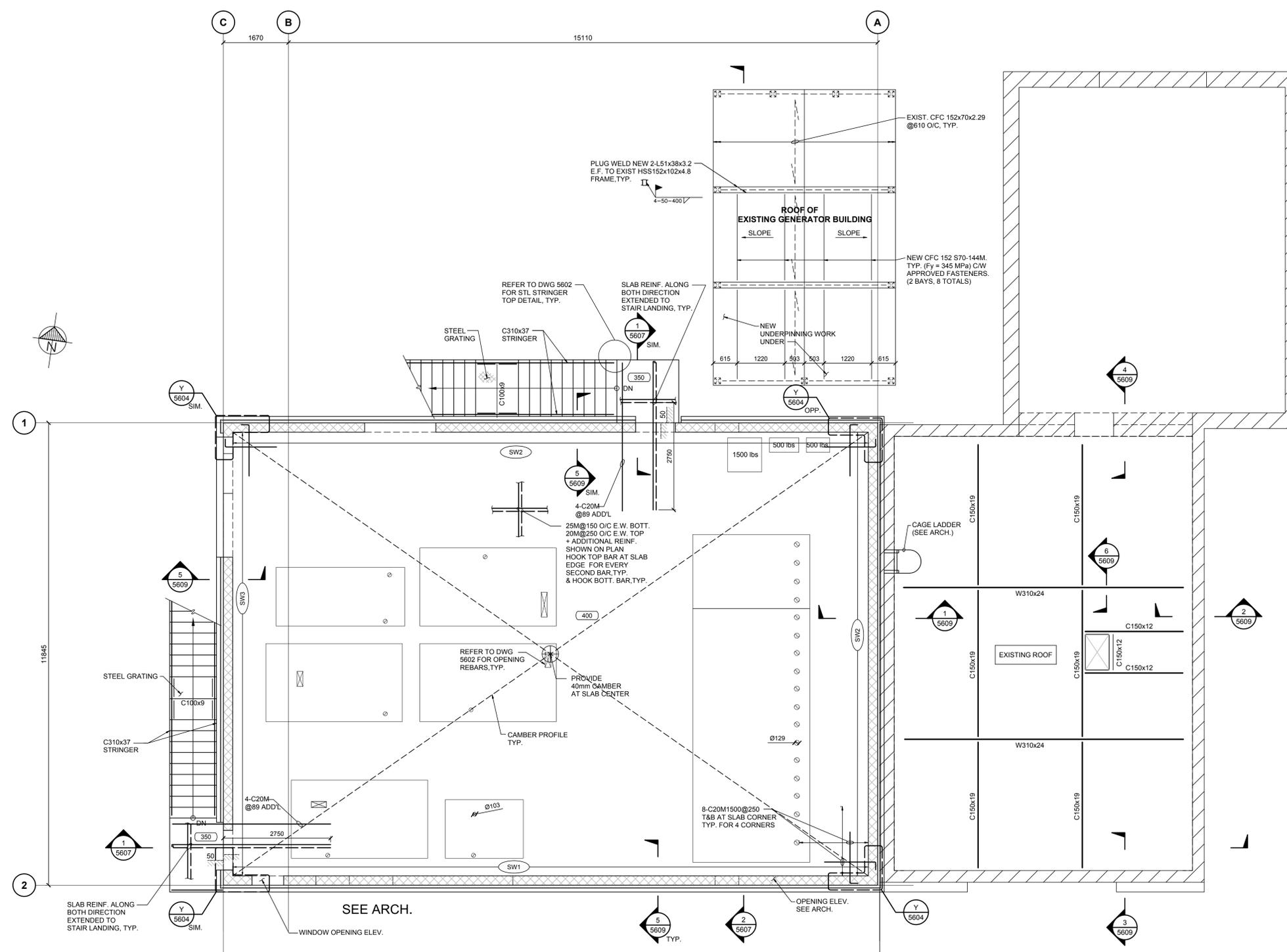
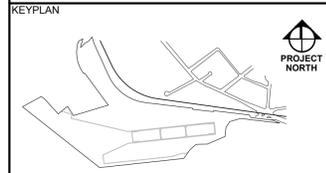
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M.C.

