

GENERAL NOTES

DESIGN LOADS


- THIS BUILDING HAS BEEN DESIGNED IN ACCORDANCE WITH THE NATIONAL BUILDING CODE OF CANADA 2015 EDITION & THE CORRECTIONAL SERVICE CANADA TECHNICAL CRITERIA FOR CORRECTIONAL INSTITUTIONS, APRIL 2015.
- THIS BUILDING HAS BEEN ASSIGNED AN IMPORTANCE CATEGORY OF "NORMAL". Is=1.0, Iw=1.0, & Ie=1.0

GRAVITY LOADS		
AREA	LIVE/SNOW	SUPERIMPOSED DEAD
ROOFS	2.6 (+ SNOW DRIFTING)	1.0 kPa
MAIN FLOOR	4.8 kPa	1.0 kPa
INTERIOR SECURE CEILINGS	1.0 kPa	1.0 kPa
STORAGE / SUPPLY ROOMS	12.0 kPa	2.0 kPa

- SNOW LOADS
SPECIFIED SNOW LOAD $S = I_s (S_s(CbCwCsCa)+S_r) + \text{SNOW DRIFT AS PER CODE.}$
 $S_s = \text{GROUND SNOW} = 2.4 \text{ kPa}$ $C_a = 1.0, C_b = 0.8$
 $S_r = \text{ASSOCIATED RAIN LOAD} = 0.7 \text{ kPa}$ $C_w = 1.0, C_s = 1.0$
- WIND LOADS:
REFERENCE VELOCITY PRESSURE FOR 1/50 = 0.47 kPa
- SEISMIC LOADS:
 $S_a(0.2) = 0.457, S_a(0.5) = 0.384$
 $S_a(1.0) = 0.244, S_a(2.0) = 0.157$
 $S_a(5.0) = 0.057, S_a(10.0) = 0.020$
 $P_{G\text{Aref}} = 0.206$
 $M_v = 1.0$ (HIGH MODE FACTOR)
SITE CLASS 'D'
CALCULATED $V_f = 0.221 \text{ W}$
 $R_d = 1.5, R_o = 1.5$
(CONVENTIONAL CONSTRUCTION MASONRY SHEAR WALLS)

CALCULATED $V_f = 0.191 \text{ W}$
 $R_d = 2.0, R_o = 1.3$
(LIMITED DUCTILITY CONCENTRICALLY BRACED FRAME)



FOUNDATIONS

- FOUNDATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE RECOMMENDATIONS IN THE SOILS REPORTS PREPARED BY: BRAUN GEOTECHNICAL, DATED NOVEMBER 30, 2015.
- FOUNDATIONS TO BEAR ON NATIVE SOILS WITH A FACTORED ULTIMATE BEARING PRESSURE OF 180 kPa, AND AN ALLOWABLE BEARING PRESSURE OF 120 kPa.
- ALL EXISTING FILLS TO BE REMOVED AND REPLACED WITH COMPACTED STRUCTURAL FILL TO GEOTECHNICAL ENGINEER'S APPROVAL.
- FOUNDATION BEARING MATERIAL TO BE APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE.
- FOOTINGS TO BE CENTERED UNDER WALLS AND COLUMNS U.N.O. ON THE DRAWINGS.
- WALLS AND COLUMNS TO BE DOWELED TO FOUNDATIONS WITH DOWELS HOOKED ONE END OF THE SAME SIZE AND SPACING AS VERTICAL REINFORCEMENT.
- FOOTING TYPES SHOWN THUS:  SEE SCHEDULE FOR FOOTING SIZES AND REINFORCEMENT.
- ELEVATIONS SHOWN THUS $\frac{00.000}{00.000}$ ARE TOP OF FOOTING ELEVATIONS AND ARE FOR ESTIMATING PURPOSES ONLY. FINAL ELEVATIONS ARE TO BE DETERMINED BY SITE CONDITIONS.
- STEPS BETWEEN UNDERSIDES OF ADJACENT FOOTINGS SHALL BE A MAXIMUM SLOPE OF 2 HORIZONTAL: 1 VERTICAL.
- SLAB ON GRADE TO BE UNDERLAIN BY 0.25mm POLY OVER 150mm OF CLEAN 19mm MINUS CRUSHED GRANULAR BASE MATERIAL OR WELL GRADED SAND COMPACTED TO 100% OF THE MATERIAL SPD MAX. DRY DENSITY, U.N.O. IN THE SOILS REPORT.

CONCRETE

- CONCRETE, CONCRETE MATERIALS AND METHODS OF CONSTRUCTION SHALL CONFORM TO CSA A23.1-14 AND A23.3-14
- CONCRETE PROPERTIES TO BE AS PER TABLE ON THESE DRAWINGS.
- FOR AREAS OF CONGESTED REINFORCEMENT, AND THIN CONCRETE SECTIONS, USE A REDUCED AGGREGATE SIZE IN THE CONCRETE MIX AND ADD SUPERPLASTICIZER TO MIX ON SITE TO INCREASE WORKABILITY.
- CONCRETE MIX DESIGNS TO BE SUBMITTED TO THE DEPARTMENTAL REPRESENTATIVE FOR REVIEW PRIOR TO COMMENCING THE WORK.
- CURING AND PROTECTION OF CONCRETE FOR HOT, COLD, OR DRY WEATHER TO BE IN ACCORDANCE WITH CSA A23.1-14, SECTION 7.4. PROVIDE MOIST CURE FOR 3 DAYS MINIMUM (ANY ALTERNATIVE METHOD MUST BE REVIEWED BY THE DEPARTMENTAL REPRESENTATIVE).
- LOCATION AND DETAILS OF CONSTRUCTION JOINTS TO BE REVIEWED BY THE DEPARTMENTAL REPRESENTATIVE. SEE DETAILS ON THESE DRAWINGS.
- HORIZONTAL CONSTRUCTION JOINTS IN WALLS TO BE CLEAN AND INTENTIONALLY ROUGHENED TO A MINIMUM 5mm AMPLITUDE.
- ALL EXTERIOR EXPOSED CONSTRUCTION JOINTS SHALL BE DESIGNED USING A WATER STOP SYSTEM. CONSTRUCTION JOINT CONCRETE SURFACES TO BE COATED WITH WATER STOP SLURRY AT THE MANUFACTURERS SPECIFIED RATE. A CONTINUOUS WATER STOP BLOCK OUT 45mm x 45mm TO BE PROVIDED AT THE JOINT SURFACE FILLED WITH A DRY MIX OF WATER STOP GROUT PRIOR TO BE PLACING ADJACENT CONCRETE. SUBMIT PROPOSED PRODUCTS AND PROCEDURES TO THE DEPARTMENTAL REPRESENTATIVE FOR REVIEW.
- CALCIUM CHLORIDE IS NOT PERMITTED IN CONCRETE MIXES.
- SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR EXTENT OF FLOOR HARDENERS AND ARCHITECTURAL CONCRETE FINISHES.
- SEE ARCHITECTURAL DRAWINGS FOR LOCATION AND EXTENT OF ALL REVEALS, DRIPS, RECESSES AND OTHER ADDITIONAL FEATURES.
- CONCRETE COVER TO BE AS PER TABLE ON THESE DRAWINGS.
- CONCRETE TESTING TO BE IN ACCORDANCE WITH CSA A23.2-14. TESTING TO BE PAID FOR BY CONTRACTOR.
- INTENTIONALLY ROUGHEN THE INTERFACE BETWEEN POURS TO 6mm AMPLITUDE.

REINFORCING STEEL

- BARS SHOWN THUS:  INDICATE TOP REINFORCING STEEL.
- BARS SHOWN THUS:  INDICATE BOTTOM REINFORCING STEEL.
- ALL REINFORCING STEEL TO BE DEFORMED BARS CONFORMING TO CSA G30.18-09, GRADE 400W.
- MINIMUM LAPS OF REINFORCEMENT TO BE AS PER TABLE ON THESE DRAWINGS.
- HOOKS AND FABRICATION DETAILS TO CONFORM TO CSA A23.1-14.
- ALL HOOKS TO BE 'STANDARD' IN ACCORDANCE WITH CSA A23.1-14 U.N.O.
- 'H.I.E.' DENOTES HOOK ONE END. THE LENGTH NOTED INCLUDES HOOK.
- CLEAR SPACING BETWEEN REINFORCING BARS PLACED IN ONE LAYER OR MINIMUM CLEAR SPACING BETWEEN LAYERS OF REINFORCEMENT TO BE AS FOLLOWS:
(UNLESS NOTED OTHERWISE)
20M AND SMALLER - 30mm
25M - 35mm
- PROVIDE MINIMUM REINFORCEMENT IN ALL CAST-IN-PLACE CONCRETE WALLS AS PER TABLE ON THESE DRAWINGS.
- REINFORCE EXTERIOR CONCRETE PAVING, SLABS, AND SIDEWALKS WITH 10M@350 O.C. E.W. MINIMUM.

CHAIRING OF REINFORCEMENT

- PROVIDE SPACER BARS FOR BEAMS WITH MULTIPLE LAYERS OF REINFORCEMENT AT MAXIMUM 1200mm ON CENTRE.
- USE 15M SUPPORT BARS AT 1200mm MAXIMUM ON CENTRE AS WELL AS ONE 15M EACH SIDE OF SUPPORTING WALL OR BEAMS.
- FLYING ENDS OF TOP BARS NOT TO EXCEED 450mm.
- CHAIR SUPPORT BARS AT 1200mm ON CENTRE MAXIMUM.
- CHAIR BOTTOM REINFORCING AT 1200mm ON CENTRE MAXIMUM EACH WAY.
- CHAIRS AND BOLSTERS TO BE PURPOSE MADE NON-METALLIC.
- POSITION CHAIRS FOR EXPOSED CONCRETE SLAB AND BEAM SOFFITS IN A REGULAR PATTERN CONFORMING WITH FINAL ARCHITECTURAL FINISH.
- PLASTIC TIES OR PLASTIC-COATED WIRES SHALL BE USED FOR TYING EPOXY-COATED REINFORCEMENT.
- PROVIDE REBAR CHAIRS FOR TOP BARS IN FOOTINGS.

EXTRA REINFORCEMENT

- PROVIDE TWO 15M CONTINUOUS AT THE ENDS AND TOPS OF WALLS AND EDGES OF ALL SLABS, MINIMUM.
- PROVIDE ON EACH FACE TWO 15M EXTRA TOP, BOTTOM AND EACH SIDE OF OPENINGS IN WALLS AND SLABS. RUN BARS 600mm MINIMUM BEYOND OPENING. PROVIDE ONE 15M x 1200mm DIAGONAL BAR AT EACH CORNER OF THE OPENING ON EACH FACE.
- FOR OPENINGS UP TO 450mm WIDE, FLARE REINFORCEMENT AROUND OPENINGS. FOR OPENINGS OVER 450mm WIDE, TERMINATE REINFORCEMENT AT OPENING. PROVIDE BARS, OF EQUAL NUMBER AND AREA TO THAT TERMINATED, ON EACH SIDE OF OPENING IN ADDITION TO THAT SPECIFIED ABOVE. RUN ALL EXTRA BARS CONTINUOUS TO THE SUPPORTS.
- PROVIDE CORNER BARS x 1200mm LONG MINIMUM (H1E 600mm) TO MATCH SIZE AND SPACING OF HORIZONTAL REINFORCEMENT IN WALLS, FOOTINGS & GRADE BEAMS UNLESS NOTED OTHERWISE. REFER TO TYPICAL DETAIL ON THESE DRAWINGS.
- PROVIDE 10mx900 LONG DIAGONAL BARS AT ALL CORNERS, COLUMNS AND OPENINGS IN SLABS ON GRADE.

CONDUITS, PIPES & SLEEVES EMBEDDED IN CONCRETE

- PIPES, CONDUITS, AND SLEEVES EMBEDDED IN CONCRETE SHALL BE ALLOWED ONLY IF INSTALLED IN ACCORDANCE WITH THE FOLLOWING GUIDELINES.
- SUBMIT LAYOUT OF CONDUITS AT POINTS OF CONGESTION AND PROVIDE ADDITIONAL REINFORCING AND/OR THICKEN SLAB AND/OR RE-ROUTE AS DIRECTED BY THE DEPARTMENTAL REPRESENTATIVE, AT CONTRACTOR'S EXPENSE.
- SLABS AND WALLS (CONDUITS IN PLANE OF):
 - LOCATE BETWEEN TOP AND BOTTOM REINFORCING IN SLAB OR EACH FACE OF WALL.
 - MAXIMUM SIZE OF CONDUIT IN ONE LAYER TO BE NOT MORE THAN ONE-QUARTER (1/4) CONCRETE THICKNESS.
 - CENTRE-LINE SPACING BETWEEN PARALLEL CONDUITS TO BE NOT LESS THAN 3 DIAMETERS.
 - MAXIMUM TOTAL SIZE OF CONDUITS CROSSING SHALL BE NOT MORE THAN ONE-THIRD (1/3) CONCRETE THICKNESS.
 - THREE OR MORE LAYERS CROSSING WILL NOT BE PERMITTED.
- SLEEVES THROUGH SLABS ARE NOT ALLOWED NEAR SUPPORTS WITHOUT PRIOR APPROVAL OF THE CONSULTANT. MINIMUM DISTANCE FROM FACE OF SUPPORT TO THE EDGE OF SLEEVE IS TWICE THE SLAB THICKNESS.

STRUCTURAL STEEL

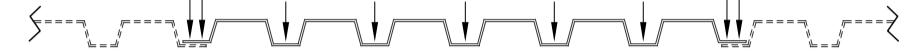
- DESIGN OF STRUCTURAL STEEL MEMBERS AND CONNECTIONS TO BE IN ACCORDANCE WITH CSA S16-14.
- SHOP DRAWINGS OF STRUCTURAL STEELWORK TO BE SUBMITTED FOR REVIEW PRIOR TO FABRICATION. PROVIDE LETTERS OF ASSURANCE AND SHOP DRAWINGS SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF BRITISH COLUMBIA FOR CONNECTIONS AND WELDING DESIGNS.
- WELDING TO CSA W59. FABRICATOR AND ERECTOR TO BE FULLY APPROVED BY THE CANADIAN WELDING BUREAU TO CSA W47.1. ALL WELDERS TO BE CWB CERTIFIED.
- STRUCTURAL WIDE-FLANGE MEMBERS TO CONFORM TO CSA G40.21-13 GRADE 350W.
- STRUCTURAL STEEL ANGLES, PLATES, WELDED WIDE FLANGES, AND CHANNELS TO CONFORM TO CSA G40.21-13, GRADE 350W.
- HOLLOW STRUCTURAL SECTIONS SHALL CONFORM TO CSA G40.21-13, GRADE 350W, CLASS C FOR SQUARE AND RECTANGULAR SECTIONS AND A-500, CLASS 'C' (Fy=307 MPa) FOR ROUND SECTIONS. DRY AND SEAL HOLLOW STRUCTURAL SECTIONS IN THE FABRICATION SHOP PRIOR TO SHIPPING TO THE SITE.
- UNLESS NOTED OTHERWISE ALL BOLTS TO BE HIGH TENSILE BOLTS AND SHALL CONFORM TO ASTM A325 AND ASTM A490. USE CORROSION INHIBITING PLATED BOLTS ON ALL PRE-PAINTED STEEL AND ARCHITECTURAL EXPOSED STEEL.
- ANCHOR BOLTS AND ORDINARY BOLTS TO CONFORM TO ASTM A307.
- ALL EXTERIOR EXPOSED BOLTS, NUTS, AND WASHERS TO BE HOT-DIPPED GALVANIZED, U.N.O.
- PROVIDE POSITIVE DEAD LOAD CAMBER TO ALL SIMPLE SPAN FLOOR BEAMS 6m IN LENGTH OR GREATER. CAMBER TO BE L/460 WHERE L IS THE MEMBER LENGTH. MOMENT-CONNECTED BEAMS AND ROOF BEAMS TO BE UNCAMBERED EXCEPT FOR MILL CAMBER, CAMBERED UP.
- MEMBERS MARKED WITH AN ASTERISK (*) SHALL BE MOMENT-CONNECTED AT THE END MARKED WITH AN ASTERISK. MOMENT CONNECTION SHALL BE FOR THE FULL MOMENT CAPACITY OF THE BEAM (BOTH POSITIVE AND NEGATIVE) U.N.O. PROVIDE ADDITIONAL STIFFENERS AND/OR WEB DOUBLER PLATES AND PROVIDE FULL PENETRATION WELDS AS REQUIRED.
- SPLICES SHALL BE AT THE LOCATIONS INDICATED ON THE DRAWINGS OR AS APPROVED BY THE DEPARTMENTAL REPRESENTATIVE.
- MEMBERS NOTED AS CONTINUOUS ON THE DRAWINGS SHALL HAVE FULL-STRENGTH WELD SPLICES EXCEPT WHERE A SPLICE FORCE IS NOTED ON THE DRAWINGS.
- COLUMN SPLICES SHALL BE DESIGNED FOR FULL STRENGTH.
- BUILT-UP MEMBERS SHALL BE FABRICATED IN THE SHOP PRIOR TO SHIPPING TO THE SITE.
- ALL EXPOSED STRUCTURAL STEEL IS TO BE OF ARCHITECTURAL QUALITY. CONNECTION DETAILS, LAYOUTS, WELDING, BOLTING PATTERNS, AND FIT-UP TO BE REVIEWED BY THE DEPARTMENTAL REPRESENTATIVE UPON COMPLETION OF SHOP DRAWINGS AND APPROVED UPON COMPLETION OF ERECTION.
- FOR SURFACE PREPARATION, PRIMING, AND PAINTING OF EXPOSED STEEL, SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS. PRIMER TO BE COMPATIBLE WITH SPECIFIED PAINT FINISH.
- SHOP INSPECT AND TEST ALL STUDS APPLIED TO BEAMS OR PLATES IN THE SHOP. FIELD INSPECT AND TEST ALL FIELD-APPLIED STUDS OR BOLT STUDS. STUDS IN COMBINATION WITH STEEL DECK SHALL BE INSTALLED IN THE FIELD. INSPECTION AND TESTING PROCEDURE SHALL CONFORM TO CSA W59.
- GROUT UNDER ALL BASE PLATES OF COLUMNS WITH A NON-SHRINK FLOWABLE, HIGH-STRENGTH GROUT. ENSURE FULL AREA IS GROUTED. PROVIDE 25mm U.N.O. ON THE DRAWINGS. MAXIMUM SHIM AND GROUT THICKNESS 50mm
- ALL 'STANDARD' AND 'TYPICAL DETAILS' SHOWN ON THE DRAWINGS APPLY TO ALL STEELWORK WHETHER SPECIFICALLY REFERENCED ON PLANS OR NOT.
- THE STEEL CONTRACTOR SHALL PROVIDE FRAMING TO SUPPORT ROOFTOP MECHANICAL EQUIPMENT OR MECHANICAL AND ELECTRICAL EQUIPMENT SUSPENDED FROM THE ROOF USING A MINIMUM L100x100x6 EACH SIDE OF THE EQUIPMENT SPANNING TO ADJACENT SUPPORTS OR AS DETAILED ON THE DRAWINGS.
- STEEL TESTING TO BE IN ACCORDANCE WITH THE CSA S16 AND THE SPECIFICATIONS.

STEEL DECK

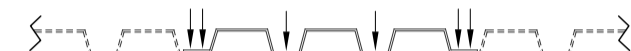
- METAL DECK SHALL BE FORMED FROM SHEET STEEL CONFORMING TO CSSBI 101-M (ZINC COATED STRUCTURAL QUALITY STEEL SHEET FOR ROOF AND FLOOR DECK). MINIMUM GRADE A, WITH A BASE STEEL THICKNESS AS NOTED AND A MINIMUM ZINC COATING DESIGNATION OF Z275 FOR ROOF DECK, AND ZF75 FOR FLOOR DECK.
- SUBMIT PROPOSED STEEL DECK PRODUCTS TO THE DEPARTMENTAL REPRESENTATIVE FOR APPROVAL PRIOR TO DELIVERY TO THE SITE.
- APPROVALS TO BE DELIVERED TO THE SITE IN GOOD CONDITION WITH THE MANUFACTURER'S LABELS AND SEALS INTACT. CARE SHOULD BE TAKEN IN STACKING AND STORING MATERIALS ON SITE AND DURING INSTALLATION. ALL DAMAGED SHEETS SHALL BE REPLACED.
- DECKING SHALL BE CONTINUOUS OVER AT LEAST THREE SPANS WHERE POSSIBLE.

SHOP DRAWINGS OF STEEL DECK TO BE SUBMITTED FOR REVIEW AND APPROVAL TO THE DEPARTMENTAL REPRESENTATIVE PRIOR TO INSTALLATION.

- FOR 40mm DECK FASTEN EACH FLUTE AT ALL SUPPORTS USING MECHANICAL CONNECTORS. USE 2 CONNECTORS AT SEAMS TO GIVE 36/9 FASTENING PATTERN - SEE DETAIL BELOW.



- FOR 75mm DECK FASTEN EACH FLUTE AT ALL SUPPORTS USING MECHANICAL CONNECTORS. USE 2 CONNECTORS AT SEAMS TO GIVE 32/7 OR 24/6 FASTENING PATTERN. - SEE DETAIL BELOW.

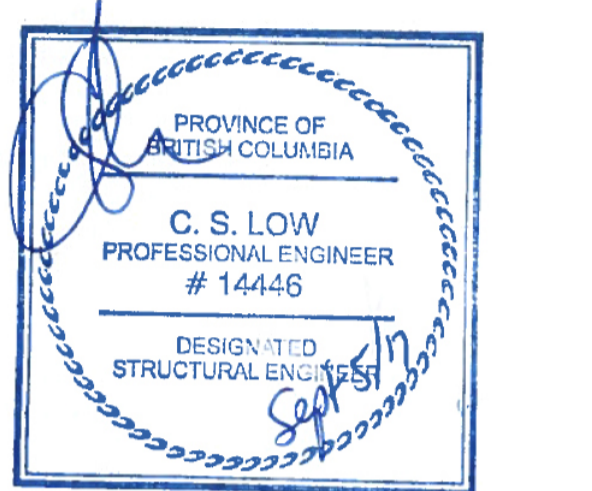


SUBMIT PROPOSED MECHANICAL STEEL DECK FASTENER PRODUCTS TO THE DEPARTMENTAL REPRESENTATIVE FOR REVIEW AND APPROVAL PRIOR TO DELIVERY TO THE SITE.

- PROVIDE MINIMUM 50mm LAP AT END LAPS. ADJUST SCREW THREAD PITCH AND DRILL POINT TO SUIT METAL DECK THICKNESS.
- FASTEN TO MEMBERS THAT ARE PARALLEL TO DECK SPAN AND INFIELD DRAG STRUTS WITH MECHANICAL CONNECTORS AS ABOVE AT 150mm O.C. U.N.O.
- FASTEN DECK TO SHEAR LUGS WITH MECHANICAL CONNECTORS SEE SHEAR LUG DETAIL.
- FASTEN DECK TO PERIMETER ANGLES PARALLEL TO DECK SPAN WITH MECHANICAL CONNECTORS AT 150mm O.C. USE FILLERS AS NECESSARY TO PROVIDE CONTACT BETWEEN DECK AND PERIMETER ANGLES.
- FASTEN SIDE LAPS WITH SCREWS AT 150mm O.C. AT SIDE LAPS. DECK PROFILE TO SUIT SIDE LAP SCREWING. PROVIDE MIN. 12mm EDGE DISTANCE PAINT EXPOSED TIPS OF SIDE LAP SCREWS TO MATCH DECK WHERE DECK IS EXPOSED INCLUDING ACOUSTIC DECK.
- ALL CONNECTORS INCLUDING SIDE LAP SCREW TO BE INSPECTED BY THE CONTRACTOR'S QUALITY CONTROL PERSONNEL AND SUBSTANDARD FASTENINGS REPLACED OR REDONE AT NO ADDITIONAL COST.
- TOUCH UP WELDS WITH ZINC-RICH PRIMER PAINT.
- DECK SUPPLIER SHALL PROVIDE CELL AND EDGE CLOSURES TO CONTAIN THE CONCRETE SLAB AS REQUIRED OF A SIMILAR MATERIAL AND COATING AS THE DECK. THE DECK SUPPLIER SHALL PROVIDE ALL FLASHING AND SUPPORT ANGLES REQUIRED TO CLOSE OFF THE SUPPORT DECK AT COLUMNS.
- WHERE BOLTED CONNECTIONS ARE USED BETWEEN VERTICAL ELEMENTS AND BEAM TOP FLANGES, THE DECK SHALL BE TRIMMED TO PREVENT INTERFERENCE WITH BOLTS. PROVIDE ANGLES OR PLATES AS REQUIRED TO MAINTAIN MINIMUM 75mm BEARING SURFACE FOR STEEL DECK.
- FLOOR AND ROOF OPENINGS:
 - CUT FLOOR DECK OPENINGS AFTER CONCRETE SLAB HAS BEEN PLACED AND HAS REACHED 28 DAY STRENGTH. OR TEMPORARILY SHORE THE DECK. REINFORCE OPENINGS IN FLOOR DECK IN ACCORDANCE WITH SPECIFICATIONS AND STRUCTURAL DETAILS.
 - REINFORCE OPENINGS IN ROOF DECK AS FOLLOWS:
 - FOR OPENINGS UP TO 150mm ACROSS THE FLUTE, NO REINFORCEMENT IS NECESSARY PROVIDED THAT NOT MORE THAN TWO VERTICAL WEBS ARE REMOVED.
 - FOR OPENINGS OVER 150mm TO 300mm ACROSS THE FLUTES, PROVIDE NOT LESS THAN A L50x50x6 ANGLE REINFORCEMENT TO FRAME ACROSS EACH SIDE OF THE OPENING PERPENDICULAR TO THE FLUTES. THE ANGLE SHALL BE WELDED BOTH SIDES TO AT LEAST TWO FLUTES ON EACH SIDE OF THE OPENING.
 - OPENINGS GREATER THAN 300mm SHALL BE REINFORCED WITH L100x100x6 EACH SIDE OF THE OPENING SPANNING TO ADJACENT STRUCTURAL MEMBERS OR AS DETAILED ON THE DRAWINGS.
- MAXIMUM CONSTRUCTION LOAD PRIOR TO PLACING AND STRENGTH GAIN OF CONCRETE SLABS TO BE IN ACCORDANCE TO DECK MANUFACTURER'S CRITERIA FOR THE SPANS INDICATED.

DRAWING LIST

DRAWING NUMBER	TITLE NAME
S-100	GENERAL NOTES SHEET 1
S-101	GENERAL NOTES SHEET 2
S-102	TYPICAL DETAILS SHEET 1
S-103	TYPICAL DETAILS SHEET 2
S-104	TYPICAL DETAILS SHEET 3
S-105	TYPICAL DETAILS SHEET 4
S-201	FOUNDATION AND CRAWL SPACE PLAN
S-202	MAIN FLOOR PLAN
S-203	LOWER ROOF PLAN AND SECURED CEILING PLAN
S-204	UPPER ROOF PLAN
S-300	SECTIONS AND DETAILS SHEET 1
S-301	SECTIONS AND DETAILS SHEET 2
S-310	SECTIONS AND DETAILS SHEET 3
S-320	SECTIONS AND DETAILS SHEET 4
S-400	BRACE BAY ELEVATION AND DETAILS



Revision/	Description/Description	Date/Date
5		
4		
3		
2		
1		
0	ISSUED FOR TENDER	17/09/01

Client/client

CORRECTIONAL SERVICE OF CANADA

Project title/Titre du projet
AGASSIZ, BRITISH COLUMBIA
4732 CEMETERY ROAD PO BOX 1600
MOUNTAIN INSTITUTE

MOUNTAIN INSTITUTE HEALTH CARE EXPANSION

Consultant Signature Only

Designed by/Concept par
CSL
Drawn by/Dessiné par
MSH
PSPC Project Manager/Administrateur de Projets SPAC
TONY TANG
Regional Manager, Architectural and Engineering Services
Gestionnaire régionale, Services d'architectural et de génie, SPAC
PREETIPAL PAUL

Drawing title/Titre du dessin

GENERAL NOTES SHEET 1

OPEN WEB STEEL JOISTS

- SHALL BE DESIGNED, FABRICATED AND BRIDGED AND CAMBERED IN ACCORDANCE WITH CSA S16-14 FOR LOADS INDICATED ON THE DRAWINGS, INCLUDING SNOW DRIFTING IN ACCORDANCE WITH THE BUILDING CODE, SUSPENDED EQUIPMENT AND PARTITION LOADS, MECHANICAL EQUIPMENT AND ROOF PADS.
- SHOP DRAWINGS OF OPEN WEB STEEL JOISTS TO BE SUBMITTED FOR REVIEW, INDICATING DESIGN LOADS, DIMENSIONS, MATERIAL PROPERTIES, MEMBER SIZES, BRIDGING REQUIREMENTS, AND ANY OTHER NECESSARY DESIGN DATA. PROVIDE LETTERS OF ASSURANCE AND SHOP DRAWINGS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF BRITISH COLUMBIA.
- OPEN WEB STEEL JOISTS TO BE GIVEN ONE COAT OF SHOP PRIMER PAINT.
- PROVIDE BOTTOM CHORD EXTENSIONS ON COLUMN LINES, OR AS NOTED ON THE DRAWINGS.
- DESIGN ALL OPEN WEB STEEL JOISTS AND BRIDGING DETAILS FOR A MINIMUM NET FACTORED UPLIFT OF 1.0 kPa.
- COORDINATE BRIDGING DETAILS WITH MECHANICAL DUCTS, ETC.
- ALLOW FOR 4.5 kN DEAD POINT LOAD ON TOP CHORD AND BOTTOM CHORD AT ANY LOCATION OF O.W.S.J.
- LIMIT SNOW LOAD DEFLECTION TO L/360. LIMIT DEAD LOAD + SNOW LOAD DEFLECTION TO L/240.
- ATTACHMENTS FOR MECHANICAL, ELECTRICAL, AND OTHER SERVICES SHALL BE MADE BY USING APPROVED CLAMPING DEVICES OR U-BOLT TYPE CONNECTORS, NO DRILLING WELDING OR CUTTING SHALL BE ALLOWED WITHOUT WRITTEN APPROVAL FROM THE JOIST DESIGNER.

NEW OPENING THRU EXIST. CONCRETE

- PRIOR TO CUTTING OR CORING ANY NEW OPENINGS THROUGH THE EXISTING CONCRETE STRUCTURE, THE PROPOSED LOCATION OF PENETRATIONS SHOULD BE REVIEWED AND APPROVED BY THE DEPARTMENTAL REPRESENTATIVE.
- PRIOR TO CUTTING, ALL LOCATIONS OF PROPOSED PENETRATIONS ARE TO BE SURVEYED BY THE USE OF GROUND PENETRATING RADAR TO DETERMINE THE PRESENCE OF EXISTING REINFORCEMENT OR SERVICES WITHIN THE CONCRETE.
- ALL OPENINGS THROUGH SLABS AND WALLS TO HAVE CORNERS OF OPENING CORE DRILLED FIRST. DO NOT OVERCUT WITH CONCRETE SAW.

REINFORCED MASONRY

- UNLESS NOTED OTHERWISE, MATERIALS SHALL CONFORM TO THE FOLLOWING STANDARDS:
 - CONCRETE BLOCK: H30/AM H BLOCK
 - MASONRY MORTAR: TYPE S, SITE MIXED BY PROPORTION SPECIFICATION OR PRE-MIXED BY PROPERTY SPECIFICATION.
 - MASONRY GROUT: 20 MPa AT 28 DAYS BY CYLINDER TEST UNDER PROPERTY SPECIFICATION. SLUMP 200mm TO 250mm.
- PROVIDE CERTIFIED BLOCKS WHERE FIRE RATINGS ARE REQUIRED.
- ALL EXTERIOR SECURE WALLS AND INTERIOR MASONRY WALLS SHALL BE H BLOCK FULLY GROUTED.
 - UNLESS NOTED OTHERWISE ON THE DRAWINGS, MINIMUM HORIZONTAL WALL REINFORCEMENT SHALL BE AS FOLLOWS:
 - 2-15M IN BOND BEAMS AT LOCATIONS SHOWN, AT ALL FLOOR LINES, ROOF LINES, AND TOP OF WALLS.
 - PROVIDE HORIZONTAL CORNER BARS AS PER TYPICAL WALL CORNER TIE DETAIL AT ALL CORNERS AND INTERSECTIONS.
 - PROVIDE 400mm DEEP LINTEL BEAMS REINFORCED WITH 2-15M BOTTOM OVER OPENINGS UP TO 2000mm WIDE. PROVIDE 600mm DEEP LINTEL BEAMS REINFORCED WITH 2-20M BOTTOM OVER OPENINGS GREATER THAN 2000mm, U.N.O. ON THE DRAWINGS. EXTEND BARS 900mm BEYOND OPENINGS OR HOOK END IF INSUFFICIENT LENGTH AVAILABLE.
 - UNLESS NOTED OTHERWISE ON THE DRAWINGS, MINIMUM VERTICAL WALL REINFORCEMENT SHALL BE AS FOLLOWS:
 - 1-15M VERTICAL AT 400mm O.C.
 - 1-15M VERTICAL AT ENDS, CORNERS AND INTERSECTION OF WALLS
 - 1-15M VERTICAL EACH SIDE OF DOOR AND WINDOW OPENINGS AND CONTROL JOINTS.
- REFER TO ARCHITECTURAL DRAWINGS FOR ADDITIONAL PERIMETER SECURITY DETAILS.
- PROVIDE STARTER DOWELS FROM SLABS AND FOOTINGS TO MATCH VERTICAL WALL REINFORCEMENT. TIE ALL VERTICAL BARS TO DOWELS OR PROVIDE METHODS TO ENSURE ACCURATE PLACEMENT ACCEPTABLE TO THE DEPARTMENT REPRESENTATIVE.
- POUR HEIGHT NOT TO EXCEED 4m. POURS OF 3m OR LESS MAY BE PLACED IN ONE LIFT.
- GROUT POUR HEIGHT NOT TO EXCEED 1.2m.
- NOTIFY THE DEPARTMENTAL REPRESENTATIVE A MINIMUM OF 24 HOURS PRIOR TO GROUT POURS.
- PROVIDE CONTROL JOINTS AT A MAXIMUM SPACING OF 7.5m FROM CORNER LOCATIONS TO BE REVIEWED WITH DEPARTMENTAL REPRESENTATIVE. MORTAR JOINTS SHALL BE RAKED BACK, READY FOR CAULKING. BOND BEAM REINFORCEMENT SHALL PROJECT FROM ONE SIDE 300mm THROUGH JOINT INTO TIGHT-FITTING PLASTIC TUBES. JOINT REINFORCEMENT SHALL BE TERMINATED, AT THE CONTROL JOINT.
- MASONRY WALLS TO BE LAID IN RUNNING BOND, TO TOLERANCES FOR LINE, PLUMBNESS, LEVEL AND JOINTS AS PER CSA A371. NOMINAL JOINT WIDTH IS 10mm.
- EXPOSED MORTAR JOINTS SHALL BE TOOLED AS SPECIFIED IN THE SPECIFICATIONS. CONCEALED JOINTS MAY BE STRUCK FLUSH.
- PROVIDE LATERAL TOP SUPPORT TO NON-LOAD BEARING WALLS AS PER DETAILS ON DRAWINGS. LOCATE BOND BEAM IN SECOND COURSE FROM THE TOP AS SHOWN ON THE DRAWINGS.
- ALL MASONRY MATERIALS SHALL BE PROTECTED FROM THE WEATHER (RAIN, SNOW, COLD AND HOT TEMPERATURE) UNTIL BUILDING IS ENCLOSED IN ACCORDANCE WITH CSA A371 REQUIREMENTS.
- PROVIDE TEMPORARY LATERAL SUPPORT TO WALLS IN ACCORDANCE WITH THE LATEST 'WORKSAFE' OCCUPATIONAL HEALTH AND SAFETY REGULATIONS.
- WALLS TO BE TESTED FOR VOIDS AFTER GROUTING. WALLS THAT ARE NOT FULLY GROUTED TO BE REMOVED AND RE-DONE.

SECONDARY STRUCTURAL AND NON-STRUCTURAL COMPONENTS

- SECONDARY STRUCTURAL AND NON-STRUCTURAL COMPONENTS INCLUDE, BUT ARE NOT LIMITED TO :
 - WINDOWS, SKYLIGHTS, GLASSBLOCK, AND CLADDING;
 - EXTERIOR AND INTERIOR STEEL STUD WALLS, BULKHEADS, AND CEILINGS;
 - HANDRAILS, GUARDRAILS, AND BALCONY RAILINGS;
 - BRICK OR MASONRY VENEERS AND THEIR SERVICES;
 - MECHANICAL AND ELECTRICAL EQUIPMENT, WINDOW WASHING COMPONENTS, & THEIR CONNECTIONS.
- SECONDARY STRUCTURAL AND NON-STRUCTURAL COMPONENTS INCLUDING THEIR CONNECTIONS TO THE BASE BUILDING SHALL BE DESIGNED AND REVIEWED IN THE FIELD BY A SPECIALTY STRUCTURAL ENGINEER REGISTERED IN BRITISH COLUMBIA.
- THE SPECIALTY STRUCTURAL ENGINEER SHALL BE EMPLOYED BY THE CONTRACTOR OR THE SUPPLIER OF THE COMPONENT, AND SHALL PROVIDE SEALED DRAWINGS, FIELD REVIEW, AND LETTERS OF ASSURANCE STATING THE WORK HAS BEEN DESIGNED TO THE APPLICABLE CODES, AND HAS BEEN INSTALLED IN ACCORDANCE WITH THE DESIGN.
- SEALED SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW PRIOR TO COMMENCING THE WORK. THE DRAWINGS MUST SHOW ALL DESIGN LOADS, MEMBER SIZES, MOVEMENT DETAILS, AND CONNECTION DETAILS.

POST-INSTALLED CONCRETE ANCHORS

- PROVIDE DRILLED IN ANCHORS AT LOCATIONS INDICATED ON STRUCTURAL DRAWINGS TO DEPTHS OF EMBEDMENT INDICATED.
- FASTENING SYSTEMS PROPOSED BY THE CONTRACTOR SHALL BE SUBMITTED TO THE DEPARTMENTAL REPRESENTATIVE FOR REVIEW AND APPROVAL. ALL PROPOSED SYSTEMS MUST BE APPROVED IN WRITING BY THE DEPARTMENTAL REPRESENTATIVE PRIOR TO USE.
- FASTENING SYSTEMS SHALL BE CAPABLE OF ACHIEVING THE PERFORMANCE REQUIREMENTS AS INDICATED ON THE DRAWINGS, AND SPECIFICATIONS.
- CONTRACTOR TO SUBMIT MANUFACTURER PUBLISHED DATA, INCLUDING BUT NOT LIMITED TO:
 - PRODUCT SPECIFICATIONS WITH RECOMMENDED DESIGN VALUES AND PHYSICAL CHARACTERISTICS FOR EPOXY DOWELS, EXPANSION AND UNDERCUT ANCHORS;
 - SAMPLES REPRESENTATIVE OF THE LENGTH AND DIAMETERS OF EACH TYPE OF ANCHOR SHOWN ON THE DRAWINGS;
 - CERTIFIED TEST REPORTS SHOWING COMPLIANCE WITH SPECIFIED PERFORMANCE CHARACTERISTICS AND PHYSICAL PROPERTIES;
 - MANUFACTURER'S INSTALLATION INSTRUCTIONS;
 - INSTALLER QUALIFICATIONS AND PROCEDURES.
- INSTALL ANCHORS PER THE MANUFACTURER'S INSTRUCTIONS, AS INCLUDED IN THE ANCHOR PACKAGING.
- INJECTION OF ADHESIVE SHALL BE PERFORMED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS ACCOMPANYING PRODUCT TO PRODUCE AN AIR-VOID FREE INJECTION.
- DRILLING SHALL BE PERFORMED WITH A ROTARY HAMMER DRILL AND CARBIDE TIPPED DRILL BIT IN ACCORDANCE WITH INSTRUCTIONS ACCOMPANYING ADHESIVE CARTRIDGES. ALTERNATE DRILLING METHODS, SUCH AS DIAMOND CORING, MUST BE APPROVED BY DEPARTMENTAL REPRESENTATIVE. AND ANCHOR MANUFACTURER.
- SPECIAL CONDITIONS SUCH AS WATER SATURATED CONCRETE, WATER-FILLED HOLES, UNDERWATER AND OVERHEAD INSTALLATIONS MUST BE APPROVED BY THE DEPARTMENTAL REPRESENTATIVE. AND ANCHOR MANUFACTURER.
- OVERHEAD ANCHORS MUST BE INSTALLED USING COMPATIBLE ACCESSORIES FROM THE MANUFACTURER TO ENSURE CORRECT ADHESIVE INJECTION.
- PRIOR TO CONSTRUCTION, TRAIN ALL PERSONNEL INVOLVED IN INSTALLATION ON-SITE BY ANCHOR MANUFACTURER'S REPRESENTATIVE. ANCHORS SHALL BE INSTALLED IN STRICT ACCORDANCE TO MANUFACTURER'S SPECIFICATIONS. PROCEDURES SHALL INCLUDE INSTALLATION OF ADHESIVE RODS OR REINFORCING IN DOWNWARD, HORIZONTAL AND OVERHEAD CONFIGURATIONS. INSTALLERS TO PROVIDE PROOF OF TRAINING UPON REQUEST.
- DEPARTMENTAL REPRESENTATIVE TO BE NOTIFIED FOR SITE REVIEW OF DRILLED ANCHORS DURING INSTALLATION PROCESS.
- INTENTIONALLY ROUGHEN THE INTERFACE BETWEEN POURS TO 6mm AMPLITUDE.

