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	DUT) GENERAL	· IRREGULARITY REVIEW: (2015 NBCC CLAUSE 4.1.8.6)
	DUT-T GENERAL INFORMATION THE INFORMATION PRESENTED ON THESE DRAWINGS HAS BEEN DESIGNED AND ANALYZED IN ACCORDANCE WITH THE SALE NATIONAL DUILD DUE OF CAMADA CONSTRUCTION IS TO BE DESIGNED AND ANALYZED IN ACCORDANCE WITH THE	1. VERTICAL STIFFNESS: YES 2. WEIGHT: YES
	2015 NATIONAL BUILDING CODE OF CANADA. CONSTRUCTION IS TO BE PERFORMED IN ACCORDANCE WITH THIS AND ALL OTHER APPLICABLE CODES.	3. VERTICAL GEOMETRIC: 4. IN PLANE DISCONTINUITY: 5. OUT OF PLANE: YES
	 1.1 CONCRETE STRUCTURE DESIGNED IN ACCORDANCE WITH CSA A23.3-14 1.2 STEEL STRUCTURE DESIGNED IN ACCORDANCE WITH CAN/CSA-S16-14 	6. WEAK STOREY: 7. TORSIONAL: BNS = 1.8
	2. GUARDRAILS/HANDRAILS SHALL BE DESIGNED AND CERTIFIED BY THE FABRICATOR'S PROFESSIONAL ENGINEER LICENSED IN ONTARIO IN ACCORDANCE WITH LOADS PROVIDED IN (4.1.5.15	BEW = 1.9 8. NON-ORTHOGONAL: YES
	IN ONTARIO IN ACCORDANCE WITH LOADS PROVIDED IN (4.1.5.15, 3.4.6.4 AND 3.4.6.5 OF THE 2015 NATIONAL BUILDING CODE OF CANADA. STAMPED SHOP DRAWINGS TO BE SUBMITTED. IN ADDITION, GLASS IN GUARDS SHALL COMPLY WITH "SUPPLEMENTARY STANDARD SB-13".	CONCLUSION: BUILDING IS ☐ REGULAR ■ IF DYNAMIC ANALYSIS: ■ REQUIRED □ N
	3. THE ROOF HAS BEEN DESIGNED FOR THE REQUIRED STORM WATER FLOW RESTRICTION IN ACCORDANCE WITH 2015 NATIONAL BUILDING CODE OF CANADA REQUIREMENTS	
	 CONTRACTOR IS TO VERIFY/COORDINATE ALL DIMENSIONS/PENETRATIONS WITH ADDIVISED TO VERIFY/COORDINATE ALL DIMENSIONS/PENETRATIONS WITH 	± 0.10 Dix (4.1.8.
	PROCEEDING WITH WORK. ANY OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS ARE TO BE APPROVED BY DEPARTMENTAL REPRESENTATIVE IN WRITING PRIOR TO CONSTRUCTION.	STRUCTURAL SEPARATION: ■ THE ADJACENT
	5. LADDERS SHALL BE DESIGNED AND CERTIFIED BY THE FABRICATOR'S PROFESSIONAL ENGINEER LICENSED IN ONTARIO IN ACCORDANCE WITH LOADS PROVIDED IN PART 4 AND PART 3 OF THE 2015 NATIONAL BUILDING CODE OF CANADA	4.1.8.14(1) OF TH
	STAMPED SHOP DRAWINGS TO BE SUBMITTED.	• BUILDING WEIGHT FOR SEISMIC DESIGN: W = 21,000
	 STEEL STAIRS SHALL BE DESIGNED AND CERTIFIED BY THE FABRICATOR'S PROFESSIONAL ENGINEER LICENSED IN ONTARIO IN ACCORDANCE WITH LOADS PROVIDED IN PART 4 AND PART 3 OF THE 2015 NATIONAL BUILDING CODE OF CANADA. STAMPED SHOP DRAWINGS TO BE SUBMITTED. 	BASE SHEARS/MOMENTS:
	7. SEISMIC RESTRAINT OF ARCH/MECH/ELECT ELEMENTS NOT NOTED ON THE DRAWINGS ARE THE RESPONSIBILITY OF THE CONTRACTOR'S ENGINEER RESTRAINT DETAILS ARE TO BE DEVELOPED IN ACCORDANCE WITH THE 2015 NBCC	VSTATIC = S(Ta)Mv I EW/(RdRo) = W * 0.16 = 3360 KN
