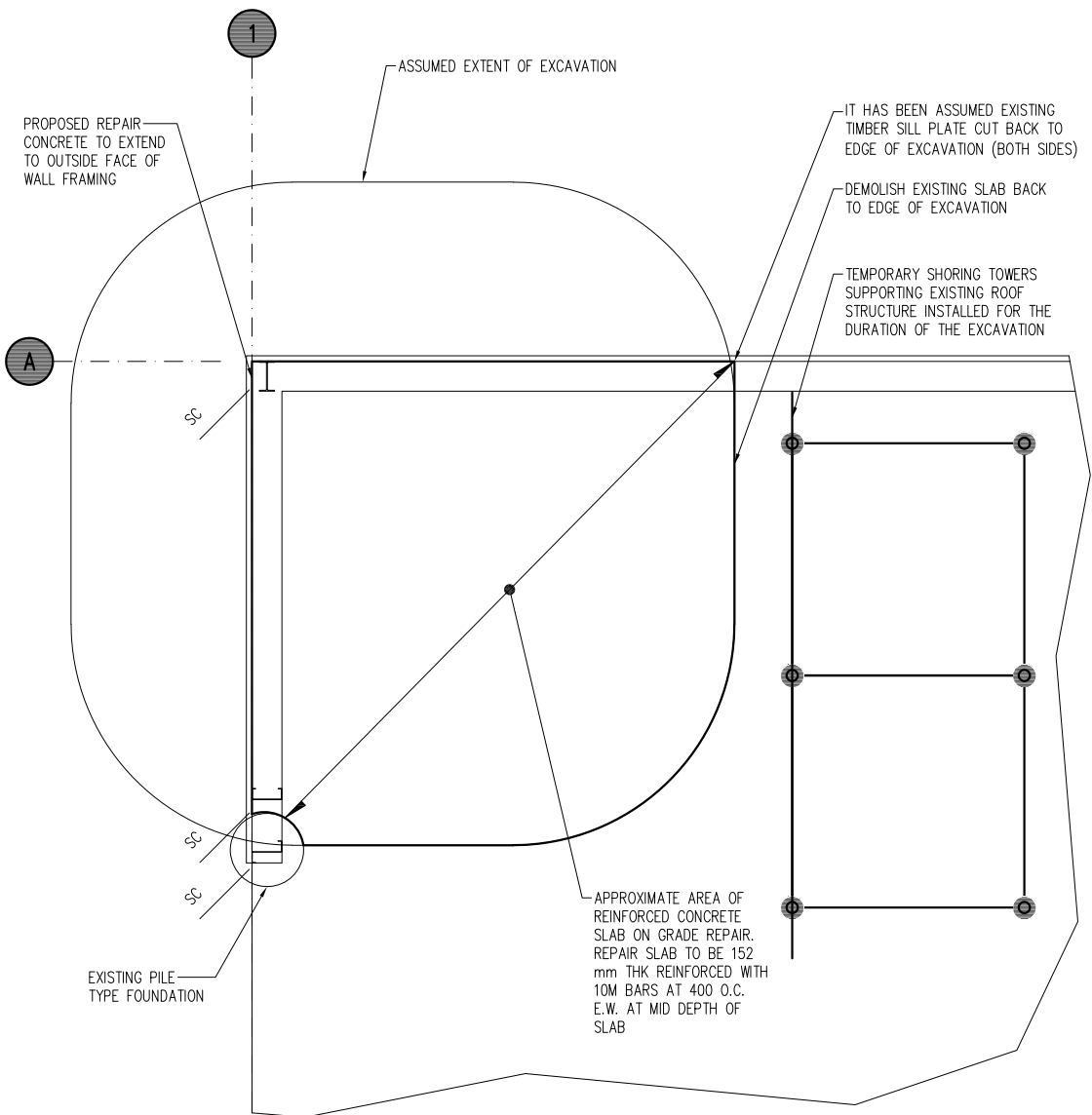


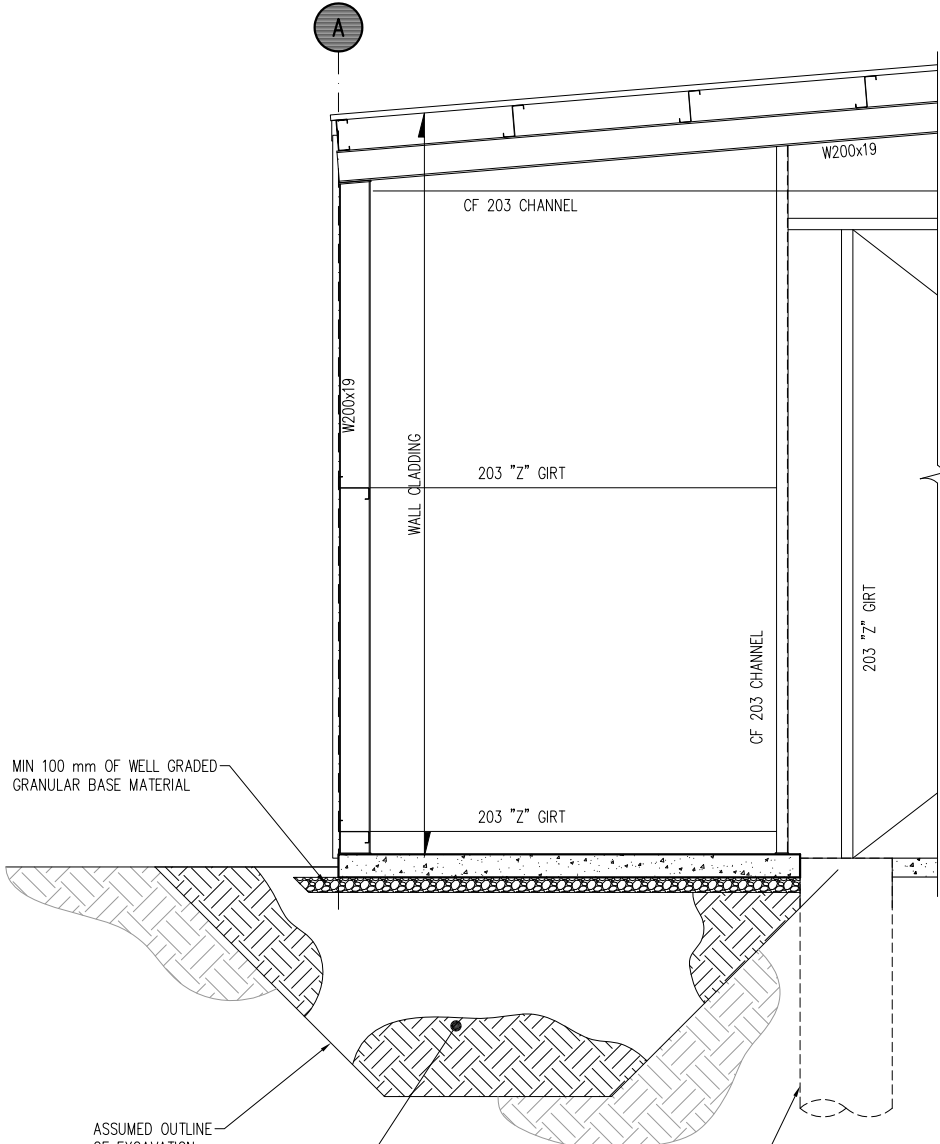
APPENDIX C

Structural Investigation





PARTIAL PLAN
1:50



END WALL ELEVATION BAYLINE 1
1:50

REINFORCING STEEL CONCRETE COVER TABLE	
EXPOSURE CONDITION	F & S
CAST AGAINST & PERMANENTLY EXPOSED TO EARTH	75 mm [3"]
SLABS, WALLS, JOISTS, SHELLS & FOLDED PLATES	40 mm [1 1/2"]
RATIO OF COVER TO NOMINAL BAR DIAMETER	1.5
RATIO OF COVER TO MAXIMUM AGGREGATE SIZE	1.5