DEMOLITION OF EXISTING STRUCTURE

- THE STRUCTURE OF THE EXISTING BUILDING MAY HAVE BEEN MODIFIED SINCE THE ORIGINAL CONSTRUCTION, AND WHILE THE EXISTING STRUCTURE SHOWN ON THESE DRAWINGS IS BASED ON PAST RECORD DRAWINGS AND WHERE POSSIBLE SITE OBSERVATIONS, VARIATIONS IN ACTUAL STRUCTURE IN THE FIELD ARE TO BE EXPECTED. THE DEPARTMENT REPRESENTATIVE TAKES NO RESPONSIBILITY FOR THE ACCURACY OF PAST RECORD DRAWINGS SHOWING THE BUILDING STRUCTURE THAT IS TO BE DEMOLISHED OR MODIFIED. THE CONTRACTOR HAS A RESPONSIBILITY TO FAMILIARIZE THEMSELVES WITH THE EXISTING STRUCTURE PRIOR TO PERFORMING THE DEMOLITION OR OTHER WORK SHOWN ON THESE DRAWINGS. REPORT ANY VARIATIONS IN THE EXISTING STRUCTURE TO THE DEPARTMENTAL REPRESENTATIVE PRIOR TO PROCEEDING WITH THE WORK IN THE AFFECTED AREA. THESE DRAWINGS SHOW STRUCTURAL WORK ONLY. REFER TO OTHER DRAWINGS AND REPORTS FOR HAZARDOUS MATERIALS AND WORK OF OTHER DISCIPLINES.
- THESE DRAWINGS SHOW A PARTIAL DEMOLITION/MODIFICATION TO AN EXISTING BUILDING, AS WELL AS NEW WORK REQUIRED TO SUPPORT PORTIONS OF NEW AND EXISTING STRUCTURE. IN SOME CASES, NEW WORK IS REQUIRED TO BE INSTALLED BEFORE A PORTION OF THE EXISTING BUILDING IS DEMOLISHED. THE CONTRACTOR IS RESPONSIBLE FOR SEQUENCING, SHORING AND INSTALLATION OF NEW STRUCTURE REQUIRED TO SAFELY REMOVE/MODIFY THE PORTIONS SHOWN. DEMOLITION AND CONSTRUCTION METHODS ARE THE RESPONSIBILITY OF THE CONTRACTOR. WHERE SHORING IS REQUIRED, THE CONTRACTOR IS TO RETAIN A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF BRITISH COLUMBIA AND FAMILIAR WITH THE DESIGN OF SHORING. PORTIONS OF THE EXISTING BUILDING ARE IN CLOSE PROXIMITY TO THE CONSTRUCTION SITE AND WILL BE OCCUPIED DURING THE CONSTRUCTION PERIOD. THE CONTRACTOR SHALL CONDUCT WORK IN A WAY THAT DOES NOT REDUCE THE LOAD CARRYING CAPACITY OF OCCUPIED PORTIONS OF THE BUILDING OR ENDANGER THOSE USING THE OCCUPIED PORTIONS OF THE BUILDING. SITE SAFETY IS THE RESPONSIBILITY OF THE CONTRACTOR.

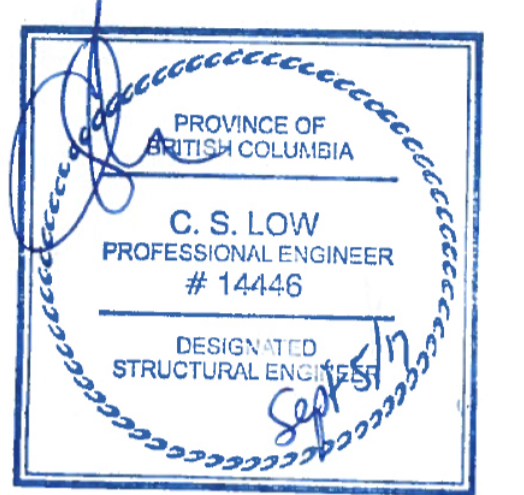
MINIMUM REINFORCEMENT SPLICE LENGTHS					UNLESS NOTED OTHERWISE	
BAR SIZE	VERT. OR BOT. REINFORCING	HORIZ. OR TOP REINFORCING	REINFORCING WITHIN WITHIN TIED ZONES	MASONRY WALL REINFORCING		
10M	450	550	500			500
15M	600	800	700			700
20M	750	950	850			1100
25M	1200	1550	1400			1750

MINIMUM WALL REINFORCEMENT			UNLESS NOTED OTHERWISE	
WALL SIZE	VERTICAL REINF.		HORIZONTAL REINF.	
150 WALL	15M@400		10M@150	
200 WALL	15M@400		15M@300	
250 WALL	15M@400 E.F.		15M@400 E.F.	

CONCRETE COVER (mm)	CONCRETE COVER TO BE MEASURED FROM INSIDE FACE OF REVEALS.			
	ELEMENT	FIRE RATING		
		1 HR.	2 HR.	3 HR.
SURFACES CAST AGAINST GROUND		75	75	75
FORMED SURFACES OF BEAMS, COLUMNS AND PILASTERS (TO PRINCIPAL REINFORCEMENT)		40	40	50
FORMED SURFACES OF BEAMS, COLUMNS AND PILASTERS EXPOSED TO GROUND OR WEATHER (TO PRINCIPAL REINFORCEMENT)		50	50	50
WALLS, SLABS AND SLABBANDS -20mm BARS AND SMALLER -25mm BARS		20 25	25 25	32 32
FORMED SURFACES OF WALLS EXPOSED TO GROUND OR WEATHER -20mm BARS AND SMALLER -25mm BARS AND LARGER		30 50	30 50	32 50

CONCRETE PROPERTIES							UNLESS NOTED OTHERWISE	
ELEMENT	28 DAY STRENGTH MIN. MPa	EXPOSURE CLASS	AIR CONTENT	MAX. AGGREGATE (mm)	SLUMP (mm)	CEMENT REDUCTION		
FOUNDATIONS	25 *	N	1 to 3%	20	80± 20	40%	**	**
FOUNDATION WALLS	25	F2	4 to 7%	20	80± 20	25%		
INTERIOR SLAB ON GRADE	25	N	1 to 3%	20	80± 20	15%		
EXTERIOR EQUIPMENT PADS	35	C2	5 to 8%	20	80± 20	25%		
INTERIOR EQUIPMENT PADS	25	N	1 to 3%	20	80± 20	15%		
EXTERIOR WALLS, PEDESTALS, AND COLUMNS	35	F2	4 to 7%	20	80± 20	25%		
MASONRY FILL	20	-	1 to 3%	10	200 - 250	25%		
CONCRETE TOPPINGS AND SLABS ON METAL DECK	25	N	1 to 4%	14	80± 20	**	**	**

- * 25 Mpa AT 56 DAYS - HIGH FLYASH CONC.
- ** SLUMP TO BE 50mm PRIOR TO ADDITION OF SUPER PLASTICIZER
- *** CEMENT REDUCTION IS THE REDUCTION OF CEMENT CONTENT BY REPLACEMENT OF FLY ASH OR EQUIVALENT COMPARED TO A LEED BASE DESIGN MIX WITH NO FLY ASH.



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AGASSIZ, BRITISH COLUMBIA
4732 CEMETERY ROAD PO BOX 1600
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HEALTH CARE EXPANSION

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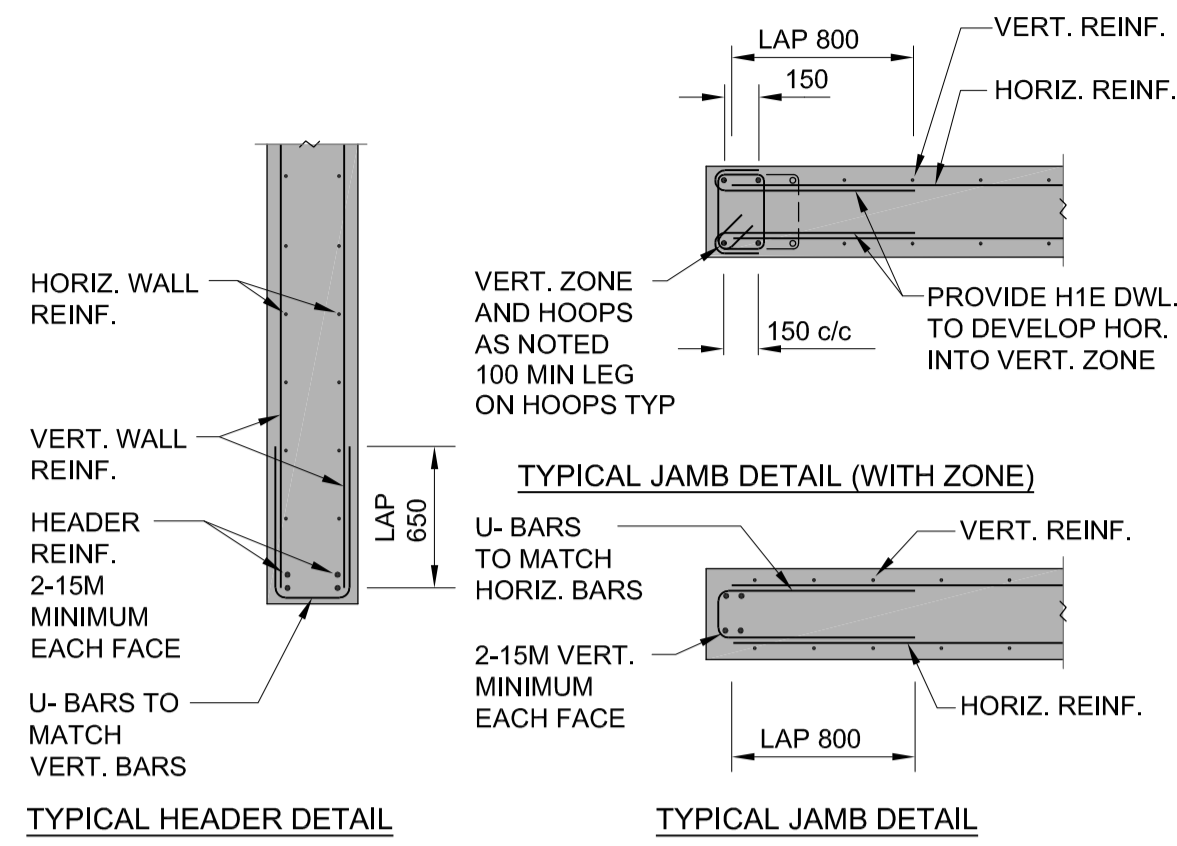
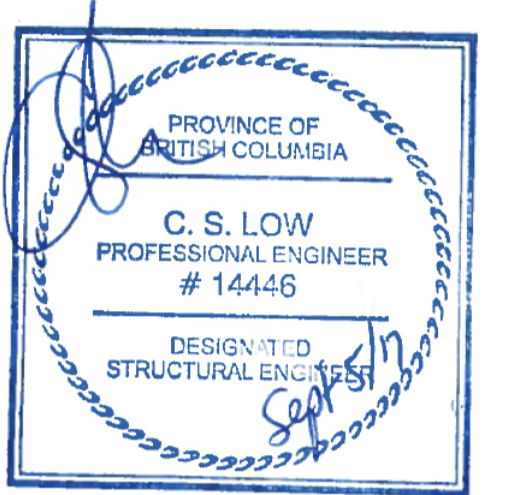
Designed by/Concept par
CSL
Drawn by/Dessine par
MSH
PSPC Project Manager/Administrateur de Projets SPAC
TONY TANG

Regional Manager, Architectural and Engineering Services
Gestionnaire régionale, Services d'architectural et de génie, SPAC
PREETIPAL PAUL

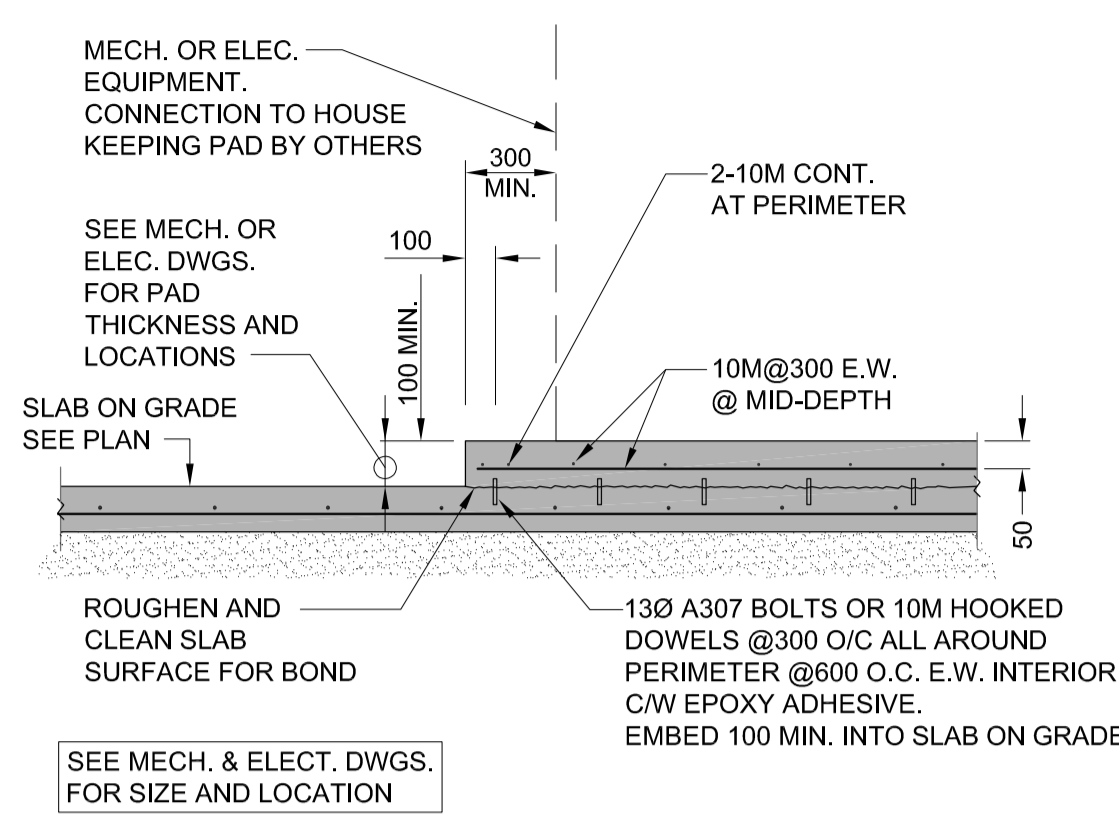
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GENERAL NOTES SHEET 2

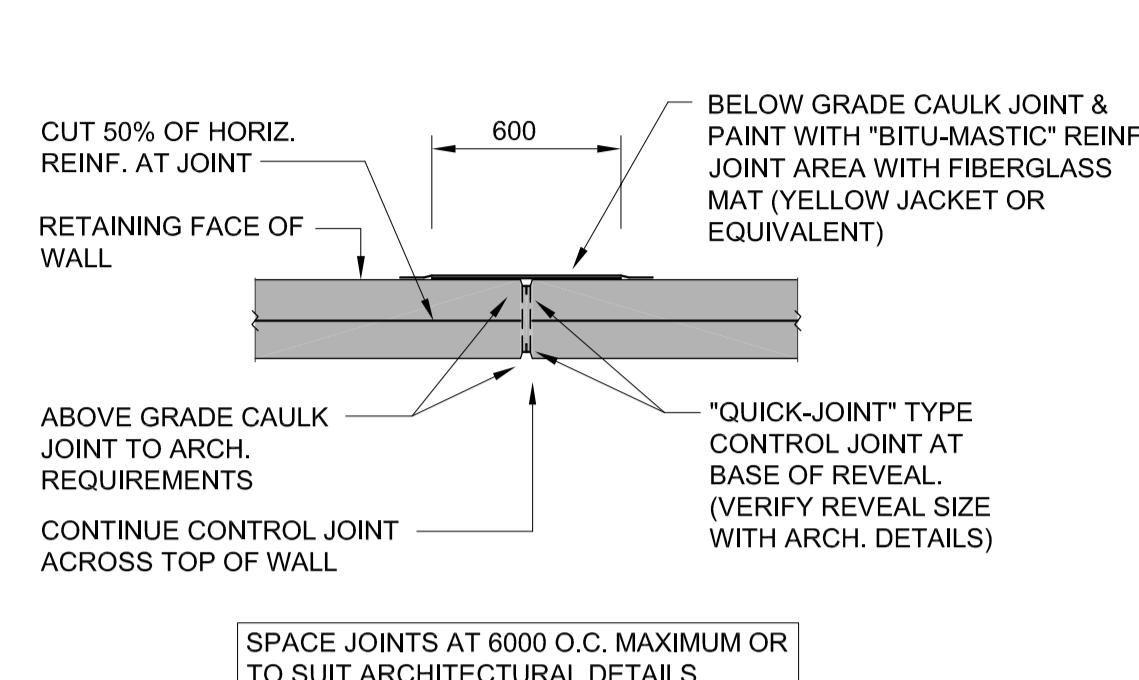
Project No./No. du projet R.077724.001	Sheet/Feuille S101 OF XX	Revision no./La Révision no. 0
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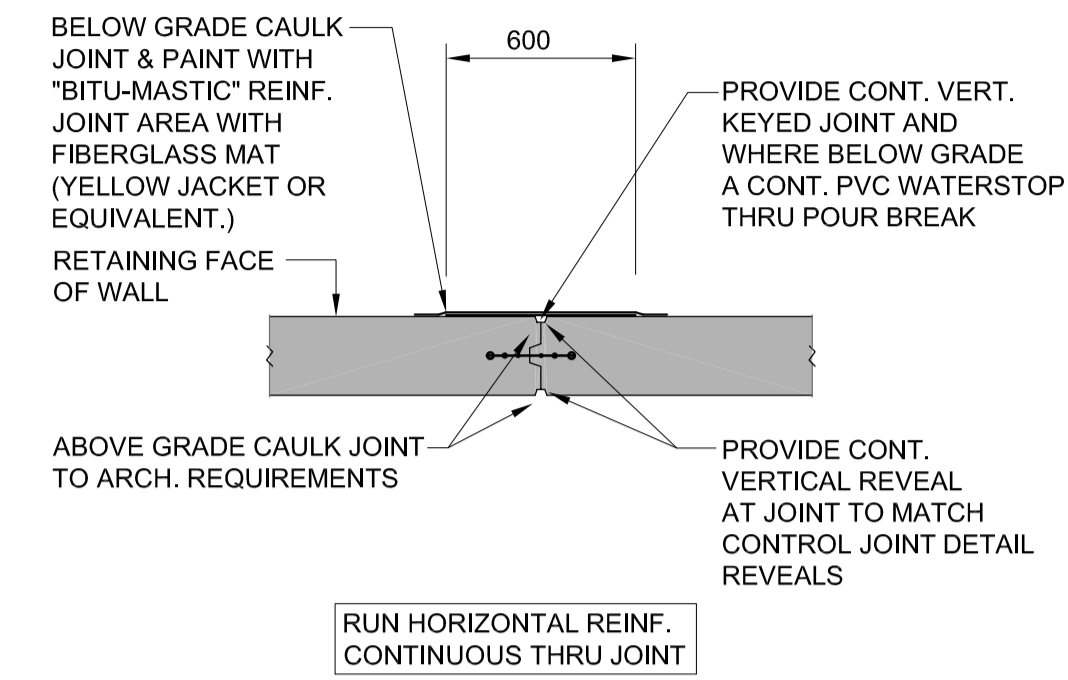
1 TYPICAL WALL OPENING JAMB AND HEADER DETAILS NTS



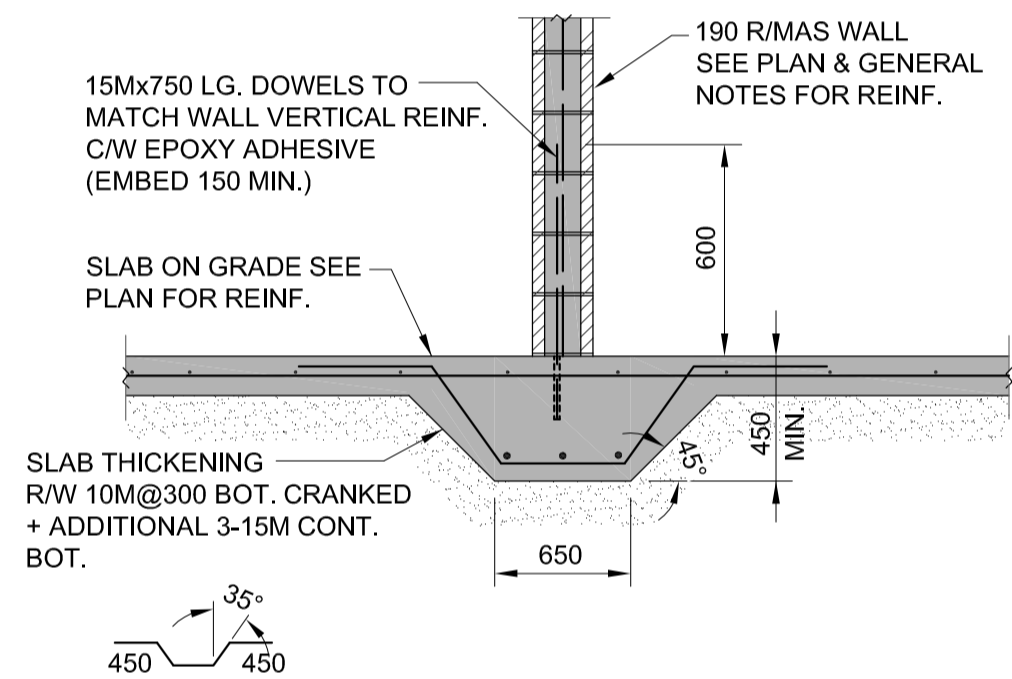
2 TYPICAL HOUSE KEEPING PAD AT SLAB ON GRADE NTS



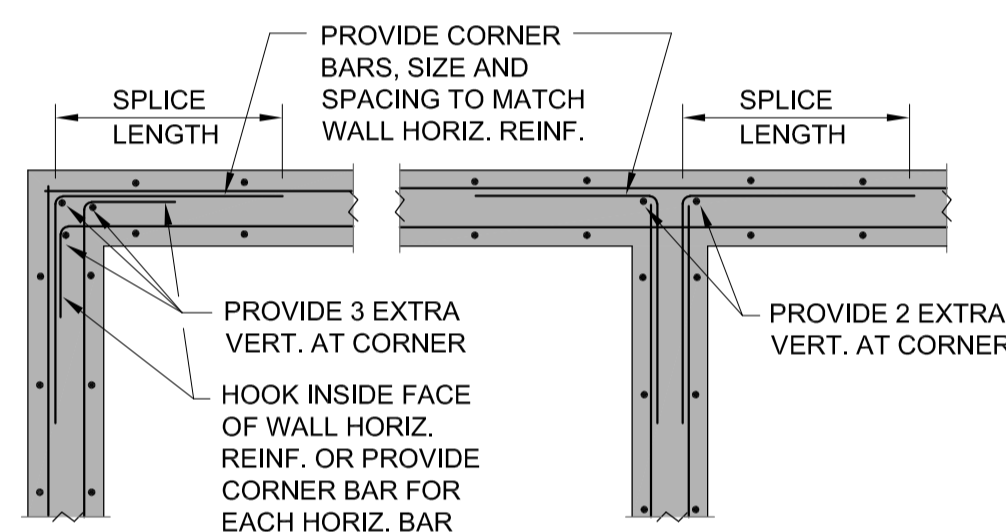
3 FOUNDATION WALL CONTROL JOINT BELOW GRADE SPACE NTS



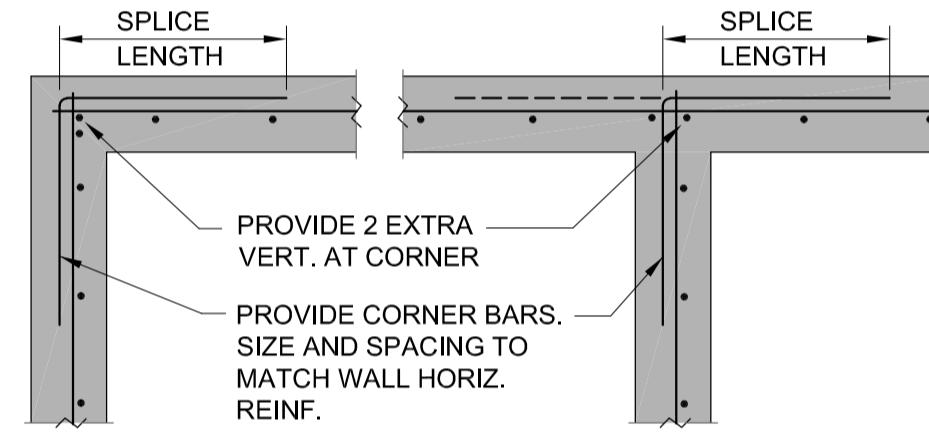
4 FOUNDATION WALL CONSTRUCTION JOINT BELOW GRADE OCCUPIED SPACE NTS



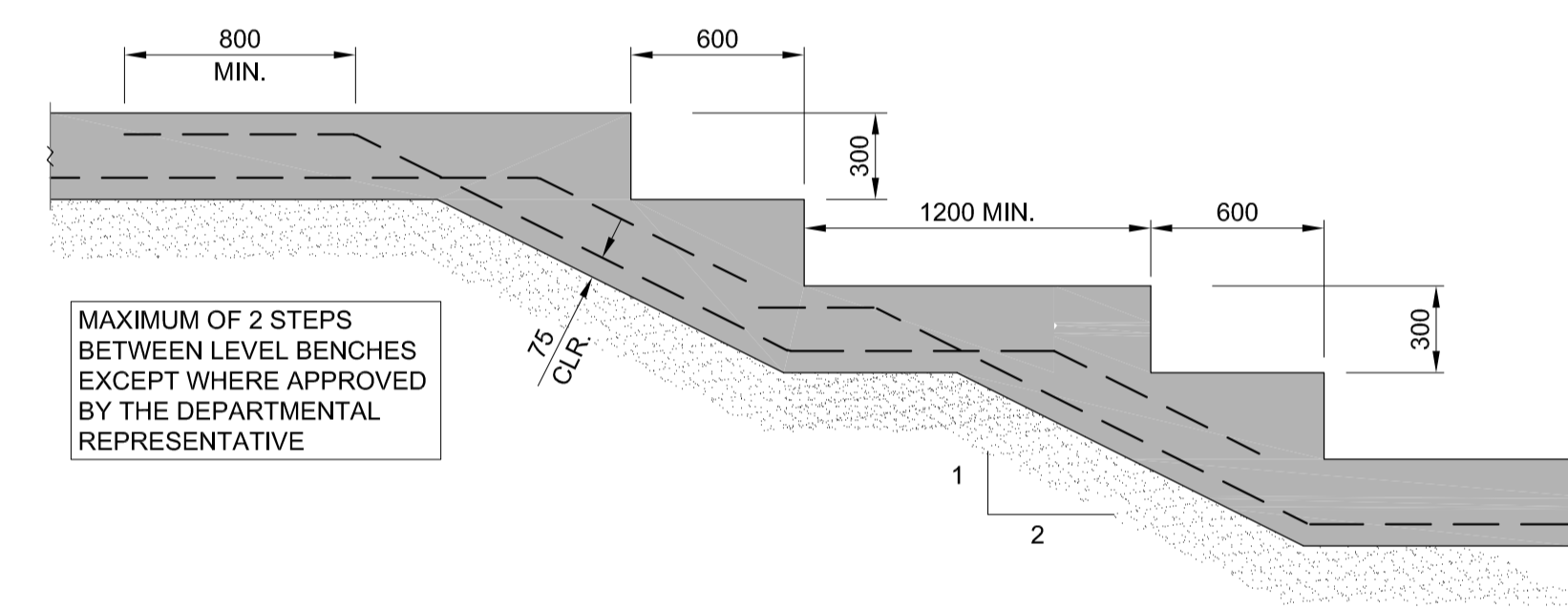
5 TYPICAL INTERIOR MASONRY WALL SLAB THICKENING NTS



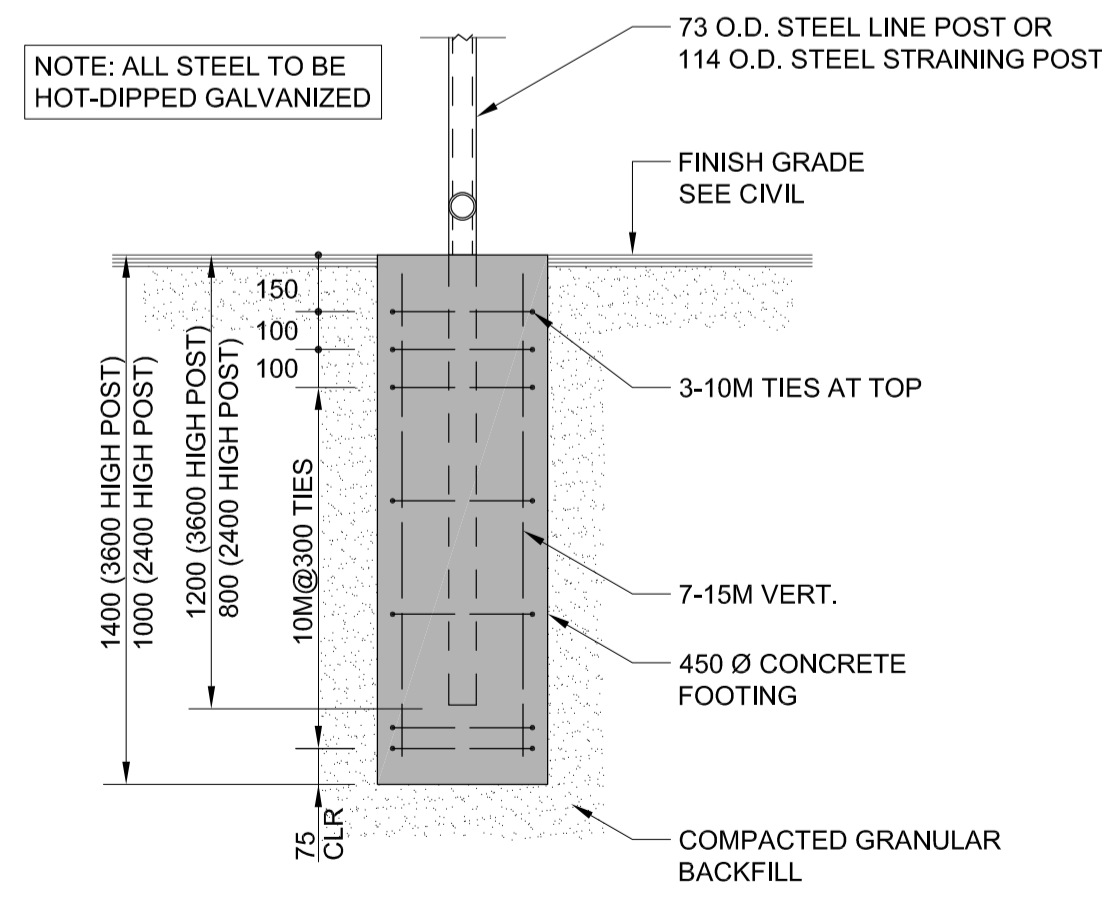
6 TYPICAL WALL CORNER TIE DETAILS FOOTING CORNER TIE DETAIL NTS



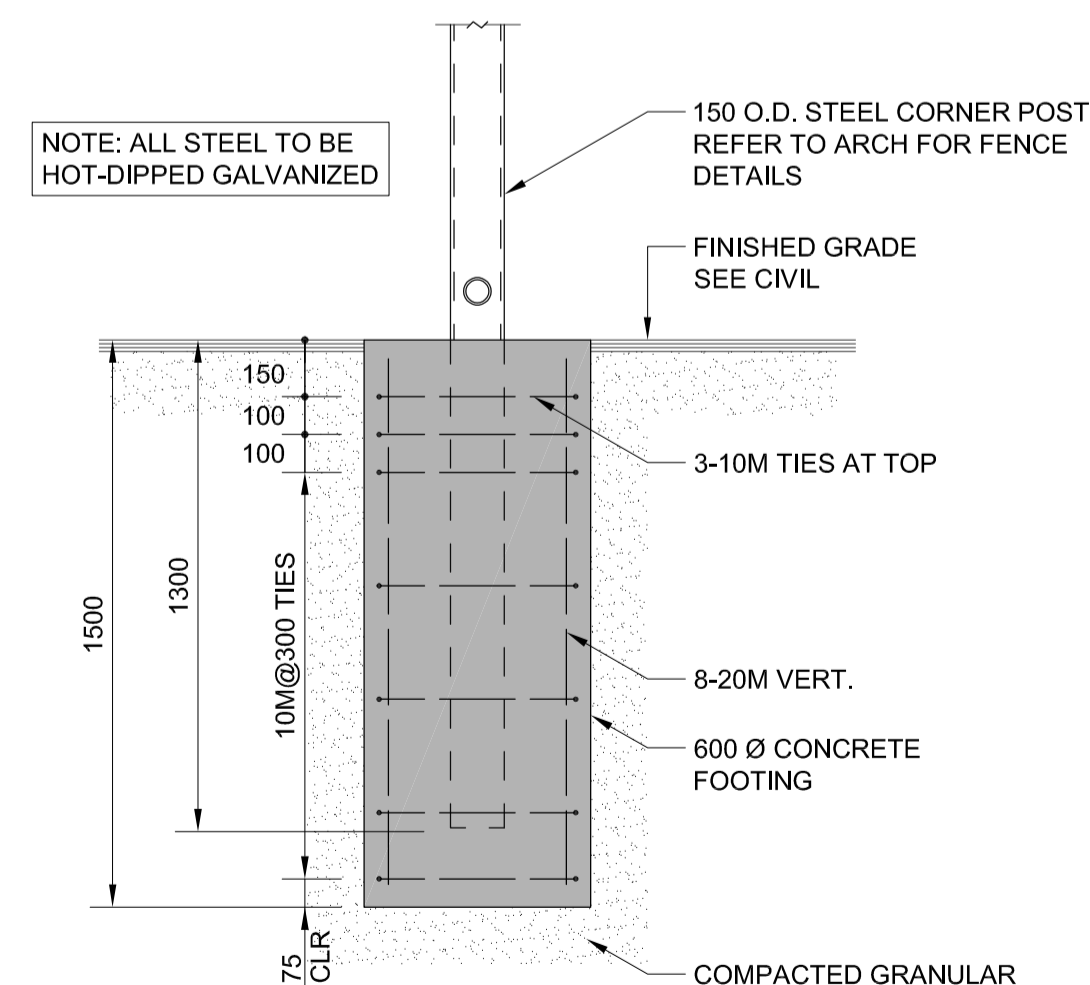
SINGLE LAYER OF WALL REINFORCEMENT



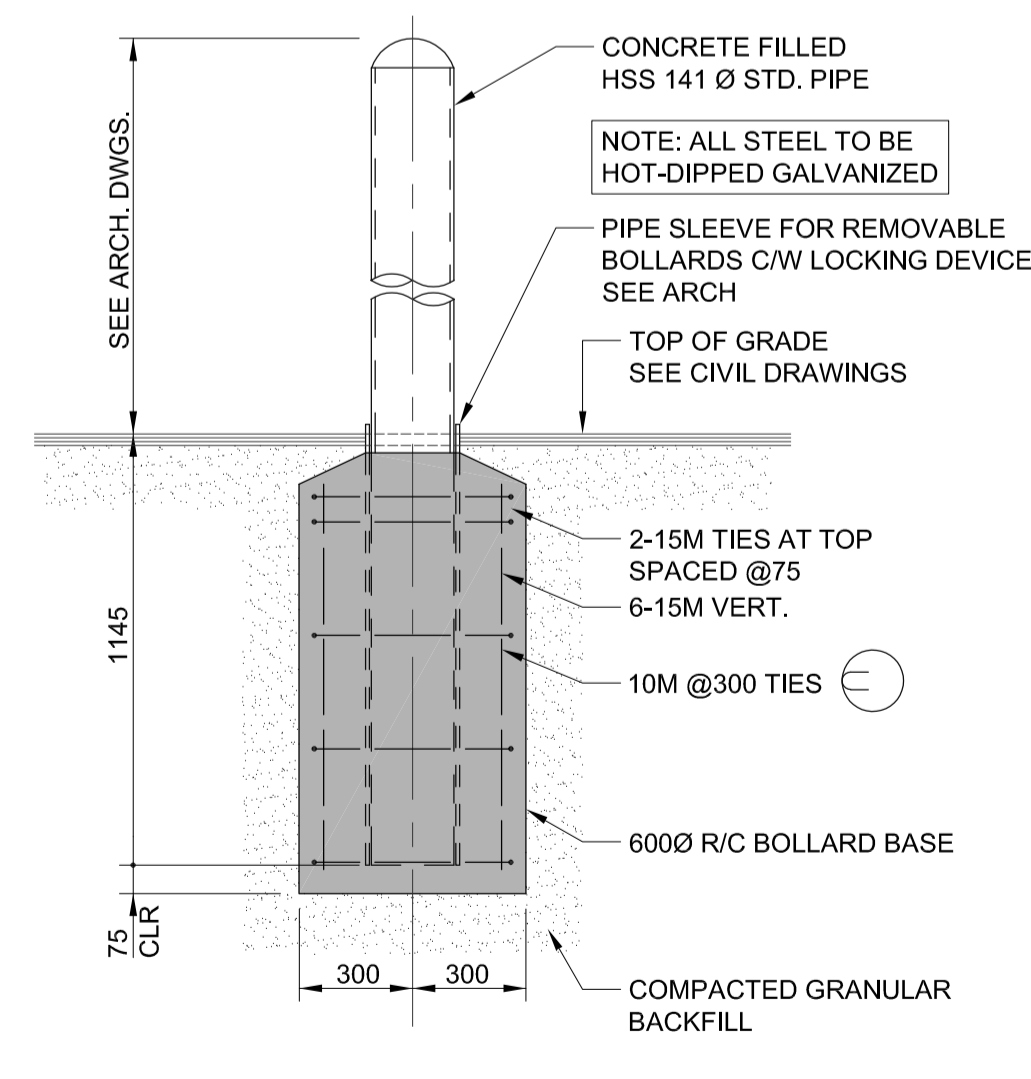
7 TYPICAL WALL FOOTING STEP DETAIL NTS



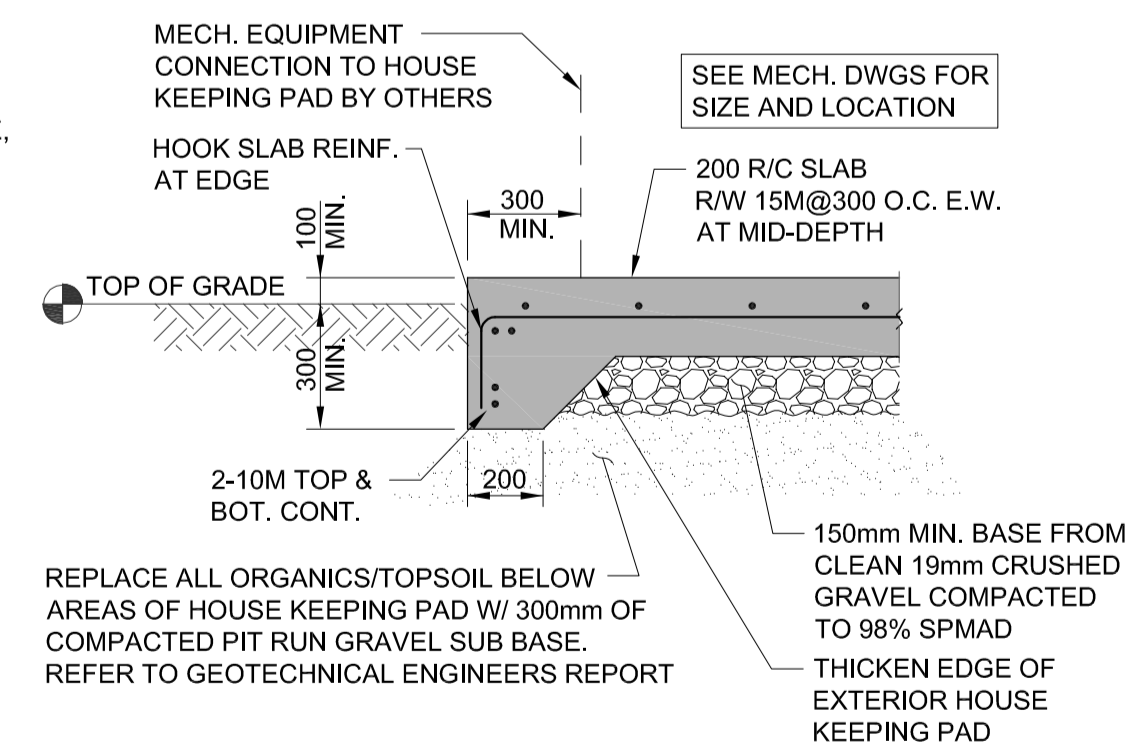
8 73mm & 114mm O.D. POST FENCE FOOTING DETAIL (SEE ARCH. FOR LOC'N.) NTS