	CONTRACTOR'S ENGINEER IS RESPONSIBLE FOR THE DESIGN AND DETAILING OF SEISMIC RESTRAINTS AND ISOLATIONS AS REQUIRED BY SPECIFICATIONS INCLUDING THE VERIFICATION THAT THE EXISTING/NEW STRUCTURE IS CAPABLE OF	
	SAFELY SUPPORTING THE IMPOSED LOADS IN ACCORDANCE WITH THE 2015 NBCC. NO ELEMENTS MAY BE CONSTRUCTED WITHOUT WRITTEN CONFIRMATION OF THESE CONDITIONS BY CONTRACTOR'S ENGINEER.	$V_{MIN} = S(4.0) M \vee IEW/(RdRo) = W * 0.0154 = 325 KI$
	8. NO FOUNDATION ELEMENTS ARE TO BE CONSTRUCTED UNTIL WRITTEN APPROVAL OF THE BEARING SURFACES AND PRESSURES IS PROVIDED BY A GEOTECHNICAL ENGINEER THROUGH ON-SITE INVESTIGATION. FAILURE TO COMPLETE THIS WORK COULD DESENT A TO COMPLETE THIS WORK COULD DESENT A TO CONTRACTORS.	$V_{MAX} = \frac{2}{3} S(0.2)IEW/(RdRo) = W * 0.162 = 3,405$
	OWN COST.	VMAX = S(0.5) I EW/(RdRo) = W * 0.129 = 2,713
	 CONTRACTOR TO PROVIDE PRE-ENGINEERED SHORING AS REQUIRED TO ACCOMMODATE THE CONTRACTOR'S CONSTRUCTION ACTIVITIES AND TO PREVENT DAMAGE TO ANY ADJACENT PROPERTY. ALL CONSTRUCTION ACTIVITIES TO BE LIMITED TO THE LIMITS OF THE CONSTRUCTION SITE AND ALL DAMAGE TO EXISTING PROPERTIES MUST BE 	$\underline{EAST-WEST}: (\Longleftrightarrow)$
	REINSTATED.	VMIN = S(4.0)MvIeW/(RdRo) = W * 0.0154 = 325 KI VMAX = $\frac{2}{3}$ S(0.2)IeW/(RdRo) = W * 0.162 = 3.405
	10. CONTRACTOR IS REQUIRED TO SUBMIT CONDUIT AND SLEEVING SHOP DRAWINGS FOR ALL FLOORS/ROOFS/WALLS/COLUMNS PRIOR TO THE ERECTION/CONSTRUCTION/FABRICATION OF ANY OF THESE ELEMENTS. THE DRAWINGS ARE TO LOCATE/DIMENSION THE CLEAR SIZES OF OPENINGS/SLEEVES/CONDUITS IN PLAN	3 VMAX = S(0.5) I EW/(RdRo) = W * 0.129 = 2,713
	(FLOORS/ROOFS/COLUMNS) AND ELEVATION (WALLS/BEAMS). THE COORDINATION OF THE VARIOUS DISCIPLINES/SUBTRADES TO ENSURE ALL ITEMS ARE CLEARLY INDICATED IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR, DRAWINGS ARE TO BE SUBMITTED A MUMICIPAL AWEEKS PRIOR TO THE CONSTRUCTION OF THE	
	AFFECTED ELEMENT.	SEISM
		EQUIVALENT STATIC (ES) DYNAMIC ANALYSIS FORCE PROCEDURE (INITIAL SCA
	DU1-2 GRAVITY LUADS:	2015 NBCC CLAUSE 4.1.8.11(1)-(10) 2015 NBCC CLA NORTH-SOUTH: (↑) NORTH-S
	Importance factors: Snow Load Parameters: SNOW: Is ULS = 1.0, SLS = 0.9 S = Is [Ss(Cb*Cw*Cs*Ca) + Sr] WIND: Iwr III S = 1.0, SLS = 0.75 Ss = 2.4 kPa (1/50)	VESNS = W * 0.16 = 3226 KN VDYINS = 2
\bigcap	SEISMIC I_{E} : ULS = 1.0 Cb = 0.4 kPa (1/50) Cb = 0.8 Ca = VARIES	MESNS = 17,400 KN • m MDYINS = MPMR =
	Cw = 1.0 Cs = 1.0	
	DESIGN LOADS	
\bigcap	REFER TO PLANS	
	D01-3 LATERAL LOADING DATA	EAST-WEST: (↔) EAST-