CONCRETE SPECIFICATIONS TABLE						
CONCRETE ELEMENT	EXPOSURE CLASS	28-DAY COMPRESSIVE STRENGTH	MAX AGGREGATE SIZE	AIR CONTENT	MAX W/C RATIO	CEMENT TYPE (1)
SLABS-ON-GRADE (3)	F-2	32 MPa [4600 psi]	20 mm [3/4"]	5-8%	0.45	GU

(1) BLENDED GENERAL USE HYDRAULIC CEMENT (TYPE GuB) OR HIGH-EARLY-STRENGTH HYDRAULIC CEMENT (TYPE HE OR HeB) MAY BE USED IF APPROVED BY THE ENGINEER

LAP, HOOK, AND DEVELOPMENT LENGTHS TABLE			
LAP LENGTH			
TENSION			
VERTICAL BARS (2)		HORIZONTAL BARS (2)	
≤ 20M	≥ 25M	≤ 20M	≥ 25M
34d	43d	44d	56d
≥ 300 mm [1'-0"]			

GENERAL NOTES

1. DESIGN PARAMETERS:

1.1. DUE TO THE CONDITION AND STANDARDS OF CONSTRUCTION OF THE EXISTING SLAB-ON-GRADE THE DESIGN OF THE PROPOSED SLAB REPAIR HAS BEEN BASED ON GOOD ENGINEERING PRACTICE ONLY.

2. GENERAL:

- 2.1. REFERENCED CODES SHALL BE THE EDITIONS DESIGNATED IN TABLE 1.3.1.2 OF DIVISION B OF THE BRITISH COLUMBIA BUILDING CODE (BCBC) 2012 UNLESS NOTED OTHERWISE.
- 2.2. THESE NOTES SHALL BE READ IN CONJUNCTION WITH ALL OTHER CONTRACT DOCUMENTS.
- 2.3. THE DRAWINGS ARE FOR THE COMPLETED PROJECT. FORMWORK, FALSEWORK, SCAFFOLDING AND THE SUPPORT OF NEW AND EXISTING STRUCTURES DURING CONSTRUCTION ARE THE RESPONSIBILITY OF THE CONTRACTOR.
- 2.4. DIMENSIONS ARE IN MILLIMETRES UNLESS NOTED OTHERWISE.
- 2.5. DO NOT SCALE DRAWINGS.
- 2.6. THE CONTRACTOR SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS IN THE FIELD. THE ENGINEER SHALL BE INFORMED OF ANY VARIATIONS FROM THE DIMENSIONS AND CONDITIONS SHOWN ON THE DRAWINGS. THE CONTRACTOR SHALL CONTROL THE PRODUCTION OF WORK ON SITE AND ELSEWHERE TO FULFILL THE INTENT OF THE DRAWINGS.
- 2.7. THE ENGINEER'S REVIEW OF SHOP DRAWINGS DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY FOR ERRORS OR OMISSIONS ASSOCIATED WITH THE PREPARATION OF THE SHOP DRAWINGS.

3. FOUNDATIONS:

- 3.1. ASSUMED SOIL BEARING CAPACITY IS NOT KNOWN. THE PROPOSED WORK ASSUMES CONSISTENT GROUND CONDITIONS ACROSS THE BUILDING FOOTPRINT
- 3.2. EXCAVATIONS SHALL BE BACKFILLED WITH GRANULAR MATERIAL OR APPROVED NATIVE SOIL FREE OF ORGANIC MATERIAL, FOREIGN MATTER AND ROCKS LARGER THAN 75 mm [3"]. THE BACKFILL SHALL BE COMPACTED TO 95 % OF STANDARD PROCTOR MAXIMUM DRY DENSITY IN LAYERS NOT EXCEEDING 300 mm [1'] UNLESS NOTED OTHERWISE. NATIVE MATERIAL IS NOT ACCEPTABLE IF IT IS IMPRACTICABLE TO CONTROL THE WATER CONTENT OR COMPACT TO THE SPECIFIED DENSITY.

4. CONCRETE:

- 4.1. CONCRETE SHALL CONFORM TO AND BE PLACED IN ACCORDANCE WITH CSA-A23.1.
- 4.2. REFER TO TABLE ON THIS DRAWING FOR CONCRETE SPECIFICATIONS. MIX DESIGNS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW AT LEAST ONE WEEK PRIOR TO BATCHING.
- 4.3. CONCRETE QUALITY SHALL BE TESTED IN ACCORDANCE WITH CSA-A23.2. CONCRETE TESTING SHALL BE CARRIED OUT BY A THIRD PARTY TESTING AGENCY CERTIFIED IN ACCORDANCE WITH CSA-A283. A MINIMUM OF THREE CYLINDERS SHALL BE TAKEN FOR STRENGTH TESTING FOR EVERY 35 m³ [45 yd³] OF EACH CLASS OF CONCRETE POURED PER DAY. TEST REPORTS SHALL BE SUBMITTED TO THE ENGINEER.
- 4.4. FORMWORK SHALL BE LEFT IN PLACE UNTIL THE CONCRETE HAS ATTAINED SUFFICIENT STRENGTH TO CARRY ITS OWN WEIGHT AND ALL IMPOSED LOADS. CONTACT THE ENGINEER FOR RECOMMENDED STRIPPING TIMES.
- 4.5. REFER TO ARCHITECTURAL, STRUCTURAL, MECHANICAL, AND/OR ELECTRICAL DRAWINGS (AS APPLICABLE) FOR HOLES, NAILERS, INSERTS, ETC TO BE CAST INTO THE CONCRETE.

5. REINFORCEMENT:

- 5.1. REINFORCEMENT, HARDWARE AND EMBEDDED ITEMS SHALL BE FABRICATED AND PLACED IN ACCORDANCE WITH CSA-A23.1.
- 5.2. REINFORCING STEEL SHALL CONFORM TO CSA-G30.18 GRADE 400 UNLESS NOTED OTHERWISE.
- 5.3. WELDING OF REINFORCING STEEL IS NOT PERMITTED UNLESS SPECIFIED OR AUTHORIZED BY THE ENGINEER. REINFORCING STEEL TO BE WELDED SHALL CONFORM TO CSA-G30.18 GRADE 400W UNLESS NOTED OTHERWISE. WELDING OF REINFORCING BARS SHALL CONFORM TO AND BE CARRIED OUT BY COMPANIES CERTIFIED IN ACCORDANCE WITH CSA-W186.
- 5.4. REFER TO TABLES ON THIS DRAWING FOR LAP, HOOK AND DEVELOPMENT LENGTHS FOR REINFORCING BARS, AND CONCRETE COVER.
- 5.5. REINFORCEMENT SPICES IN SLABS AND WALLS SHALL BE STAGGERED WITH AN OFFSET EQUAL TO OR GREATER THAN THE REQUIRED LAP LENGTH.
- 5.6. REINFORCEMENT, HARDWARE AND EMBEDDED ITEMS SHALL BE SECURED AND SUPPORTED TO ENSURE PROPER CONCRETE COVER AND SPACING WITHIN ALLOWABLE TOLERANCES BEFORE AND DURING PLACING OF CONCRETE. WIRE NOT LESS THAN 1.5 mm [1/16"] IN DIAMETER SHALL BE USED TO SECURE REINFORCEMENT. BAR SUPPORTS SHALL BE MADE OF PRECAST CONCRETE OR PLASTIC.
- 5.7. CONCRETE SHALL NOT BE POURED UNTIL THE ENGINEER, OR HIS/HER REPRESENTATIVE, HAS INSPECTED THE REINFORCING STEEL AND POUR CONDITIONS.