9 150mm O.D. POST FENCE FOOTING DETAIL (SEE ARCH. FOR LOC'N.) NTS



10 TYPICAL BOLLARD DETAIL (SEE ARCH. FOR LOC'N.) NTS



11 TYPICAL EXTERIOR HOUSE KEEPING PAD NTS

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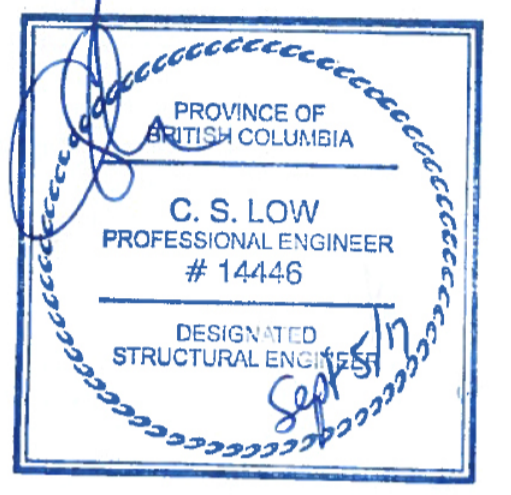
CORRECTIONAL SERVICE OF CANADA

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AGASSIZ, BRITISH COLUMBIA
 4732 CEMETERY ROAD PO BOX 1600
 MOUNTAIN INSTITUTE

MOUNTAIN INSTITUTE
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Consultant Signature Only
 Designed by/Concept par
 CSL
 Drawn by/Dessine par
 MSH
 PSPC Project Manager/Administrateur de Projets SPAC
 TONY TANG
 Regional Manager, Architectural and Engineering Services
 Gestionnaire régionale, Services d'architectural et de génie, SPAC
 PREETIPAL PAUL
 Drawing title/Titre du dessin
TYPICAL DETAILS SHEET 1

Project No./No. du projet	Sheet/Feuille	Revision no./La Révision no.
R.077724.001	S102 OF XX	0



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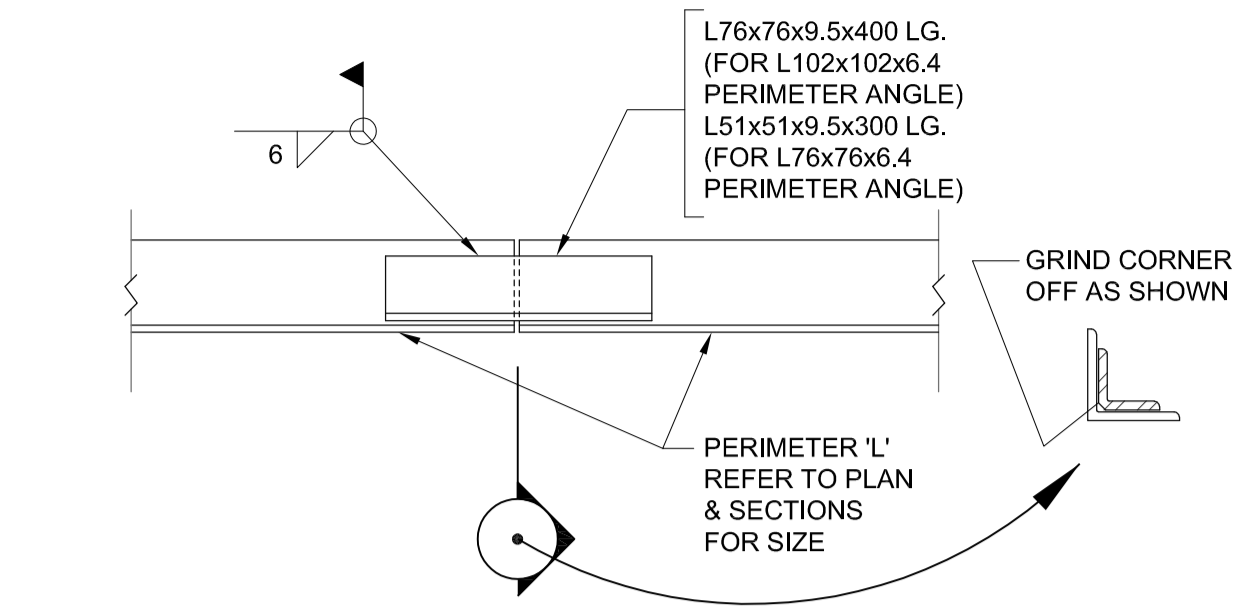
PSPC Project Manager/Administrateur de Projets SPAC
TONY TANG

Regional Manager, Architectural and Engineering Services
Gestionnaire régionale, Services d'architectural et de génie, SPAC
PREETPAL PAUL

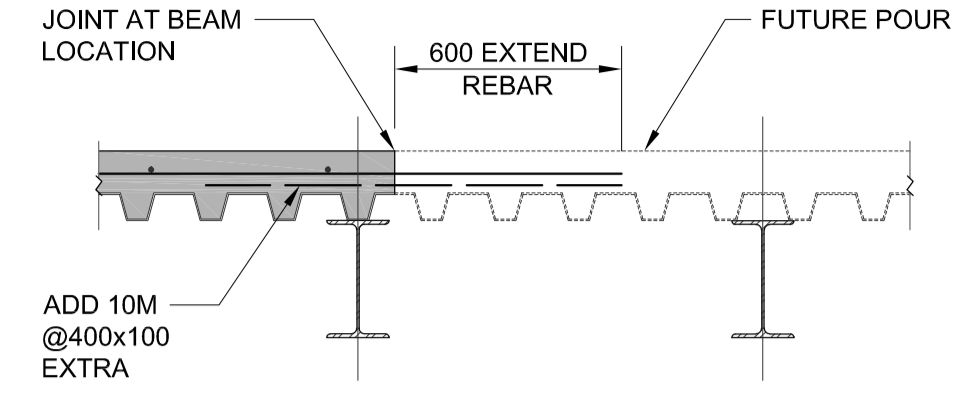
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TYPICAL DETAILS SHEET 2

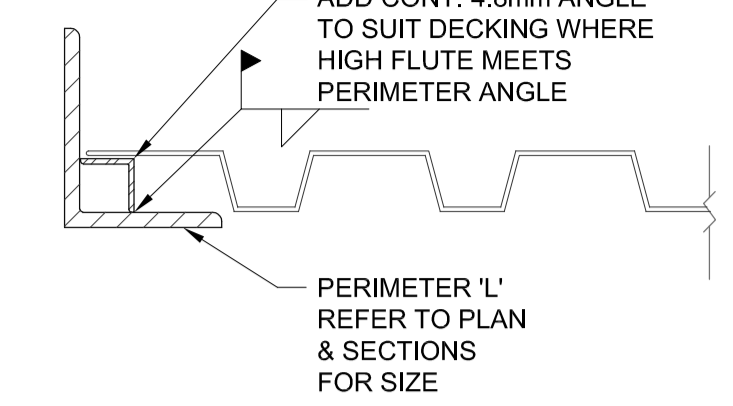
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R.077724.001	S103 OF XX	0



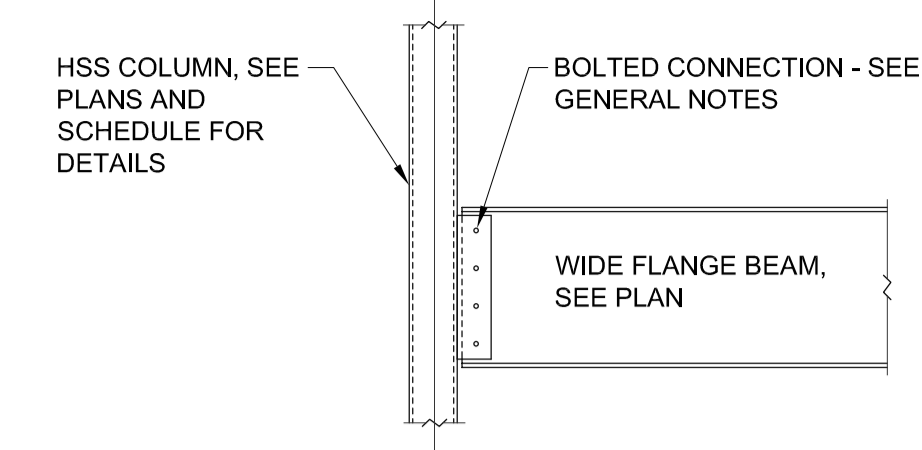
1 TYPICAL PERIMETER ANGLE SPLICE NTS



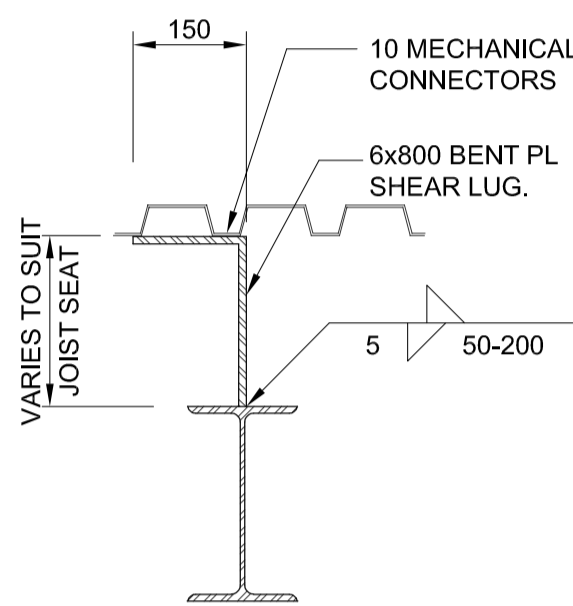
2 TYPICAL POUR JOINT IN DECK & TOPPING NTS



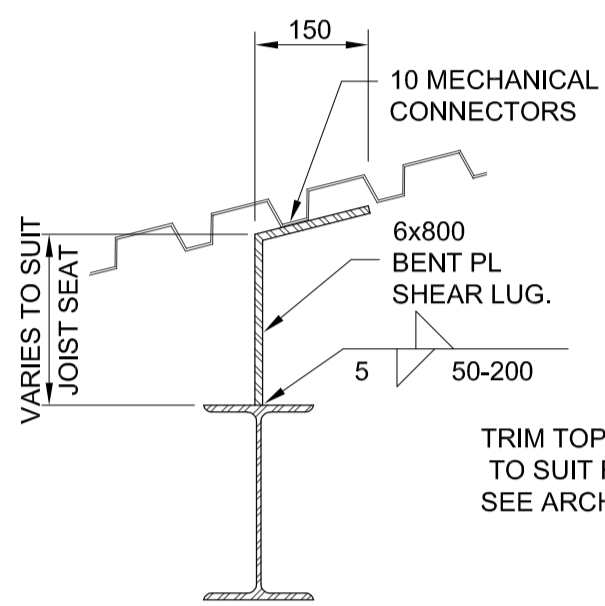
3 HIGH RIB CLOSURE DETAIL NTS



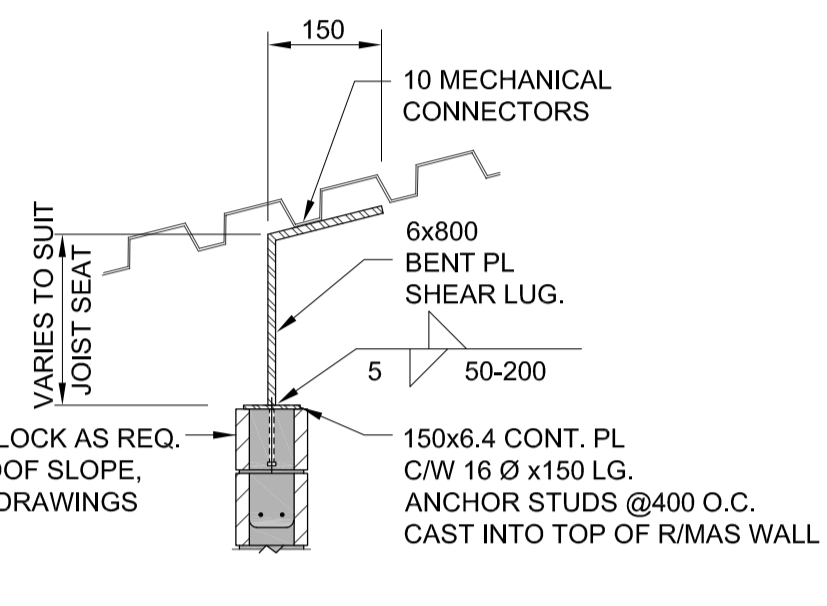
4 TYPICAL HSS COLUMN TO BEAM CONNECTION DETAIL NTS



TYPICAL SHEAR LUG AT FLAT ROOFS



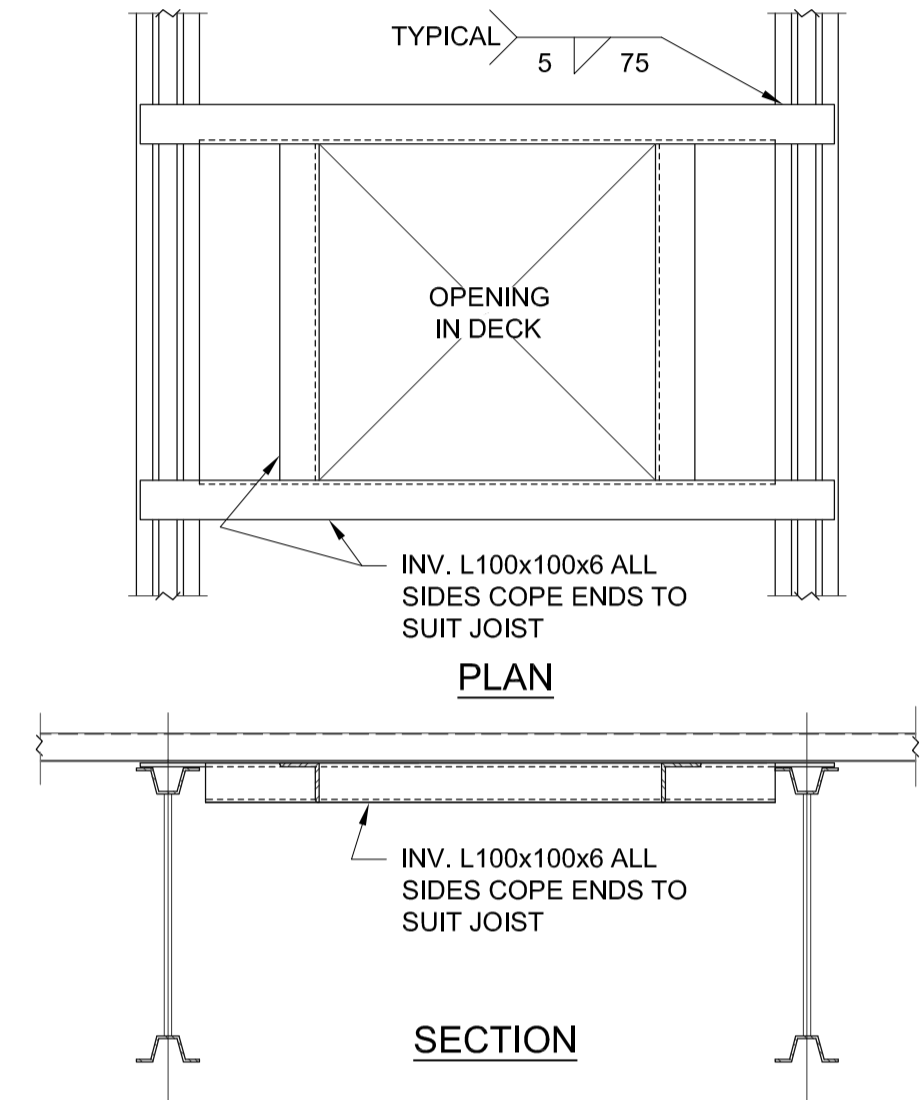
TYPICAL SHEAR LUG AT SLOPING ROOFS



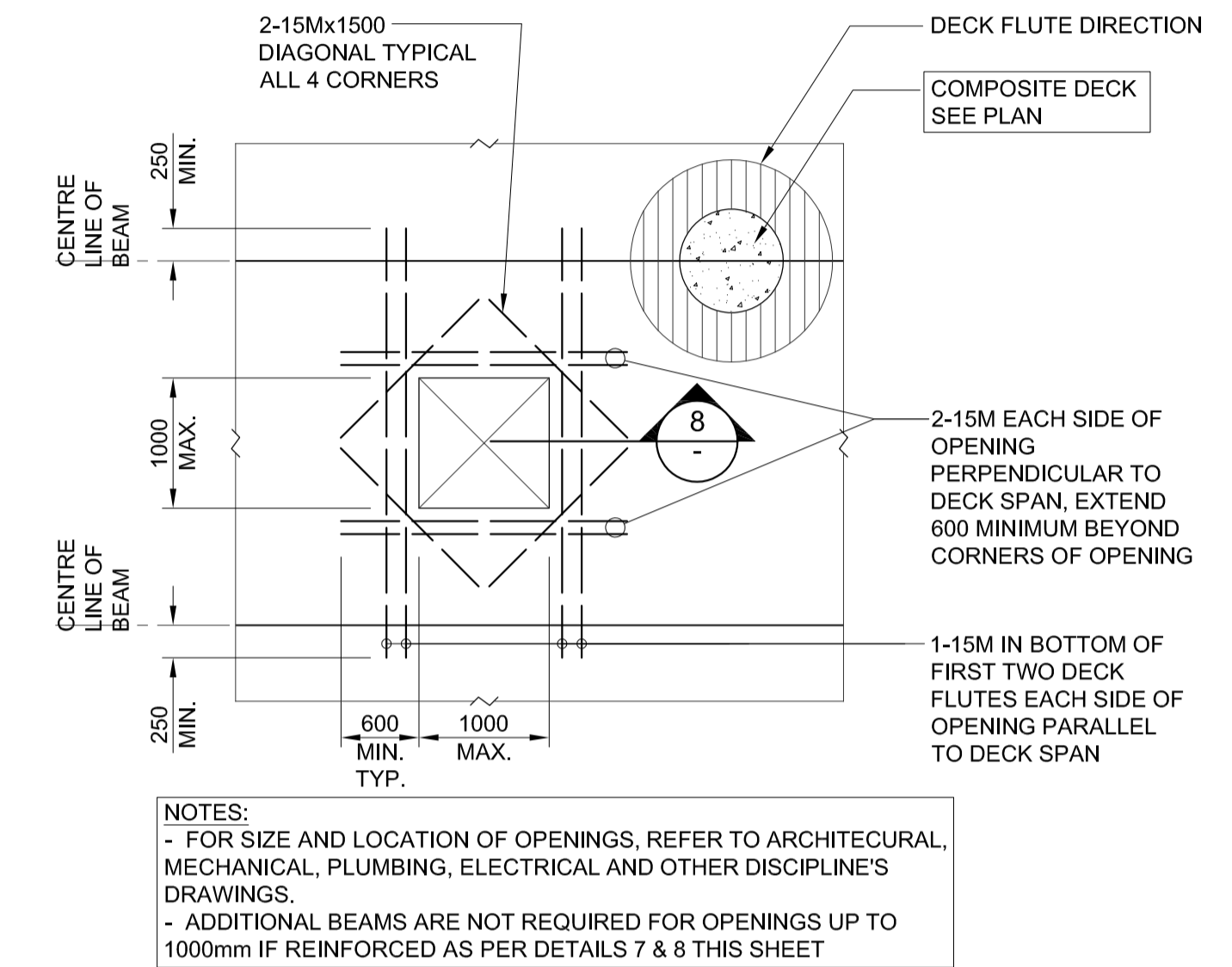
TYPICAL SHEAR LUG AT CMU WALL

NOTE:
PLACE OR CUT STEEL DECK, BEFORE WELDING, TO INSURE THAT DECK FLUTES ALIGN DIRECTLY ON SHEAR LUGS TO PROVIDE FULL TRANSFER OF LOADS THROUGH DIAPHRAGM & INTO SUPPORT BEAMS AND BRACING MEMBERS.

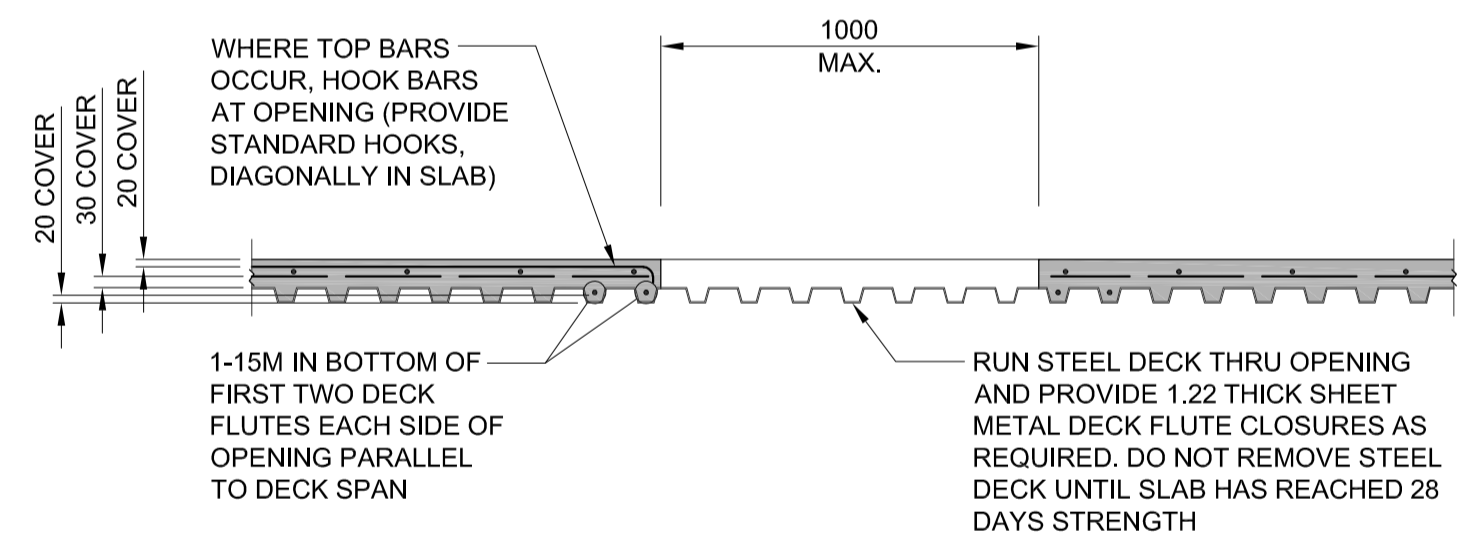
5 TYPICAL SHEAR LUG CONNECTION DETAILS NTS



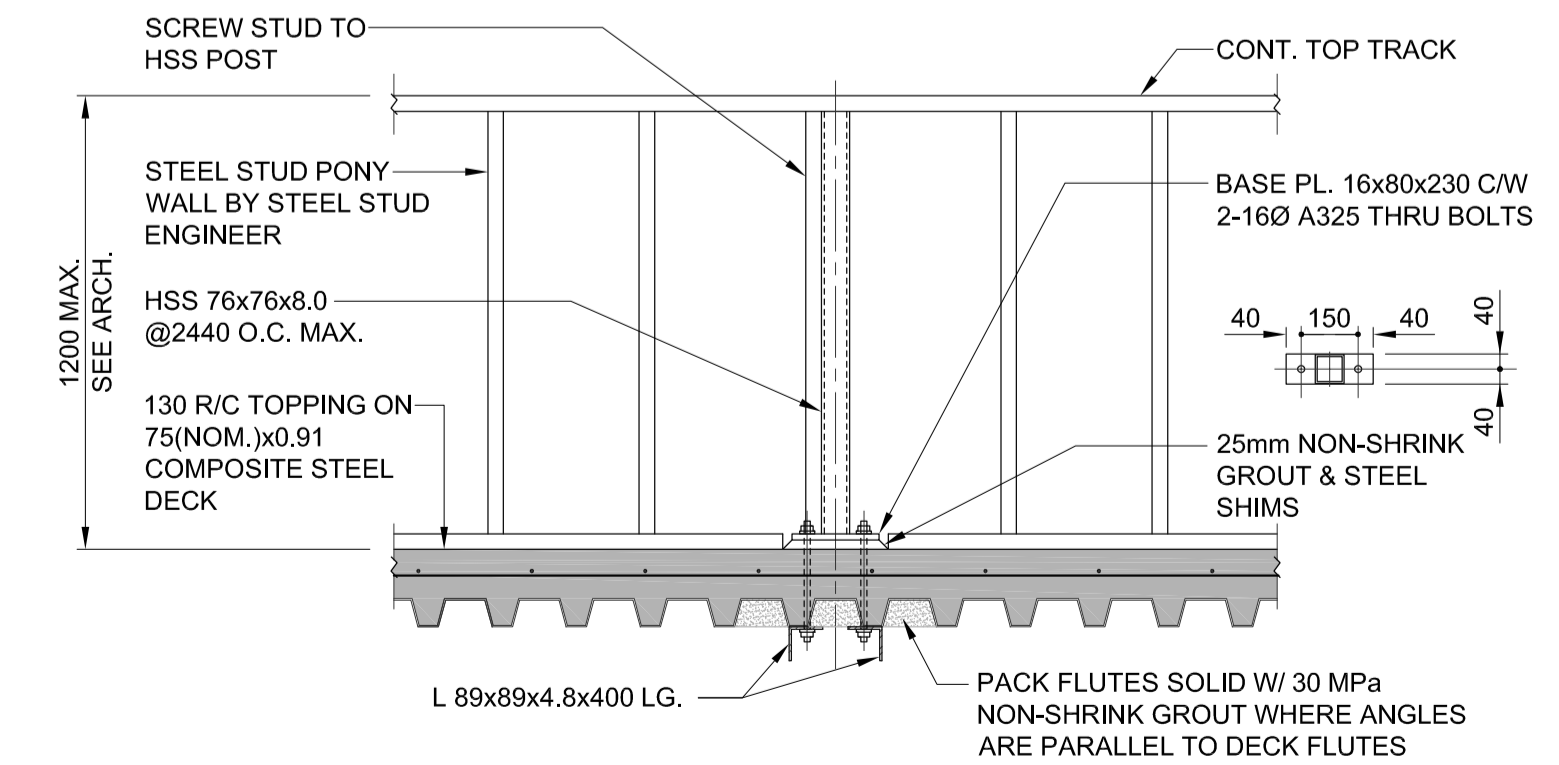
6 TYPICAL FRAMING FOR OPENINGS IN ROOF DECK DETAIL NTS



7 PARTIAL PLAN - TYPICAL SLAB ON METAL DECK OPENING (U.N.O.) NTS



8 SECTION - TYPICAL SLAB OPENING (U.N.O.) NTS



9 TYPICAL PONY WALL SUPPORT DETAIL NTS

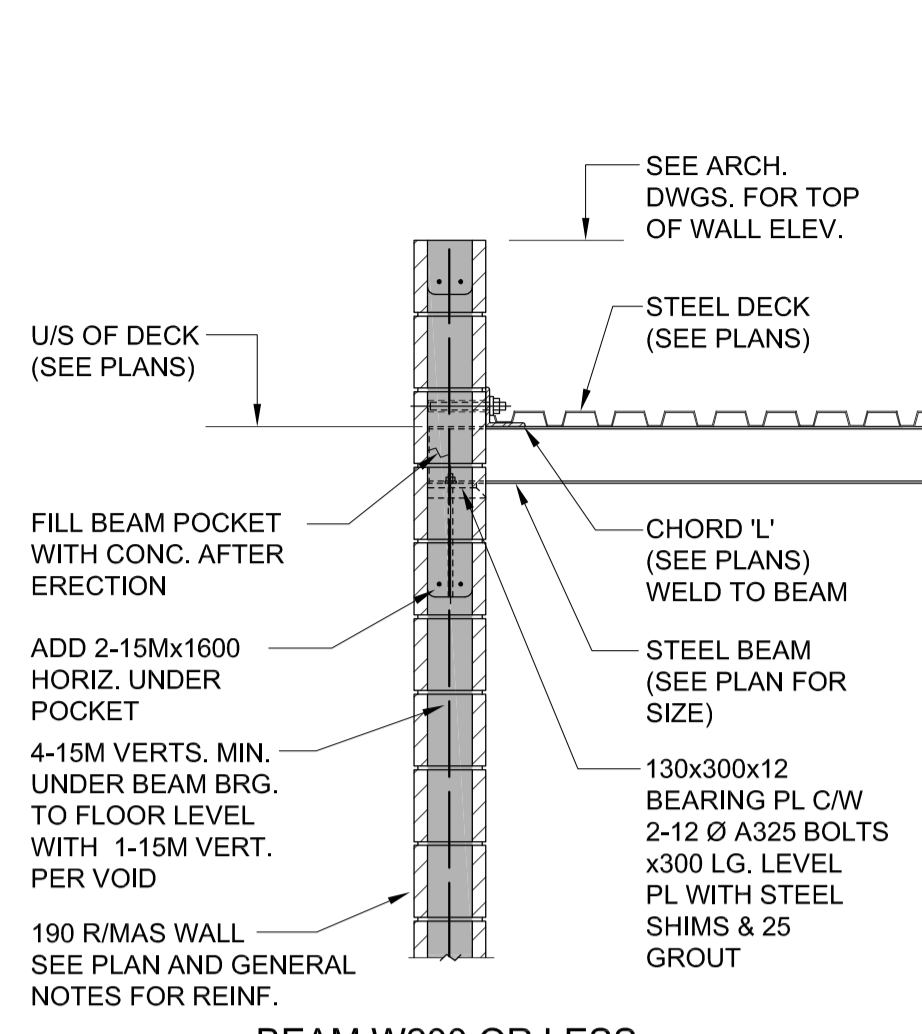
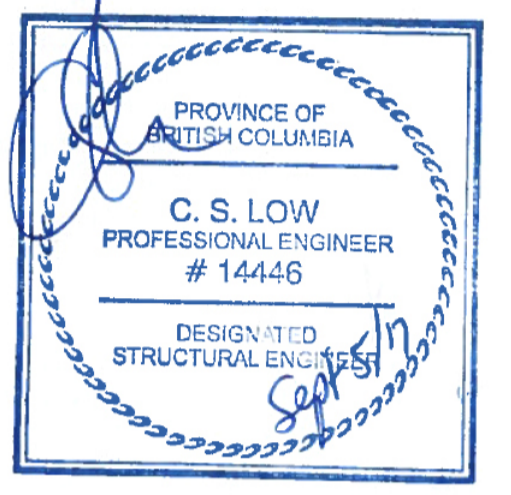
PART PLAN SHOWING PLATE DESIGNATION

TYPE	PLATE	STUDS
TYPE "A"	PL 100x12 x200 LG	2-12 Ø STUDS x100 LG.
TYPE "B"	PL 150x16 x250 LG	2-16 Ø STUDS x150 LG.
TYPE "C"	PL 150x12 x150 LG.	3-12 Ø STUDS x100 LG. (2 TOP, 1 BOTTOM)
TYPE "D"	PL 200x12 x250 LG.	3-16 Ø STUDS x150 LG. (2 TOP, 1 BOTTOM)
TYPE "E"	PL 250x16 x250 LG	CONNECTION FROM MIN 10 PL (FULL DEPTH OF CAST-IN PL.) 4-16 Ø STUDS x150 LG
TYPE "F"	PL 250x20 x300 LG	CONNECTION FROM MIN 10 PL (FULL DEPTH OF CAST-IN PL.) 4-20 Ø STUDS x150 LG
TYPE "G"	PL 250x16 x400 LG	CONNECTION FROM MIN 10 PL (FULL DEPTH OF CAST-IN PL.) 6-16 Ø STUDS x150 LG
TYPE "H"	PL 300x20 x500 LG	CONNECTION FROM MIN 12 PL (FULL DEPTH OF CAST-IN PL.) 6-20 Ø STUDS x200 LG
TYPE "J"	PL 300x25 x550 LG	CONNECTION FROM MIN 12 PL (FULL DEPTH OF CAST-IN PL.) 8-20 Ø STUDS x200 LG
TYPE "K"	PL 300x25 x650 LG	CONNECTION FROM MIN 12 PL (FULL DEPTH OF CAST-IN PL.) 10-20 Ø STUDS x200 LG

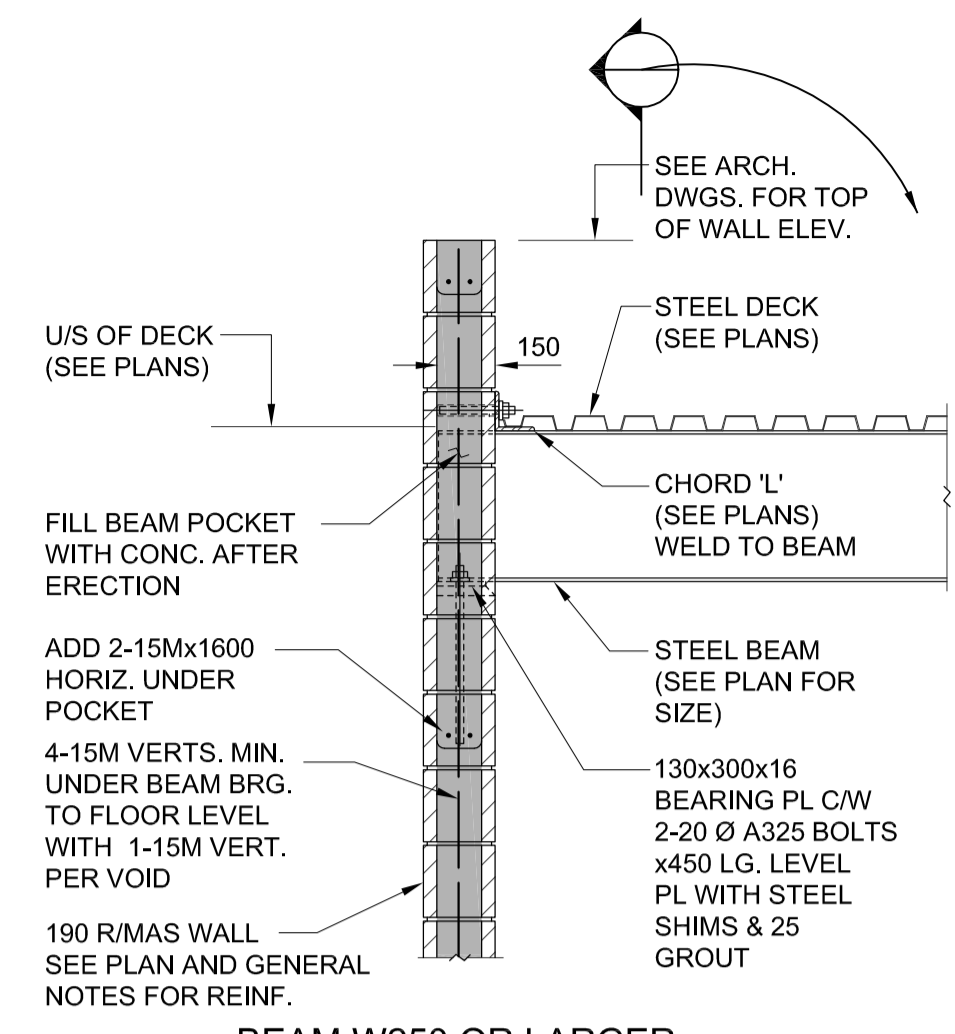
DESIGNATES PLATE TYPE AND NUMBER OF STUDS SEE DETAIL

WALL OR COLUMN SEE PLAN FOR BEAM SIZE SEE PLANS

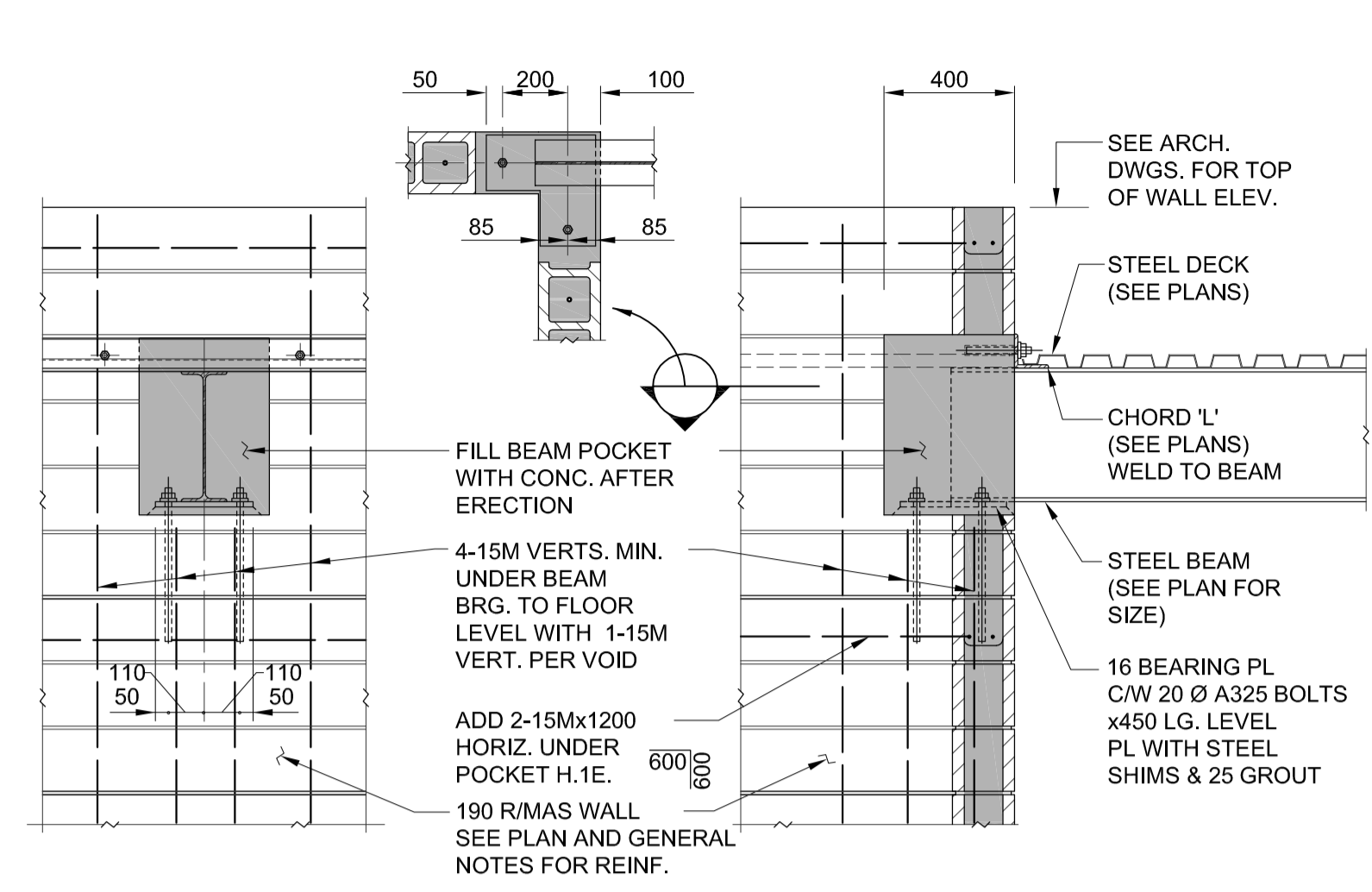
CONCRETE COVER TO EDGE OF CAST-IN PLATE REQUIRED TO DEVELOP LOADS			
TYPE	TOP	BOTTOM	SIDES
TYPE 'A'	TOP - 35mm	BOTTOM - 100mm	SIDES - 25mm
TYPE 'B'	TOP - 40mm	BOTTOM - 130mm	SIDES - 15mm
TYPE 'C'	TOP - 50mm	BOTTOM - 125mm	SIDES - 50mm
TYPE 'D','E'&'G'	TOP - 40mm	BOTTOM - 130mm	SIDES - 40mm
TYPE 'F','H','J','K'	TOP - 60mm	BOTTOM - 160mm	SIDES - 60mm