Ζ	• SEISMIC FORCE RESISTING SYSTEM (SFRS)	VESEW = W * 0.16 = 3226 KN VDYIEW = MESEW = 17,400 KN • m MDYIEW = MPMP =
	SFRS: SYSTEM & CONNECTIONS: (2015 NBCC CLAUSE 4.1.8.9/4.1.8.10)	
	Rd = 1.5 Ro = 1.3	
	CSA STANDARD: CAN/CSA A23.3-14 APPLICABLE CLAUSE(S): 21.6	
	SFRS: DIAPHRAGMS & CONNECTIONS: (2015 NBCC CLAUSE 4.1.8.15) CSA STANDARD: CAN/CSA A23.3-14	
U	APPLICABLE CLAUSE(S): 21.9	(1) INITIAL DYNAMIC LOAD SCALING FACTOR
	SFRS: SYSTEM FOUNDATIONS: (2015 NBCC CLAUSE 4.1.8.16) CSA STANDARD: CAN/CSA A23.3-14	S.F. = $g \cdot \frac{Ie}{RdRo} = g \cdot 0.87 = 5.031$
ш	APPLICABLE CLAUSE(S): 21.10	DYNAMIC ANALYSIS PROCEDURE LOADS ARE BASE SCALING FACTOR APPLIED. WHEN USED THESE ARE
	CONFIRMATION: FOUNDATIONS HAVE BEEN DESIGNED TO RESIST THE LATERAL FORCES APPLIED TO THE SFRS IN ACCORDANCE WITH THE 2015 NBCC INCLUDING ALL APPLICABLE AMPLIFICATION FACTORS.	WITH 4.1.8.12 OF THE 2015 NBCC TO DETERMINE DE
	• <u>SEISMIC IMPORTANCE FACTOR</u> : (2015 NBCC CLAUSE 4.1.8.5) TE = 1.0	OF THE 2015 NBCC. LOADS INDICATED SHOW THE D MOMENT.
\bigcirc	· <u>REFERENCE CITY:</u> OTTAWA	(3) N/A = NOT USED IN THE DESIGN OF THE BUILDING.
	SITE CLASS: THE NOTED SITE CLASSIFICATION FOR SEISMIC SITE RESPONSE AND SHEAR STRENGTH PARAMETERS INDICATED ARE AS REPORTED IN THE GEOTECHNICAL REPORT: EP168-123367	
7	BY: STANTEC DATED: MARCH, 2015	D01-3.1 WIND:
	· PGA: 0.304 PGV: 0.208	q= 0.41 KPa Vbase = 140 KN
	RESPONSE SPECTRUM DATA:	(1 IN 50 YEARS) Middle = 750 KN $Iw = 1.0 (ULS)$ EAST-WEST: (\leftarrow
	<u>5% DAMPED SPECTRAL RESPONSE</u> <u>ACCELERATION VALUES FOR REFERENCE CITY:</u> (2015 NBCC SUPPLEMENTARY STANDARD SB-1)	Iw = 0.75 (SLS) Vbase = 170 KN Mbase = 900 KN
	Sa(0.2) = 0.474 Sa(0.5) = 0.252	
	Sa(1.0) = 0.124 Sa(2.0) = 0.058 a = 0.014	D01-4 SHOP DRAWINGS
	Sa(3.0) = 0.014 Sa(10.0) = 0.0056	1. SUBMIT SHOP DRAWINGS FOR ALL STRUCTURAL WORK DEPARTMENTAL REPRESENTATIVE. OBTAIN DEPARTM
	DESIGN SPECTRAL RESPONSE ACCELERATION VALUES (DSRAV): (2015 NBC CLAUSE 4.1.8.4)	2. EACH OF THE FOLLOWING SHOP DRAWINGS MUST BEA
111	■ CLASS 'C': (Fa = 1.00: Fv = 1.00) Sa(0,2) = 0.474	DEPARTMENTAL REPRESENTATIVE REGISTERED IN TH
	Sa(0.5) = 0.252 Sa(1.0) = 0.124	 b) DRAWINGS FOR ANY STRUCTURAL PARTS DESIGN BUILDING ENVELOPE.
7	$\begin{array}{llllllllllllllllllllllllllllllllllll$	d) FORMWORK
		 SHOP DRAWINGS MUST BE REVIEWED AND STAMPED F DEPARTMENTAL REPRESENTATIVE. SHOP DRAWINGS DELAYS IN THE CONSTRUCTION SCHEDULE DUE TO NO
Ш	 <u>SYSTEM RESTRICTION VALUE</u>: IEFaSa(0.2) = 0.474 ≥ 0.35 ■ YES (DYNAMIC ANALYSIS REQUIRED) NO (STATIC ANALYSIS MAY BE USED) 	RESPONSIBILITY OF THE CONTRACTOR.