6. POST-INSTALLED ADHESIVE ANCHORS

- 6.1. ADHESIVE ANCHORS SHALL CONFORM TO ACI 355.4 AND BE MADE BY HILTI INC. OR APPROVED EQUIVALENT.
- 6.2. ADHESIVES ANCHORS SHALL BE STORED AND INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS. CLEANED HOLES SHALL BE PROTECTED FROM CONTAMINATION UNTIL ADHESIVE IS INSTALLED.
- 6.3. CONCRETE TO HAVE A MINIMUM AGE OF 21 DAYS AND A MINIMUM COMPRESSIVE STRENGTH OF 17.5 MPa [2,500 psi] AT THE TIME OF ANCHOR INSTALLATION.
- 6.4. THREADED AND REINFORCING BARS SHALL NOT BE BENT AFTER BEING INSTALLED UNLESS SPECIFIED OR AUTHORIZED BY THE ENGINEER.



ISSUED FOR TENDER

DRAWING NOTES

REFERENCE DRAWINGS

DWG No.	REV	PROJECT No.	PRODUCED BY
-	-	-	-

DRAWING REVISIONS

REV	DATE	DRW	DES	APP	REVISION

B	16.08.03	CH	BC	BC	ISSUED FOR TENDER
A	16.07.20	CH	BC	BC	ISSUED FOR PRICING

REV	DATE	DRW	DES	APP	REVISION
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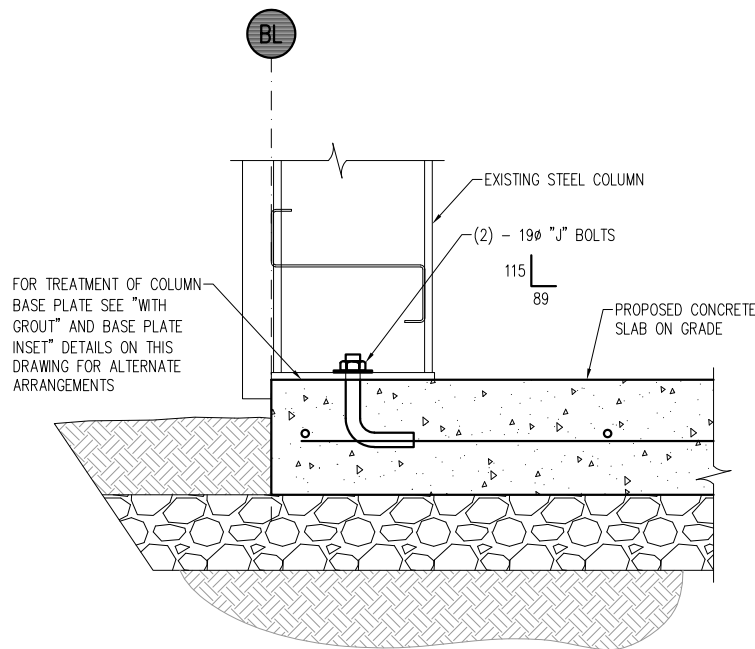
CLIENT

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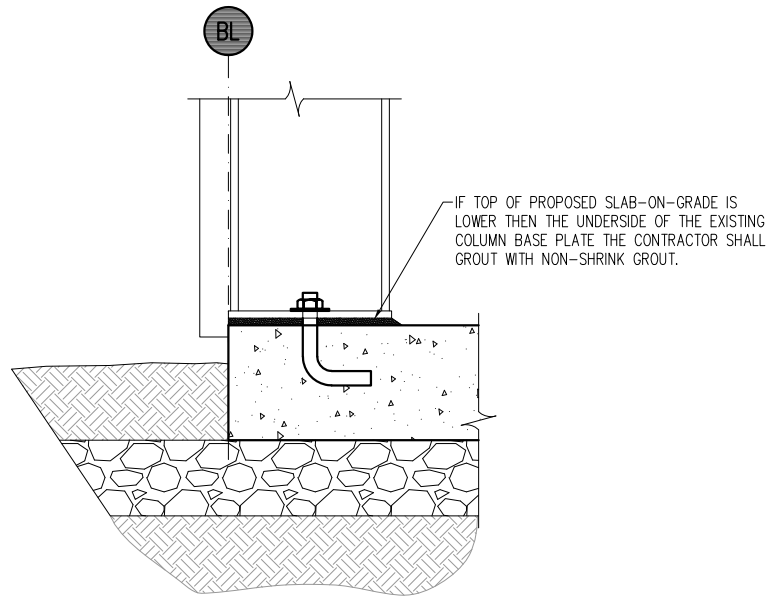
PROJECT INFORMATION

SNC LAVALIN		
TOAD RIVER, BC HIGHWAY MAINTENANCE BUILDING ASSESSMENT		
PROPOSED SLAB-ON-GRADE REPAIR DETAILS		
PROJECT No.	DRAWING No.	REV
427-04	105	B

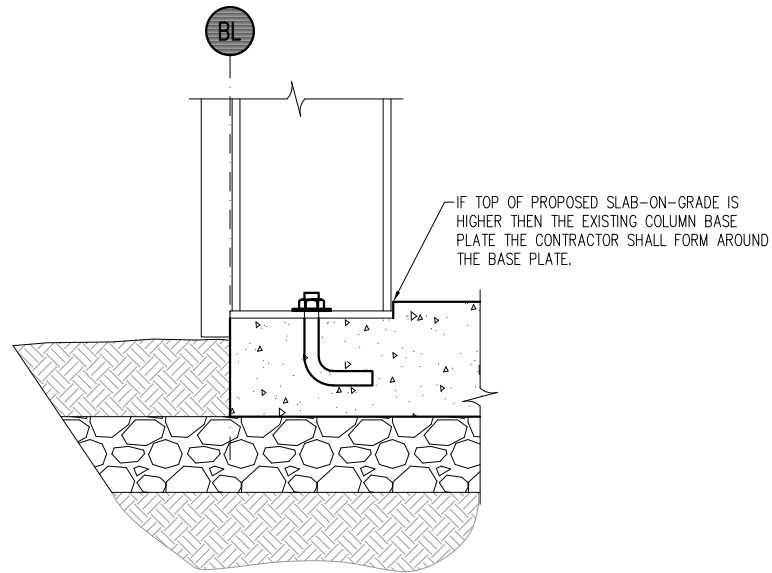
FILE NAME: 427-04.dwg
LAST UPDATED: 2017.16
DISCARD ALL PREVIOUS REVISIONS



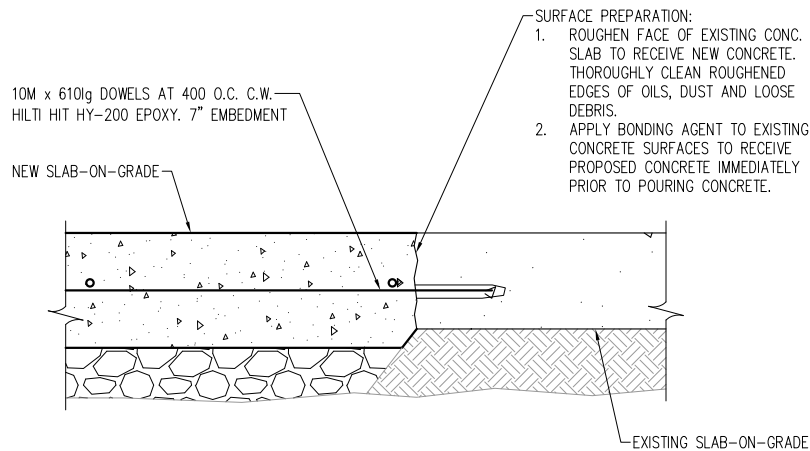
TYPICAL EDGE OF SLAB AND BASE PLATE DETAIL
1:10