PWSC Project Manager/Administrateur de Projets TPSGC
France Veillette

PWSC, Regional Manager, Architectural and Engineering Services/
Gestionnaire régional, Services d'architecture et de génie, TPSGC
Preetipal Paul

Drawing title/Titre du dessin
MAIN FLOOR PLAN & DETAILS

Project No./No. du projet	Sheet/Feuille	Revision no./ Loi Révision
12355	R.062548.002	5604
12320		3

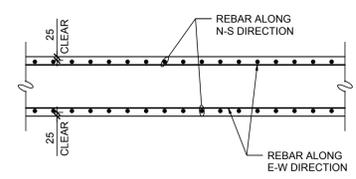


- LEGEND:**
- DENOTES 240 CMU LOAD-BEARING WALLS CLASSIFICATION TO BE H20/A/M
 - DENOTES 240 CMU LOAD-BEARING WALL BELOW
 - DENOTES CONCRETE SLAB THICKNESS
 - DENOTES SHEARWALL TYPE. SEE SCHEDULE THIS DWG. 5604 FOR DETAILS
 - DENOTES TOP REINFORCING
 - DENOTES BOTTOM REINFORCING

- NOTES:**
1. LAPPING OF REBARS TO BE APPROVED BY DEPARTMENTAL REPRESENTATIVE.

FORMWORK SHORING FOR 2ND FLOOR SLAB SHALL BE KEPT IN PLACE UNTIL CONCRETE REACHES ITS 28 DAYS COMPRESSIVE STRENGTH BUT NOT LESS THAN 14 DAYS & PROVIDE RE-SHORING FOR A MINIMUM OF 3 MONTHS.

SECOND FLOOR PLAN
1:50



SLAB REINFORCEMENT PLACEMENT SECTION
1:20

Revision/Revisions	Description/Description	Date/Date
-	-	-
3	ISSUED FOR TENDER	2016-01-28
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ESQUIMALT GRAVING DOCK

825 ADMIRALS ROAD
VICTORIA, BC, V9A 2P1

Project title/Titre du projet
825 ADMIRALS ROAD VICTORIA BC
ESQUIMALT GRAVING DOCK
ELECTRICAL SAFETY UPGRADE

SOUTH SUBSTATION SWITCHGEAR REPLACEMENT PROJECT (SSSR)

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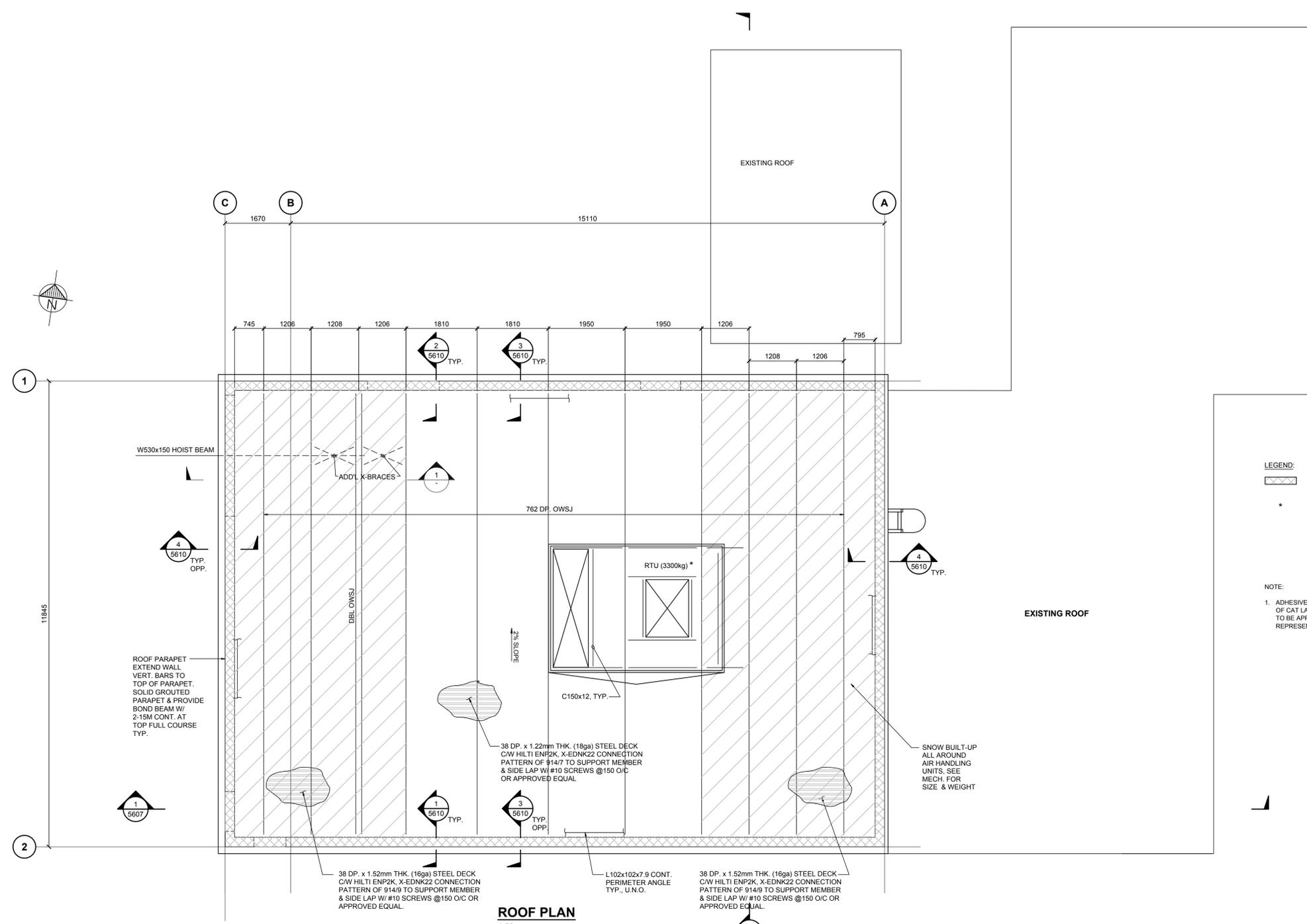
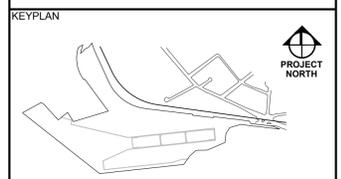
PWSC Project Manager/Administrateur de Projets TPSGC
France Veillette

