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**FONDEX** LTD.  
LTEE  
CONSULTING ENGINEERS

GEOTECHNICAL STUDY  
RIDEAU CANAL  
JONES FALLS, ONTARIO

# 14  
Spec

PREPARED FOR

PARKS CANADA

PROJECT NO.: 0-6941

FEBRUARY 1985.

30 Concourse Gate, Nepean, K2E 7N7, 727-0895  
1124 CUMMINGS AVENUE, OTTAWA, ONTARIO K1J 7R8 TELEPHONE 748-1122

FAX 727-0581

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SUMMARY

The monoliths, at the lower locks (40 to 42), are made up of sandstone and granite blocks cemented with mortar. The quality of the stone varies significantly throughout the monoliths. It is considered to be in poor to fair condition.

The monoliths are resting on granitic bedrock of poor quality for the uppermost 0.5 m thick layer. The bedrock is, generally, more competent below the upper weathered layer.

Next to the log sill, the bottom of the lock is composed of a layer of concrete over sandstone and granite blocks of, generally, poor to fair condition. The blocks are founded on a granitic bedrock of fair to excellent quality.

Clayey silt fill was found underlying the stone masonry at the stair locations.

The recommended safe-net-bearing value for the weathered bedrock is 1000 kPa.

The recommended safe-net-bearing value for competent bedrock is 2000 kPa.

The clean clayey silt fill is considered to be acceptable as a foundation material, if the restoration does not impose additional pressure on the fill.

The foundation factor,  $F$ , for earthquake considerations is equal to 1.0 for foundations on bedrock.

The excavation side slope in the fill or natural ground should be cut-back at 45 degrees above a height of 1.2 m in order to comply with the Occupational Health and Safety Act.

The earth pressure on shoring is assumed to have a trapezoidal distribution, as given on p. 7.

The earth pressure on retaining walls is assumed to have a trapezoidal distribution with an earth pressure coefficient equal to 0.33.

The above points are discussed in detail in the following report.

## 1. SITE AND PROJECT DESCRIPTION

All components of the Jones Falls locks, located 40 km north of Kingston, Ontario, have experienced significant deterioration. The locks are used to commute between Sand Lake and Whitefish Lake. Parks Canada will be reconstructing and restoring the locks.

The locks are, primarily, divided into two sections. A first set of three locks, numbers 40 to 42, is used to travel from Whitefish Lake to the Turning Basin, while north of the Basin, lock 39 serves as an access from the Turning Basin to Sand Lake.

Physiographically, the Jones Falls area is characterized by shallow till overburden and outcropping rock ridges. The Pre-Cambrian rock in the study area is of the Grenville Formation consisting of granite-gneiss.

A geotechnical study has been undertaken to determine the bearing capacity and conditions beneath the monoliths in locks 40 to 42 and beneath the upper stop log sill in lock 39.

## 2. FIELD WORK AND SUBSOIL

The field work, consisting of 12 vertical boreholes and 2 horizontal boreholes, was carried out during the period of November 19, 1984, to January 11, 1985, using CME 55 drilling equipment and a portable diamond drill.

Core samples of the bedrock were taken in all vertical boreholes, except at boreholes numbers 11 to 14, inclusive, which were put down through the stone stairs on the north side of lock numbers 40 and 41. It was decided to replace the two proposed test pits along the stairs by boreholes 11 to 14 to avoid excessive damage to the property. Upon completion of the boreholes, they were filled with cement grout.

The logs of boreholes 1 through 8 with A and B, show that the monoliths, at the lower locks (40 to 42), are made up of sandstone and granite blocks cemented with mortar. A concrete layer of variable thickness was found covering the surface of the monoliths at all borehole locations. While the quality of the stone varies significantly throughout the monoliths, it is considered to be in poor to fair condition.

The monoliths are resting on granitic bedrock, which is of poor quality for the uppermost 0.5 m thick layer. The bedrock is, generally, more competent below the upper weathered layer.

The information obtained from borehole numbers 9 and 10 shows that, next to the log sill, the bottom of the lock is composed of a 75 mm to 125 mm thick layer of concrete over sandstone and granite blocks of, generally, poor to fair condition. The blocks are founded on a granitic bedrock of fair to excellent quality.