TYPICAL BASE PLATE DETAIL (WITH GROUT)
1:10



TYPICAL BASE PLATE DETAIL (BASE PLATE INSET)
1:10



TYPICAL SLAB JOINT DETAIL
1:10



ISSUED FOR TENDER

DRAWING NOTES

1. FOR GENERAL NOTES SEE DRAWING 105.

REFERENCE DRAWINGS

DWG No.	REV	PROJECT No.	PRODUCED BY
-	-	-	-

DRAWING REVISIONS

REV	DATE	DRW	DES	APP	REVISION
B	16.08.03	CH	BC	BC	ISSUED FOR TENDER
A	16.07.20	CH	BC	BC	ISSUED FOR PRICING

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CLIENT



PROJECT INFORMATION

SNC LAVALIN
TOAD RIVER, BC
HIGHWAY MAINTENANCE
BUILDING ASSESSMENT
PROPOSED SLAB-ON-GRADE
REPAIR DETAILS

PROJECT No.	DRAWING No.	REV
427-04	106	B

FILE NAME: 427-04.dwg
LAST UPDATED: 2007.16
DISCARD ALL PREVIOUS REVISIONS

June 29th, 2016
Project: 427-04

Alana Duncan
Senior Project Manager
SNC-Lavalin Environment
3440 Douglas Street
Victoria, BC
V8Z 3L5

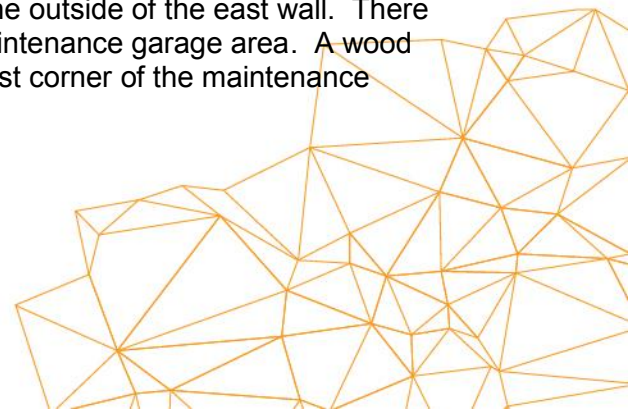
Re: Condition and Relocation Suitability Assessment, and Temporary Shoring Advice
for an Existing Maintenance Garage at Toad River Maintenance Camp
at Km 649 on the Alaska Highway

Dear Alana:

At your request we visited the Toad River Maintenance Camp at Km 649 on the Alaska Highway on June 9th, 2015. During our visit we recorded the current condition and structural arrangement of the existing maintenance garage building and foundations. We understand our visit and review of the existing building was carried out within the context of the possible relocation of the structure as part of a future environmental remediation project for the maintenance site. Our letter report is below and copies of photographs taken during our inspection and drawings of the existing building are attached.

The existing building is rectangular on plan, nominally 40 feet wide by 96 feet long, enclosing approximately 3800 square feet of maintenance shop space. The existing building is a conventionally framed, pre-engineered style, steel structure with corrugated metal wall and roof cladding, integral batt roof insulation and spray foam wall insulation. The south facing elevation contains an overhead door, nominally 18 feet wide by 14 feet high, and a conventionally sized man door located on the east wall. The building structure is founded on what might be considered to be unconventional foundations. The columns which support the building bear onto an arrangement of rough sawn two-ply 2 inch by 12 inch timbers below the perimeter of the building, which bear directly onto the ground. The shop floor consists of a concrete slab-on-grade.

The interior of the building contains; a number of equipment and tool racks, work benches, shelving and used oil storage. A wood-framed storage structure with an overhead mezzanine floor and stairs is located in the northwest corner of the building. A wood framed lean-to office structure is located on the outside of the east wall. There is a small window opening between the office and maintenance garage area. A wood framed washroom enclosure is located in the southeast corner of the maintenance shop space.



It appears the building structure has been altered in the past.

- The framing of the north wall suggests it used to accommodate a large door opening. This door opening no longer exists and is now infilled with the same corrugated metal cladding and spray foam insulation as the rest of the building.
- The framing of the south wall suggests it may have accommodated a larger door in the past.
- The profile of the wall cladding at each end of the building steps out at high level. The building may well have had large sliding doors at each end. The rails from which the slid doors where hung may have been housed within the step in the cladding. The maintenance building now has one overhead door as described previously.

The building superstructure is in relatively good condition. There is some minor local damage consistent with the buildings use as a maintenance shop and heavy equipment storage garage. We recorded some missing steel connection bolts and a number of roof purlins appear to have twisted. However, overall, the building superstructure is in good condition.

The building foundations and floor slab are in a poor condition. The timber plates below the perimeter of the building are substantially rotten, resulting in the settlement of some of the steel columns. This settlement is visible in Photo 17. The concrete floor slab surface is uneven, possibly as a result of being formed at different times, and possibly to allow for some type of floor drainage in the past. See Photos 10 and 18.

We understand the mechanical and electrical systems, including; heating, ventilation, power supply, lighting, and plumbing are all reportedly in good condition throughout the building.

We understand that our building condition assessment forms part of a broad assessment of the whole Toad River Highway Maintenance Camp site to aid the development an environmental remediation strategy for the camp site. Depending on the extent of environmental remediation proposed we foresee the following treatment options for the existing maintenance building.

- Retain the existing maintenance building in its current location.

If no significant environmental remediation work is required below or adjacent to the building, then as discussed above, the existing building superstructure is in a good condition and could be retained as it is.

However, given the poor condition of the building foundations we would recommend that the building Owner consider replacing these foundations with new in the near future.

- Relocate the existing maintenance building.

Should environmental remediation work require significant work to the ground beneath and around the existing maintenance building then relocating the building would be a viable option.

The robust nature of steel construction means it would be feasible to relocate the existing building superstructure without completely disassembling the structure. Part of the building relocation work should include the construction of new and appropriately designed foundations.

Preparation for relocating the building should also include the demolition of the interior washroom and mezzanine floor enclosures, demolition of the little lean-to office structure, and the appropriate disconnection and removal of mechanical and electrical equipment.

We would also recommend for either option that the site Owner considered reviewing the existing mechanical and electrical services, and upgrading these where appropriate.

We also understand you would like to excavate a small hole in one corner of the building and require advice on whether temporary support of the building structure is required, and if so, details of the temporary shoring.

We recommend, because of the shallow nature of the existing building foundations and their poor condition that temporary shoring of the building is required for the duration of the excavation works. Refer to the drawings attached for the extent of excavation we have currently assumed, and a possible shoring plan using scaffolding.

We trust our letter report provides you with enough information for this stage of the project. Please let us know if you would like to discuss our recommendations further or if you would like more detailed information on building relocation and foundation options, and whether you would like us to develop the proposed temporary shoring scheme in more detail.

Sincerely,

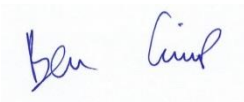
Prepared by:



SCOUTEN ENGINEERING

Craig Hadley – Civil Technologist
chadley@scoutenengineering.com

Reviewed by:



SCOUTEN ENGINEERING

Ben Crimp P.Eng., MStructE. – Senior Structural Engineer
bcrimp@scoutenengineering.com



Attach: Photographs
Scouten Maintenance Shop Drawings 427-04-100 to 104



Photo 1: View of maintenance shop from southeast.