	<u>PERIOD DATA</u> : EMPIRICAL PERIOD: (2015 NBCC CLAUSE 4.1.8.11.(3)) (a).(b)or(c)	4. SUBMIT STRUCTURAL STEEL, STEEL JOIST AND STEEL REVIEW BEFORE FABRICATION. ALL SHOP DRAWINGS ENGINEER IN THE PROVINCE OF ONTARIO
C	Ta(EMPIRICAL)NS = 0.20 sec	5. SHOP DRAWINGS ARE REVIEWED FOR CONFORMANCE
		RESPONSIBILITY FOR THE QUANTITIES AND DETAILED PROVIDE THE COMPLETE AND SATISFACTORY JOB DES
	Ta(MODAL)NS = 0.056 sec	CONTRACTOR.
	Ta(MODAL)EW = 0.053 sec	D01-5 DEFINITIONS THE FOLLOWING ABBREVIATIONS HAVE BEEN LISED IN THE
	$\frac{DESIGN FERIODS/MODE a MOMENT FACTORS}{20 < \frac{Sa(0.2)}{20} = 24 < 40$	@ AT (SPACING c/c)
	$-3 - \frac{1}{Sa(5.0)} - \frac{34}{2} + 0$	ARCH. ARCHITECTURAL M. B BOTTOM M.
	Ta(DESIGN)NS = 0.056 sec Mv = 1.0 J = 1.0 Ta(DESIGN)EW = 0.053 sec Mv = 1.0 J = 1.0	BF BRAGE FRAME M BLL BOTTOM LOWER LAYER NI BUL BOTTOM UPPER LAYER N
	· DESIGN FUNDAMENTAL PERIOD BASED DSRAV: (2015 NBCC CLAUSE 4.1.8.11(2))	BP BEAM POCKET OU cic CENTRE TO CENTRE PO
	S(Ta)NS = 0.474	CONT. CONTINUOUS SI CW CORE WALL
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F2	1500x1500x400	12-15M B.E.W. ∟ SLS 1000 kPa ULS 1000 kPa		600x600 12-25M VERT. 3-10M TIES @ 200o.c. (MAX.)	
F3	500x300 STRIP FOOTING	3-15M BOT. CONT. SLS 1000 kPa ULS 1000 kPa			
F4	2000x800	20M @ 100 BLL TRANS 20M @ 100 BUL LONG 20M @ 100 TUL TRANS 20M @ 100 TUL LONG			
F5	1000x500	SLS 1000 kPa ULS 1000 kPa 15M @ 200 BLL TRANS. 15M @ 200 BUL LONG. SLS 1000 kPa ULS 1000 kPa		NOTES: 1. PROVIDE FOOTING DOWEL 2. PROVIDE TIE GROUPS AS 3. BEND VERTICAL BARS INT 4. FOOTING AND BOTTOM OF REPORT. COLUMNS ARE T	LS TO MATCH VERTICAL F NOTED IN DETAIL 'A' U/N. O SLAB WHERE COLUMNS F COLUMN ELEVATIONS T FO BE TERMINATED SUCH
1. PROV 2. QUAL ALL B 3. ALL F BEAR	/IDE 75mm COVER TO BO IFIED GEOTECHNICAL D BEARING SURFACES PRI COOTINGS TO BEAR ON U ING CAPACITY AS SHOW	DTTOM REINFORCEMENT (TYP.). DEPARTMENTAL REPRESENTATIVE TO REVIEW OR TO CONCRETE PLACEMENT. JNDISTURBED SOIL WITH AN ALLOWABLE VN ABOVE.	,	5. ALL COLUMN CORNERS TO	D RECEIVE 45° CHAMFER.
		CONCF	RETE COLUMN	SCHEDULE: NOTES	& DETAILS
ROOF THE THE THE (A' SF <22	ES TO BE CONTINUOUS IROUGH SLAB/DROPS T HALF SPACING) PACING OF TIES TO BE: SXDIAM. OF LONG BARS 24xDIAM. OF TIE //2 LEAST COL. DIM. 300mm E SPACING AS DTED ON SCHEDULE	BEND VERTS INTO SLAB PLACE ADD'L SPACING AT LIFT OVER A I B > LARGES > 450mm > 1/6 HEIGI (MIN. 5 ADD'L CLASS 'B' TEN (LAPS TO BE	TIE GROUPS AT ½ TOP & BOTTOM OF EACH HEIGHT OF B T COLUMN DIMENSION HT OF STOREY GROUPS)	D 5003	DETAIL: COLUMN/ SCALE: N.T.S.