Boreholes numbers 11 to 14, inclusive, which were taken through the stone stairs, show that the stairs are made up of sandstone blocks in poor

to fair condition. The stone masonry rests on clean clayey silt fill, with frequent gravel sizes. The clayey silt fill was found overlying silty sand fill at borehole 12.

The composition of the monoliths and the bottom of the lock 39 as well as the subsoil encountered, is described in detail on the borehole logs given in Appendix 'B' of this report. The borehole locations are provided on the site plan, Dwgs. 1 and 2.

### 3. FOUNDATIONS

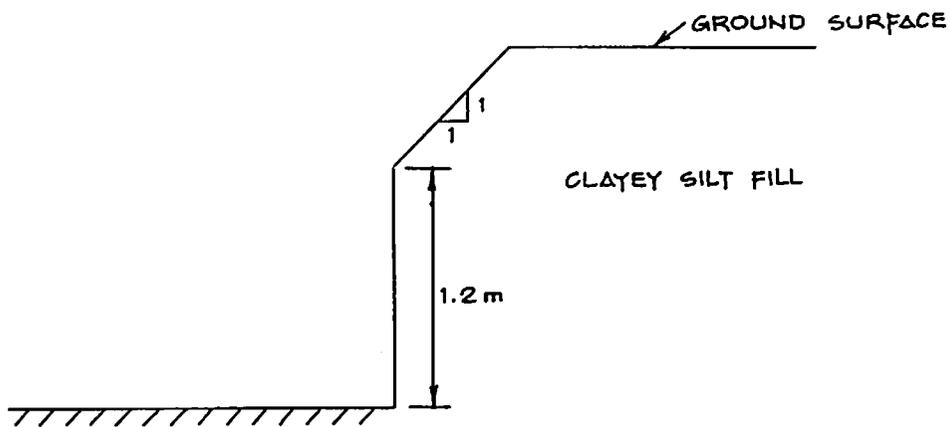
The two pairs of monoliths and the upper lock sill are founded on granitic bedrock. The surface of the bedrock is, generally, in very poor condition, a recommended safe-net-bearing value for the surface bedrock is 1000 kPa. A greater value could be obtained if the very poor quality bedrock is subexcavated. A recommended safe-net-bearing value for competent bedrock is 2000 kPa, or greater.

The monolith stairs are founded on clean clayey silt fill with frequent gravel sizes. The fill has been consolidating over the years under the structural loading and water percolation. If the restoration of the monoliths does not impose additional pressure on the fill, it is considered to be acceptable as a foundation material. Any foundation soil, which is disturbed during restoration, should be subexcavated and replaced with "engineered" fill.

The foundation factor,  $F$ , required to satisfy the conditions outlined in the Ontario Building Code for earthquake considerations, is 1.0 for foundations on bedrock.

#### 4. EXCAVATIONS AND GROUNDWATER

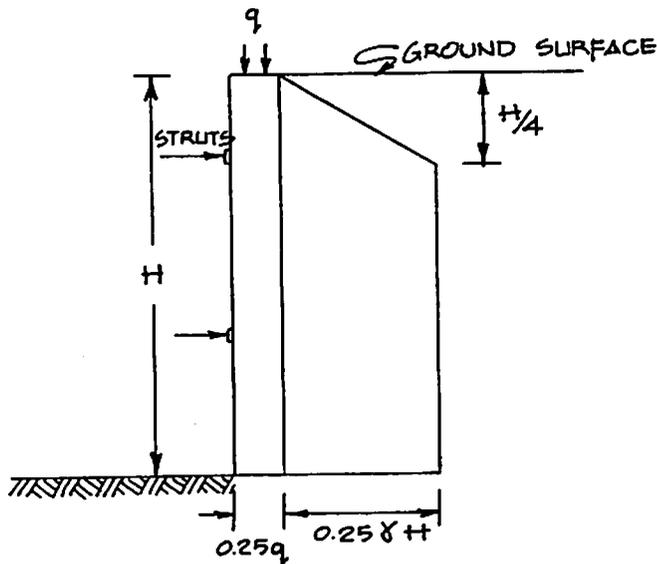
The excavation side slopes in the fill or natural ground should be cut-back at 45 degrees above a height of 1.2 m in order to comply with the Occupational Health and Safety Act. A sketch showing the recommended side slope configuration is given below.