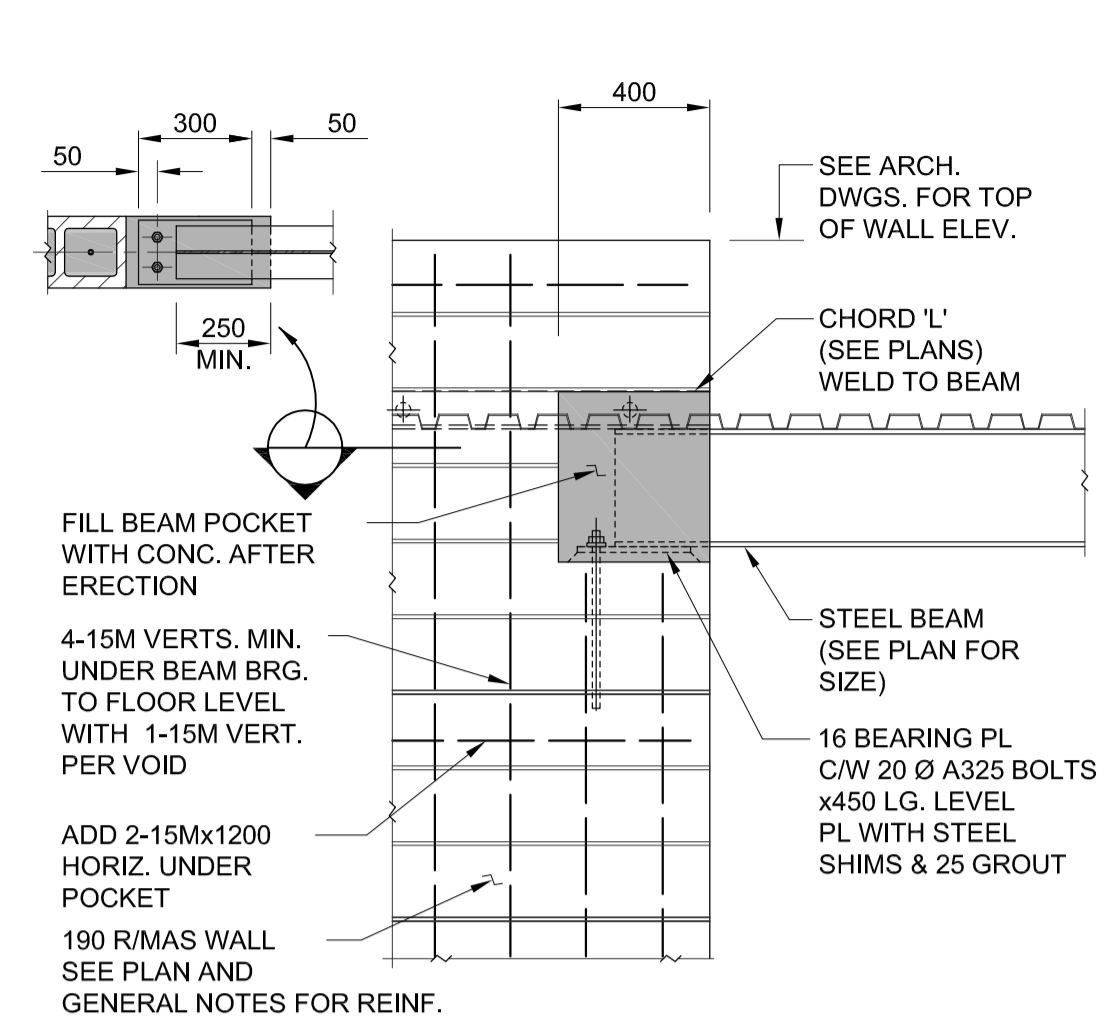
BEAM W200 OR LESS



BEAM W250 OR LARGER

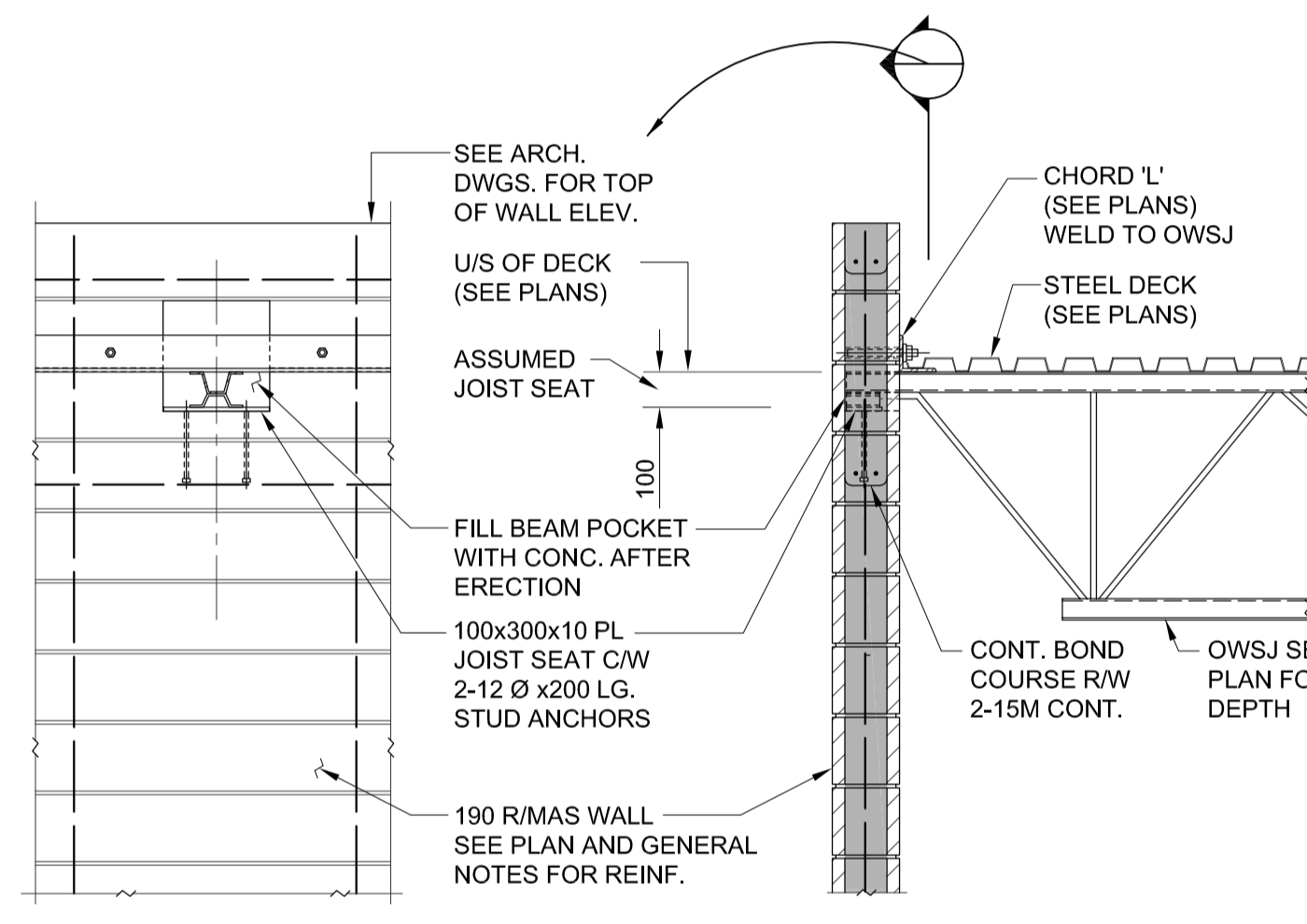


2 TYPICAL BEAM/CORNER WALL BEARING DETAIL NTS

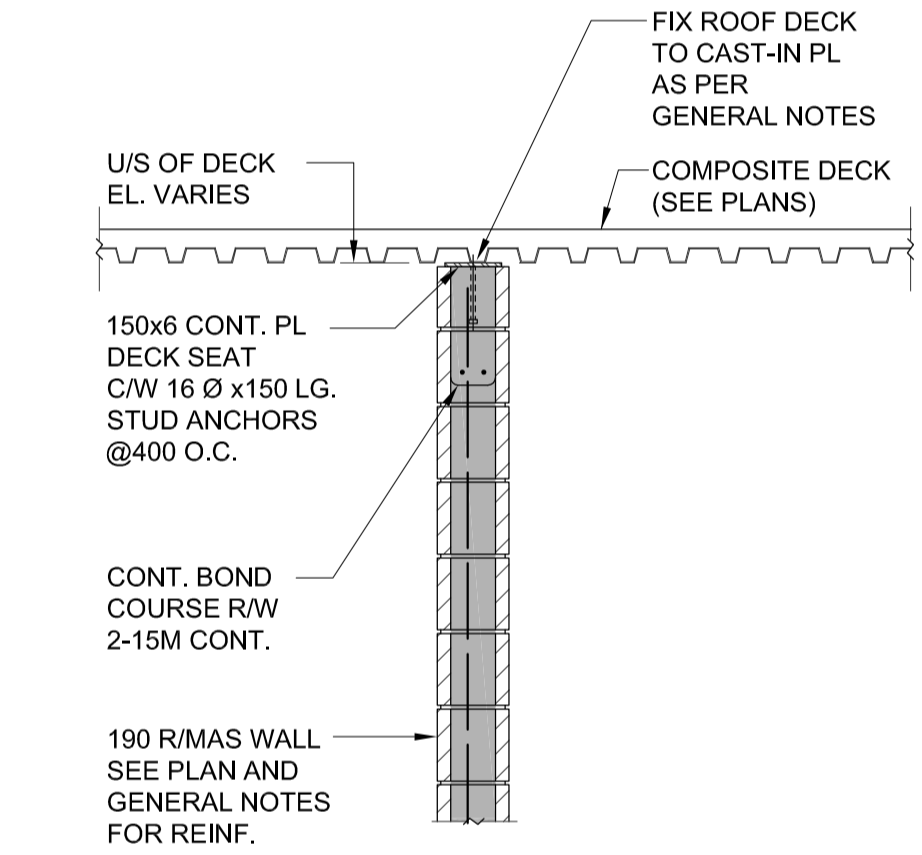


3 TYPICAL BEAM END OF WALL BEARING DETAIL NTS

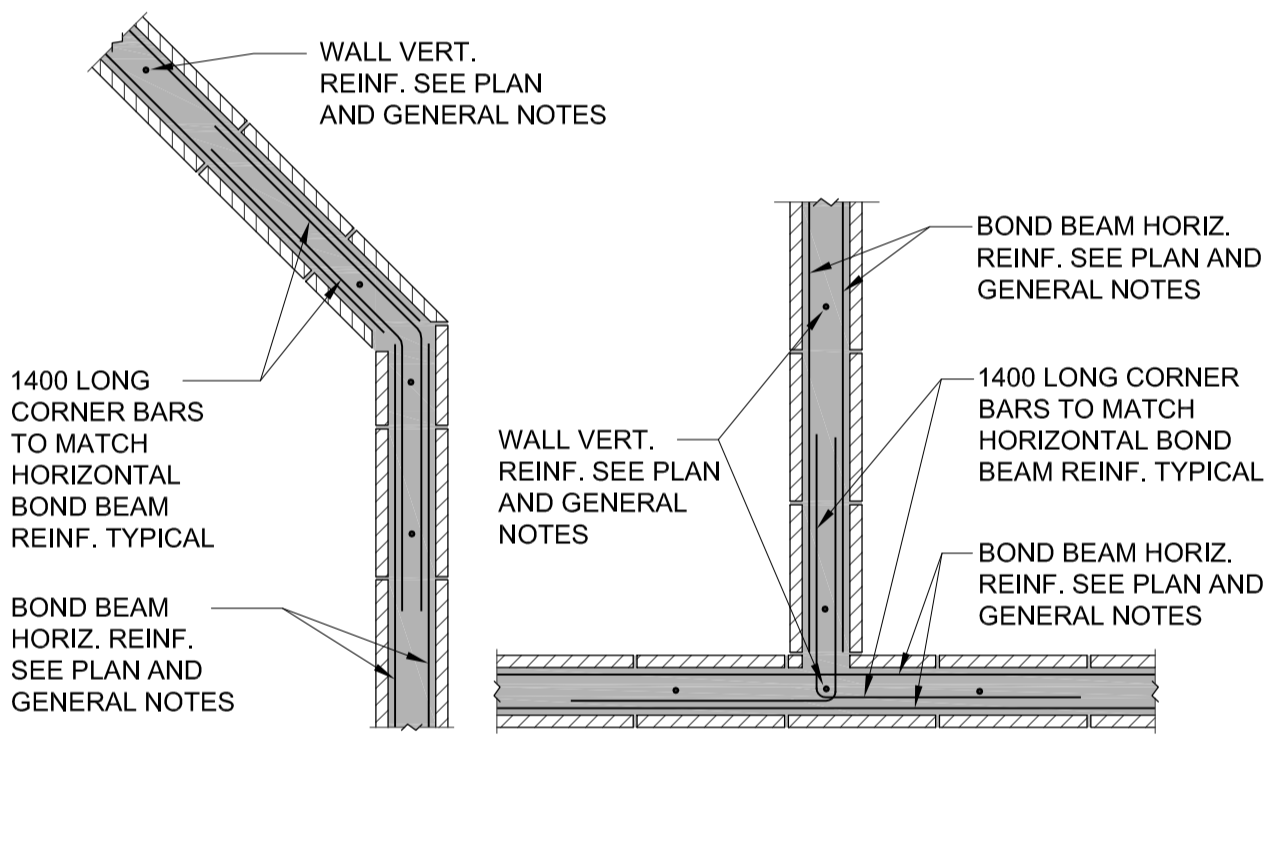
1 TYPICAL BEAM/MASONRY WALL BEARING DETAILS NTS



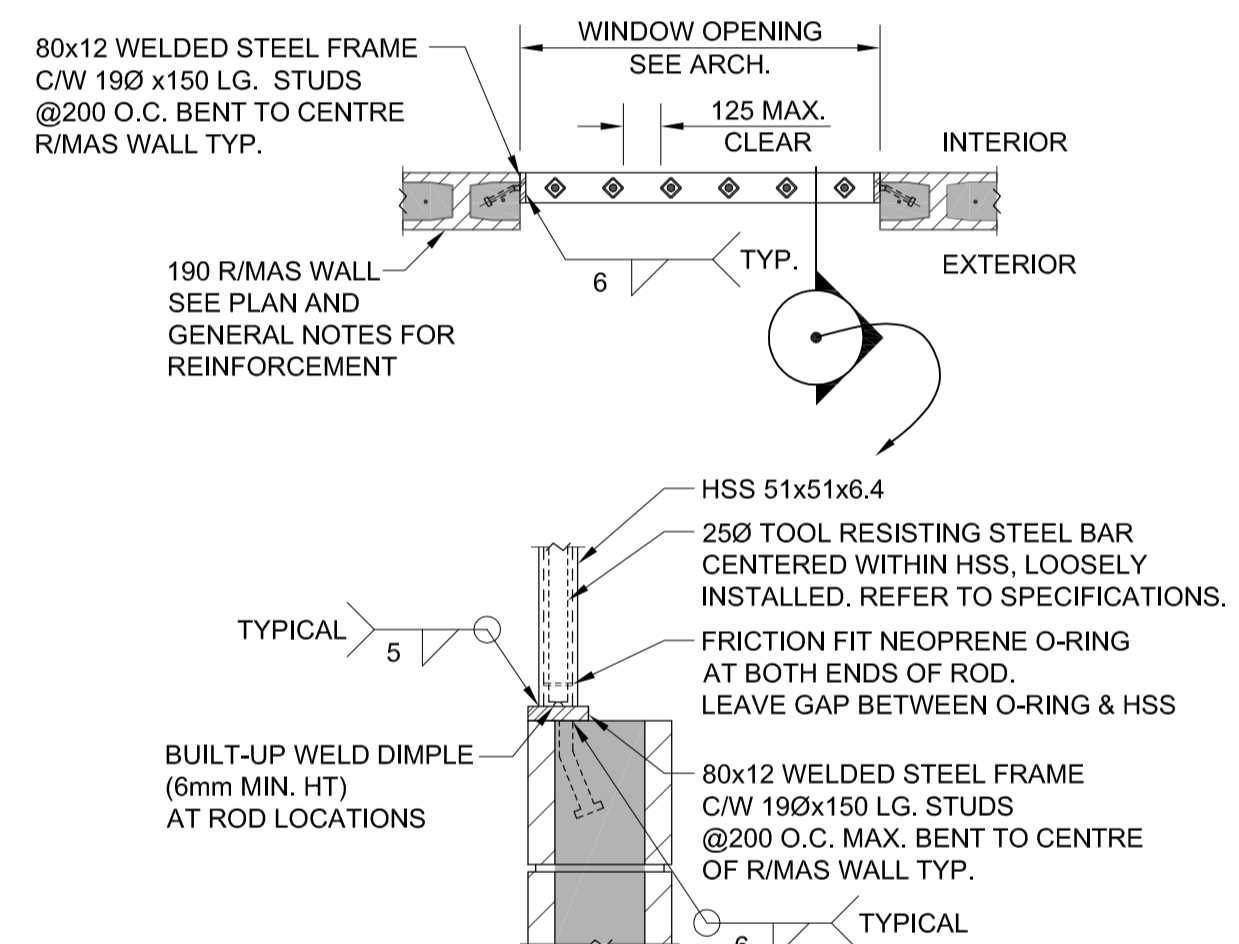
4 TYPICAL O.W.S.J./MASONRY WALL BEARING DETAIL NTS



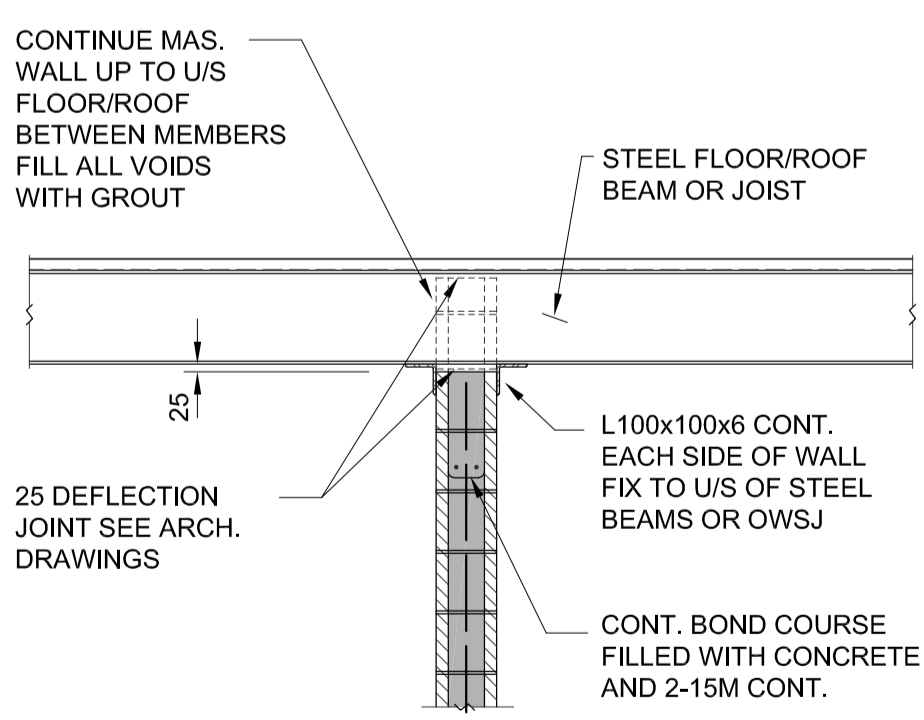
5 TYPICAL STEEL DECK/MASONRY WALL BEARING DETAIL NTS



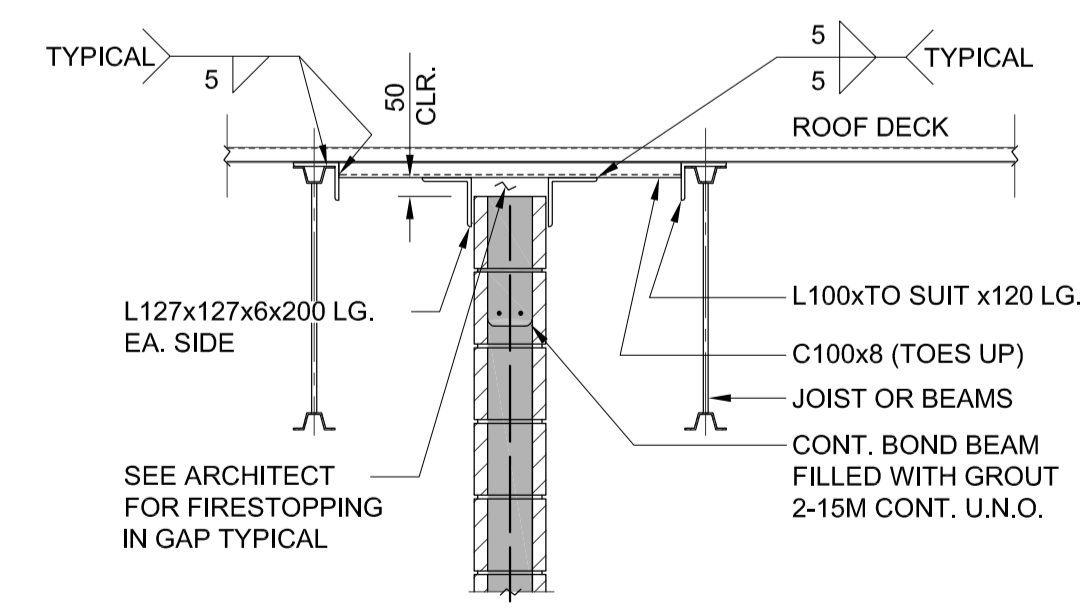
6 TYPICAL R/MAS. BOND BEAM CORNER TIE DETAILS NTS



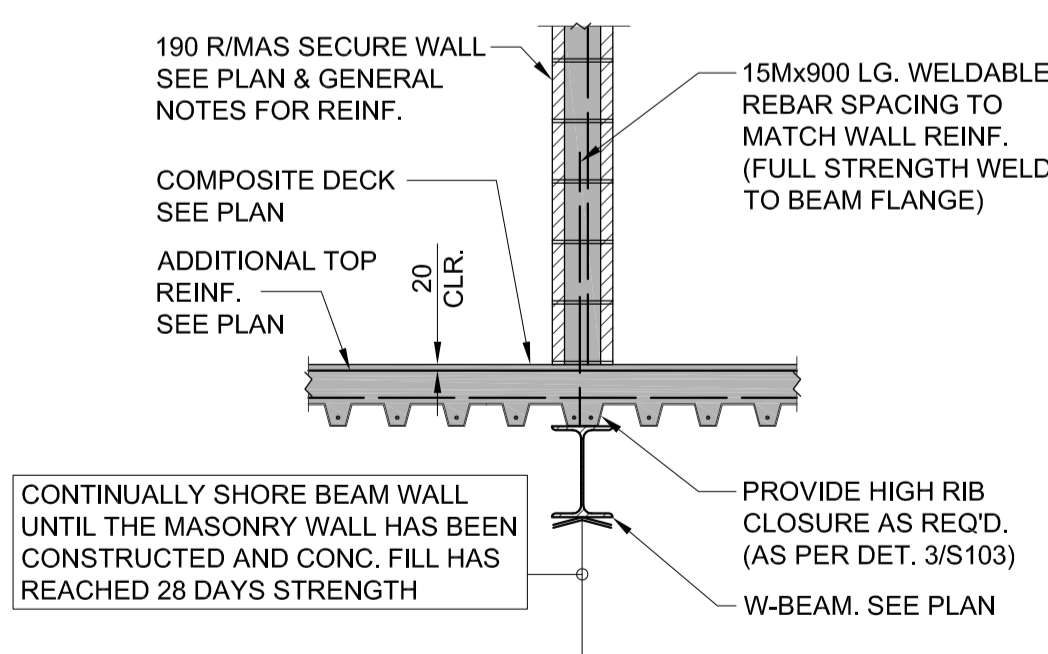
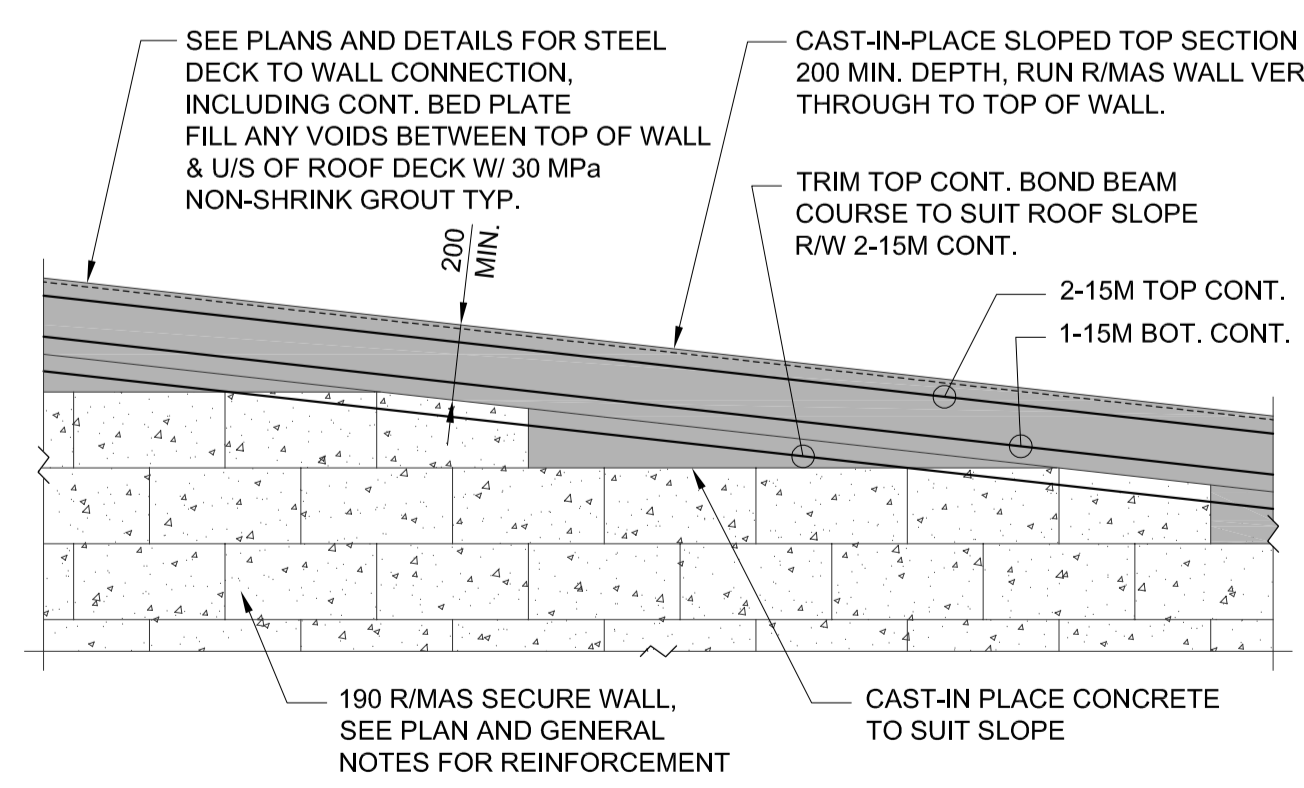
7 TYPICAL EXTERNAL WINDOW STEEL SECURITY BAR DETAIL NTS



8 TYPICAL SUPPORT AT TOP OF NON-LOADBEARING MASONRY WALLS (FOR WALL LOCATIONS SEE ARCH. DWGS.) NTS



9 TYPICAL TOP OF WALL BOND BEAM FOR SLOPED WALLS NTS



10 TYPICAL SECURE MASONRY WALL BASE AT SUSPENDED SLAB ON DECK DETAIL NTS

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CORRECTIONAL SERVICE OF CANADA

Project title/Titre du projet
AGASSIZ, BRITISH COLUMBIA
4732 CEMETERY ROAD PO BOX 1600
MOUNTAIN INSTITUTE

MOUNTAIN INSTITUTE
HEALTH CARE EXPANSION

Consultant Signature Only

Designed by/Concept par
CSL

Drawn by/Dessine par
MSH

PSPC Project Manager/Administrateur de Projets SPAC
TONY TANG

Regional Manager, Architectural and Engineering Services
Gestionnaire régionale, Services d'architecture et de génie, SPAC
PREETIPAL PAUL

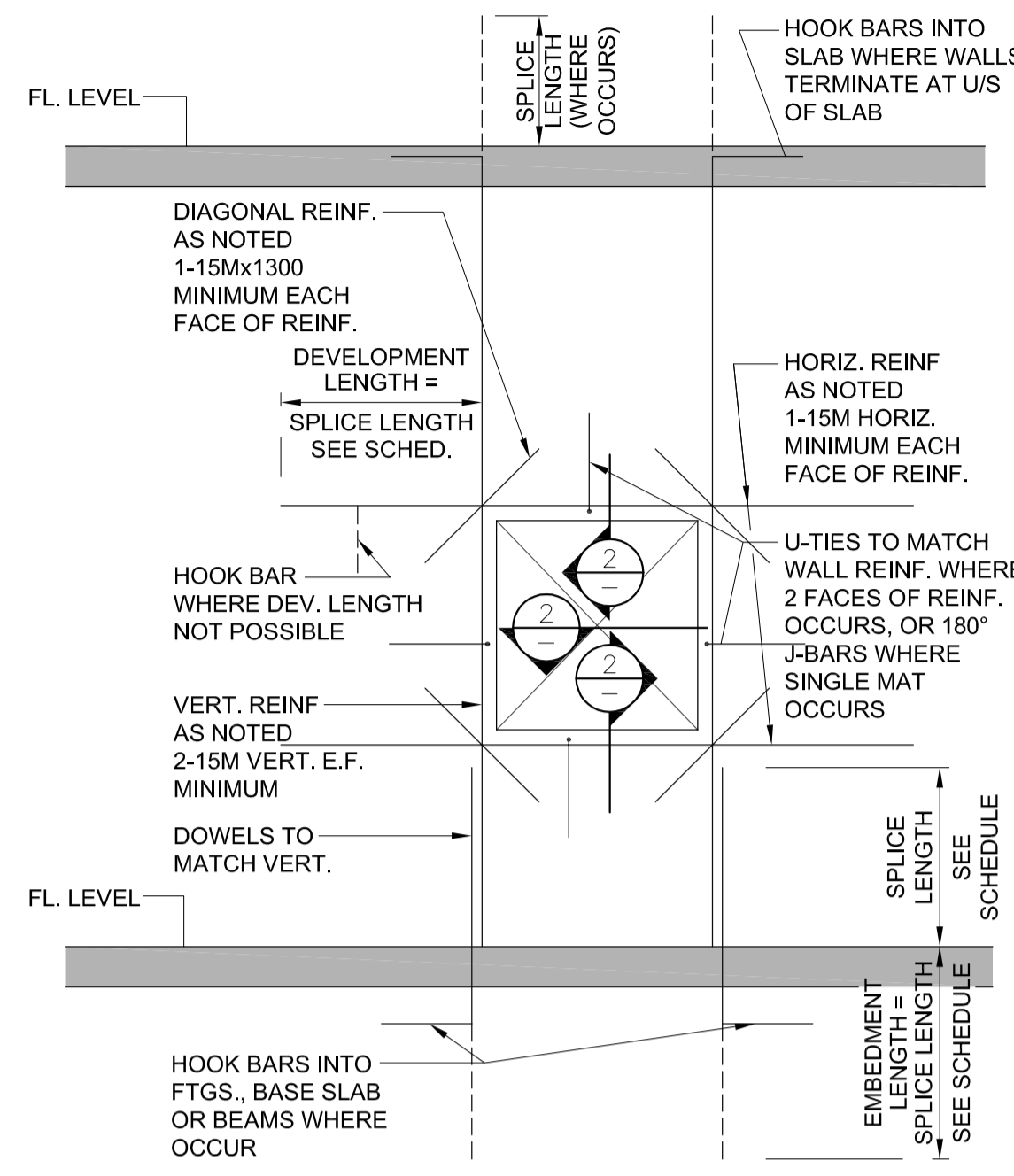
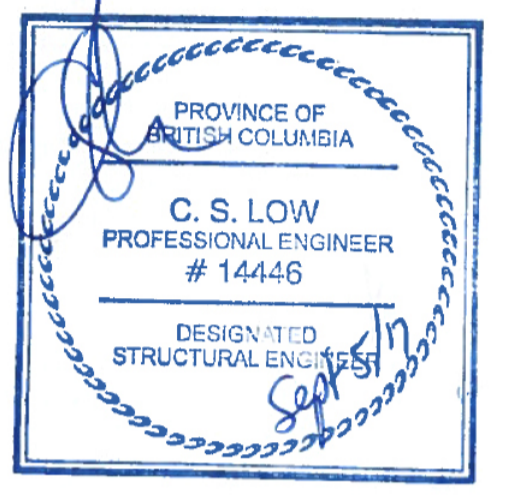
Drawing title/Titre du dessin