PWSC Regional Manager, Architectural and Engineering Services/
Gestionnaire régionale, Services d'architecture et de génie, TPSGC
Prestipal Paul

Drawing title/Titre du dessin
SECOND FLOOR PLAN

Project No./No. du projet	Sheet/Feuille	Revision no./Loi Révision
12355	R.062548.002	5605
12320		3



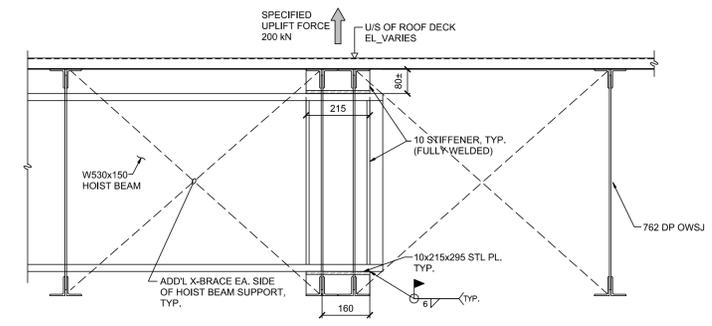
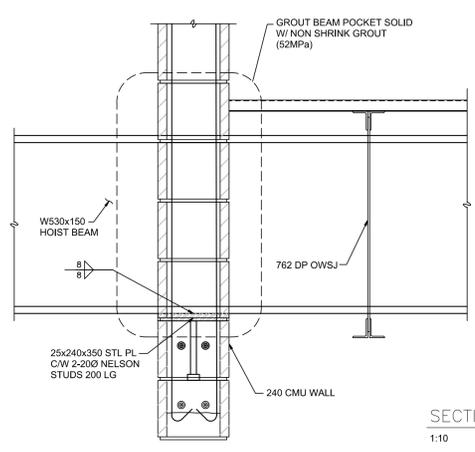


LEGEND:
 DENOTES 240 CMU LOAD-BEARING WALLS CLASSIFICATION TO BE H20/A/M
 * DENOTES DIMENSIONS TO BE CONFIRMED WITH MECHANICAL

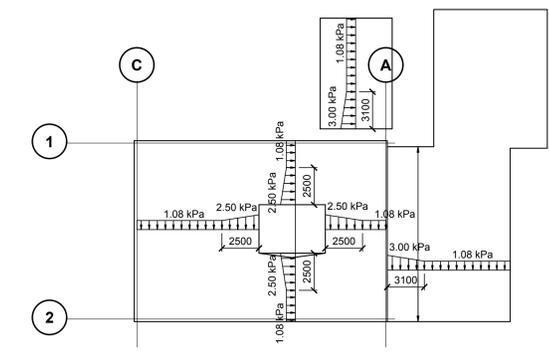
NOTE:
 1. ADHESIVE ANCHORS CONNECTING SUPPORTS OF GAT LADDER TO EXTERIOR CMU BLOCK WALL TO BE APPROVED BY DEPARTMENTAL REPRESENTATIVE.

ROOF PARAPET EXTEND WALL VERT. BARS TO TOP OF PARAPET. SOLID GROUTED PARAPET & PROVIDE BOND BEAM W/ 2-15M CONT. AT TOP FULL COURSE TYP.