SKETCH OF UNBRACED EXCAVATION

If shoring is to be used as an alternative means for excavation, the earth pressure can be assumed to have a trapezoidal distribution.

A sketch of the recommended pressure distribution is provided below:



SKETCH OF EARTH PRESSURE ON SHORING

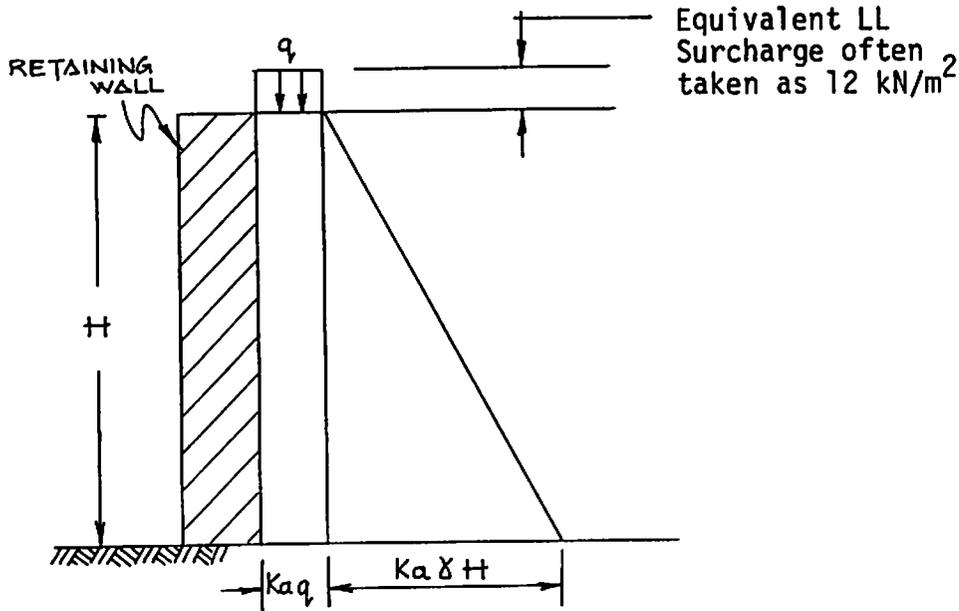
where

- $q$  = any surcharge loading acting on the surface adjacent to the shoring.  
 $\gamma$  = 20 kN/m<sup>3</sup>

### 5. EARTH PRESSURE ON RETAINING WALLS

The earth pressure acting on retaining walls can be assumed to have a triangular distribution.

A sketch of the recommended pressure distribution is shown below:



assume  $\delta = 20 \text{ kN/m}^3$   
 $Ka = 0.33$

SKETCH OF EARTH PRESSURE ON RETAINING WALLS

In the above configuration, there has been no allowance made for hydrostatic build-up against the wall. Therefore, if there is no provision made for drainage of the backfill material, it will be necessary to design the structure for the increased loading due to hydrostatic pressure.

FONDEX LIMITED

Glenn Collins, M.Sc.

Henry R. Krzywicki, P.Eng.

APPENDIX

APPENDIX 'A'

EXPLANATORY NOTES  
ON THE  
RECORDS OF BOREHOLES

EXPLANATORY NOTESON THERECORDS OF BOREHOLES

The purpose of borehole records is to assemble on a single sheet all of the field and laboratory data obtained during the investigation regarding the soil, bedrock, and groundwater conditions at the location of the borehole.

SOIL PROFILE

Elevation: This column gives the elevations of boundaries between various geological strata. The elevation refers to the datum shown in the heading of the borehole record. The corresponding depths below the ground surface are also shown.

Description: Each geological stratum is described, using standard terminology, from examination and analyses of samples.

The relative density of granular soils is defined on the basis of the Standard Penetration Test. The consistency of cohesive soils is referred to in terms of either shear strength or unconfined compressive strength. The proportion of each constituent part as

defined by the grain size is denoted by the following terms.