Photo 2: View of east face of maintenance garage.

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201 – 1968 Queensway Prince George BC V2L 1M2
info@scoutenengineering.com T 250.562.7050

PROJECT : 427-04 DATE: June 23, 2016
LOCATION: Toad Review KM 649 Alaska Highway, BC
CLIENT: SNC-Lavalin
PROJECT: Toad River – Maintenance Garage Assessment
BY: CH PAGE: 1 of 9



Photo 3: View of north face of maintenance garage.



Photo 4: View of west face of maintenance garage.

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PROJECT : 427-04 DATE: June 23, 2016

LOCATION: Toad River KM 649 Alaska Highway, BC

CLIENT: SNC-Lavalin

PROJECT: Toad River – Maintenance Garage Assessment

BY: CH PAGE: 2 of 9



Photo 5: View of inside of building along west side.



Photo 6: View of inside of building along east side.

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PROJECT : 427-04 DATE: June 23, 2016

LOCATION: Toad River KM 649 Alaska Highway, BC

CLIENT: SNC-Lavalin

PROJECT: Toad River – Maintenance Garage Assessment

BY: CH PAGE: 3 of 9



Photo 7: View of wood-framed office on east side.



Photo 8: View of wood-framed washroom and used oil storage inside building on southeast corner.

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PROJECT : 427-04 DATE: June 23, 2016
LOCATION: Toad Review KM 649 Alaska Highway, BC
CLIENT: SNC-Lavalin
PROJECT: Toad River – Maintenance Garage Assessment
BY: CH PAGE: 4 of 9



Photo 9: View of warehouse and mezzanine in northwest corner.



Photo 10: View of electrical panel, air compressor and pressure washer.

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PROJECT : 427-04 DATE: June 23, 2016
 LOCATION: Toad Review KM 649 Alaska Highway, BC
 CLIENT: SNC-Lavalin
 PROJECT: Toad River – Maintenance Garage Assessment
 BY: CH PAGE: 5 of 9



Photo 11: View of used oil unit heater.



Photo 12: Existing used oil storage and washroom.

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PROJECT : 427-04 DATE: June 23, 2016

LOCATION: Toad River KM 649 Alaska Highway, BC

CLIENT: SNC-Lavalin

PROJECT: Toad River – Maintenance Garage Assessment

BY: CH PAGE: 6 of 9



Photo 13: View of secondary framing on north wall (upper).



Photo 14: View of secondary framing on north wall (lower).

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PROJECT : 427-04 DATE: June 23, 2016

LOCATION: Toad River KM 649 Alaska Highway, BC

CLIENT: SNC-Lavalin

PROJECT: Toad River – Maintenance Garage Assessment

BY: CH PAGE: 7 of 9



Photo 15: Typical baseplate at corner column.



Photo 16: View of existing timber sill plate.

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PROJECT : 427-04 DATE: June 23, 2016
LOCATION: Toad Review KM 649 Alaska Highway, BC
CLIENT: SNC-Lavalin
PROJECT: Toad River – Maintenance Garage Assessment
BY: CH PAGE: 8 of 9



Photo 17: View of existing base plate of portal frame.



Photo 18: View of existing floor slab showing uneven nature of slab surface.

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PROJECT : 427-04 DATE: June 23, 2016
LOCATION: Toad Review KM 649 Alaska Highway, BC
CLIENT: SNC-Lavalin
PROJECT: Toad River – Maintenance Garage Assessment
BY: CH PAGE: 9 of 9

STRUCTURAL NOTES:

1. GENERAL NOTES:
- 1.1

THE EXISTING BUILDING STRUCTURE HAS BEEN ASSESSED AS BEING SUITABLE FOR RELOCATION. SEE SCOUTEN ENGINEERING LETTERS REPORT, DATED
- 1.2

THE BUILDING STRUCTURE DETAILS AND WEIGHT OF BUILDING HAVE BEEN PROVIDED FOR GUIDANCE ONLY. THE CONTRACTOR CARRYING OUT THE BUILDING RELOCATION SHALL CARRY OUT WHATEVER CHECKS THEY FEEL ARE NECESSARY DURING THE DESIGN OF A BUILDING LIFT/RELOCATION PROCEDURES.
- 1.3

NEW FOUNDATIONS FOR THE NEW BUILDING ARE TO BE DESIGNED AND BUILT IN ACCORDANCE WITH:
BRITISH COLUMBIA BUILDING CODE 2012
BUILDING LOADS SUBJECTED ONTO THE PROPOSED FOUNDATIONS ARE TO BE CALCULATED USING CLIMATE DATA FROM ENVIRONMENT CANADA AND ANALYSIS OF THE EXISTING STRUCTURE.
- 1.4

DESIGN DOCUMENTS INCLUDING DRAWINGS ARE TO BE SUBMITTED TO PWGSC FOR THEIR REVIEW A MINIMUM OF 2 WEEKS PRIOR TO COMMENCEMENT OF WORKS ON SITE.
- 1.5

SEE THESE DRAWINGS FOR BUILDING IMPROVEMENTS TO BE INCLUDED IN THIS CONTRACT.
- 1.6

INFORMATION ON THE EXISTING BUILDING STRUCTURE WAS GATHERED DURING A SITE VISIT BY SCOUTEN ENGINEERING ON JUNE 9th, 2016.

2. ESTIMATED BUILDING WEIGHT:

- 3.1

AN ESTIMATED BUILDING WEIGHT IS NOTED ON THIS DRAWING FOR ESTIMATING/CONTEXT PURPOSES ONLY. THE ENGINEER OF RECORD FOR THE BUILDING RELOCATION WORK IS RESPONSIBLE FOR CONFIRMING THE BUILDING WEIGHT USED IN THEIR DESIGN WORK.
- 3.2

MATERIAL PROPERTIES ASSUMPTIONS:
STEEL DENSITY = 78 kN/m³
WALL AND ROOF PROFILE STEEL CLADDING THICKNESS = APPROX. 0.5mm
- 3.3

IT HAS BEEN ASSUMED THAT BUILDING SERVICES (LIGHTING, OVERHEAD DOOR ASSEMBLY, VENTILATION SYSTEMS, ETC.) WILL BE REMOVED PRIOR TO BUILDING BEING RELOCATED. THE WEIGHT OF BUILDING SERVICES SHOULD BE INCLUDED IF THEY ARE NOT TO BE REMOVED PRIOR TO THE BUILDING RELOCATION.
- 3.4

IT HAS ALSO BEEN ASSUMED INTERIOR FRAMING INCLUDING THE EXISTING OFFICE AND MEZZANINE INTERNAL STRUCTURES, EQUIPMENT AND TOOL RACKS, HAVE BEEN REMOVED AND/OR DEMOLISHED PRIOR TO BUILDING RELOCATION.
- 3.5

ESTIMATED BUILDING WEIGHT:
- 3.6

WT. OF PRIMARY AND SECONDARY STRUCTURE INCLUDES A 10% ALLOWANCE FOR MISCELLANEOUS ITEMS SUCH AS BOLTS AND SMALL FRAMING MEMBERS.