TYPICAL DETAILS SHEET 3

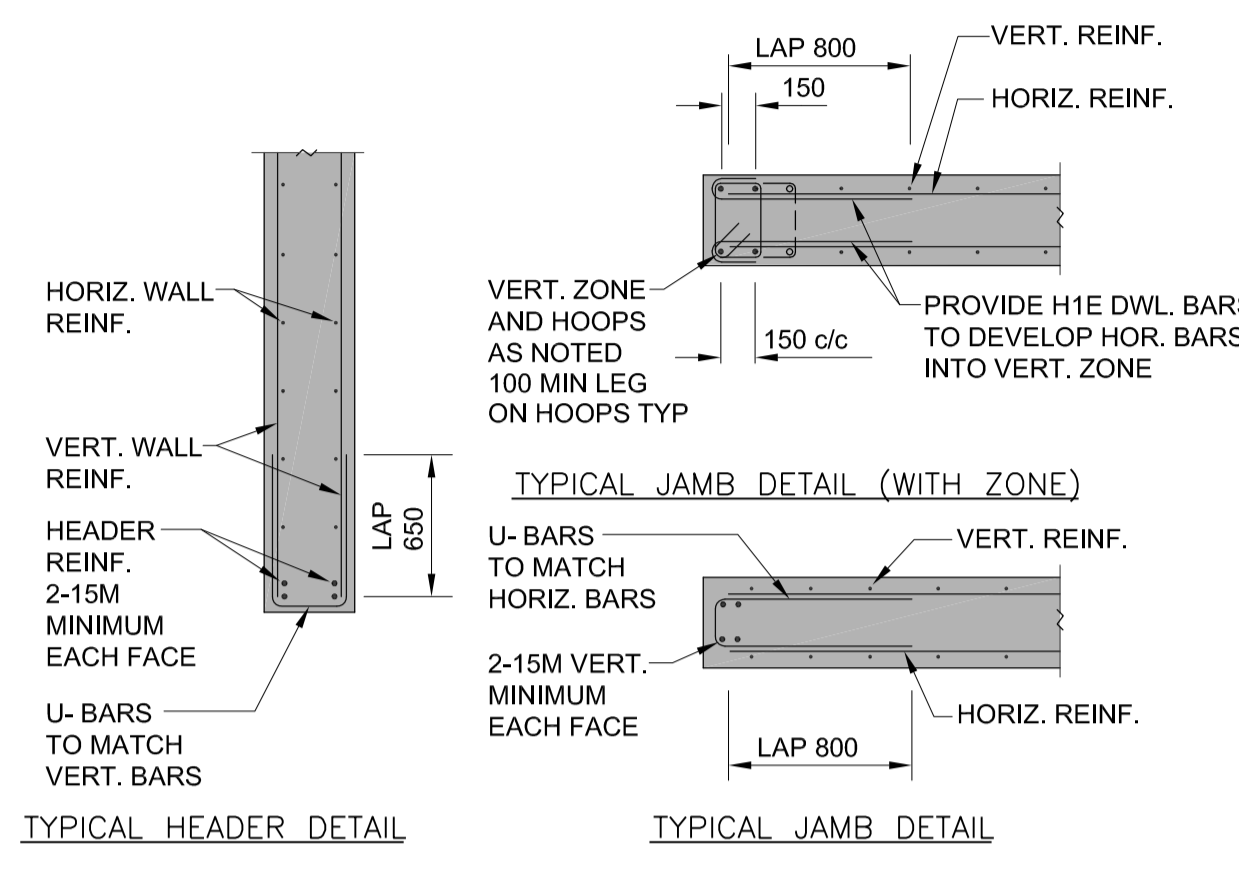
Project No./No. du projet
R.077724.001

Sheet/Feuille
S104
OF XX

Revision no./La Révision no.
0

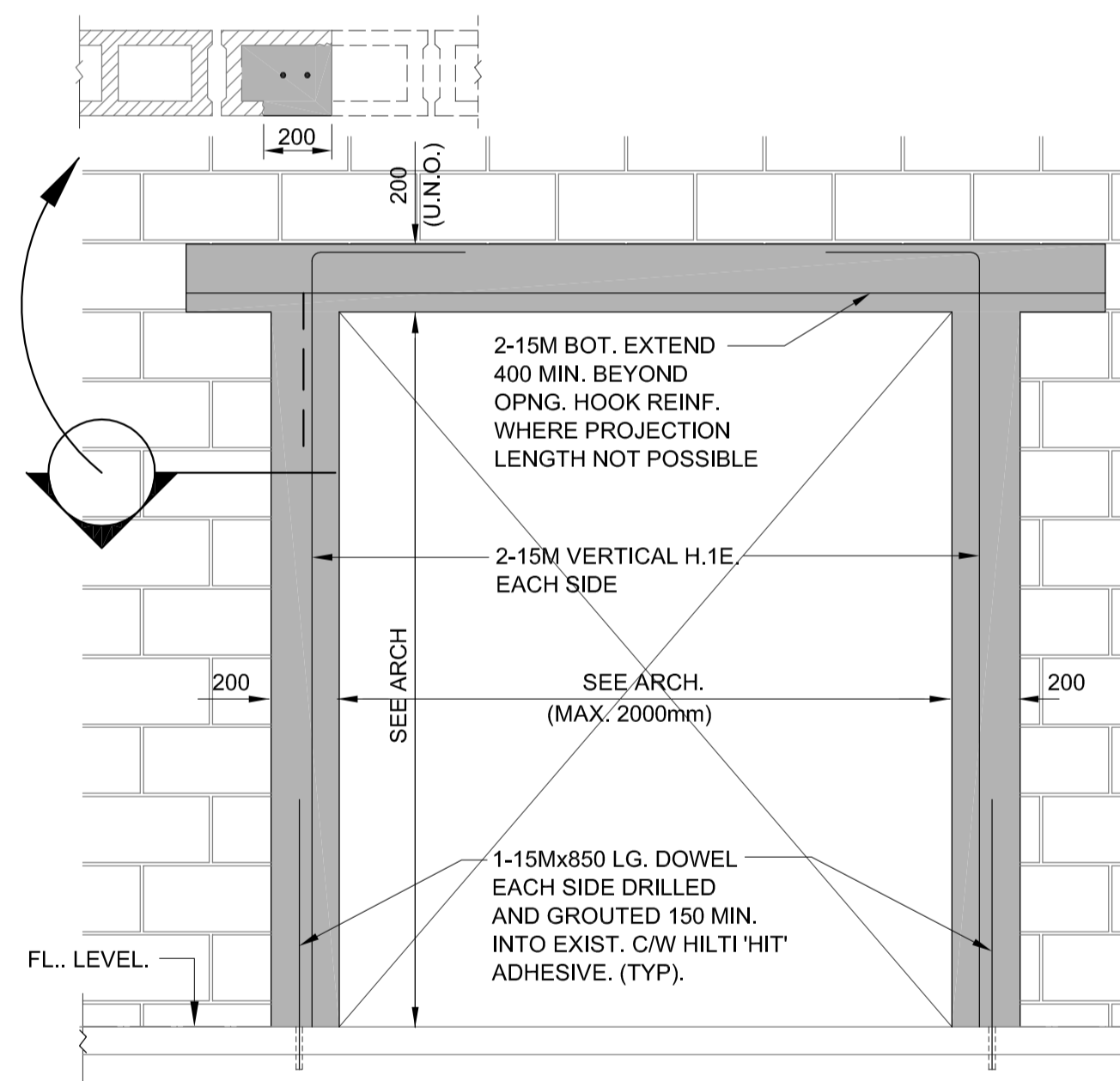


1 TYPICAL WALL OPENING
- - - - - NTS

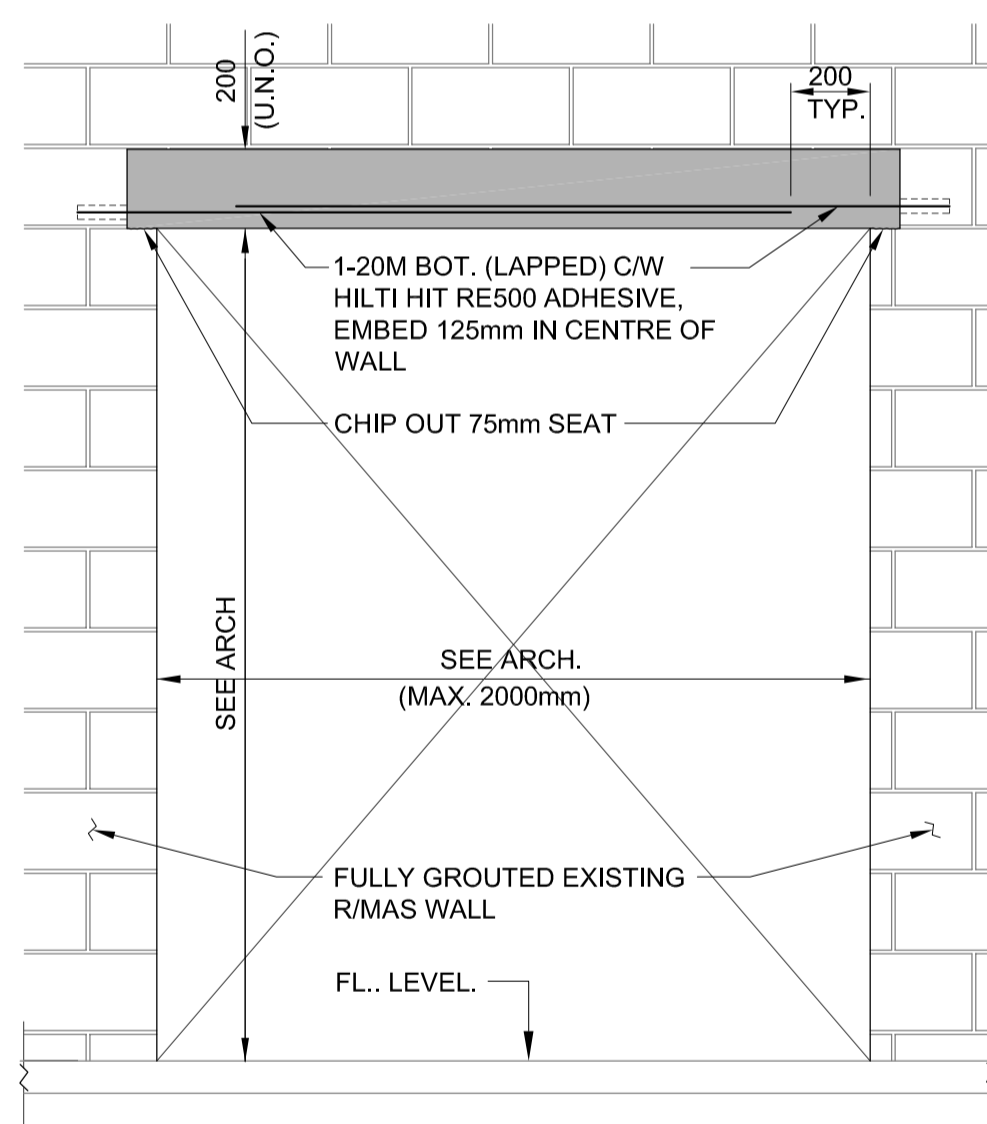


2 TYPICAL WALL OPENING
- - - - - JAMB AND HEADER DETAILS

- PROCEDURE NOTES:
1. LOCATE NEW OPENINGS, SEE ARCH. DWGS. FOR CONFIGURATIONS.
 2. CHECK FOR EXISTING OPENINGS ABOVE NEW OPENING. CONFIRM IF ADDITIONAL STRENGTHENING IS REQUIRED.
 3. BREAK BACK EXISTING MASONRY FACE SHELLS AROUND OPENING AS NOTED TO PROVIDE NEW BOND BEAMS.
 4. INSTALL NEW REINF. AS NOTED & GROUT SOLID MODIFIED CELLS.
 5. PREDRILL CORNERS TO AVOID OVERCUTTING.
 6. SAWCUT NEW OPNG. AS SPECIFIED.



3 MASONRY DOOR OPENING
- - - - - (NON-FULLY GROUTED EXISTING WALLS) NTS



4 MASONRY DOOR OPENING
- - - - - (FULLY GROUTED EXISTING WALLS) NTS

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CORRECTIONAL SERVICE OF CANADA

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AGASSIZ, BRITISH COLUMBIA
4732 CEMETERY ROAD PO BOX 1600
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HEALTH CARE EXPANSION

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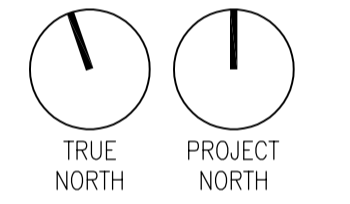
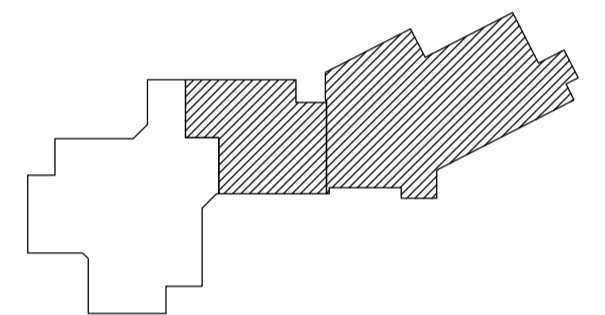
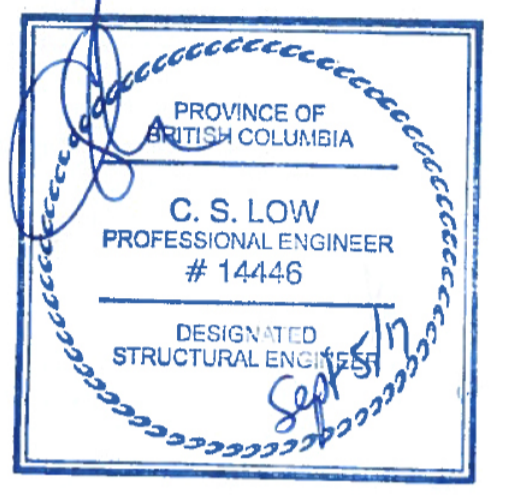
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TONY TANG

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Gestionnaire régionale, Services d'architectural et de génie, SPAC
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TYPICAL DETAILS SHEET 4

Project No./No. du projet R.077724.001	Sheet/Feuille S105 OF XX	Revision no./La Révision no. 0
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Gestionnaire régionale, Services d'architecture et de génie, SPAC
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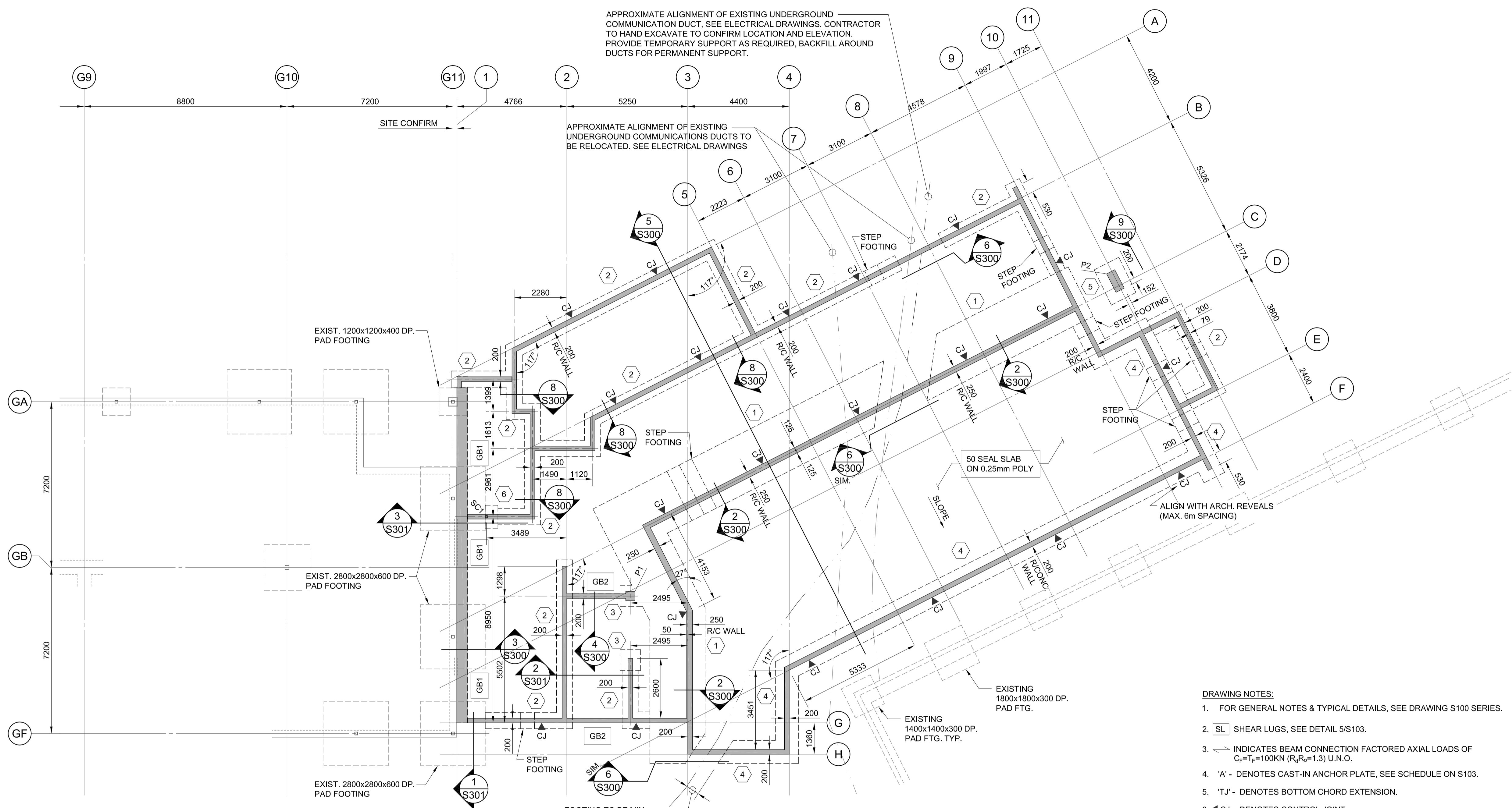
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FOUNDATION AND CRAWL SPACE PLAN

Project No./No. du projet
R.077724.001

Sheet/Feuille
S201
OF XX

Revision no./La Révision no.
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FOUNDATION AND CRAWL SPACE PLAN
1:100

STEEL COLUMN SCHEDULE

TYPE	SIZE
SC1	HSS 102x102x8.0

CONCRETE PEDESTAL SCHEDULE

TYPE	SIZE	REINFORCEMENT	TIES
P1	400x400	R/W 4-25M VERT. C/W 10M TIES @300	☐
P2	400x900	R/W 12-20M VERT. C/W 10M TIES @300	☐

FOOTING SCHEDULE

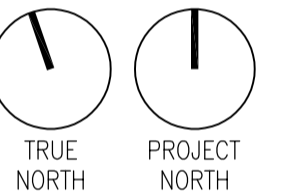
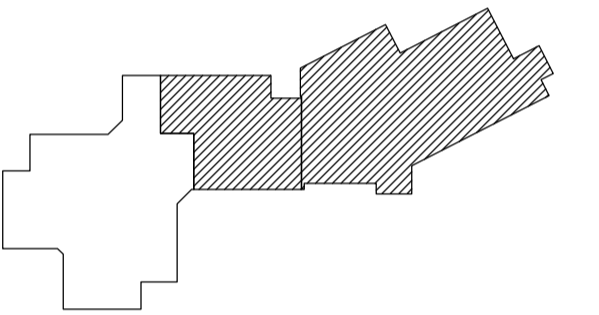
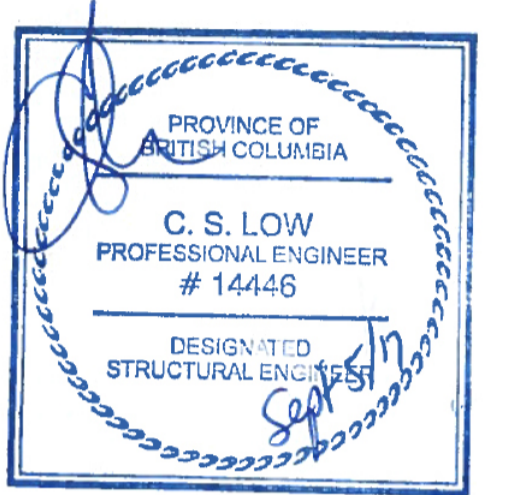
TYPE	SIZE	REINFORCEMENT
1	2400x350 DP. STRIP FTG.	15M@275 TRANSVERSE BOT. 9-15M CONT. LONG. BOT. 15M@200 H1E TRANSVERSE TOP 5-15M CONT. LONG. TOP
2	700x350 DP. STRIP FTG.	3-15M CONT. LONG. BOT. 15M@275 TRANSVERSE BOT.
3	900x900x400 DP. PAD FTG.	4-15M EACH WAY BOT.
4	1000x350 DP. STRIP FTG.	4-15M CONT. LONG. BOT., 15M@275 TRANSVERSE BOT.
5	1700x1200x500 DP. PAD FTG.	6-15M LONG. BOT., 9-15M TRANSVERSE BOT.
6	1000x550x600 DP. PAD FTG.	4-15M LONG. BOT., 6-15M TRANSVERSE BOT.

GRADE BEAM SCHEDULE

TYPE	SIZE	CONCRETE STRENGTH	REINFORCEMENT
GB1	400x400	25 MPa	SEE DETAIL 3/S300
GB2	200x1200	25 MPa	SEE DETAIL 4/S300

GRADE BEAM NOTES:
1. ALL TOP REINFORCING LAPS ARE TO BE AT MID-SPAN, ALL BOTTOM REINFORCING LAPS ARE TO BE AT SUPPORTS.
2. PROVIDE 15M H1E DOWELS EMBEDDED INTO EXISTING PAD FOOTINGS AND SLAB EDGE THICKENING.

- DRAWING NOTES:**
- FOR GENERAL NOTES & TYPICAL DETAILS, SEE DRAWING S100 SERIES.
 - SL SHEAR LUGS, SEE DETAIL 5/S103.
 - INDICATES BEAM CONNECTION FACTORED AXIAL LOADS OF $C_c = T_c = 100\text{KN}$ ($R_c, R_t = 1.3$) U.N.O.
 - 'A' - DENOTES CAST-IN ANCHOR PLATE, SEE SCHEDULE ON S103.
 - 'TJ' - DENOTES BOTTOM CHORD EXTENSION.
 - CJ - DENOTES CONTROL JOINT.



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CORRECTIONAL SERVICE OF CANADA

Project title/Titre du projet
AGASSIZ, BRITISH COLUMBIA
4732 CEMETERY ROAD PO BOX 1600
MOUNTAIN INSTITUTE

MOUNTAIN INSTITUTE HEALTH CARE EXPANSION

Consultant Signature Only

Designed by/Concept par
CSL

Drawn by/Dessine par
MSH

PSPC Project Manager/Administrateur de Projets SPAC
TONY TANG

Regional Manager, Architectural and Engineering Services
Gestionnaire régionale, Services d'architecture et de génie, SPAC
PREETPAL PAUL

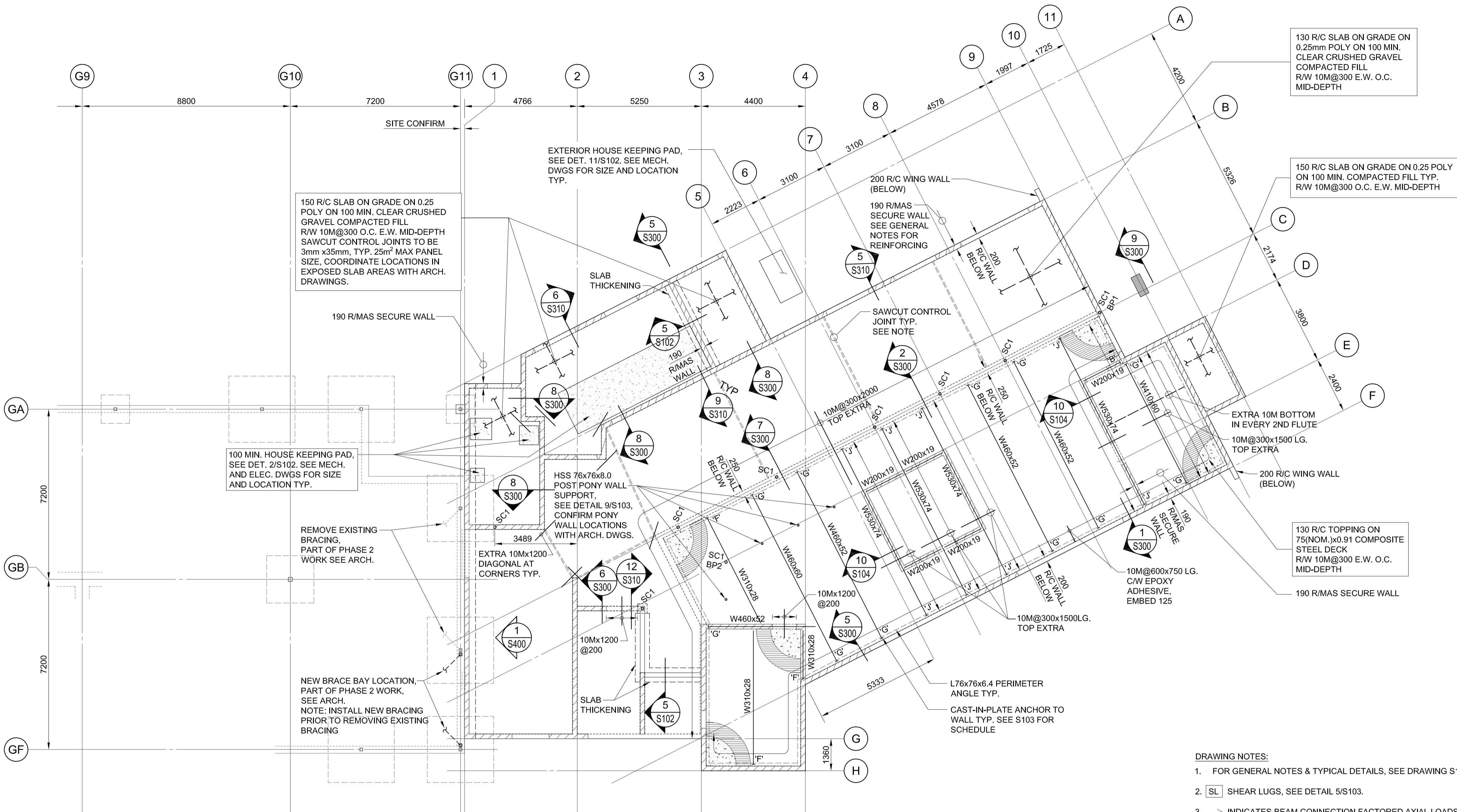
Drawing title/Titre du dessin

MAIN FLOOR PLAN

Project No./No. du projet
R.077724.001

Sheet/ Feuille
S202
OF XX

Revision no./
La Révision no.
0



MAIN FLOOR PLAN

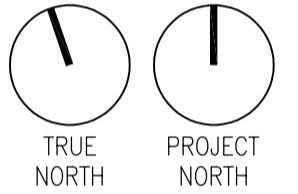
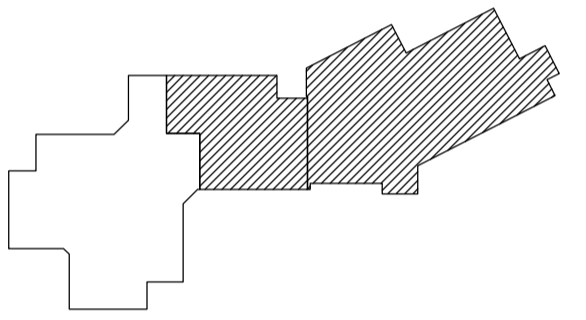
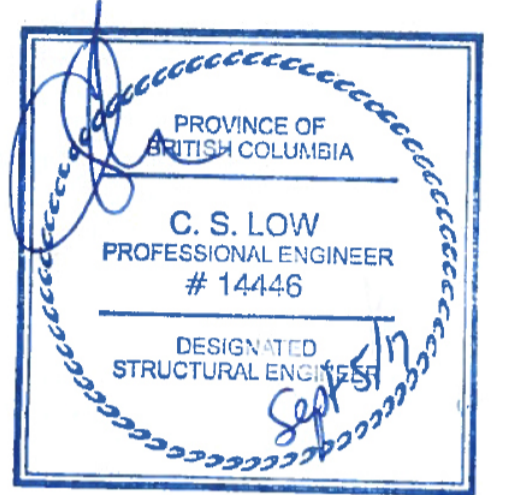
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DRAWING NOTES:

- FOR GENERAL NOTES & TYPICAL DETAILS, SEE DRAWING S100 SERIES.
- SL SHEAR LUGS, SEE DETAIL 5/S103.
- INDICATES BEAM CONNECTION FACTORED AXIAL LOADS OF $C_F = T_F = 100\text{KN}$ ($R_F/R_T = 1.3$) U.N.O.
- 'A' - DENOTES CAST-IN ANCHOR PLATE, SEE SCHEDULE ON S103.
- 'TJ' - DENOTES BOTTOM CHORD EXTENSION.
- CJ - DENOTES CONTROL JOINT.

STEEL COLUMN SCHEDULE	
TYPE	SIZE
SC1	HSS 102x102x8.0

COLUMN BASE PLATE SCHEDULE	
TYPE	SIZE
BP1	PL. 16x175x250 C/W 4-19 Ø x450 LG. A307 H.1E. ANCHOR BOLTS C/W STEEL SHIMS & GROUT
BP2	PL. 16x127 C/W 2-19 Ø THREADED ADHESIVE ANCHOR, EMBED 90 C/W STEEL SHIMS & GROUT



Revision/Revision	Description/Description	Date/Date
5		
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CORRECTIONAL SERVICE OF CANADA

Project title/Titre du projet
 AGASSIZ, BRITISH COLUMBIA
 4732 CEMETERY ROAD PO BOX 1600
 MOUNTAIN INSTITUTE

MOUNTAIN INSTITUTE
 HEALTH CARE EXPANSION

Consultant Signature Only

Designed by/Concept par
 CSL

Drawn by/Dessiné par
 MSH

PSPC Project Manager/Administrateur de Projets SPAC
 TONY TANG

Regional Manager, Architectural and Engineering Services
 Gestionnaire régionale, Services d'architectural et de génie, SPAC
 PRETIFFAL FAUL

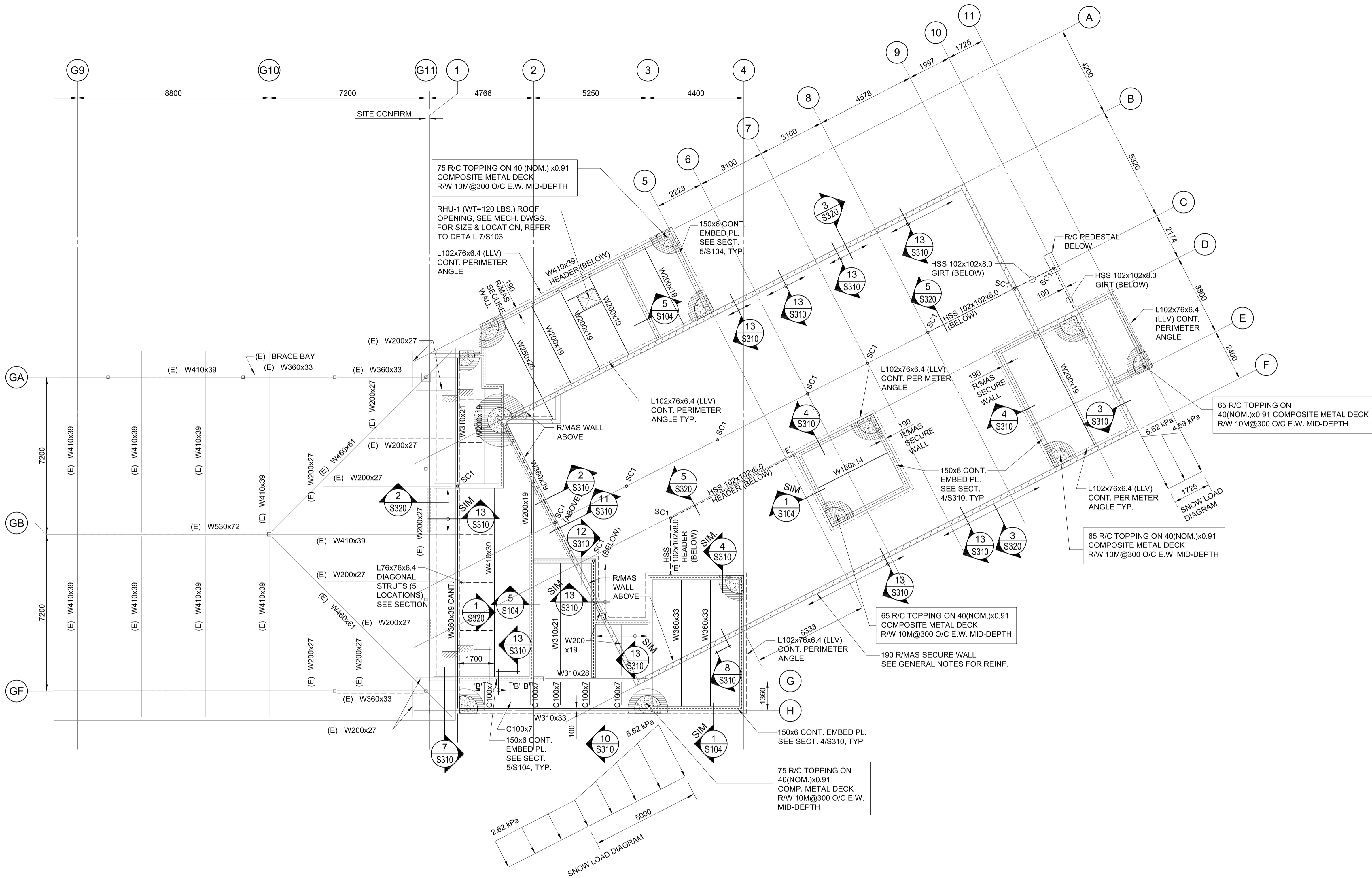
Drawing title/Titre du dessin

LOWER ROOF PLAN
 AND SECURED CEILING PLAN

Project No./No. du projet
 R.077724.001

Sheet/Feuille
 S203
 OF XX

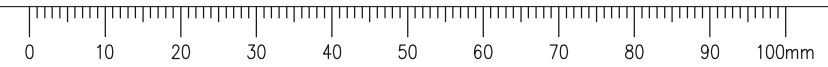
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 La Révision
 no. 0

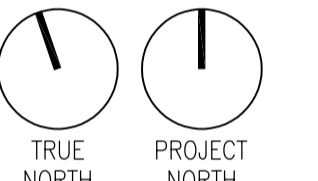
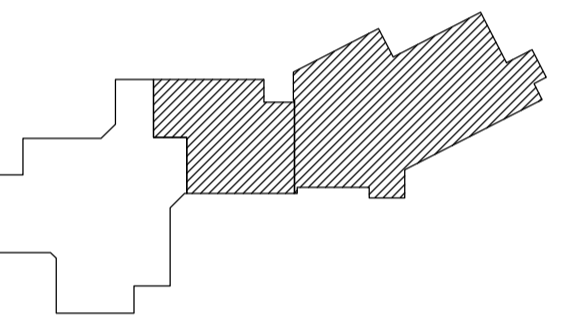
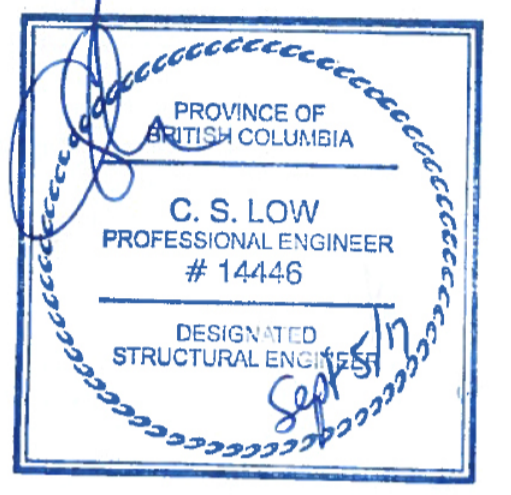


LOWER ROOF AND SECURED CEILING PLAN
 1:100

- DRAWING NOTES:**
- FOR GENERAL NOTES & TYPICAL DETAILS, SEE DRAWING S100 SERIES.
 - SL SHEAR LUGS, SEE DETAIL 5/S103.
 - \leftarrow INDICATES BEAM CONNECTION FACTORED AXIAL LOADS OF $C_c = T_c = 100\text{KN}$ ($R_c, R_t = 1.3$) U.N.O.
 - 'A' - DENOTES CAST-IN ANCHOR PLATE, SEE SCHEDULE ON S103.
 - 'TJ' - DENOTES BOTTOM CHORD EXTENSION.
 - CJ - DENOTES CONTROL JOINT.

STEEL COLUMN SCHEDULE	
TYPE	SIZE
SC1	HSS 102x102x8.0





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CORRECTIONAL SERVICE OF CANADA

Project title/Titre du projet
AGASSIZ, BRITISH COLUMBIA
 4732 CEMETERY ROAD PO BOX 1600
 MOUNTAIN INSTITUTE

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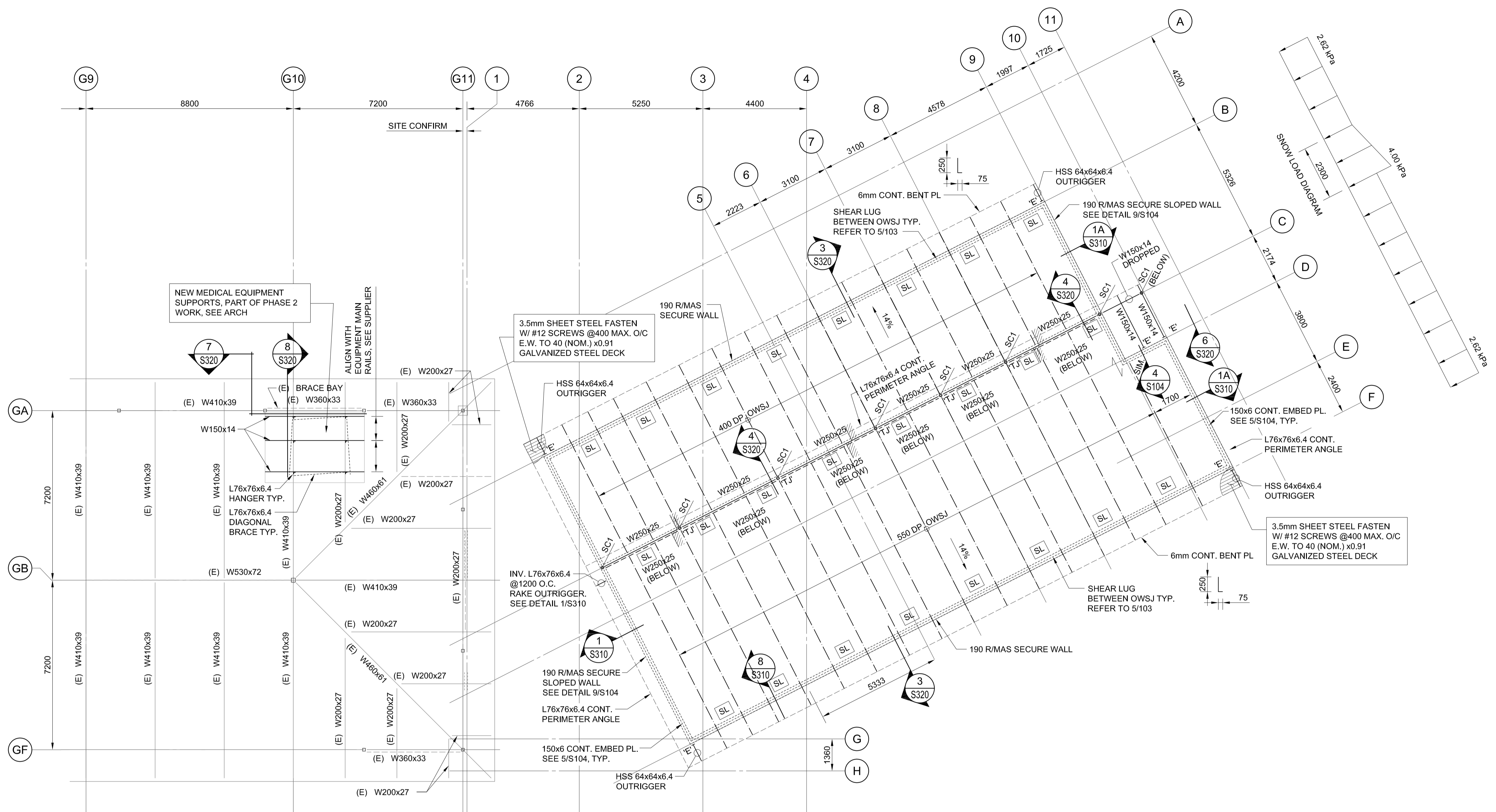
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 MSH

PSPC Project Manager/Administrateur de Projets SPAC
 TONY TANG

Regional Manager, Architectural and Engineering Services
 Gestionnaire régionale, Services d'architecture et de génie, SPAC
 PREETPAL PAUL