Relative Density  
(granular soils)

Standard Penetration  
Test Value "N"

Very loose	0 to 4
Loose	4 to 10
Compact or Medium	10 to 30
Dense	30 to 50
Very dense	over 50

Consistency  
(cohesive soils)

Undrained Shear Strength (c)  
(KN/m<sup>2</sup>)

Very soft	under 10
Soft	10 to 25
Medium or firm	25 to 50
Stiff	50 to 100
Very stiff	100 to 200
Hard	over 200

Plasticity of Cohesive SoilsLiquid Limit

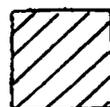
Low	under 30%
Medium	30 to 50%
High	over 50%

Descriptive TermsRange of proportion

"Trace"	1 to 10 %
"Some"	10 to 20 %
Adjective (c.g. sandy, silty)	20 to 35 %
"and" (c.g. sand and gravel)	35 to 50 %

STRATIGRAPHIC PLOT

The following stratigraphical symbols are used to denote main soils types:



Clay



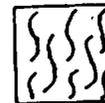
Sand

Cobbles and/or  
boulders

Silt



Gravel



Organic soil

## GROUNDWATER CONDITIONS

The groundwater level as observed in the borehole is shown by the symbol  $\nabla$

## SAMPLES

Number: Each sample taken from the borehole is numbered as shown in this column; the exact location and the length of each sample are also shown.

Type: The symbols shown are referred to the following sample types:

AS: auger sample  
SS: split spoon sample  
ST: Shelby tube sample  
WS: washed sample  
RC: core sample

(N) Standard Penetration Resistance: The number of blows required to advance a Standard Split Barrel sampler 0.3 m into the subsoil, driven by means of a hammer, having 63.5 kg ( $\pm 0.5$  kg) mass, falling freely a distance 0.76 m ( $\pm 0.02$  m)

Recovery: Rock core recoveries are given in percentages.

#### DYNAMIC PENETRATION RESISTANCE

The number of blows required to advance a 51 mm diameter 60<sup>0</sup> steel cone fitted to the end of 45 mm OD drill rods, 0.3 m into the subsoil. The driving energy being 475 J per blow.

#### STRENGTH

Results of field or laboratory strength tests on cohesive soils are shown graphically in the "Shear Strength" column using the indicated symbols.

#### CONSISTENCY

Results of moisture content, liquid limit, and plastic limit tests as determined in the laboratory are shown under "Consistency".

A P P E N D I X "B"

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BOREHOLE LOGS





0-6941

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BOREHOLE NUMBER 1

PROJECT Jones Falls Geotechnical Study

LOCATION Rideau Canal, Jones Falls, Ontario

DRILLING DATE Dec 28, 29, 31

DATUM Geodetic BOREHOLE TYPE Portable Diamond Drill

REPORT DATE Feb 18/85

DRAWN BY GC

GEOLOGIC PROFILE		SAMPLES				DYNAMIC PENETRATION RESISTANCE BLOWS					CONSISTENCY :								
Elev. Depth	DESCRIPTION	STRATIGRAPHY	NUMBER	TYPE	% RGD	% RECOVERY	RESISTANCE BLOWS					NATURAL MOISTURE CONTENT (W)							
							0	20	40	60	80	SHEAR STRENGTH		LIQUID LIMIT (W)		PLASTIC LIMIT (Wp)			
111.54 0.0	200 mm CONCRETE CAP		1	RC	19	62													
	<u>SANDSTONE</u>																		
	RED, COARSE GRAINED, WITH MORTAR, POOR CONDITION		2	RC	0	23													
1.0			3	RC	40	60													
			4	RC	100	100													
2.0	- LOST DRILLING WATER AT 2.0 m. DEPTH		5	RC	52	66													
	- GOOD CONDITION BETWEEN 2 m AND 5 m DEPTH.		6	RC	85	100													
3.0			7	RC	68	74													
4.0			8	RC	86	100													
			9	RC	50	81													
5.0	- OCCASIONAL GNEISS AND QUARTZITE BOULDERS BY 5 m DEPTH.		10	RC	0	65													
6.0			11	RC	42	57													
104.54 7.0	CONTINUED		12	RC	25	75													

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BOREHOLE NUMBER 1 (cont)