ISSUED WITH REPORT

DRAWING NOTES

REFERENCE DRAWINGS

DWG No.	REV	PROJECT No.	PRODUCED BY
-	-	-	-

DRAWING REVISIONS

B	16.06.30	CH	.	BC	ISSUED FOR REVIEW
A	16.06.27	CH	.	.	ISSUED WITH DRAFT REPORT
REV	DATE	DRW	DES	APP	REVISION

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CLIENT



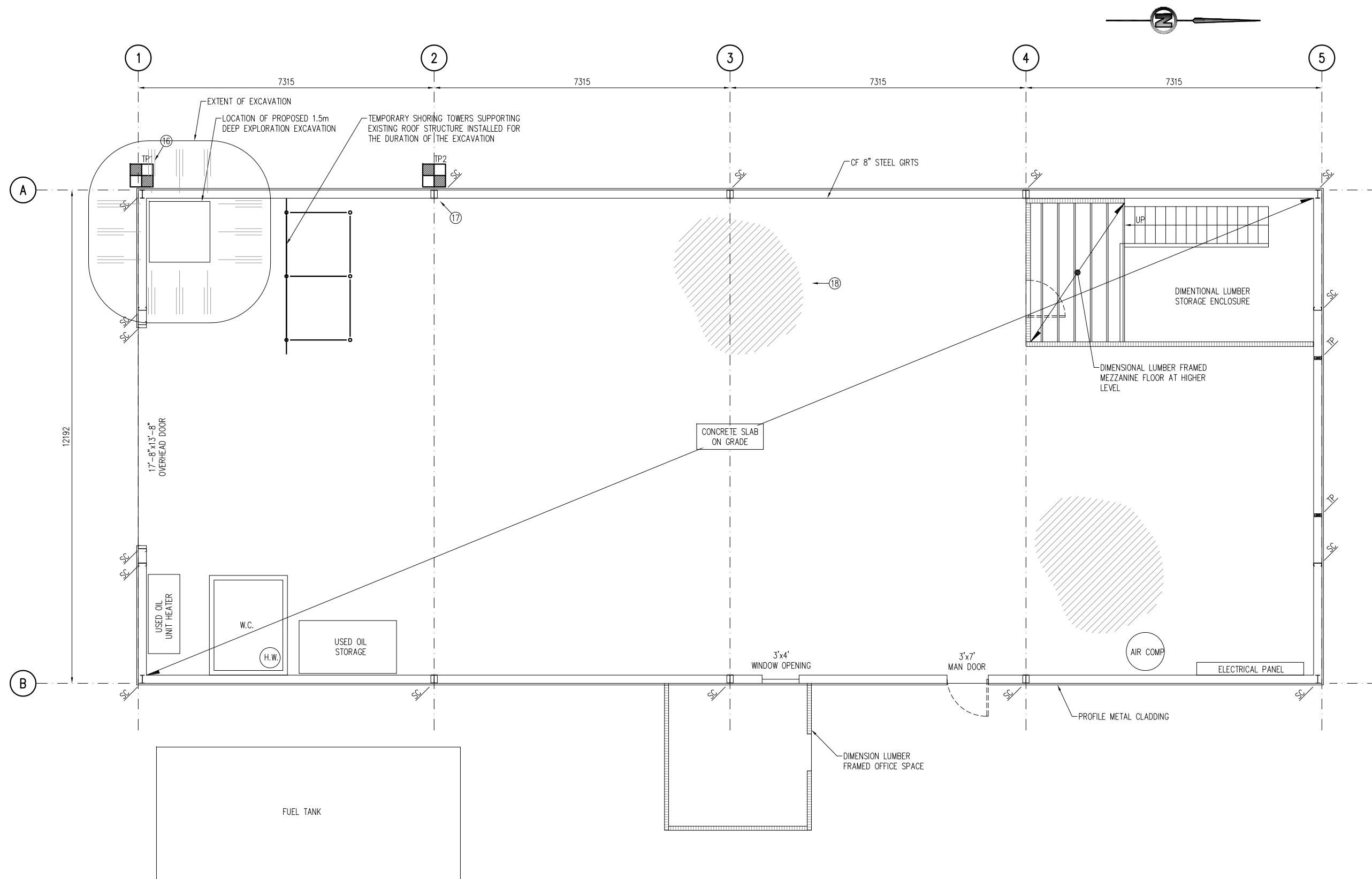
PROJECT INFORMATION

SNC LAVALIN

TOAD RIVER, BC
HIGHWAY MAINTENANCE
BUILDING ASSESSMENT

GENERAL NOTES

PROJECT No.	DRAWING No.	REV
427-04	100	B



EXISTING GROUND FLOOR PLAN
1:100

ISSUED WITH REPORT

DRAWING NOTES

1. FOR GENERAL NOTES SEE DRAWING 100.

2. LEGEND:

SC

STEEL COLUMN

TP

TIMBER COLUMN

TP#

TEST PIT

SPAN OF STEEL CLADDING

PHOTO REF. (SEE SCOUTEN ENGINEERING REPORT DATED

DIP IN EXISTING CONC. SLAB ON GRADE

REFERENCE DRAWINGS

DWG No.	REV	PROJECT No.	PRODUCED BY
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-