Drawing title/Titre du dessin
UPPER ROOF PLAN

Project No./No. du projet R.077724.001	Sheet/ Feuille S204 OF XX	Revision no./ La Révision no. 0
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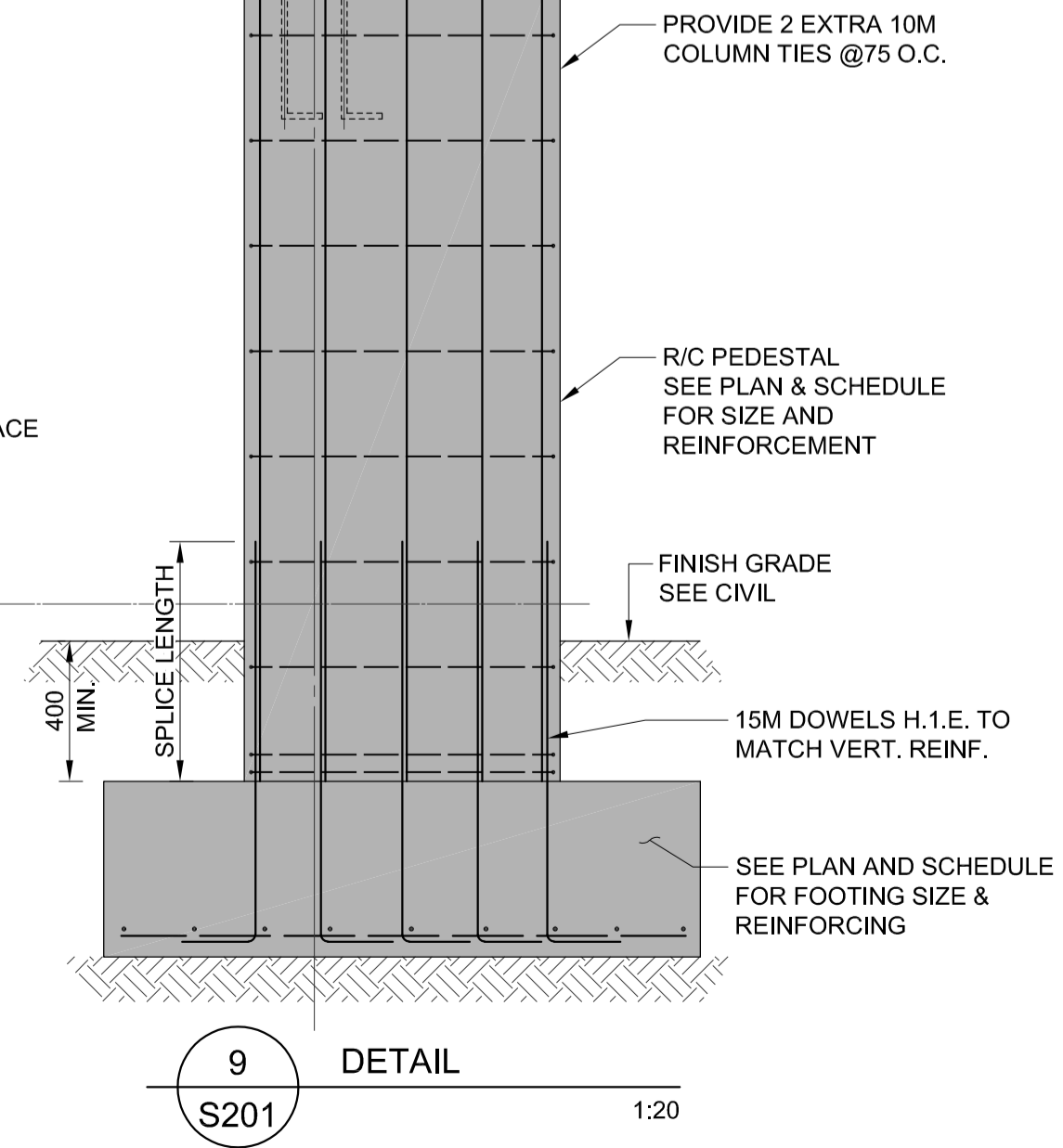
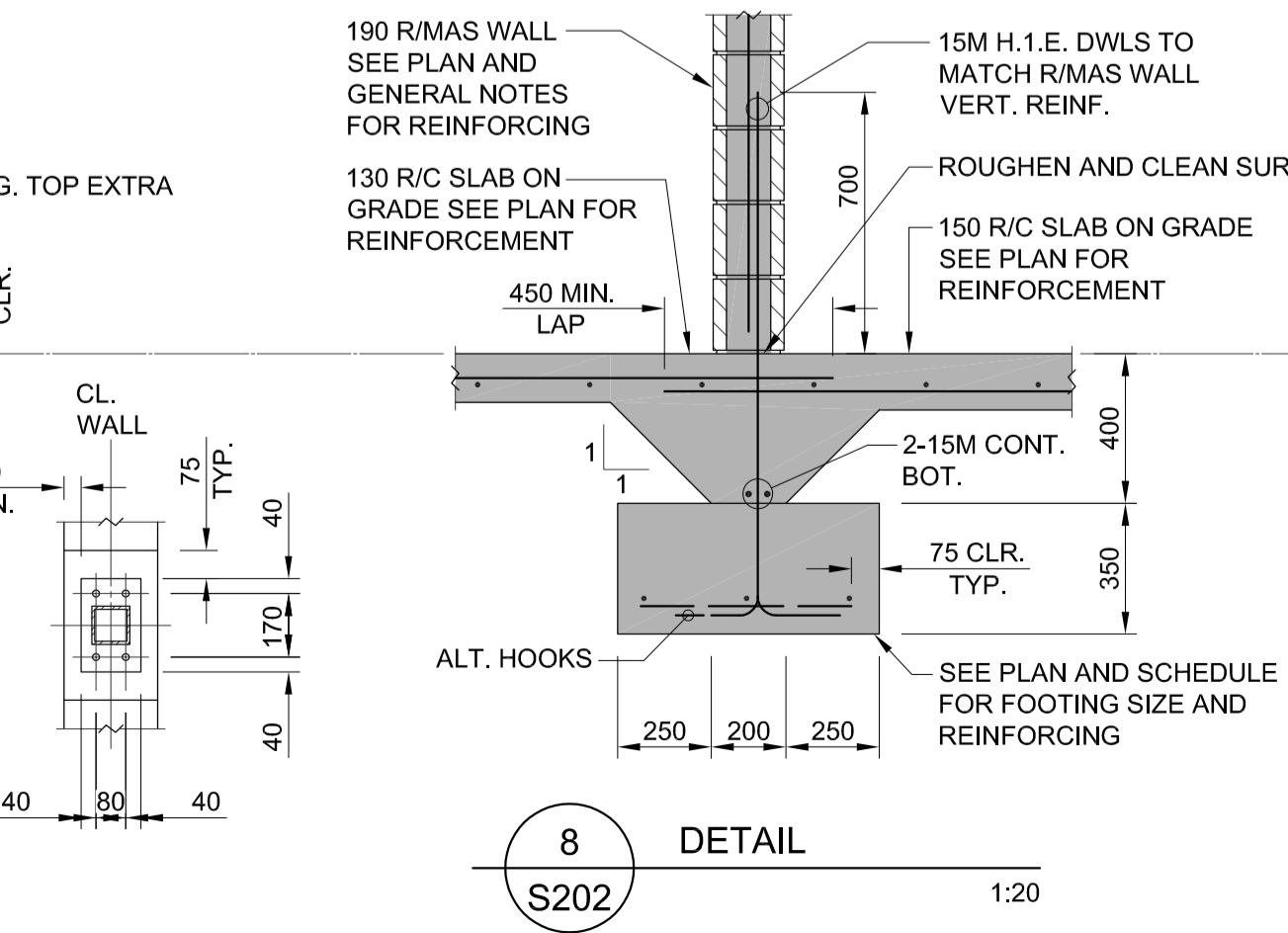
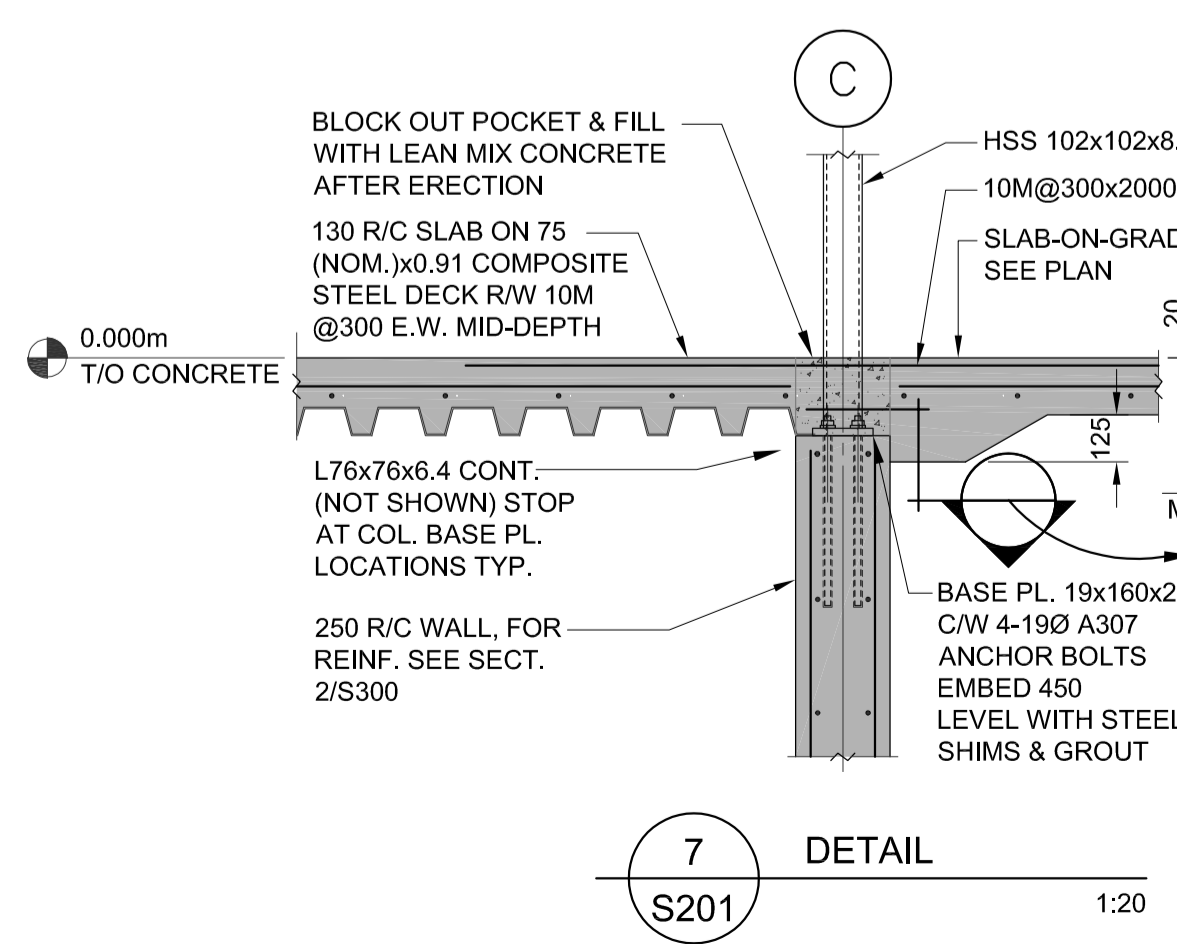
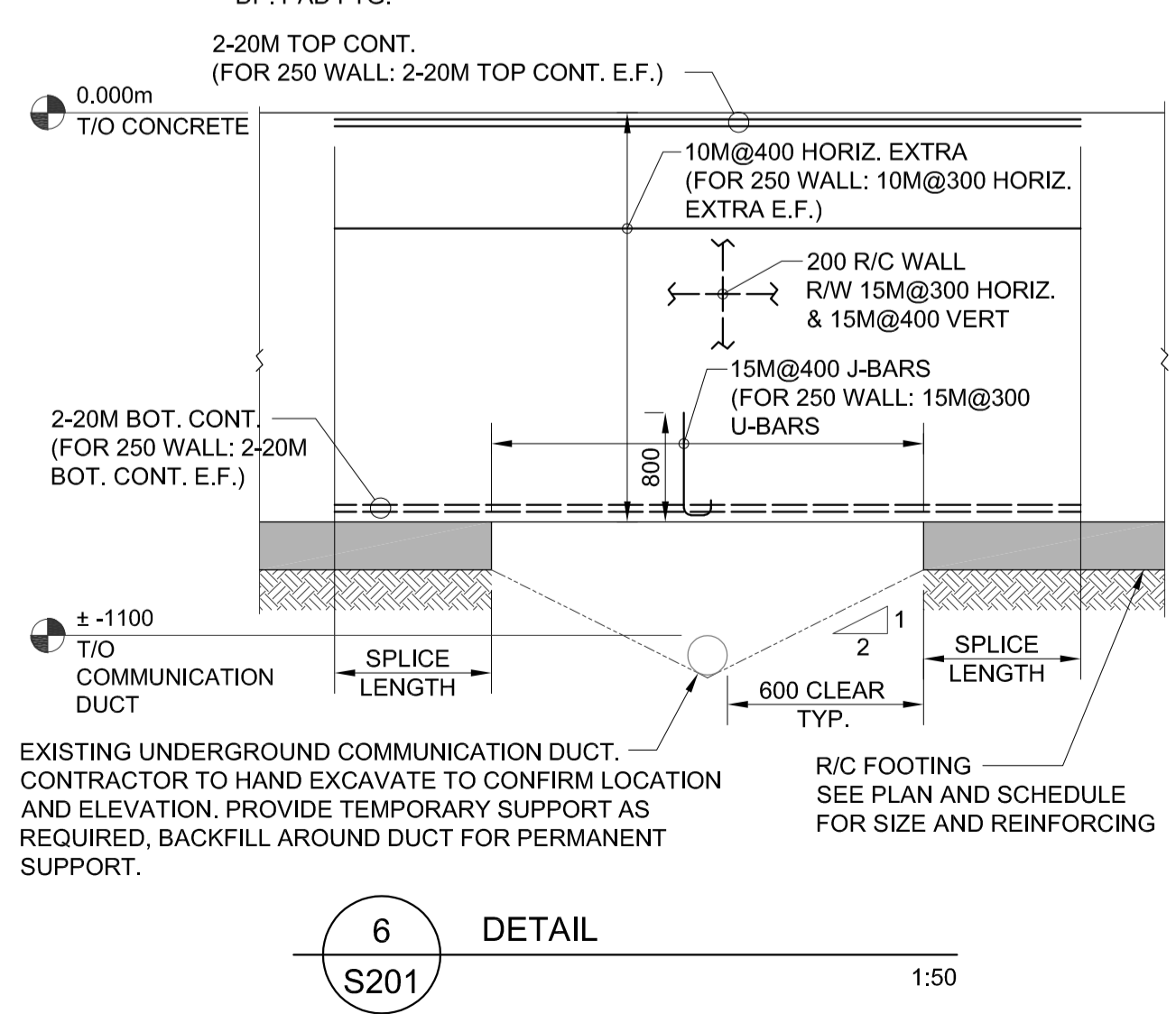
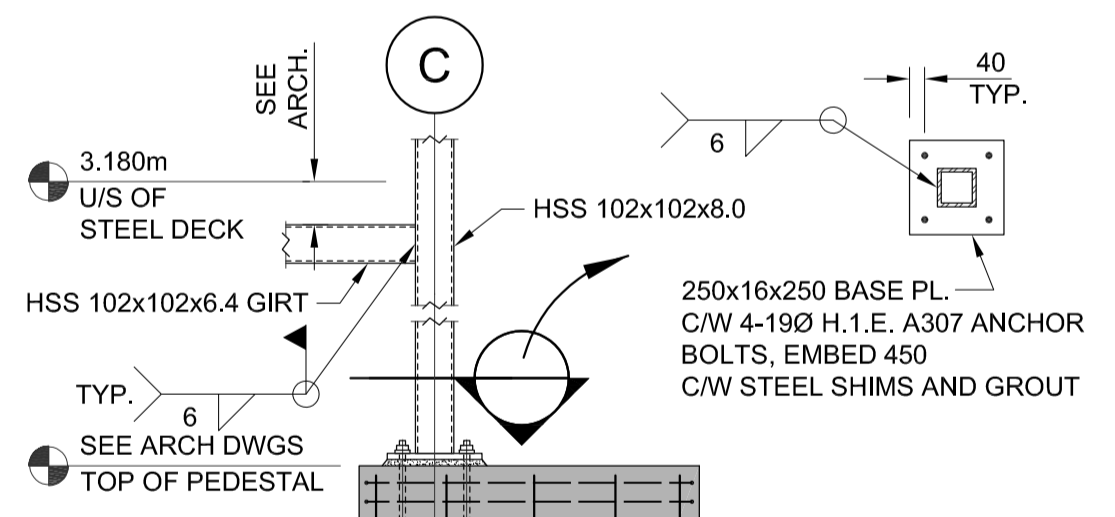
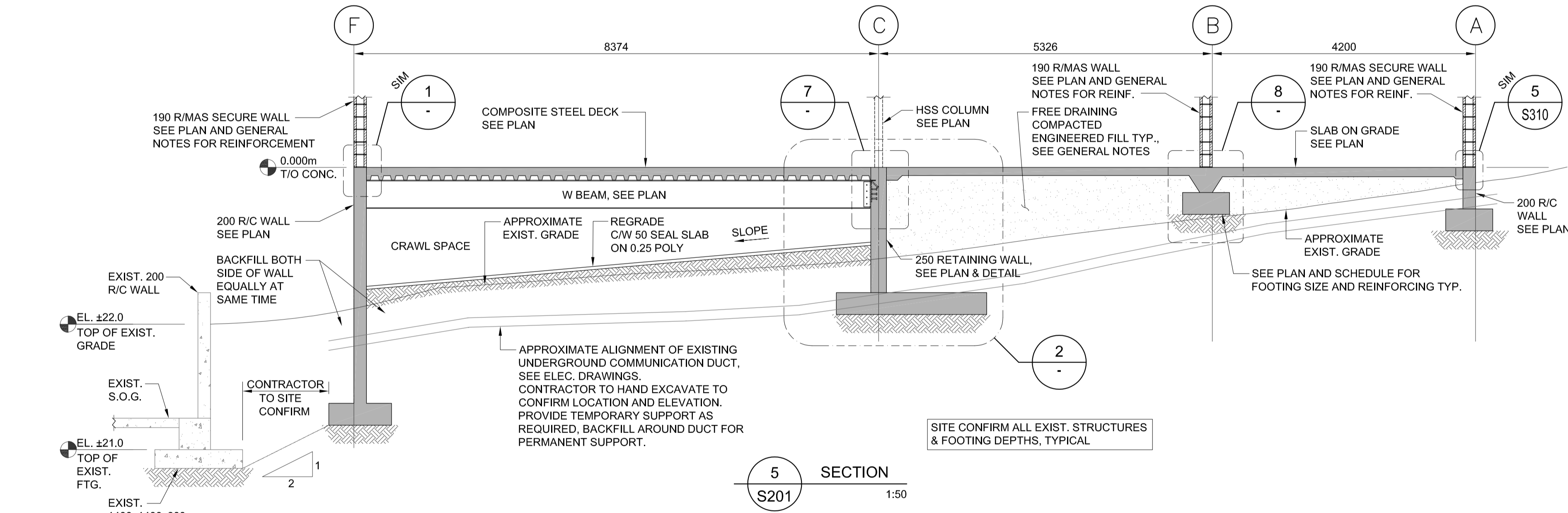
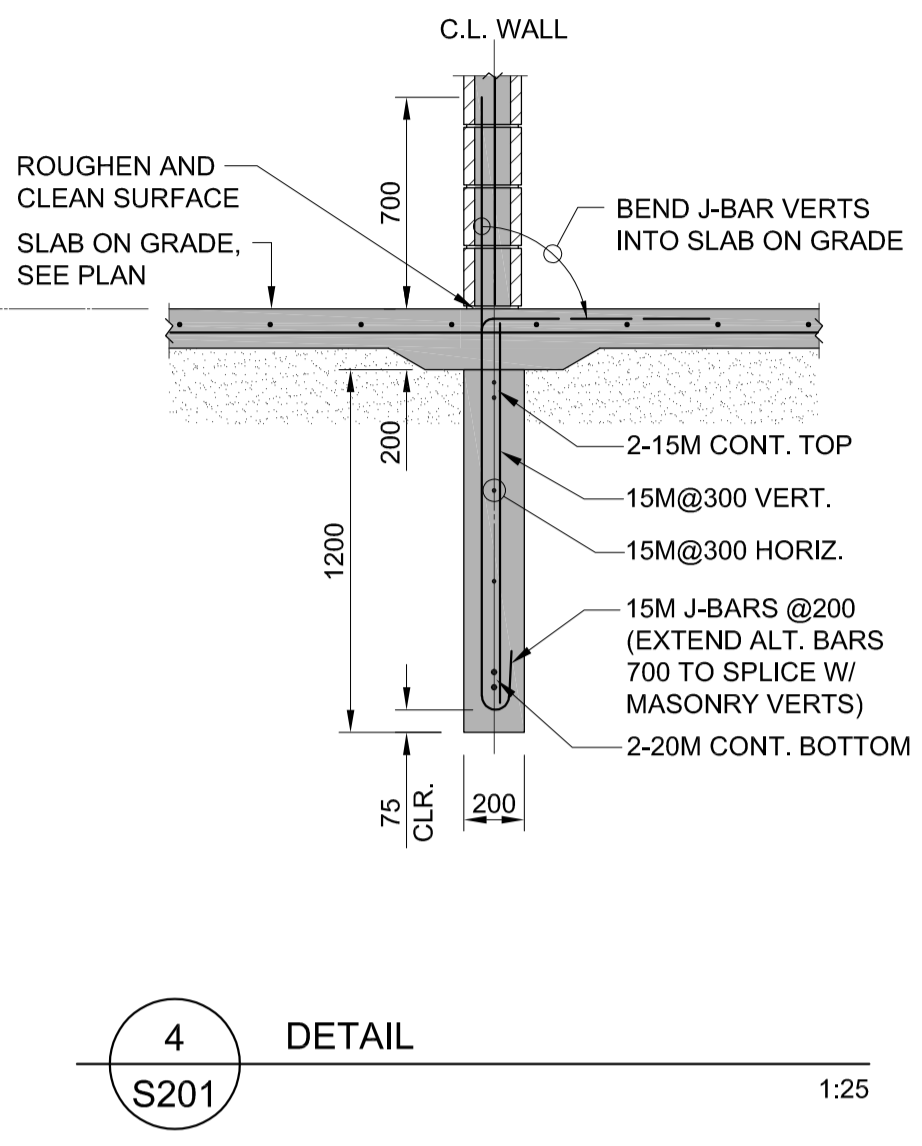
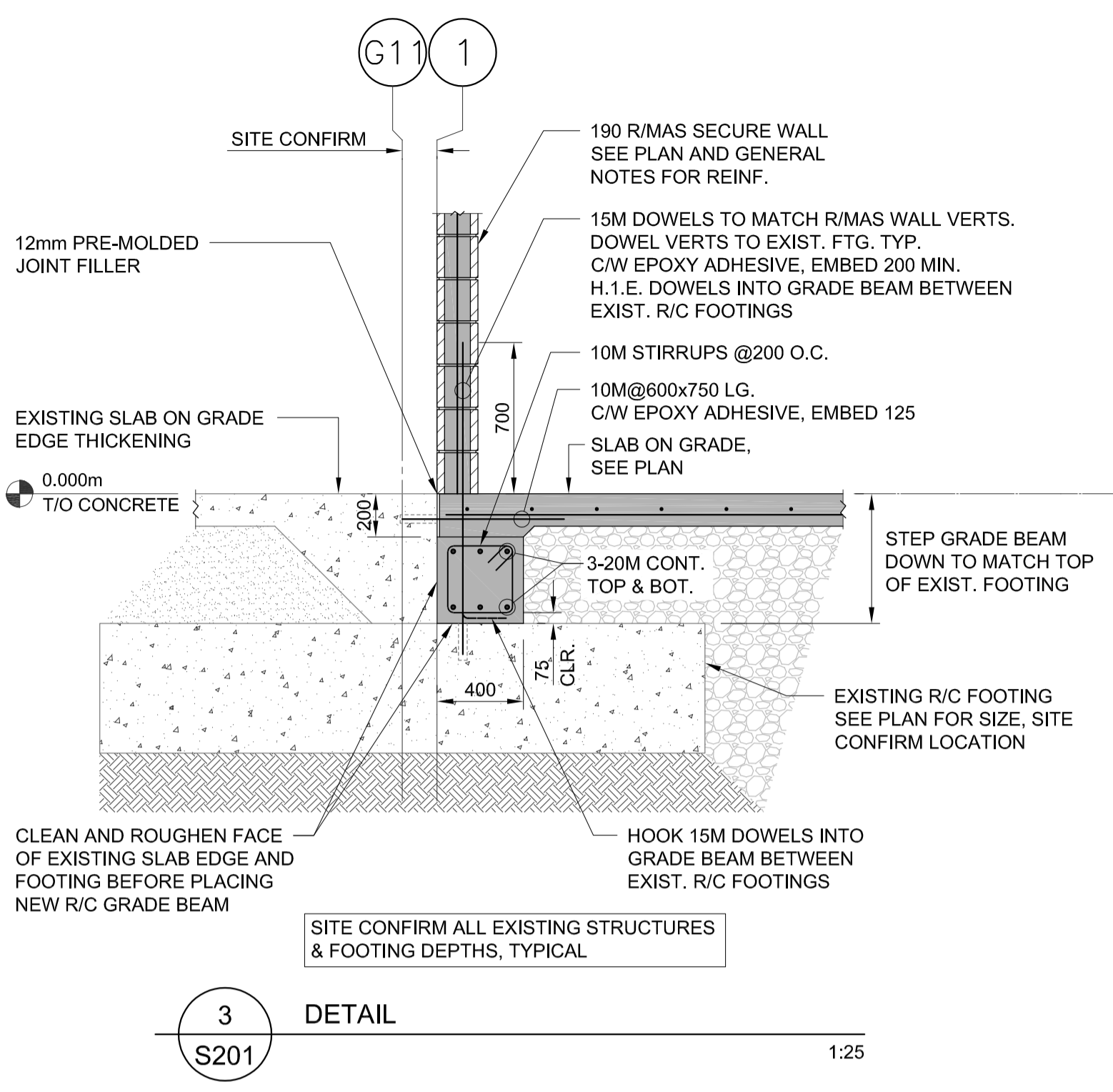
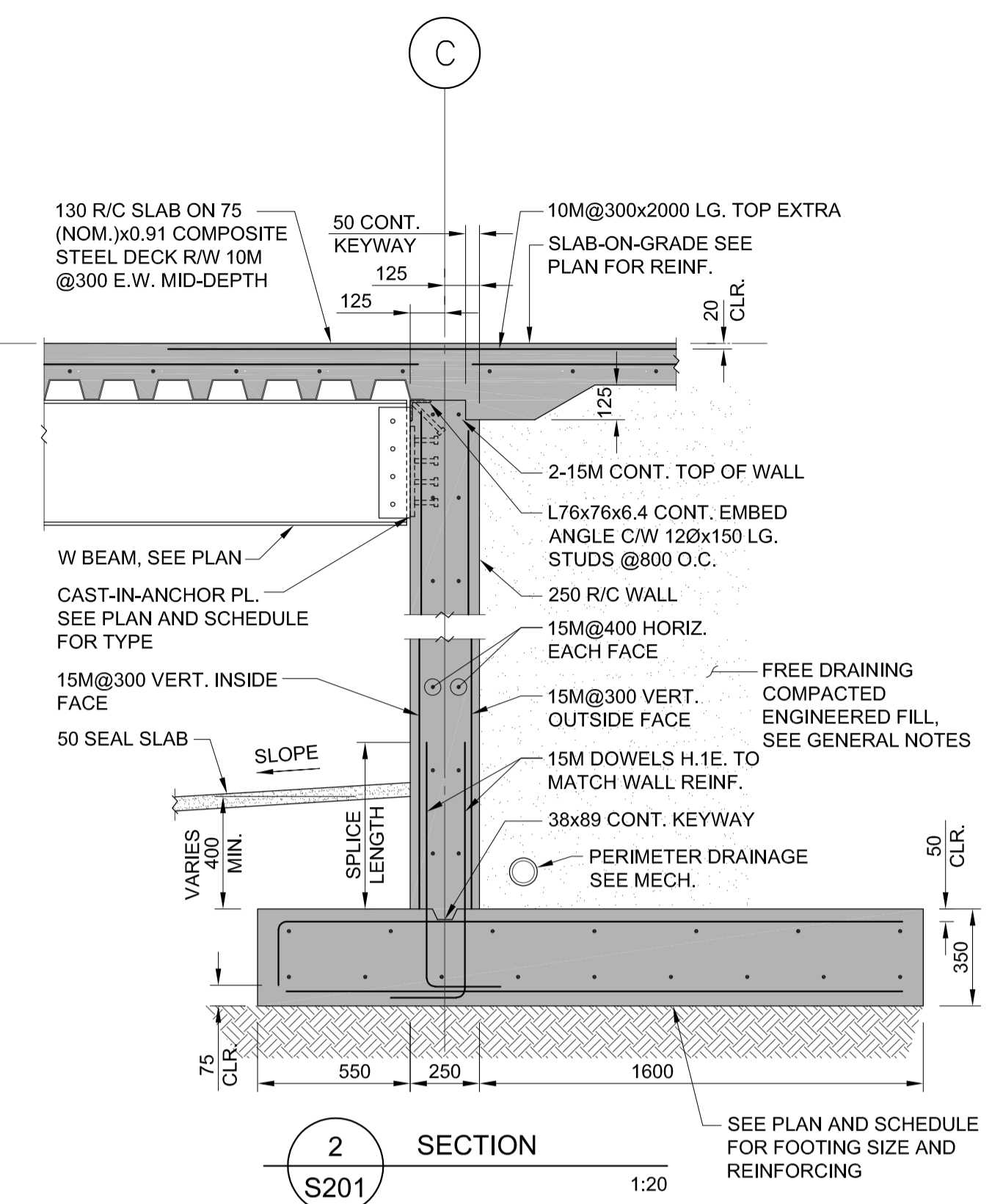
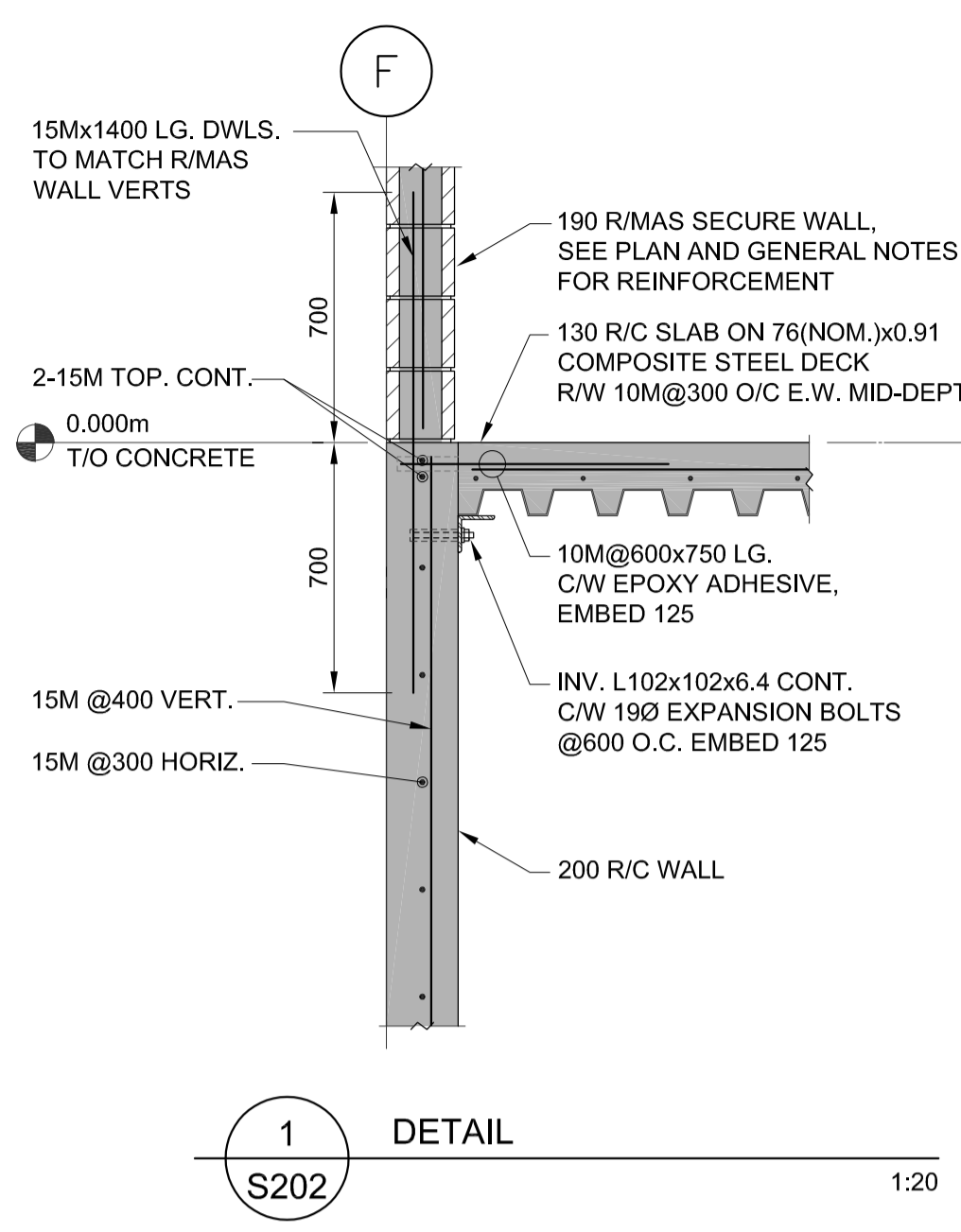
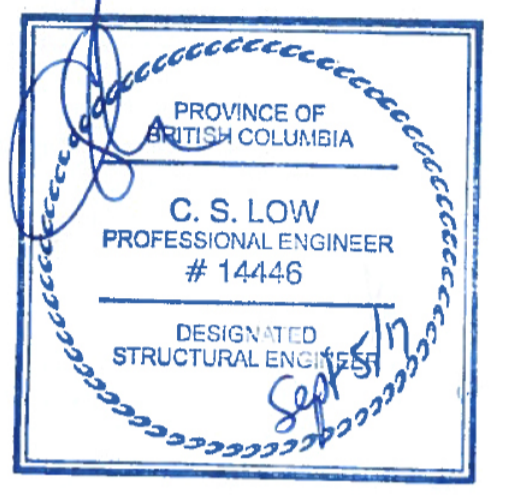


UPPER ROOF PLAN

1:100

- DRAWING NOTES:**
- FOR GENERAL NOTES & TYPICAL DETAILS, SEE DRAWING S100 SERIES.
 - [SL] SHEAR LUGS, SEE DETAIL 5/S103.
 - ← INDICATES BEAM CONNECTION FACTORED AXIAL LOADS OF $C_F = T_F = 100\text{KN}$ ($R_F R_C = 1.3$) U.N.O.
 - 'A' - DENOTES CAST-IN ANCHOR PLATE, SEE SCHEDULE ON S103.
 - 'TJ' - DENOTES BOTTOM CHORD EXTENSION.
 - ◀ CJ - DENOTES CONTROL JOINT.

STEEL COLUMN SCHEDULE	
TYPE	SIZE
SC1	HSS 102x102x8.0



Revision/	Description/Description	Date/Date
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4		
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0	ISSUED FOR TENDER	17/09/01

CORRECTIONAL SERVICE OF CANADA

Project title/Titre du projet
AGASSIZ, BRITISH COLUMBIA
4732 CEMETERY ROAD PO BOX 1600
MOUNTAIN INSTITUTE

MOUNTAIN INSTITUTE
HEALTH CARE EXPANSION

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Drawn by/Dessine par
MSH

PWSC Project Manager/Administrateur de Projets TPSPC
TONY TANG

Regional Manager, Architectural and Engineering Services
Gestionnaire régionale, Services d'architecture et de génie, TPSPC
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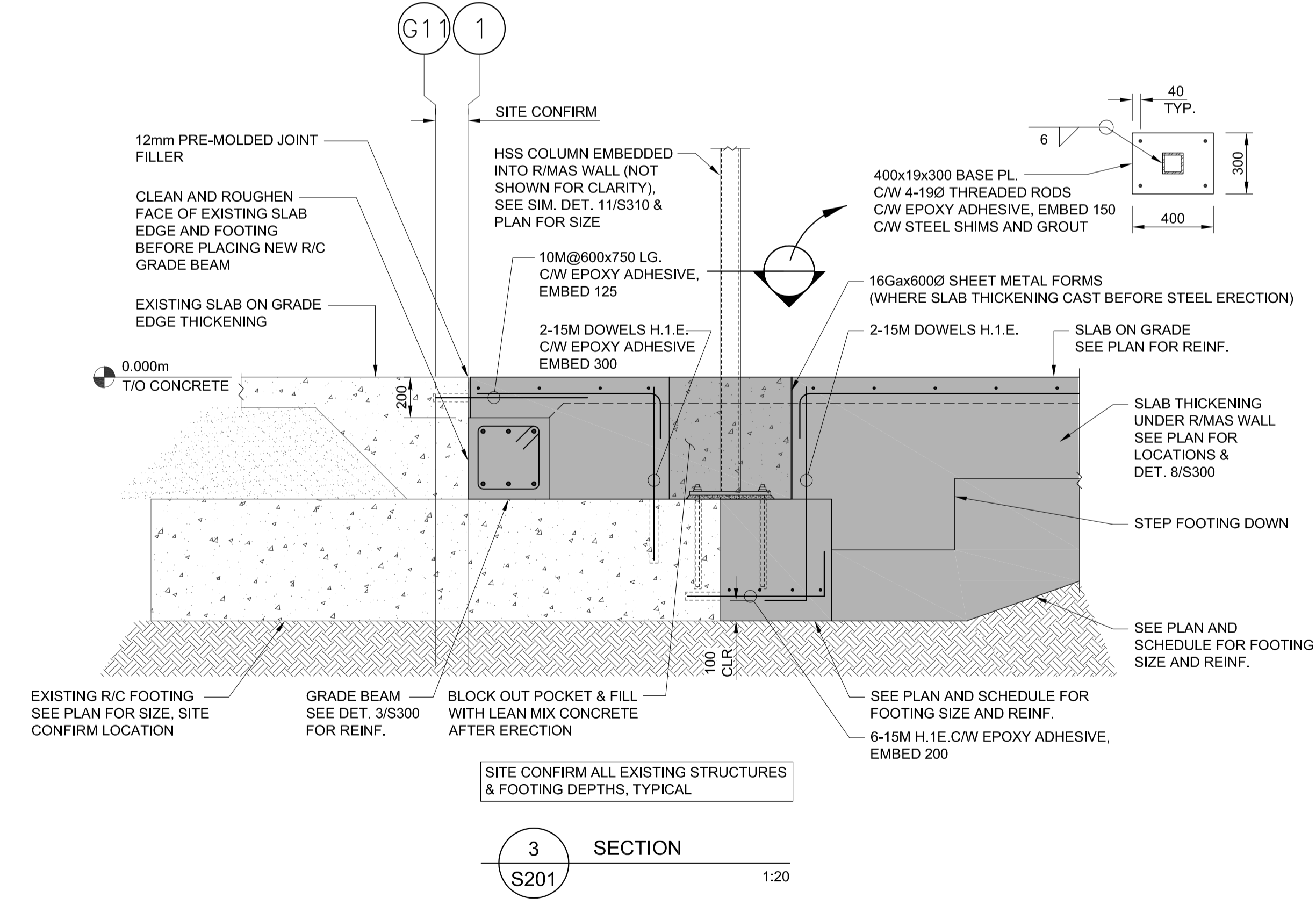
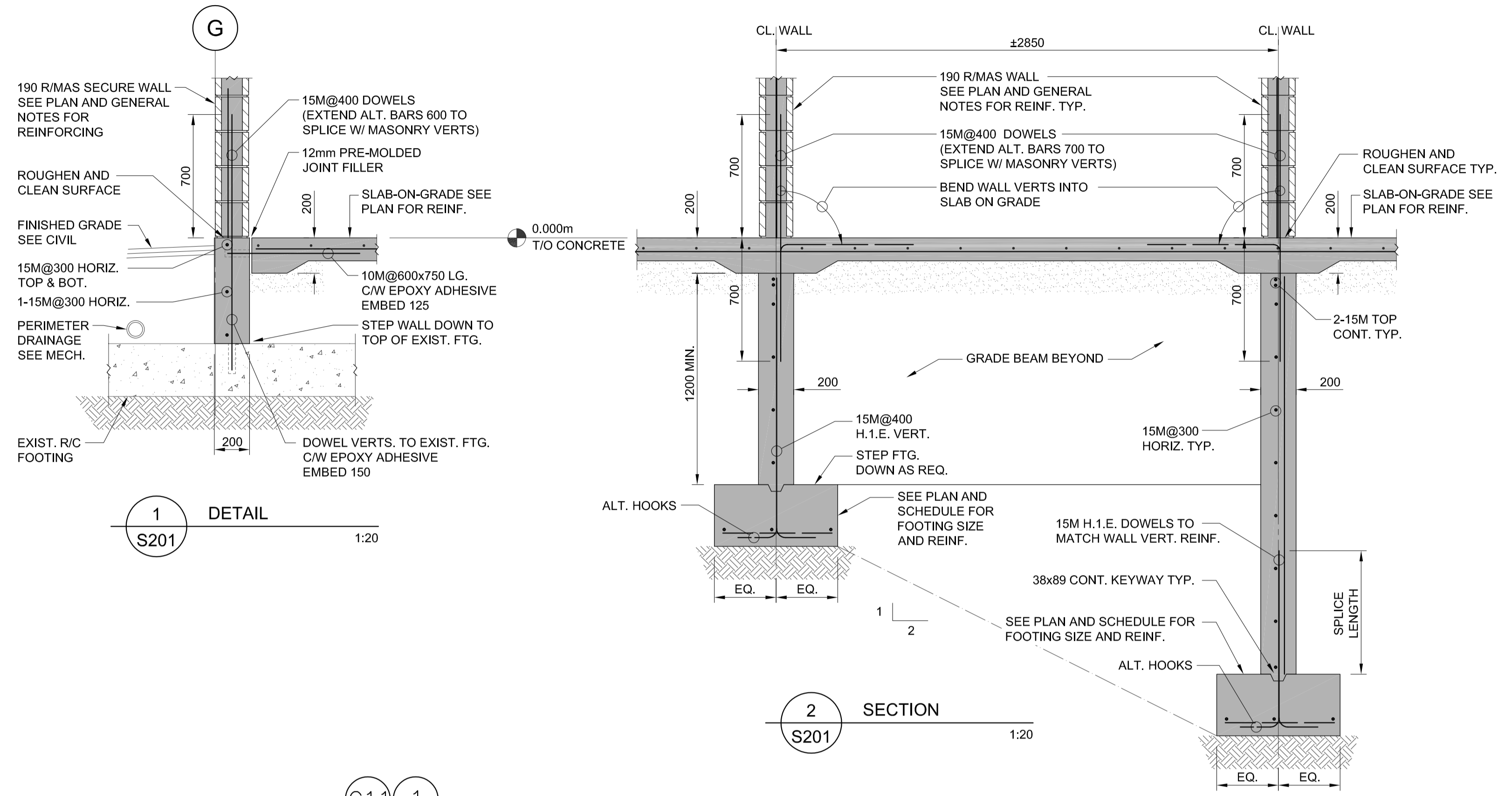
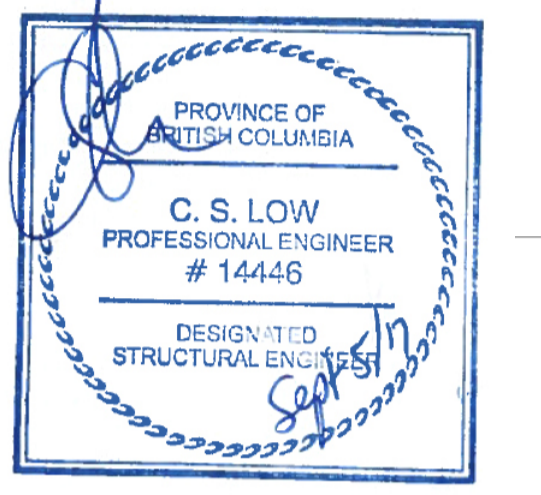
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SECTIONS AND DETAILS
SHEET 1