ROOF PLAN
1:50



SECTION 1
1:10



ROOF SNOW LOAD DIAPHRAGM
1:200

1. ROOF SNOW LOAD = 1.08 kPa U.N.O. Ig=1.0 ADJUST FACTOR AS REQUIRED
2. DESIGN OWSJ WITH NET UPLIFT = 1.35 kPa

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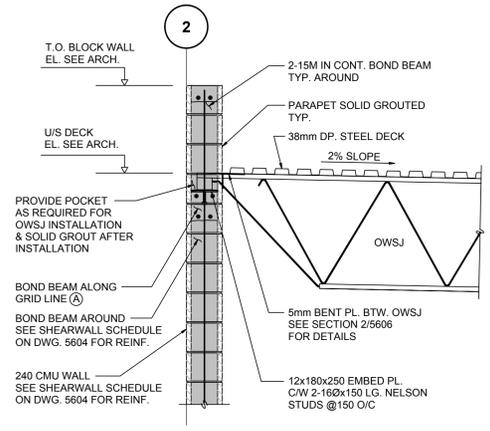
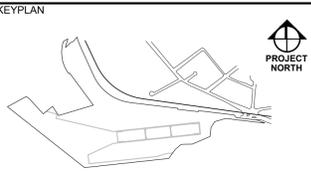
Client/client
ESQUIMALT GRAVING DOCK
 825 ADMIRALS ROAD
 VICTORIA, BC, V9A 2P1
 Project title/Titre du projet
 825 ADMIRALS ROAD VICTORIA BC
 ESQUIMALT GRAVING DOCK
 ELECTRICAL SAFETY UPGRADE
SOUTH SUBSTATION SWITCHGEAR REPLACEMENT PROJECT (SSSR)

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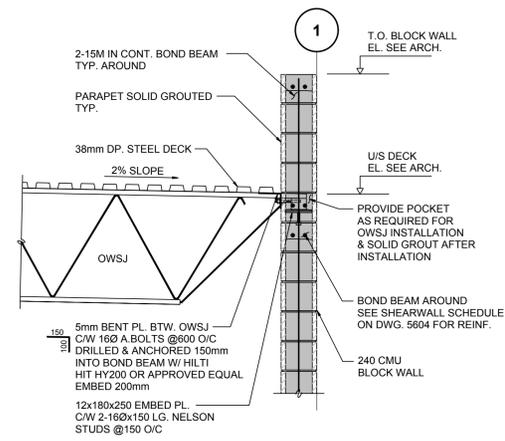
Drawing title/Titre du dessin
ROOF PLAN

Project No./No. du projet	Sheet/Feuille	Revision no./Loi Révision
12355	R.062548.002	5606
12320		3

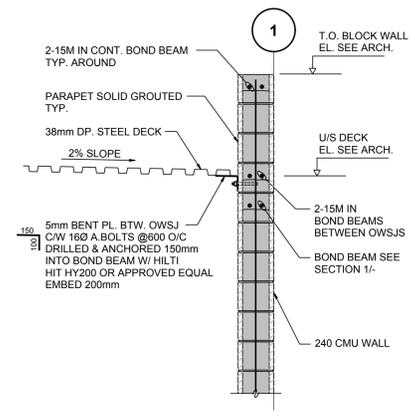




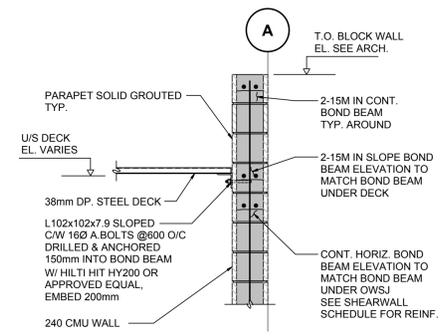
SECTION 1
1:20
5606
TYPICAL FOR OWSJ SUPPORT AT BLOCK WALL



SECTION 2
1:20
5606
TYPICAL FOR OWSJ SUPPORT AT BLOCK WALL



SECTION 3
1:20
5606
TYPICAL BETWEEN OWSJS



SECTION 4
1:20
5606
TYPICAL BETWEEN OWSJS

Revision/Revisions	Description/Description	Date/Date
-	-	-
-	-	-
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ESQUIMALT GRAVING DOCK

825 ADMIRALS ROAD
VICTORIA, BC, V9A 2P1

Project title/Titre du projet
**825 ADMIRALS ROAD VICTORIA BC
ESQUIMALT GRAVING DOCK
ELECTRICAL SAFETY UPGRADE**

**SOUTH SUBSTATION
SWITCHGEAR
REPLACEMENT PROJECT
(SSSR)**

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PWSC, Regional Manager, Architectural and Engineering Services/
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Prestipal Paul

Drawing title/Titre du dessin
**SECTIONS & DETAILS
SHEET 2**

Project No./No. du projet	Sheet/Feuille	Revision no./ Lo. Révision
12355	R.062548.002	5610
12320		3