PROJECT Jones Falls Geotechnical Study

DRILLING DATE Dec 28,29,31/84

LOCATION Rideau Canal, Jones Falls, Ontario

REPORT DATE Feb 18/85

DATUM Geodetic BOREHOLE TYPE Portable Diamond Drill

DRAWN BY GC

GEOLOGIC PROFILE		SAMPLES				DYNAMIC PENETRATION RESISTANCE BLOWS					CONSISTENCY:		
Elev. Depth	DESCRIPTION	STRATIGRAPHY	NUMBER	TYPE	% RQD	% RECOVERY	0	20	40	60	80	NATURAL MOISTURE CONTENT (w) <span style="float: right;">—•—</span>	
							<u>SHEAR STRENGTH</u>					LIQUID LIMIT (w) <span style="float: right;">—○—</span>	
							FIELD VANE SHEAR <span style="float: right;">•</span>					PLASTIC LIMIT (Wp) <span style="float: right;"> </span>	
							LAB VANE SHEAR <span style="float: right;">X</span>					%	
104.54	CONTINUED												
7.0	<p><u>SANDSTONE</u> RED, COARSE GRAINED, FREQUENT QUARTZITE AND OCCASIONAL GNEISS BOULDERS</p> <p>- FAIR CONDITION</p> <p>- SEVERAL VOIDS NOTED BETWEEN 9 AND 10.0 m DEPTH.</p>		13	RC	26	59							
8.0			14	RC	23	35							
9.0			15	RC	60	100							
101.44	<p><u>QUARTZITE BEDROCK</u> WHITE, WEATHERED AT UPPER LEVELS, BECOMING SOUND BY 11.7 m. DEPTH.</p>		16	RC	29	45							
10.1			17	RC	78	77							
11.0			18	RC	78	77							
12.0			19	RC	0	82							
99.44			20	RC	56	100							
12.1	END OF BOREHOLE												
	<u>NOTES</u>												
	1) LOSS OF DRILL WATER UPON PENETRATION OF CONCRETE CAP.												







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BOREHOLE NUMBER 3B

PROJECT Jones Falls Geotechnical Study  
 LOCATION Rideau Canal, Jones Falls, Ontario  
 DATUM Geodetic BOREHOLE TYPE CME 55

DRILLING DATE Nov 22, 23/84  
 REPORT DATE Feb 18/85  
 DRAWN BY GC

GEOLOGIC PROFILE		SAMPLES				DYNAMIC PENETRATION RESISTANCE BLOWS					CONSISTENCY:				
Elev. Depth	DESCRIPTION	STRATIGRAPHY	NUMBER	TYPE	% RGD	% RECOVERY	RESISTANCE BLOWS					MOISTURE CONTENT (W)			
							0	20	40	60	80	NATURAL MOISTURE		LIQUID LIMIT (W)	
							SHEAR STRENGTH								
							FIELD VANE SHEAR					—			
							LAB VANE SHEAR					—			
111.56 0.0	100mm CONCRETE CAP SAND FILL:														
111.06 0.5	LOOSE, BROWN, OCCASIONAL GRAVEL SIZES AND WOOD PIECES, MOIST.		1	RC	0	33									
1.0	SANDSTONE RED, COARSE GRAINED IN MORTAR  POOR CONDITION														
2.0	OCCASIONAL QUARTZITE BOULDERS BY 1.5m. DEPTH.		2	RC	21	35									
3.0	FAIR TO GOOD CONDI- TION BY 3m. DEPTH  75mm VOID AT 3.86m. DEPTH.		3	RC	75	91									
4.0															
5.0			4	RC	39	53									
6.0															
104.56 7.0	CONTINUED														





















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BOREHOLE NUMBER 8 (cont)