Project No./No. du projet
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S300
OF XX

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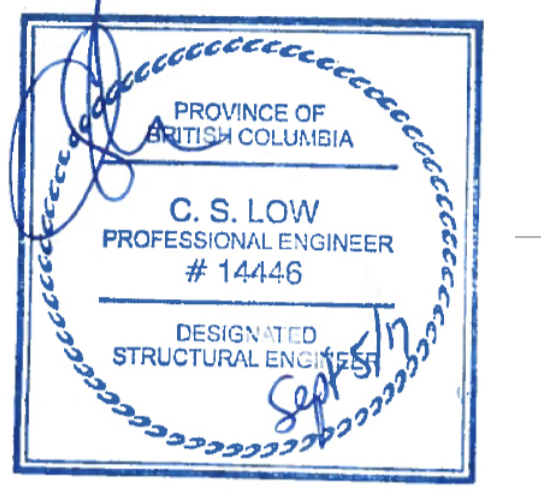
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4732 CEMETERY ROAD PO BOX 1600
MOUNTAIN INSTITUTE**

**MOUNTAIN INSTITUTE
HEALTH CARE EXPANSION**

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CSL
Drawn by/Dessine par
MSH
PWGSC Project Manager/Administrateur de Projets TPSSG
TONY TANG
Regional Manager, Architectural and Engineering Services
Gestionnaire régionale, Services d'architectural et de génie, TPSSG
PREETIPAL PAUL

Drawing title/Titre du dessin
**SECTIONS AND DETAILS
SHEET 2**

Project No./No. du projet: **R.077724.001**
Sheet/Feuille: **S301**
Revision no./La Révision no.: **0**
OF XX



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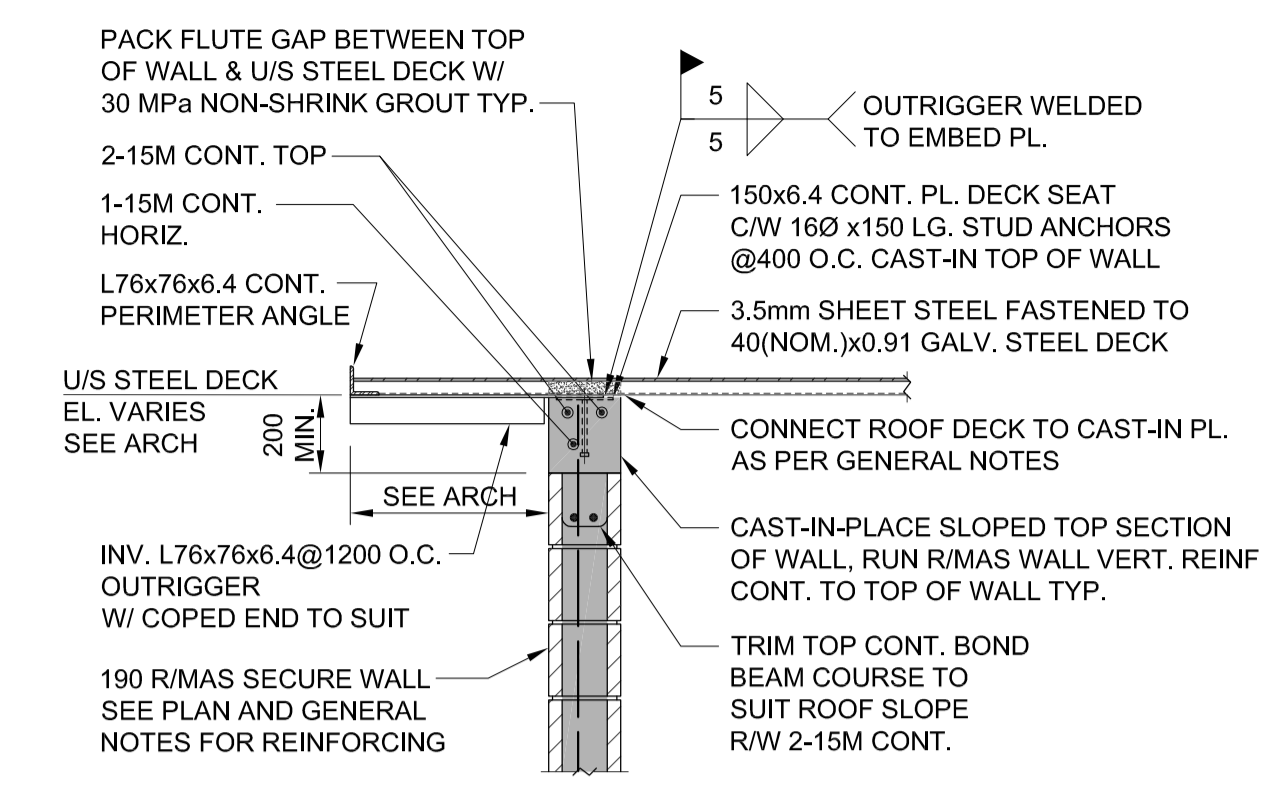
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4732 CEMETERY ROAD PO BOX 1600
MOUNTAIN INSTITUTE

MOUNTAIN INSTITUTE HEALTH CARE EXPANSION

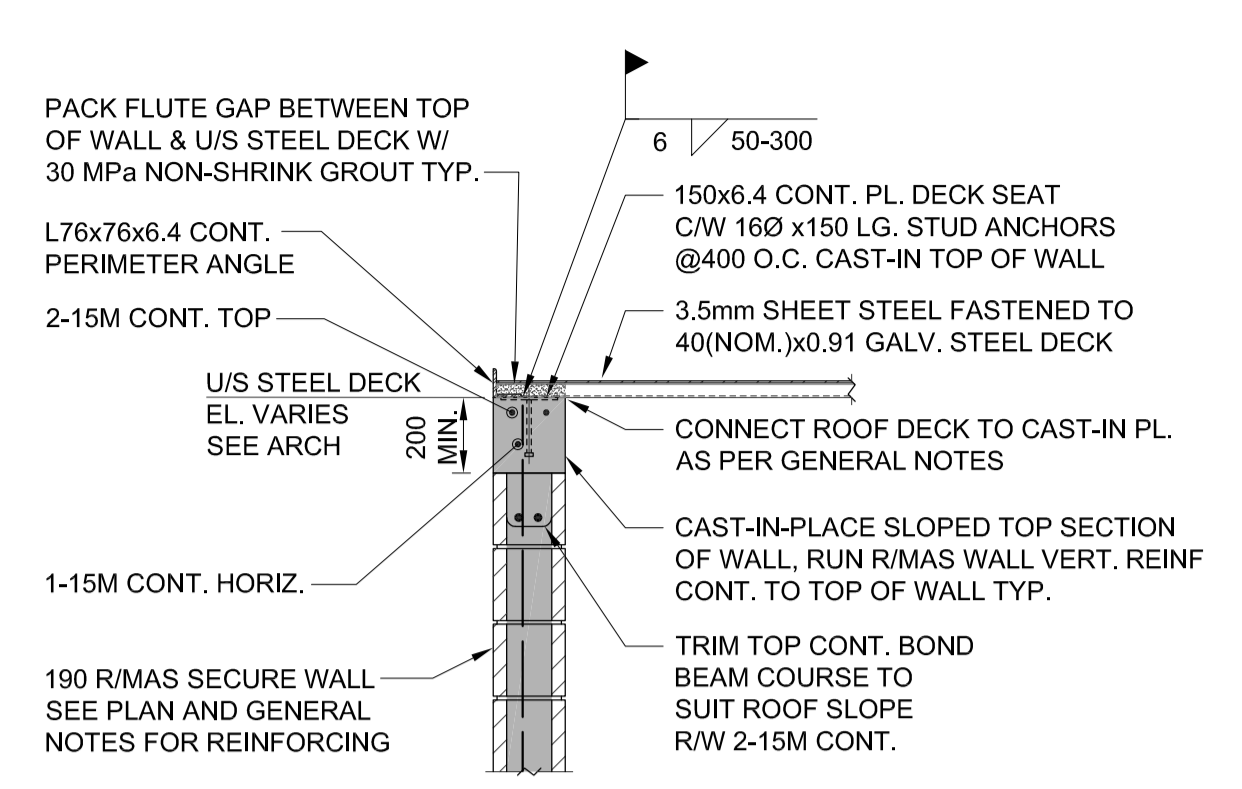
Consultant Signature Only
Designed by/Concept par
CSL
Drawn by/Dessine par
MSH
PWSC Project Manager/Administrateur de Projets TPSCG
TONY TANG
Regional Manager, Architectural and Engineering Services
Gestionnaire régionale, Services d'architectural et de génie, TPSCG
PREETIPAL PAUL
Drawing title/Titre du dessin

SECTIONS AND DETAILS SHEET 3

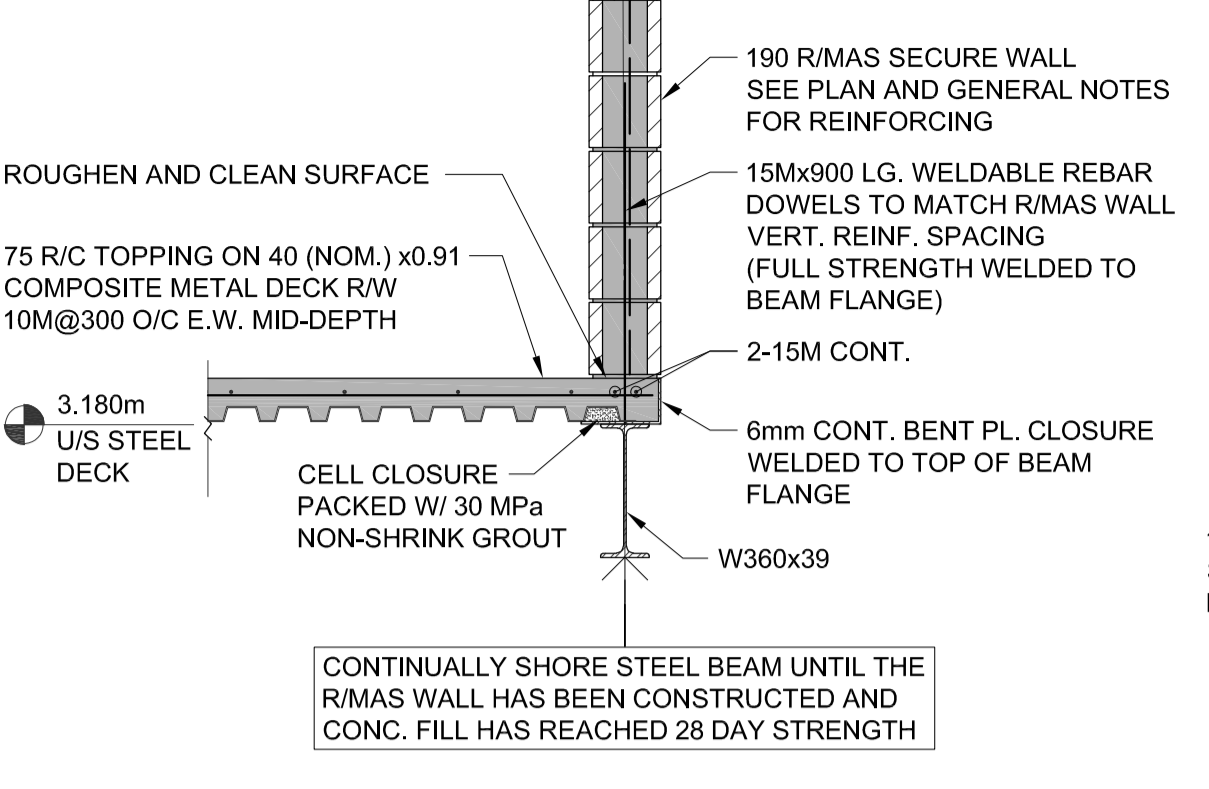
Project No./No. du projet
R.077724.001
Sheet/Feuille
S310
Revision no./La Révision no.
0
OF XX



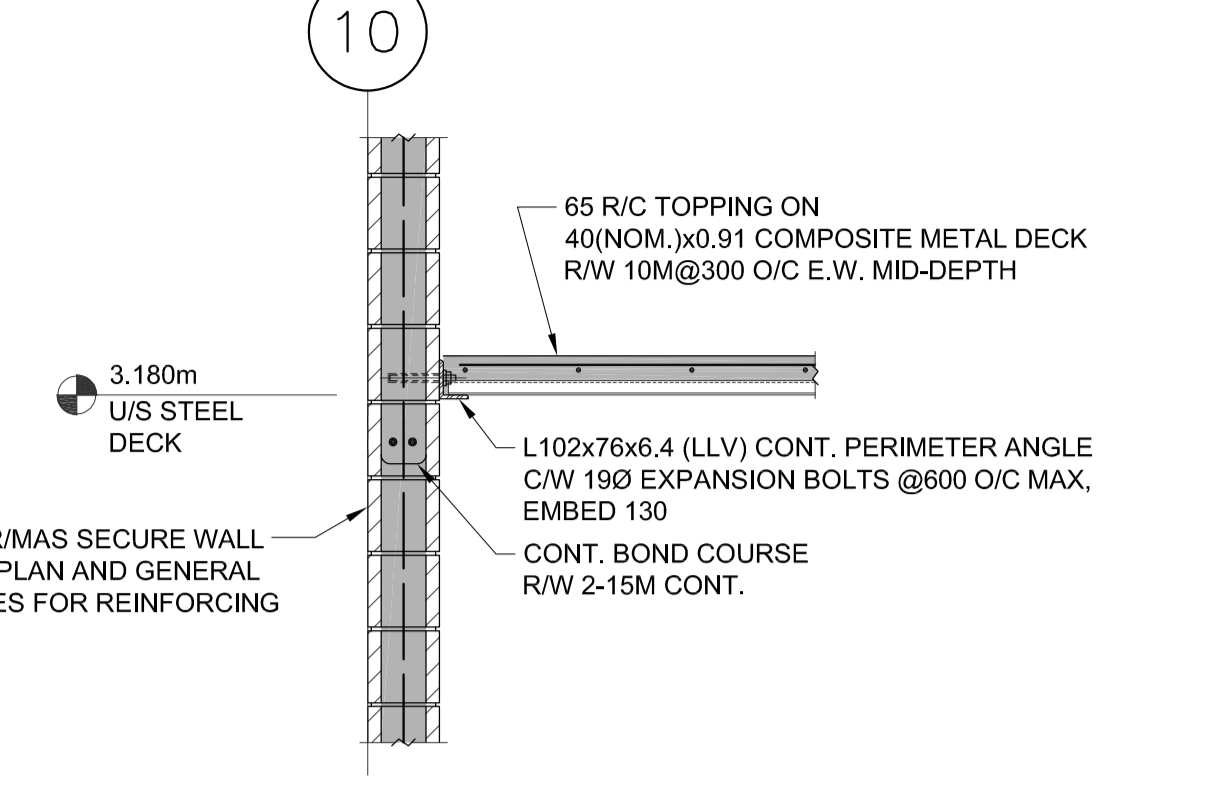
1 DETAIL
S204 1:20



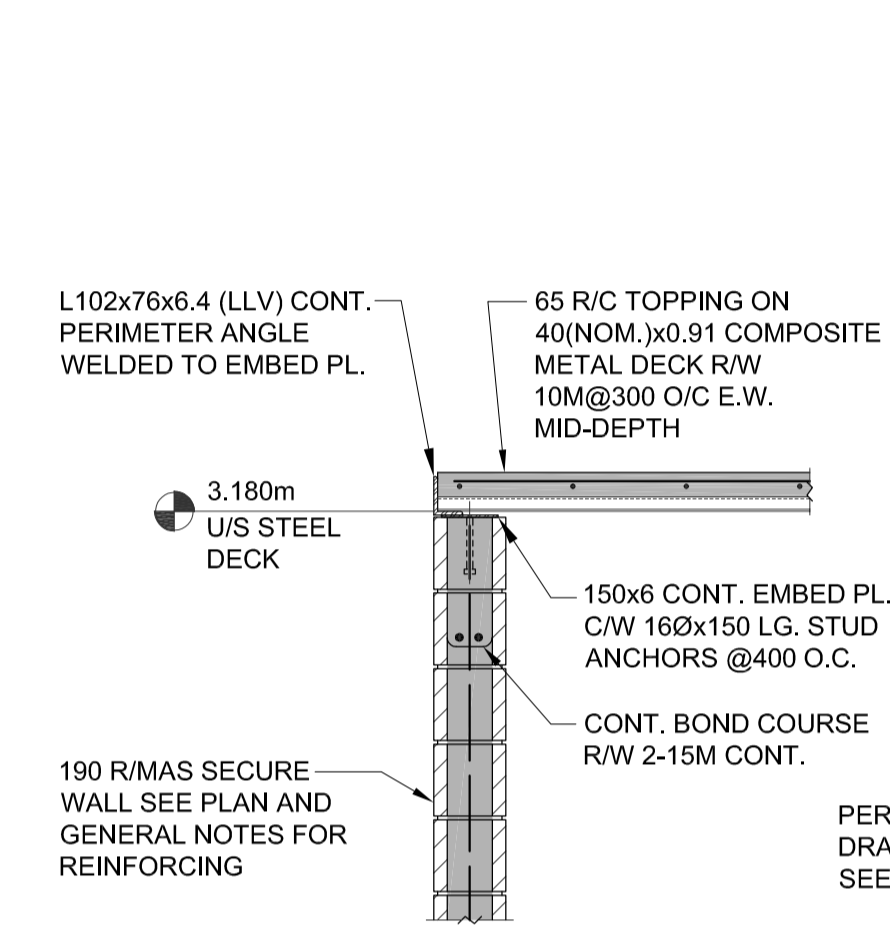
1A DETAIL
S204 1:20



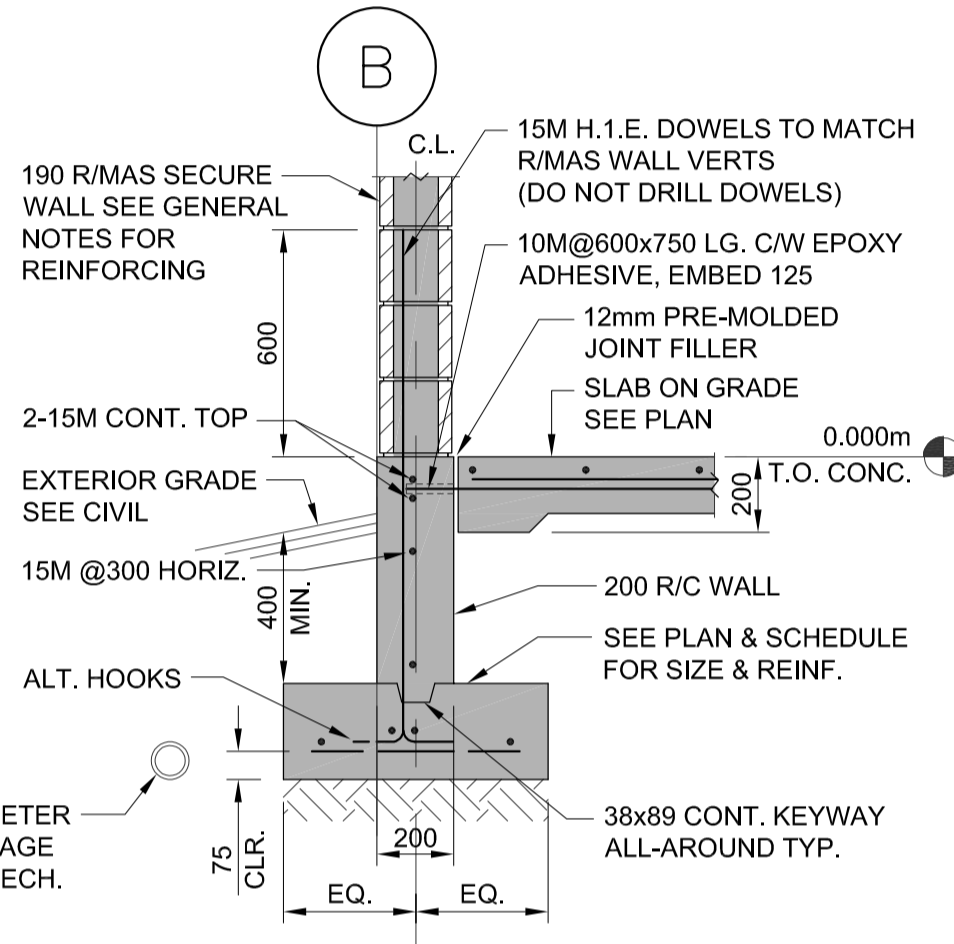
2 DETAIL
S203 1:20



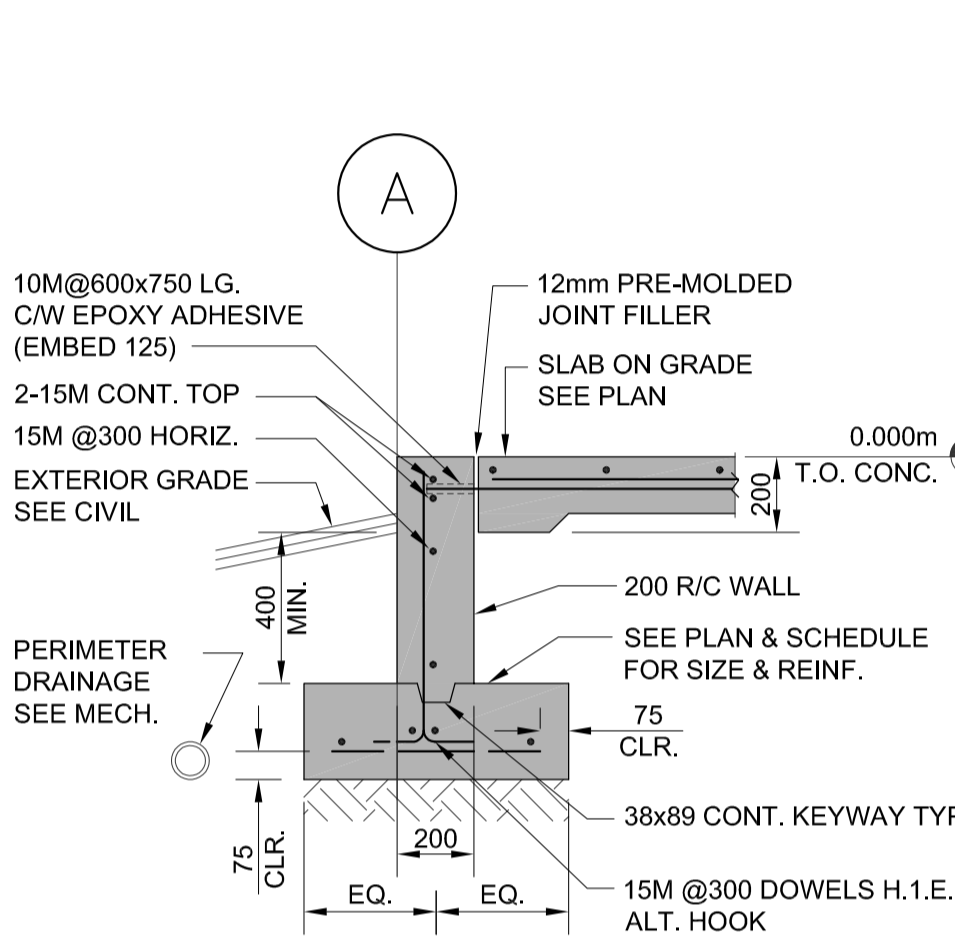
3 DETAIL
S204 1:20



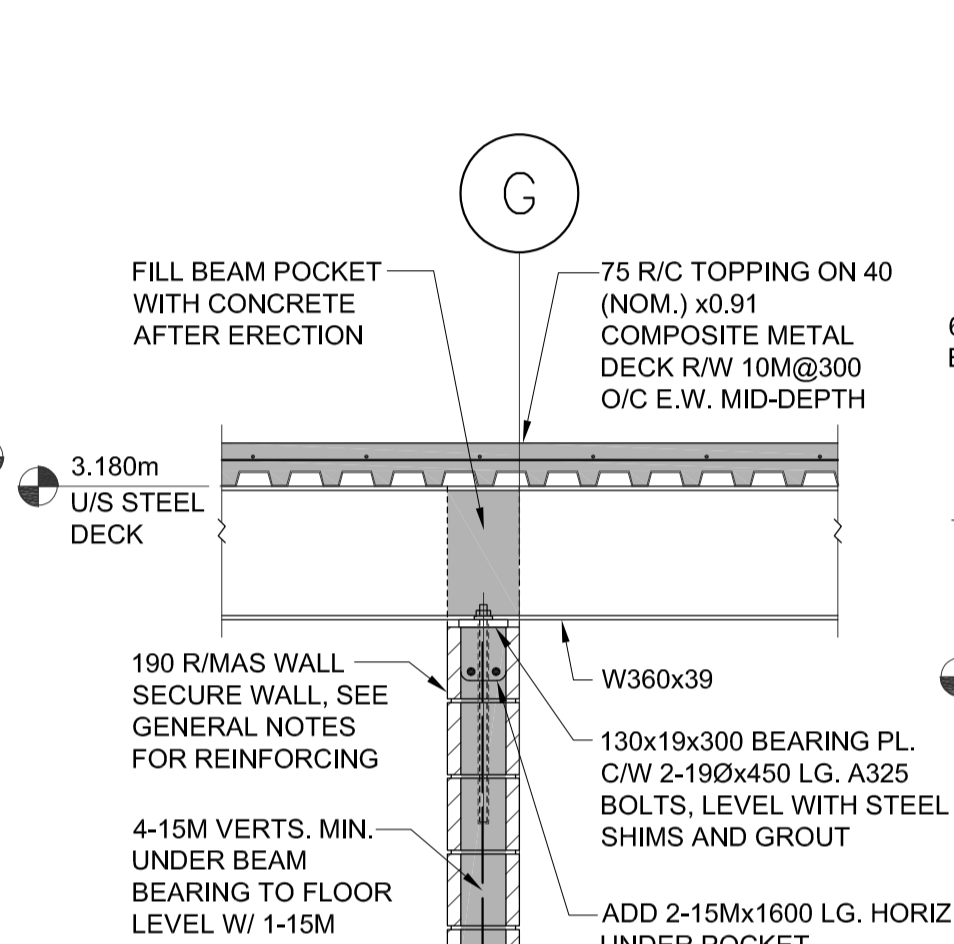
4 DETAIL
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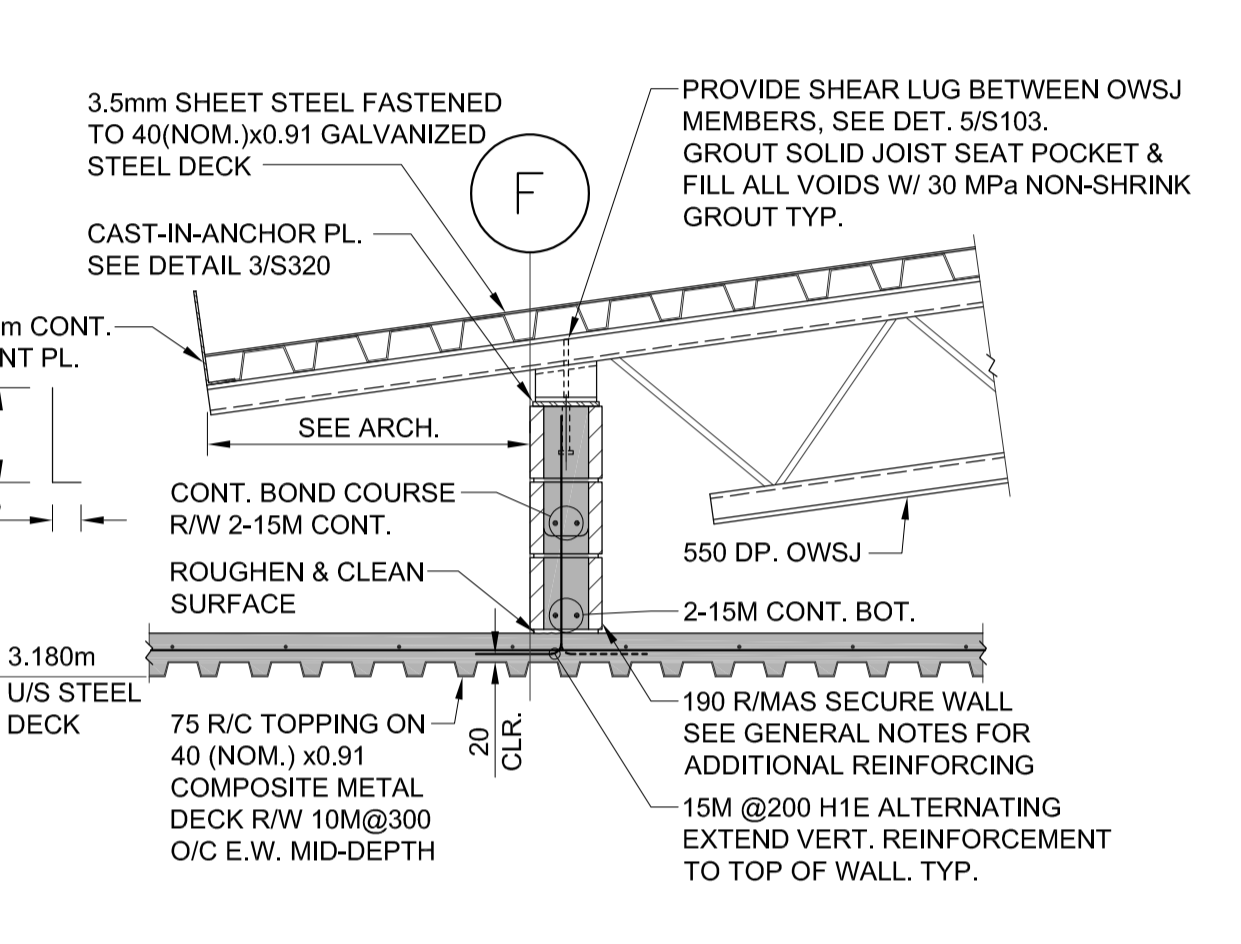
5 DETAIL
S202 1:20



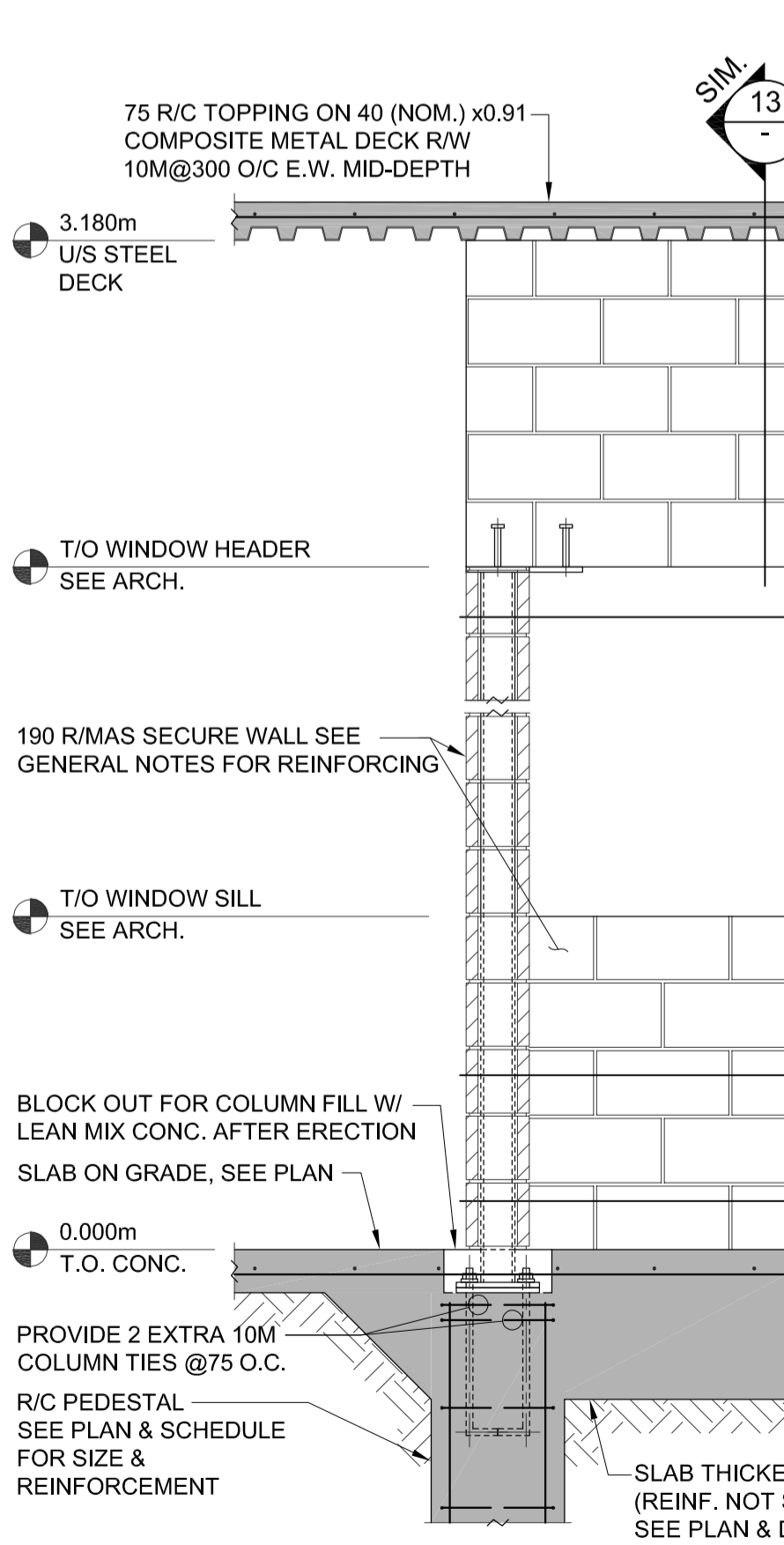
6 DETAIL
S202 1:20



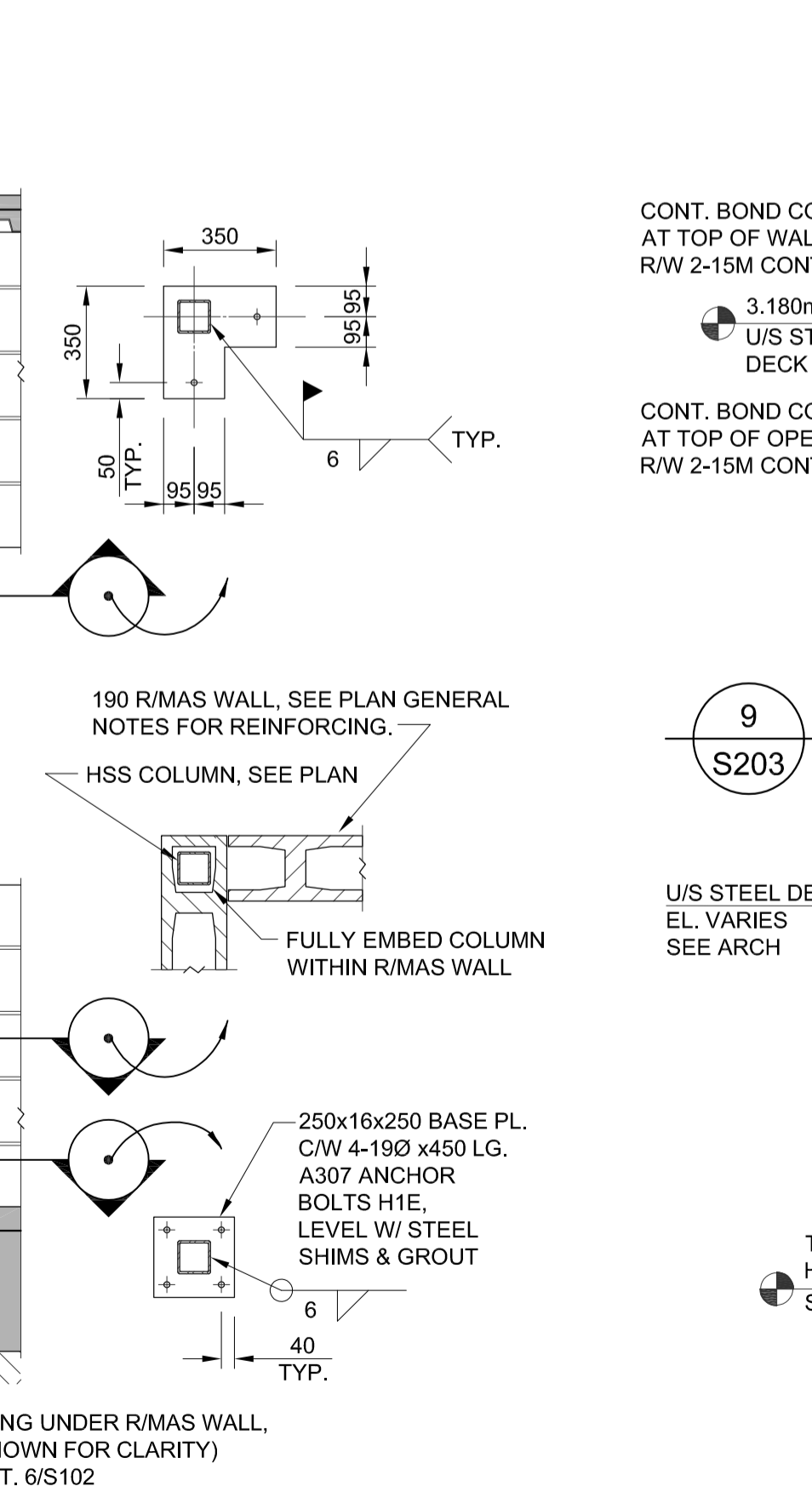
7 DETAIL
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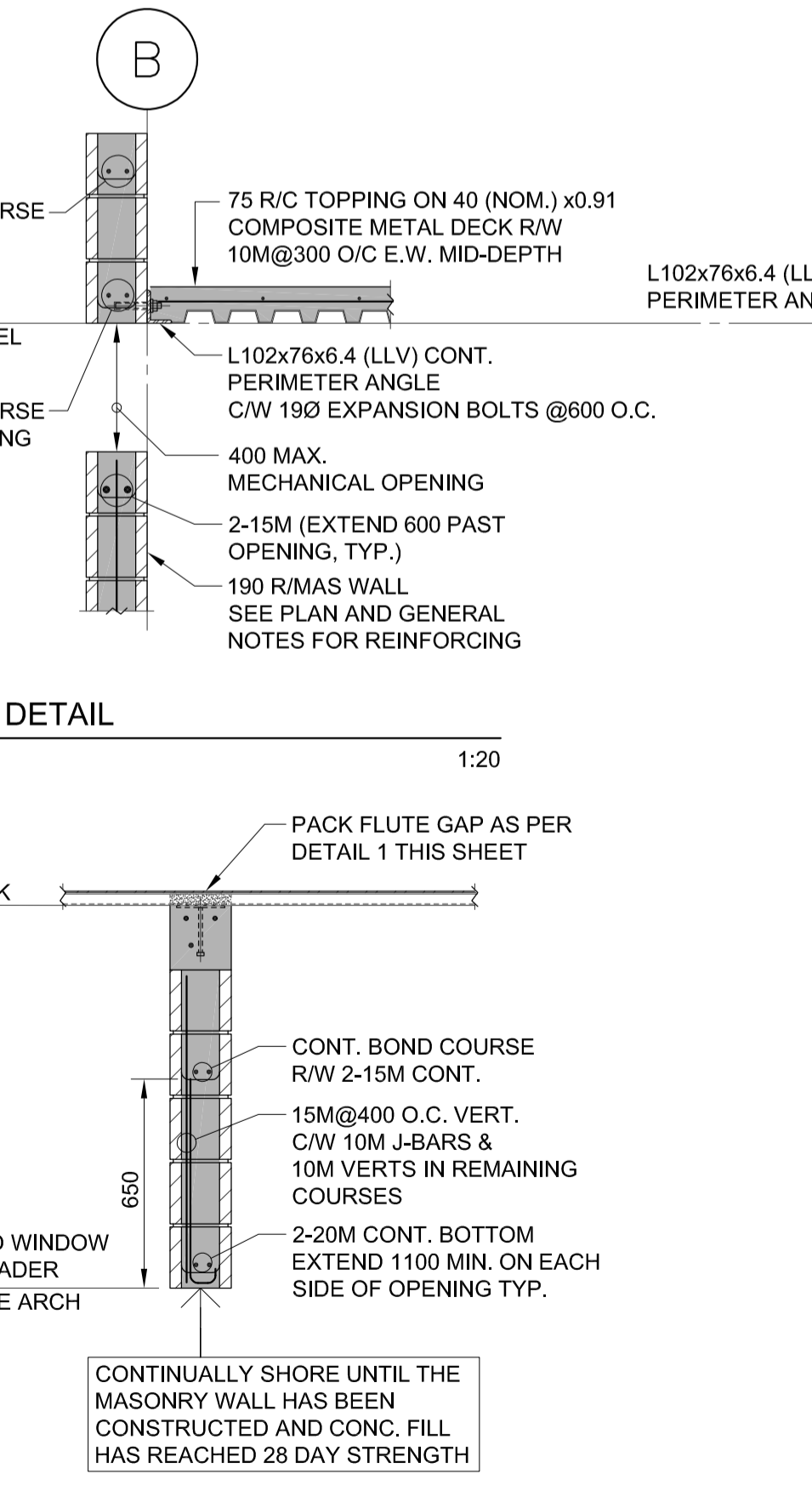
8 DEATAIL
S203 1:20