DRAWING REVISIONS

REV	DATE	DRW	DES	APP	REVISION
B	16.06.30	CH	-	BC	ISSUED FOR REVIEW
A	16.06.27	CH	-	-	ISSUED WITH DRAFT REPORT

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PROJECT INFORMATION

SNC LAVALIN

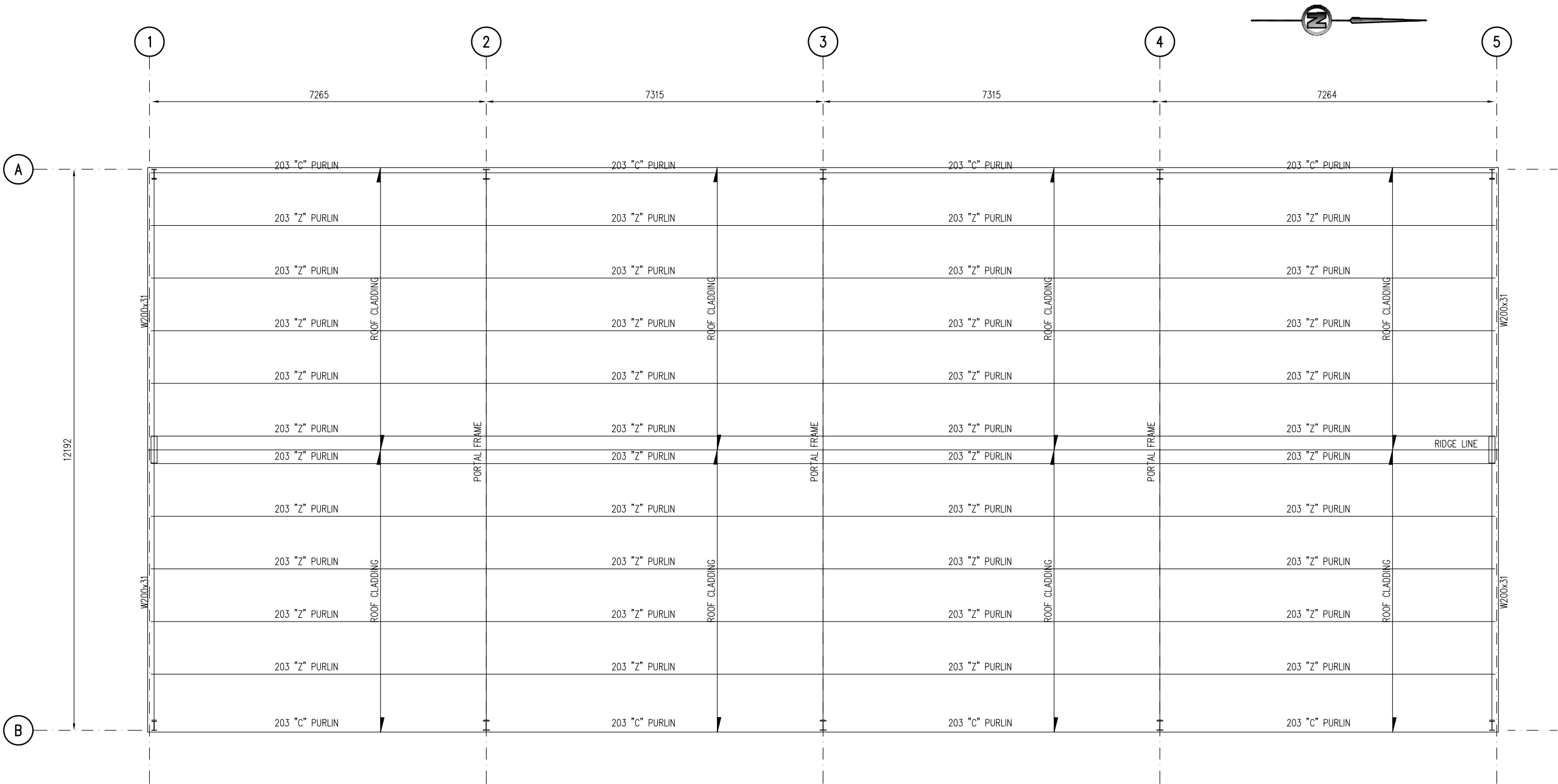
TOAD RIVER, BC
HIGHWAY MAINTENANCE
BUILDING ASSESSMENT

EXISTING GROUND FLOOR PLAN

PROJECT No.	DRAWING No.	REV
427-04	101	B

DISCARD ALL PREVIOUS REVISIONS

FILE NAME: 427-04.dwg
LAST UPDATED: 30.06.16



EXISTING ROOF PLAN
1:100

ISSUED WITH REPORT

DRAWING NOTES
1. FOR GENERAL NOTES SEE DRAWING 100.

REFERENCE DRAWINGS			
DWG No.	REV	PROJECT No.	PRODUCED BY
-	-	-	-

DRAWING REVISIONS					
B	16.06.30	CH	.	BC	ISSUED FOR REVIEW
A	16.06.27	CH	.	.	ISSUED WITH DRAFT REPORT
REV	DATE	DRW	DES	APP	REVISION

CONSULTANT

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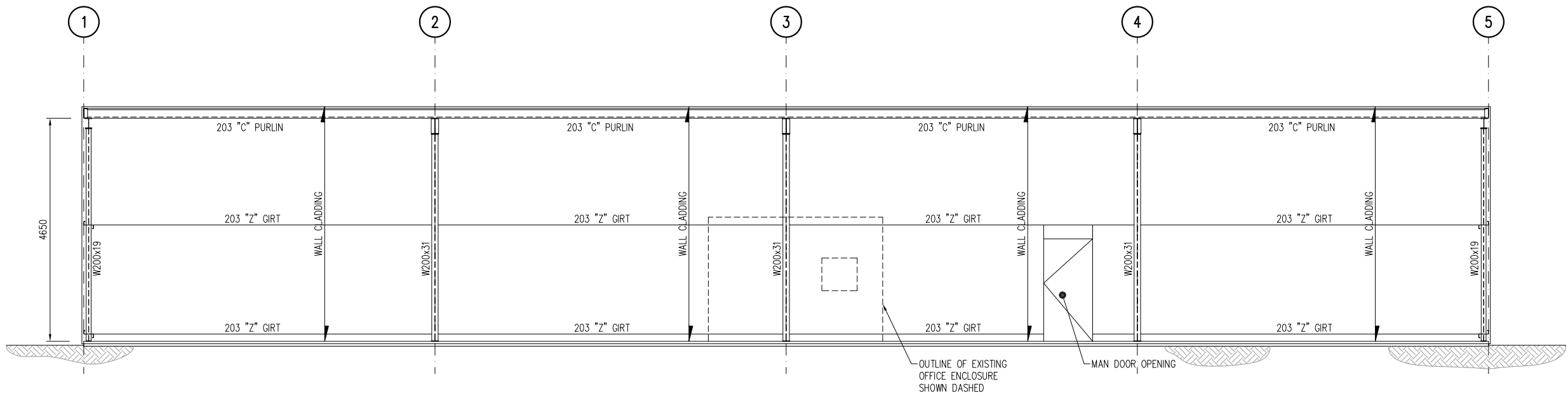
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SNC • LAVALIN

PROJECT INFORMATION		
SNC LAVALIN		
TOAD RIVER, BC HIGHWAY MAINTENANCE BUILDING ASSESSMENT		
EXISTING ROOF PLAN		
PROJECT No.	DRAWING No.	REV
427-04	102	B



SIDE WALL ELEVATION
1:100

DRAWING NOTES

1. FOR GENERAL NOTES SEE DRAWING G-100.

REFERENCE DRAWINGS

DWG No.	REV	PROJECT No.	PRODUCED BY
-	-	-	-

DRAWING REVISIONS

REV	DATE	DRW	DES	APP	REVISION
B	16.06.30	CH	.	BC	ISSUED FOR REVIEW
A	16.06.27	CH	.	.	ISSUED WITH DRAFT REPORT
REV	DATE	DRW	DES	APP	REVISION

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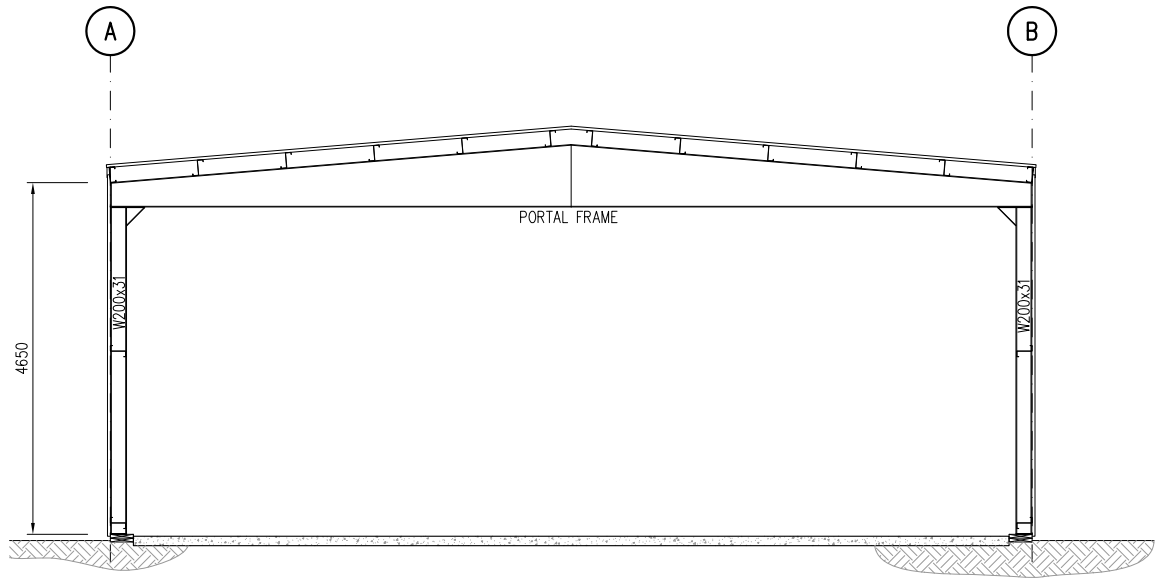