S003

SCALE: N.T.S.



S TO MATCH VERTICAL REINFORCING.

OTED IN DETAIL 'A' U/N. PROVIDE ADDITIONAL TIE GROUPS AS PER DETAIL 'A'. SLAB WHERE COLUMNS TERMINATE.

COLUMN ELEVATIONS TO BE ESTABLISHED FROM PLANS AND GEOTECHNICAL TO BE TERMINATED SUCH THAT THEY ARE FOUNDED ON UNDISTURBED NATIVE RIATE FROST COVER) OR CAISSONS APPROVED BY GEOTECHNICAL NTATIVE ON SITE. REFER ALSO TO GEOTECHNICAL REPORT.

EACH FACE 600 DETAIL: COLUMN/SLAB INTERFACE AT GRIDS (3)/(4) SCALE: N.T.S.



DETAIL: TIES: 16 BAR ARRANGEMENT (2 TIES)



(3 TIES)

HIGH ROOF EL. VARIES (106.500 H.P. / 104.975)

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1	200mm CONCRETE SLAB ON GRADE c/w: 15M @ 300 E.W. (MID-HEIGHT)	18 FW1: 175mm CONCRETE FOUNDATION WALL c/w: • 15M @ 400 VERT. + DWLS • 15M @ 300 HOB. (CONT.)
2	500mm CONCRETE SLAB ON GRADE c/w: 15M @ 200 T.E.W.	• 2-15M HOR. ADD'L AT TOP
•	15M @ 200 B.E.W.	 PRE-ENGINEERED CFC FRAMING @ 300 o.c. (MAX. CONTRACTOR'S ENGINEER TO
3	RAISED FLOOR (REFER TO ARCH. DRAWINGS)	DESIGN / DETAIL / CERTIFY ALL FRAMING / CONNECTORS / DETAILS
(4) ^	ENGINEERED FILL AS PER GEOTECHNICAL REPORT	20 PARAPET BEYOND
5	ALL FOOTINGS TO BEAR ON UNDISTURBED BEDROCK. GEOTECHNICAL DEPARTMENTAL REPRESENTATIVE TO APPROVE PRIOR TO CONCRETE PLACEMENT	21 REFER TO SLAB REINFORCING NOTE 7 FOR REBA AROUND OPENINGS (TYP.)
6	1800mm MIN. FROST COVER (REFER TO	22 HSS 152x102x9.5 (GALV.) POSTS @ 1500 o.c. (MAX.
n	CLADDING (REFER TO ARCH. DRAWINGS)	3 HSS 152x102x9.5 (GALV.) HOR. AT TOP AND BOTTO
		24) PRE-FABRICATED SCREEN (REFER TO ARCH.)
\odot	OF SLAB (TYP.) <u>900</u>	5 HSS 102x102x9.5 (GALV.) KICKERS AT EACH POST
	900	$\begin{array}{c} \hline 26 \end{array} 300 \times 200 \times 12 \text{ BP} (GALV.) \\ \hline 26 \end{array} \qquad $
9	150mm CONCRETE SLAB ON GRADE c/w: 152x152 MW18.7x18.7	
10	40mm DEEP SAWCUTS c/w: FLEXIBLE JOINT FILLER	ALL MEMBER ENDS TO BE CAPPED / ALL WELDS
(11)	RECESS FOR FLOOR GRILL (REFER TO ARCH. DRAWINGS)	1281 REFER TO SCHEDULES FOR BEAM REBAR / LAYO
(12)	REFER TO COLUMN SCHEDULE FOR REINFORCING	29 SNOW DRIFT AT PARAPET
13	REFER TO WALL SCHEDULES FOR REINFORCING	OFFSET BASEPLATE ON COLUMN TO ENSURE 150
14	DUCTING (REFER TO MECH. DRAWINGS AND SPECS)	EDGE DISTANCE TO BOLTS
15	REFER TO PLANS FOR SLAB REINFORCING / LAYOUT OF STEEL	[31] PROVIDE DOWELS / SHEAR KEY IN COLUMNS AS F COLUMN SCHEDULE (TYPICAL ALL COLUMNS ON GRIDS (3) AND (4)
16	10mm BITUMINOUS BOND BREAKER	32 REFER TO ARCH/MECH/ELECT DOCUMENTS FOR
(17)	BAND OF RF CONCRETE	TO BE CAST INTO CONCRETE (TYP.)

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CE DESSIN N'EST PAS À L'ÉCHELLE SI LES MARQUEURS NE SONT PAS ALIGNÉS AVEC RÈGLE





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180 200 mm HIS DRAWING IS NOT TO SCALE IF THIS RULER DOES NOT MEASURE CORRECTLY



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