PROJECT Jones Falls Geotechnical Study

DRILLING DATE Nov 27/84

LOCATION Rideau Canal, Jones Falls, Ontario

REPORT DATE Feb 18/85

DATUM Geodetic BOREHOLE TYPE CME 55

DRAWN BY GC

GEOLOGIC PROFILE		SAMPLES				DYNAMIC PENETRATION RESISTANCE BLOWS					CONSISTENCY:								
Elev. Depth	DESCRIPTION	STRATIGRAPHY	NUMBER	TYPE	% RQD	% RECOVERY	RESISTANCE BLOWS					MOISTURE CONTENT (W)							
							0	20	40	60	80	SHEAR STRENGTH		LIQUID LIMIT (W)		PLASTIC LIMIT (Wp)			
												FIELD VANE SHEAR		LAB VANE SHEAR					
												·		X					
100.42	CONTINUED																		
7.0	<u>SANDSTONE</u> RED, COARSE GRAINED WITH FREQUENT GNEISS AND QUARTZITE BOULDERS, IN MORTAR  - POOR TO FAIR CONDITION		6	RC	17	61													
8.0			7	RC	24	54													
9.0			8	RC	0	35													
10.0			9	RC	24	51													
11.0																			
95.72	<u>QUARTZITE BEDROCK</u> GREY, SOUND  GNEISS INTRUSION BY 13.1m DEPTH+.		10	RC	65	97													
11.7																			
94.13	END OF BOREHOLE																		
13.28																			