PROJECT INFORMATION

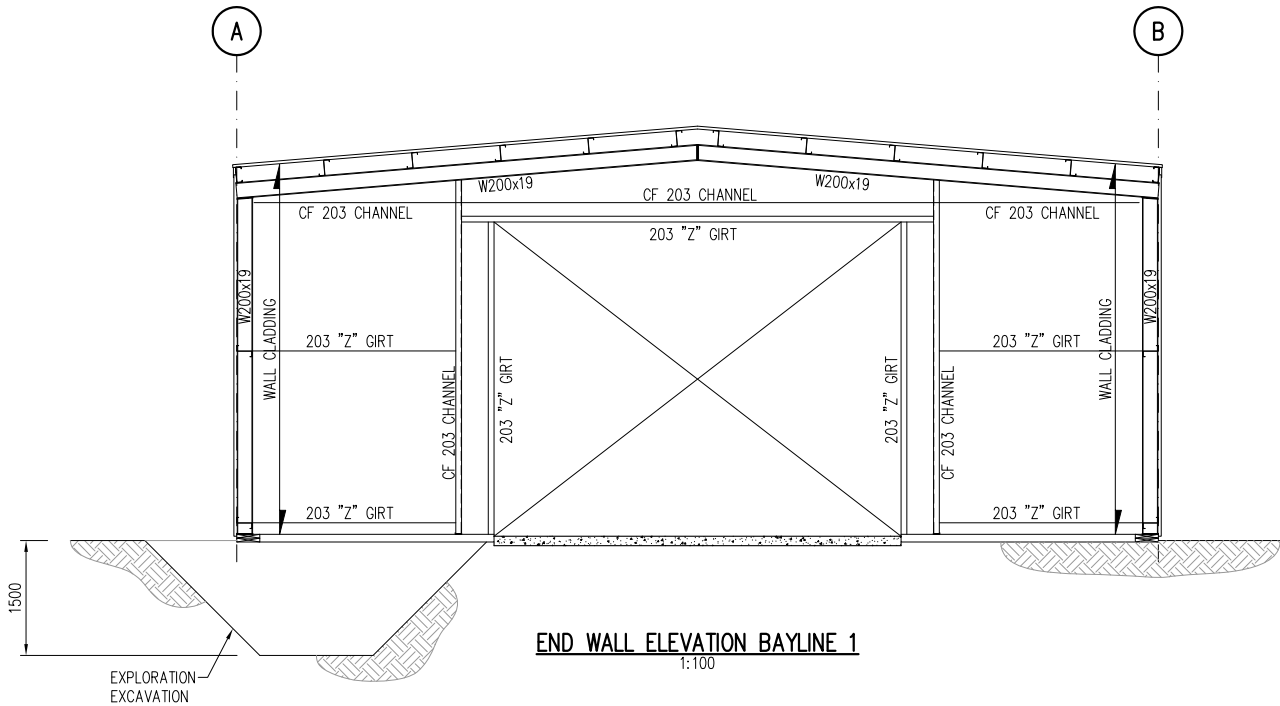
SNC LAVALIN
TOAD RIVER, BC
HIGHWAY MAINTENANCE
BUILDING ASSESSMENT
EXISTING SIDE WALL ELEVATION

PROJECT No.	DRAWING No.	REV
427-04	103	B

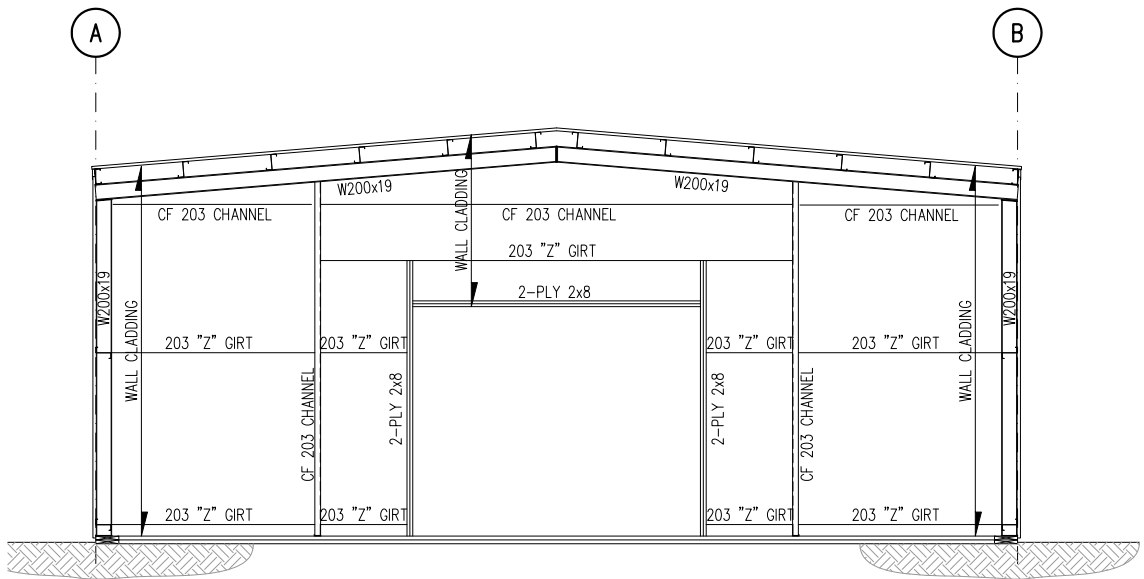
ISSUED WITH REPORT



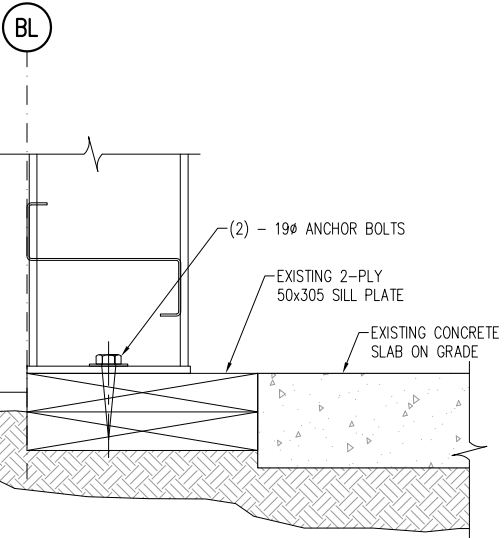
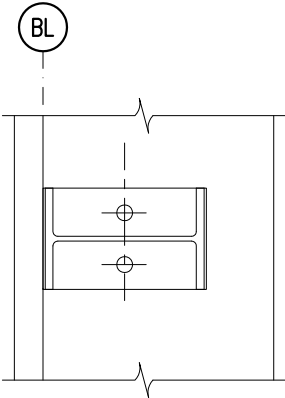
PORTAL FRAME ELEVATION
1:100



END WALL ELEVATION BAYLINE 1
1:100



END WALL ELEVATION BAYLINE 5
1:100



TYPICAL BASE PLATE DETAIL
1:10

ISSUED WITH REPORT

DRAWING NOTES

1. FOR GENERAL NOTES SEE DRAWING 100.

REFERENCE DRAWINGS

DWG No.	REV	PROJECT No.	PRODUCED BY
-	-	-	-

DRAWING REVISIONS

B	16.06.30	CH	.	BC	ISSUED FOR REVIEW
A	16.06.27	CH	.	.	ISSUED WITH DRAFT REPORT
REV	DATE	DRW	DES	APP	REVISION

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PROJECT INFORMATION

SNC LAVALIN

TOAD RIVER, BC

HIGHWAY MAINTENANCE

BUILDING ASSESSMENT

EXISTING PORTAL FRAME AND

END WALL ELEVATIONS

PROJECT No.	DRAWING No.	REV
427-04	104	B