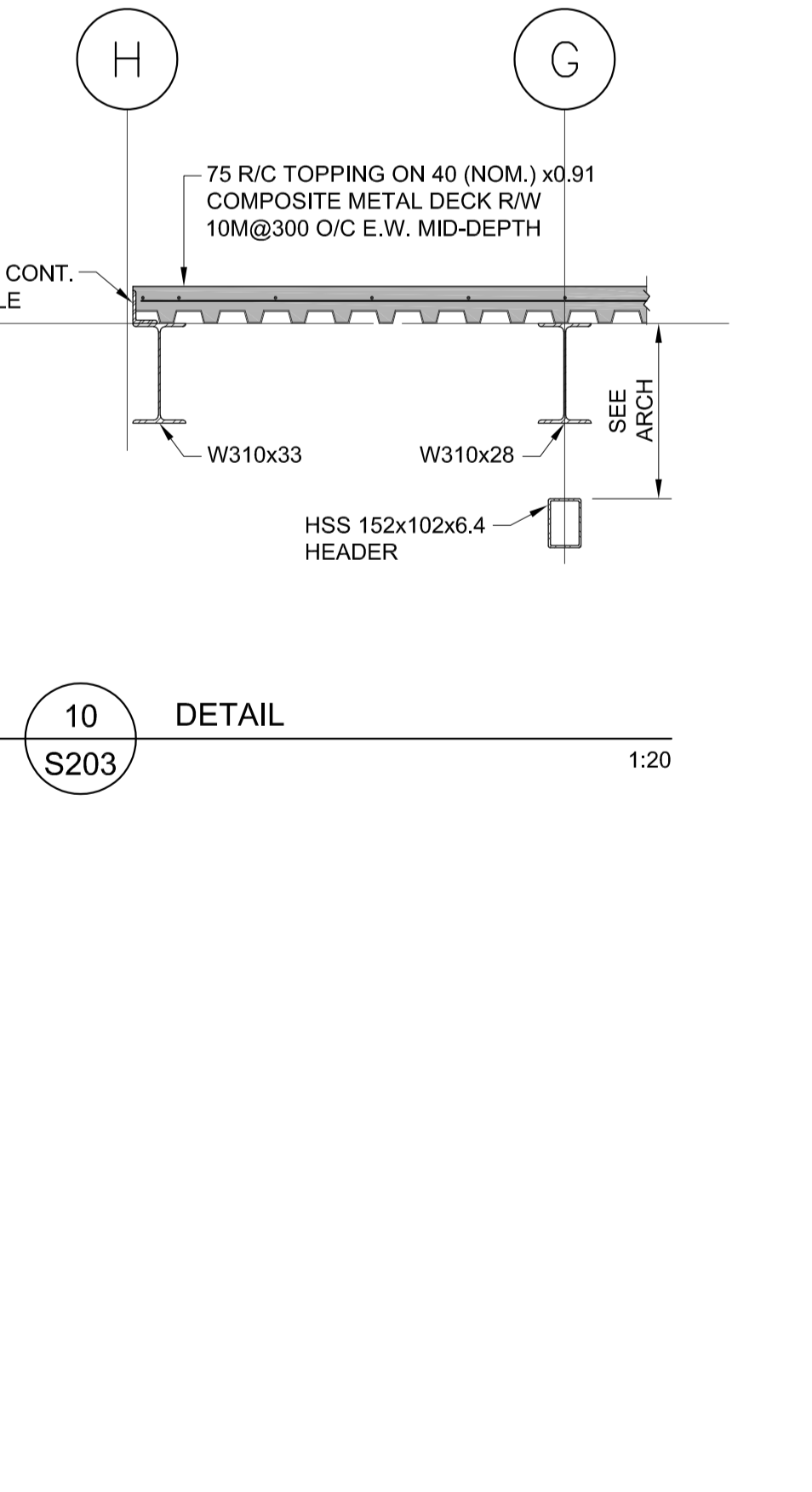
12 SECTION
S203 1:20



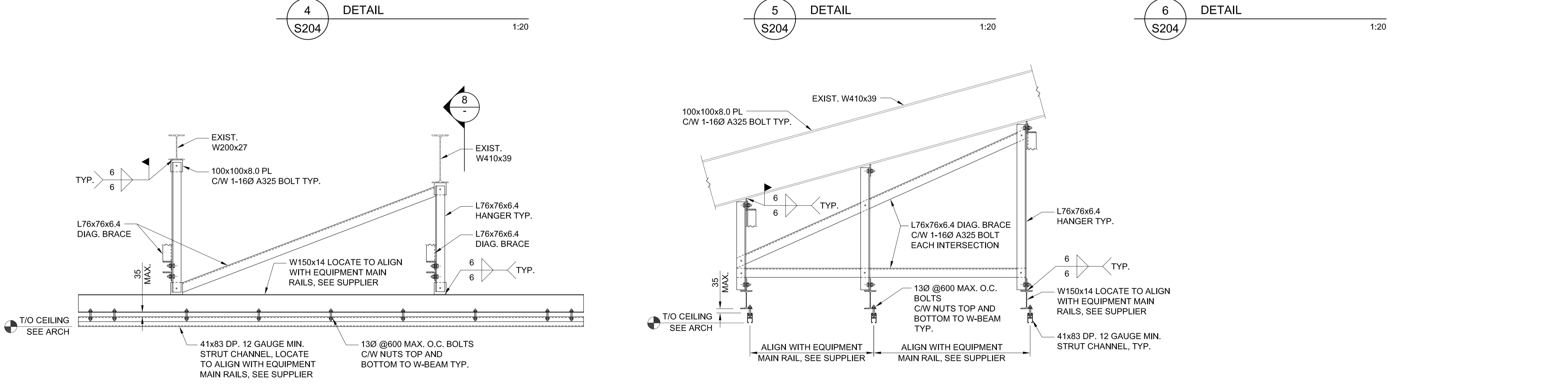
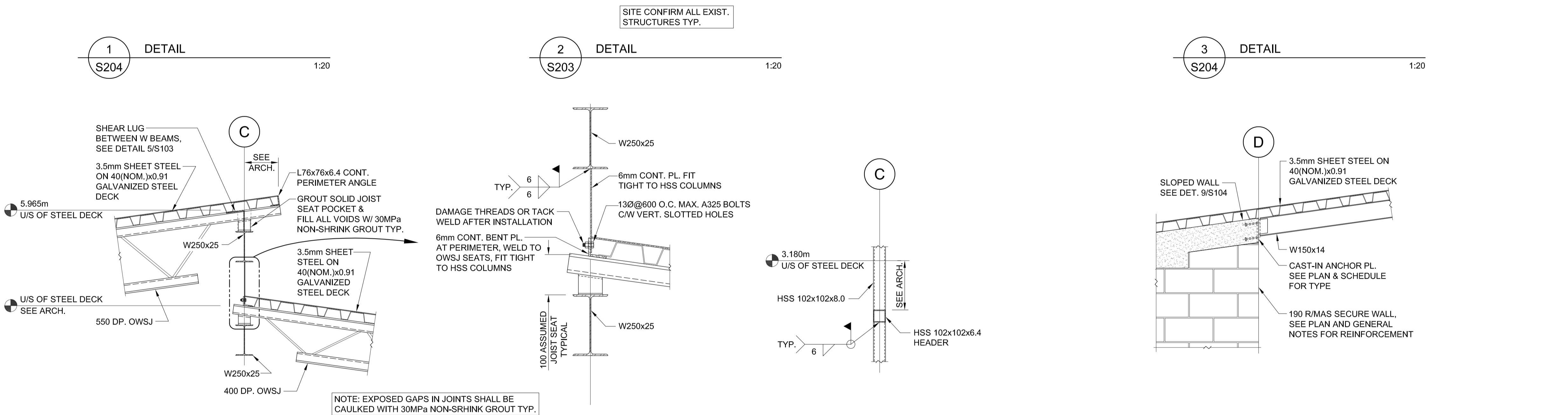
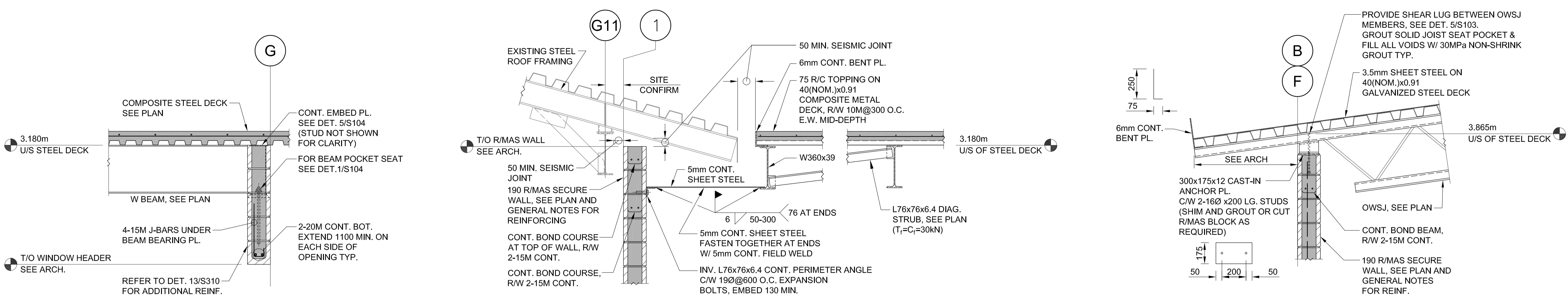
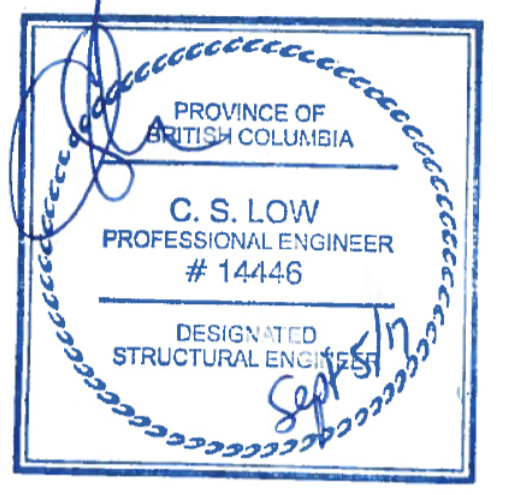
9 DETAIL
S203 1:20



10 DETAIL
S203 1:20



11 DETAIL
S204 1:20



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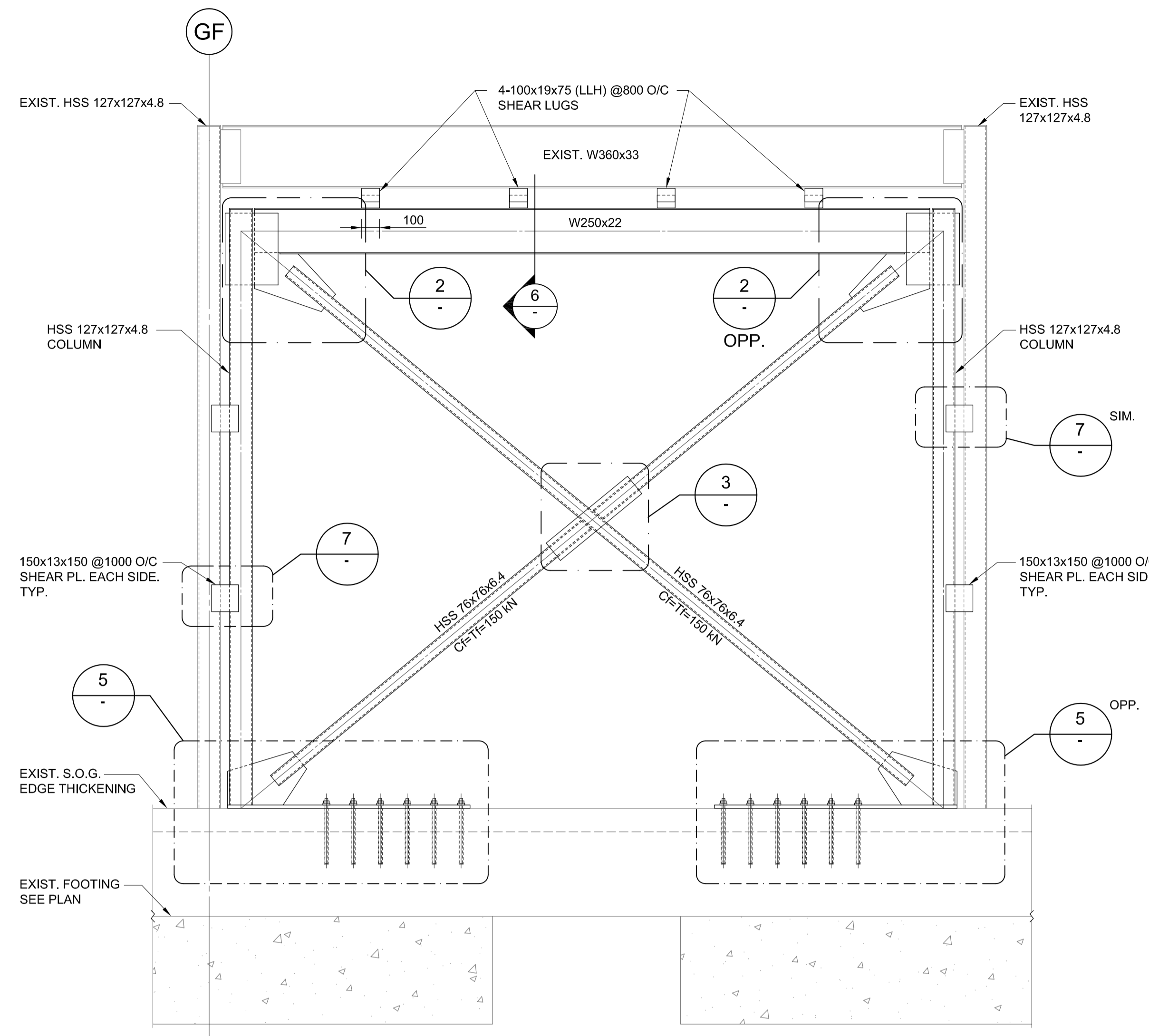
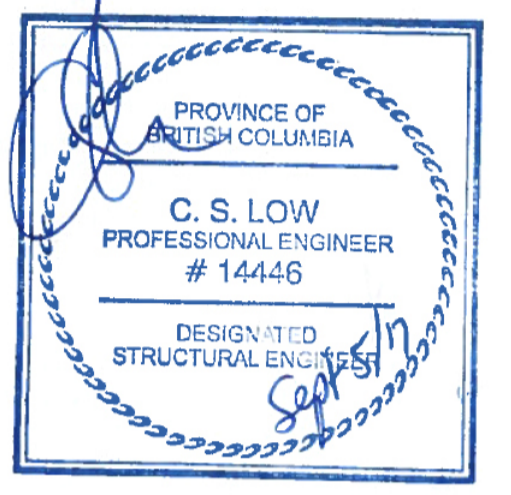
PWSSC Project Manager/Administrateur de Projets TPSSC
TONY TANG

Regional Manager, Architectural and Engineering Services
Gestionnaire régionale, Services d'architectural et de génie, TPSSC
PREETIPAL PAUL

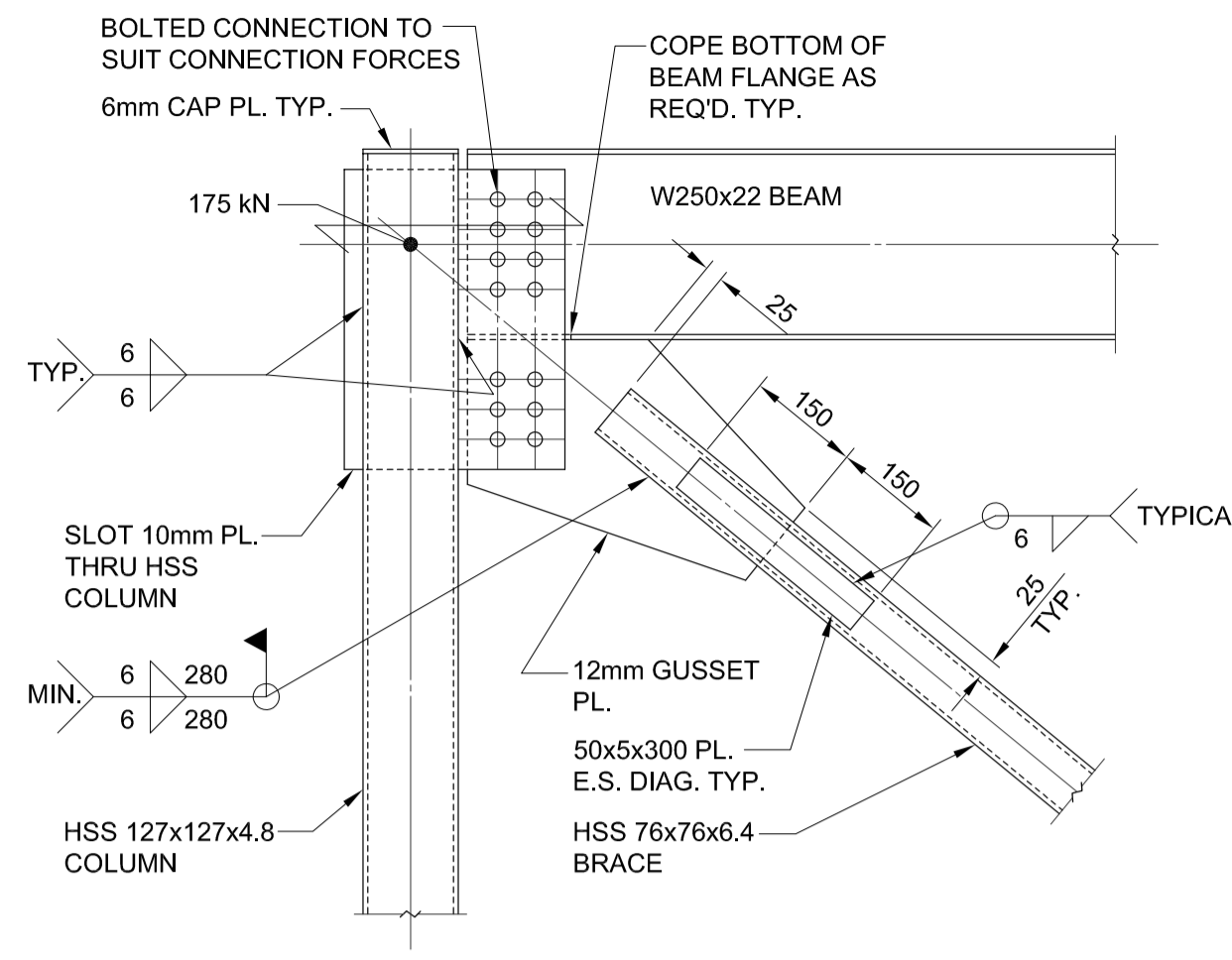
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SECTIONS AND DETAILS SHEET 4

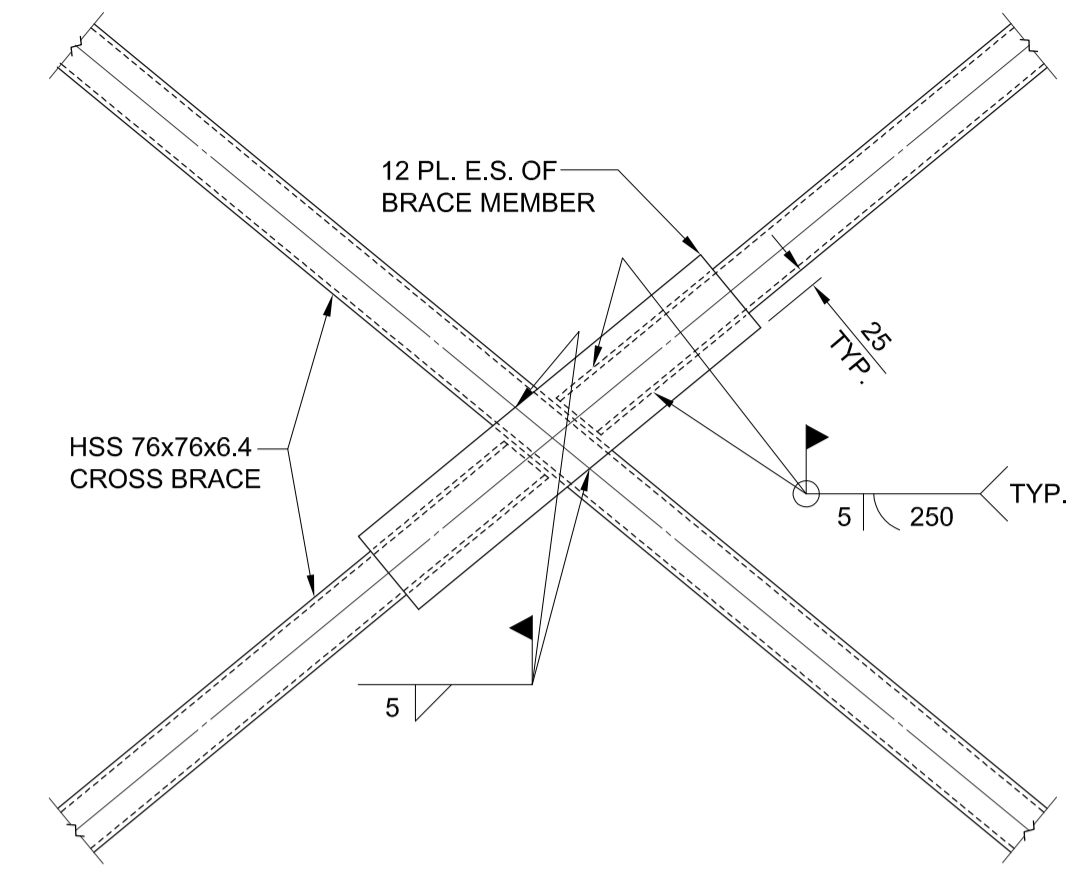
Project No./No. du projet R.077724.001	Sheet/Feuille S320 OF XX	Revision no./La Révision no. 0
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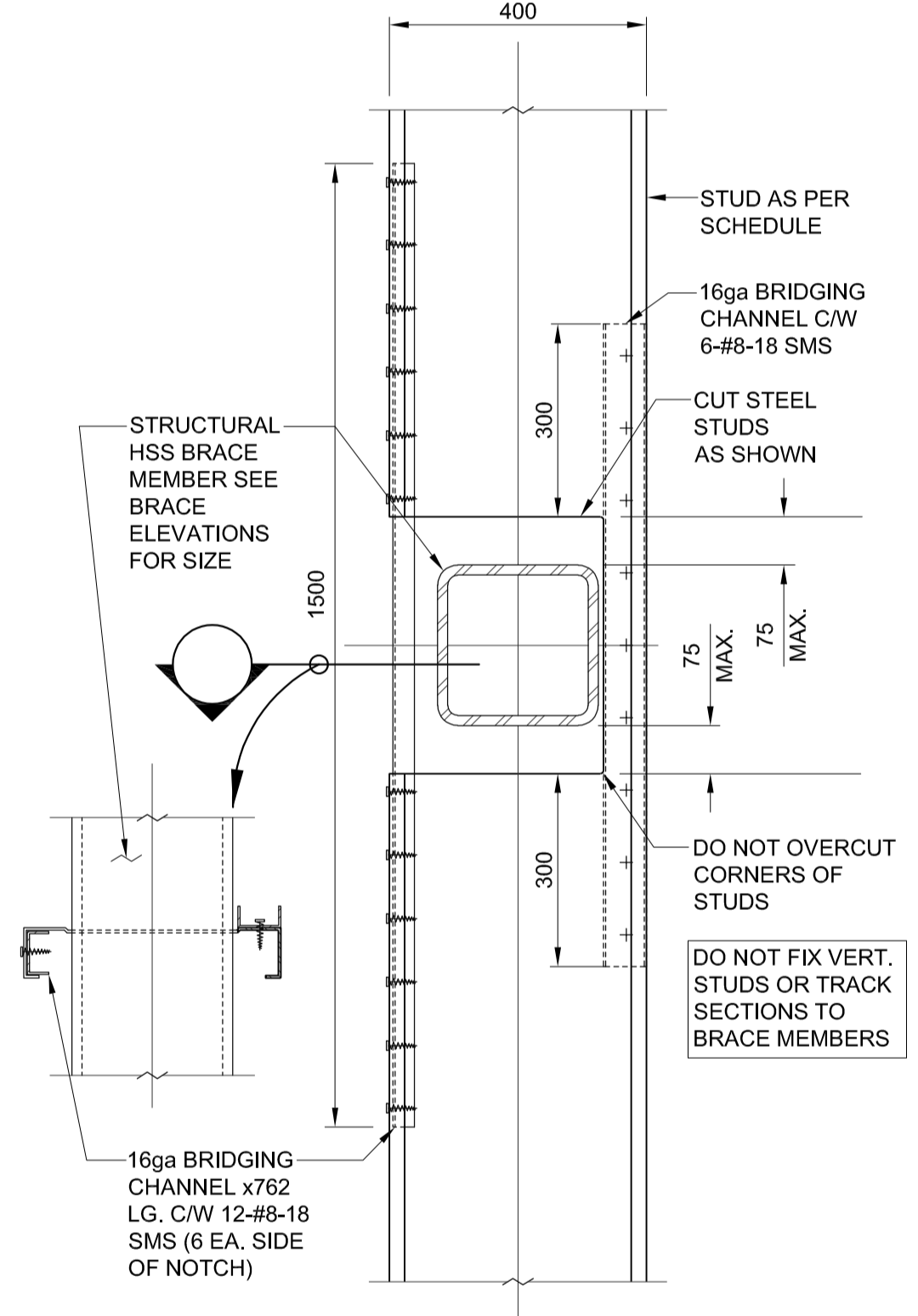
1 NEW CBF TO EXISTING STEEL FRAME
S202 1:20



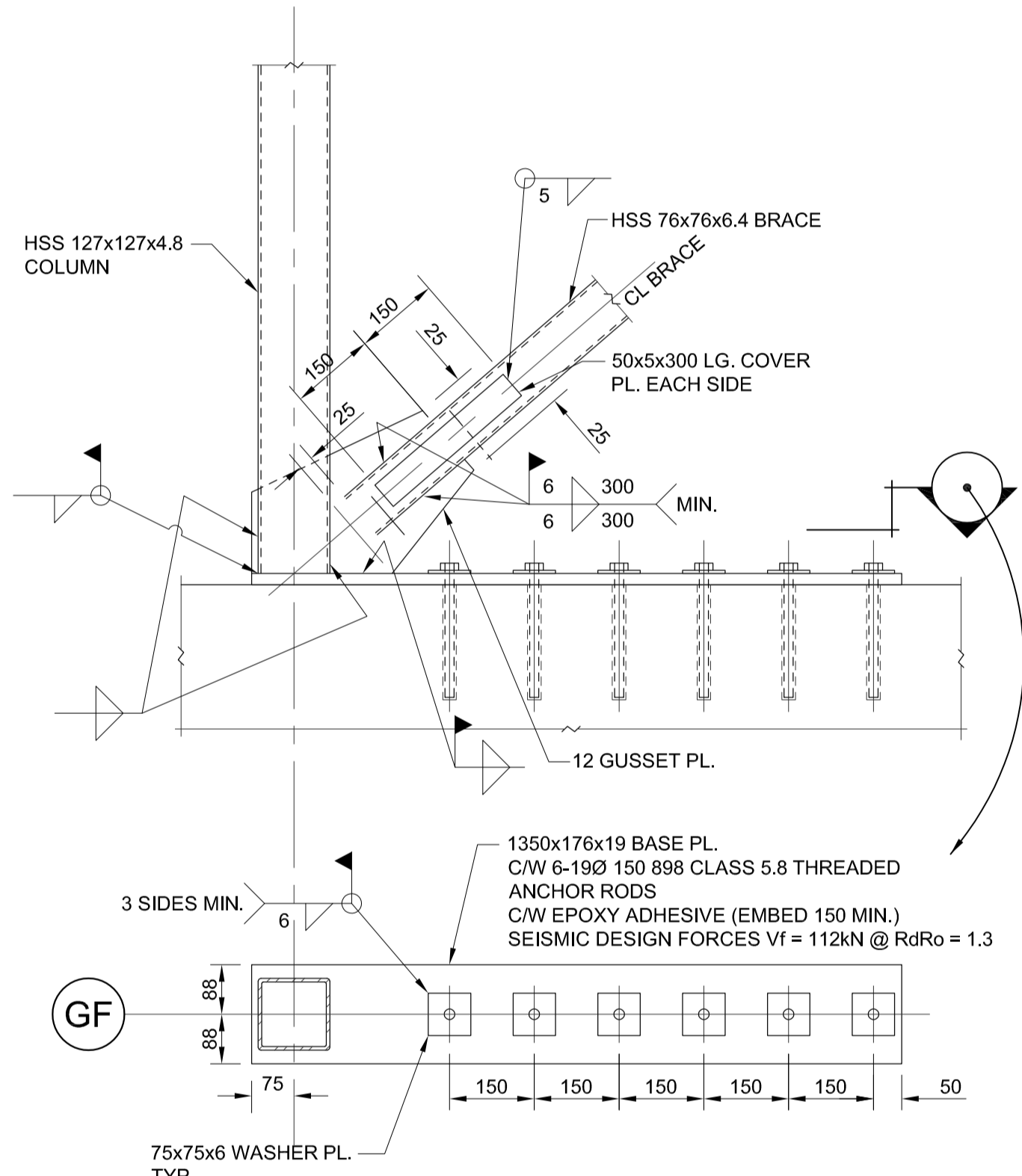
2 BRACE FRAME TOP CONNECTION DETAIL 1:10



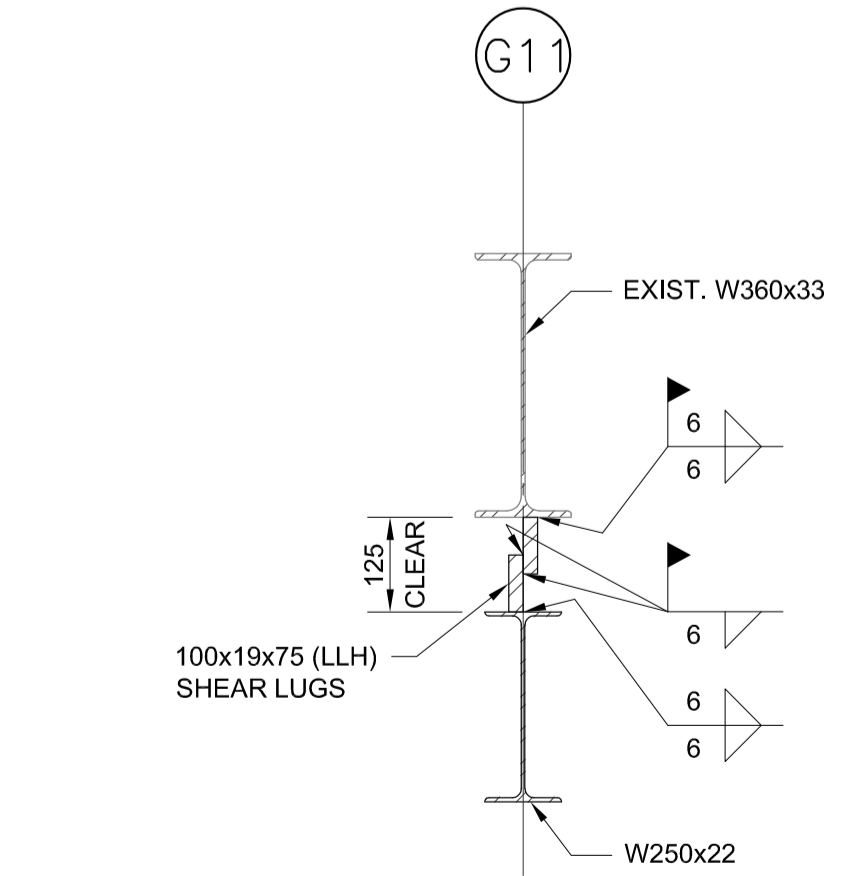
3 CROSS BRACE FRAME CENTRE CONNECTION DETAIL 1:10



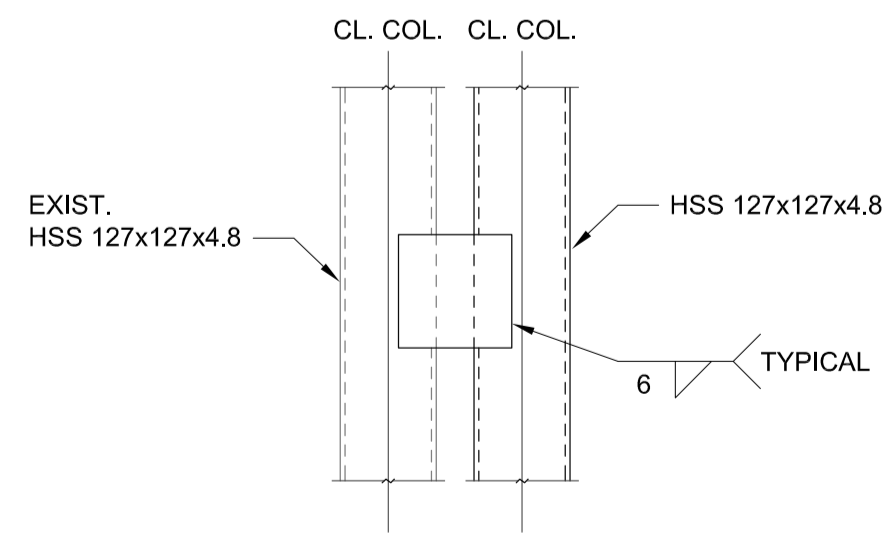
4 TYPICAL STEEL STUD WALL AT BRACE BAY DETAIL 1:10



5 BRACE BAY BOTTOM CONNECTION DETAIL 1:10



6 BEAM SHEAR LUG CONNECTION DETAIL 1:10



7 COLUMN SHEAR LUG CONNECTION DETAIL 1:10

BRACE BAY NOTES:

- FOR GENERAL NOTES, AND TYPICAL DETAILS SEE SHEET SERIES S-100.
- SET ANCHOR BOLTS USING A RIGID TEMPLATE TO KEEP BOLTS TO THEIR CORRECT LOCATION DURING THE PLACEMENT OF CONCRETE. CUTTING OF THE BASE PL. TO SUIT MISPLACED ANCHOR BOLTS WILL NOT BE PERMITTED.
- WELD GUSSET PLATE(S) TO COLUMN BASE PLATE USING CONTINUOUS WELD AT ALL CONTACT POINTS USING THE WELD SIZE SHOWN ON THE DETAILS OR AS REQUIRED TO DEVELOP THE BRACE FORCE. GAP BETWEEN THE COLUMN AND GUSSET PLATE TO BE A MAXIMUM OF 1.5mm. WELD THE COLUMN ALL AROUND TO THE GUSSET PLATE AND BASE PLATE USING THE WELD SIZE SHOWN ON THE DETAILS.
- ERECT THE COLUMN AND BRACE BAY STEEL USING SHIMS TO LEVEL COLUMN BASE PLATE. INCREASE SPECIFIED WELD SIZES TO ACCOMMODATE SHIMMING. SHIMS SHALL NOT EXCEED 6mm U.N.O.
- BRACE BAY COLUMN SHALL BE CONTINUOUS FULL HEIGHT.
- BRACE MEMBERS MAY BE CONNECTED BY BOLTS OR BY WELDING PROVIDED THAT THE BRACE CONNECTION DEVELOPS THE FACTORED CONNECTION FORCE SHOWN ON BRACING ELEVATIONS.
- CONNECTIONS FOR BEAMS SHOWN ON BRACE BAYS ARE TO BE DESIGNED TO RESIST BOTH OF THE FOLLOWING LOAD CONDITIONS:
 - A SHEAR AT EACH END OF THE BEAM EQUAL TO 50% OF THE TOTAL UNIFORMLY DISTRIBUTED FACTORED LOADS FOR A LATERALLY SUPPORTED BEAM OF THE SPAN SHOWN. NO AXIAL FORCE IS ASSUMED TO OCCUR WITH THIS SHEAR.
 - A SHEAR AT EACH END OF THE BEAM EQUAL TO 35% OF THE TOTAL UNIFORMLY DISTRIBUTED FACTORED LOADS FOR A LATERALLY SUPPORTED OF THE SPAN SHOWN ACTING SIMULTANEOUSLY WITH AN AXIAL FORCE SHOWN ON BRACE DRAWINGS. THE AXIAL FORCE FOR W-SECTION BEAMS SHOULD BE ASSUMED TO BE TENSION AND COMPRESSION.
- DRAG LINE AND BRACE BEAM CONNECTIONS WITH AXIAL FORCES EXCEEDING 100kN SHALL HAVE GUSSET PLATES SLOTTED THROUGH THE COLUMNS. SLOTTED HOLES NOT ALLOWED.
- BRACE CONNECTION DESIGN SHALL MEET THE REQUIREMENT OF CSA S16-14.
- MARK TOP OF EMBED PL.'S WITH CENTRELINE OF COLUMN IN THE SHOP. REFER TO PLAN FOR THE LOCATION OF COLUMN ON EMBED PLATES.
- DIAGONAL BRACES ARE NOT DESIGNED TO ACT AS WIND GIRTS FOR EXTERIOR WALLS. CUT AND REINFORCE EXTERIOR WALL STUDS TO FIT AROUND THE DIAGONAL BRACES. TYPICAL ALL LEVELS.
- STEEL FABRICATORS CONNECTION DESIGN ENGINEER SHALL SUBMIT SKETCHES AND CALCULATIONS FOR PROPOSED BRACE BAY CONNECTION DETAILS PRIOR TO FORMAL SUBMISSION OF SHOP DRAWINGS FOR REVIEW BY DEPARTMENTAL REPRESENTATIVE.

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5		
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Regional Manager, Architectural and Engineering Services
Gestionnaire régionale, Services d'architecture et de génie, TPSSC
PREETIPAL PAUL

Drawing title/Titre du dessin
BRACE BAY ELEVATION AND DETAILS

Project No./No. du projet
R.077724.001
Sheet/Feuille
S400
Revision no./La Révision no.
0
OF XX