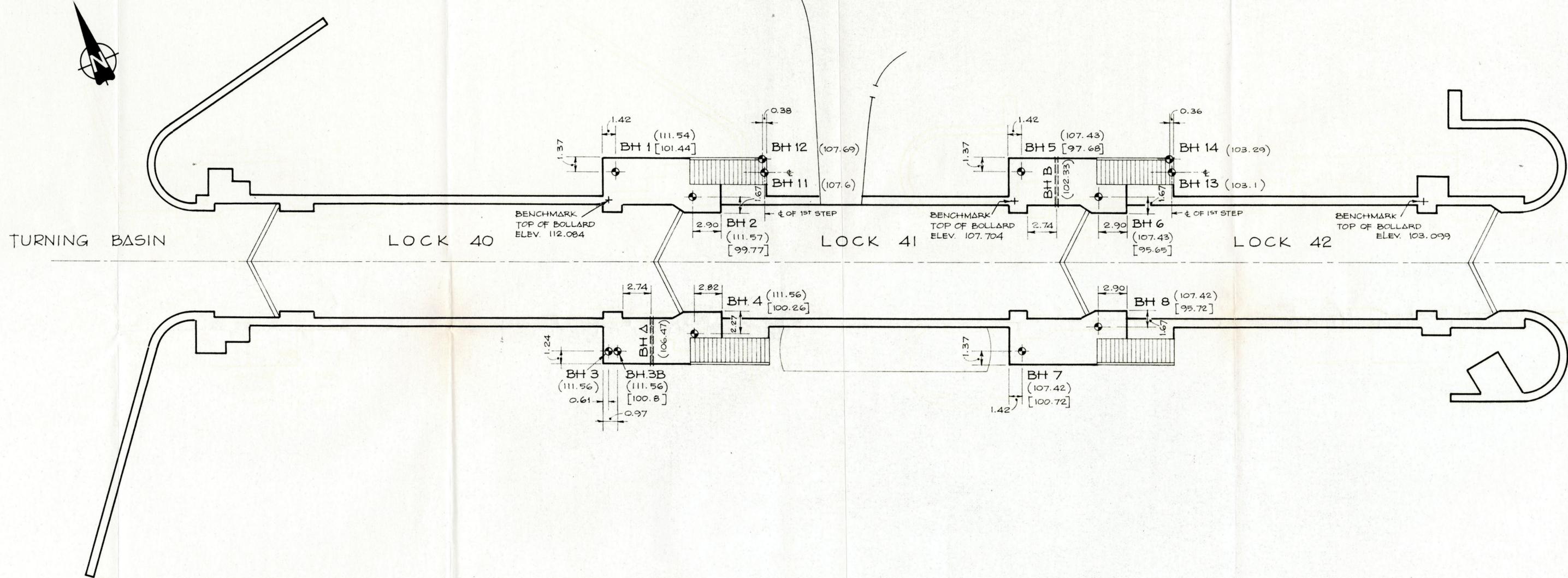








**DRAWINGS**



**LEGEND**

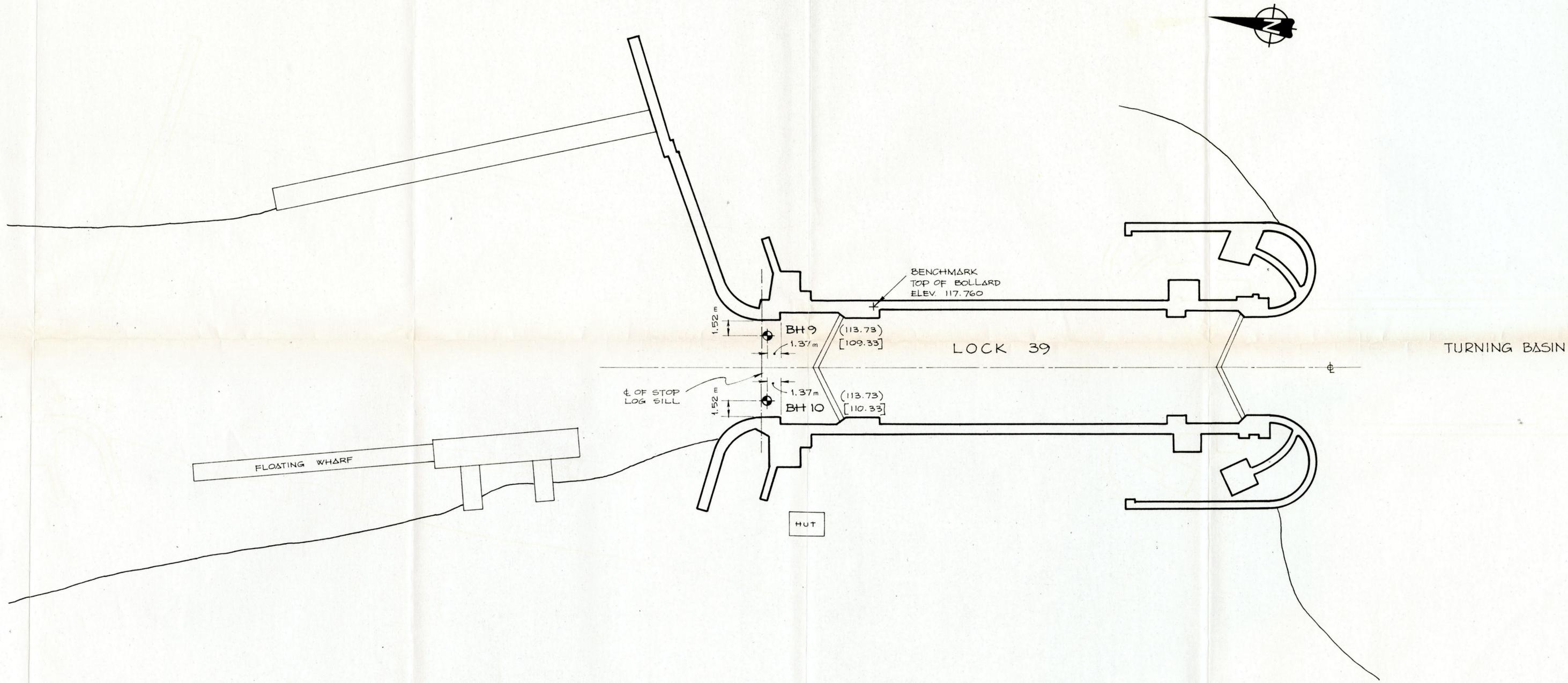
- ⊕ BH 5 - VERTICAL BOREHOLE
- ≡≡≡ BH A - HORIZONTAL BOREHOLE
- (107.43) - SURFACE ELEVATION
- [97.68] - BEDROCK ELEVATION

**JONES FALLS  
GEOTECHNICAL STUDY  
RIDEAU CANAL  
JONES FALLS, ONTARIO.**

LOCATION OF BOREHOLES

DRAWN BY: G.V.	DRAWING No: O-6941-5-1 of 2
CHECKED BY: G.C.	

<b>FONDEX LTD.</b>	SCALE: 1:240 METRIC
	DATE: FEBRUARY 1985



LEGEND

- BH 9 - VERTICAL BOREHOLE
- (113.73) - SURFACE ELEVATION
- [109.33] - BEDROCK ELEVATION

<b>JONES FALLS GEOTECHNICAL STUDY RIDEAU CANAL JONES FALLS ONTARIO.</b>	
LOCATION OF BOREHOLES	
DRAWN BY: G.V. CHECKED BY: G.C.	DRAWING No: O-6941-S-2 of 2
<b>FONDEX LTD.</b>	SCALE: 1:240 METRIC DATE: FEBRUARY 1985