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<b>Title - Sujet</b> Hampton Harbour Approach	
<b>Solicitation No. - N° de l'invitation</b> EB144-192089/A	<b>Amendment No. - N° modif.</b> 004
<b>Client Reference No. - N° de référence du client</b> EB144-19-2089	<b>Date</b> 2018-11-28
<b>GETS Reference No. - N° de référence de SEAG</b> PW-\$PWA-110-5819	
<b>File No. - N° de dossier</b> PWA-8-80086 (110)	<b>CCC No./N° CCC - FMS No./N° VME</b>
<b>Solicitation Closes - L'invitation prend fin</b> <b>at - à 02:00 PM</b> <b>on - le 2018-12-04</b>	<b>Time Zone</b> <b>Fuseau horaire</b> Atlantic Standard Time AST
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<b>Signature</b>	<b>Date</b>

La modification 004 vise à intégrer les éléments suivants :

Supprimer dans son intégralité :  
Rapport géotechnique précédente

Insérer :  
Rapport géotechnique révisée

TOUTES LES AUTRES MODALITÉS ET CONDITIONS DE MEURENT LES MÊMES.



**GEOTECHNICAL INVESTIGATION  
HAMPTON WHARF  
HAMPTON, NOVA SCOTIA**

**Submitted to:**

**Public Works and Government Services Canada**  
1713 Bedford Row  
Halifax, Nova Scotia B3H 1T6

**Submitted by:**

Amec Foster Wheeler Environment & Infrastructure  
10000 Highway 100  
Dartmouth, Nova Scotia B3A 1K1

10 October 2017  
TV177001

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## 1 INTRODUCTION

Amec Foster Wheeler Environment & Infrastructure Division of Amec Foster Wheeler Americas Limited (Amec Foster Wheeler) has been retained by Public Works and Government Services Canada (PWGSC) in accordance with the RFP dated June 1, 2017 to carry out a geotechnical investigation at the site of the existing wharf. The site is located in Milton, Ontario.

The purpose of the investigation was to determine the subsurface conditions at the site and based on these conditions to provide geotechnical design recommendations for the proposed repair of the existing wharf.

The report provides specific and clear recommendations for the proposed project described herein and includes geotechnical design recommendations.

There should also be an ongoing communication with Amec Foster Wheeler during both the detailed design and construction phases of the project to ensure that the recommendations in this report have been interpreted and implemented correctly. Additional further clarification and/or elaboration are needed concerning the geotechnical aspects of the project. Amec Foster Wheeler should be contacted immediately.

## 2 PROJECT AND SITE DESCRIPTION

The existing wharf consists of a closed face timber pile and tie beam system retaining wall. It is understood that the retaining wall is a gravity retaining wall on approximately 1 m of fill from the bridge abutment to a more recent constructed retaining wall. According to historical records, the timber pile retaining wall was constructed in 1970. The timber piles have been driven and tilted outward. The tie beam system tie rod connections were on the piles to a concrete anchor wall foundation. The existing backfill material consists of granular fill.

The wharf's site is located in Milton, Ontario. The site location and project layout are shown on Figure 1.

## 3 INVESTIGATION PROCEDURE

The field work for the investigation was carried out under the supervision of Amec Foster Wheeler personnel on August 10 and September 1, 2017. A total of three boreholes (BH1 to BH3) were drilled at the site to determine from 0.7 to 3.0 m. The three boreholes were drilled just in front of the existing timber pile retaining wall at a distance of 0.1 and 0.7 m from the intersection of the wall with the bridge. The borehole locations are shown on the attached plan in Figure 2.

The boreholes were advanced using a track-mounted drilling rig provided by No. 1 Drilling Inc. The soils encountered were composed of continuous interbedded silty sand and 0.0 mm to 0.075 mm fines. In order to determine the relative density and/or consistency of the subsoil, a Standard Penetration Test (SPT) was carried out for each borehole attempt.

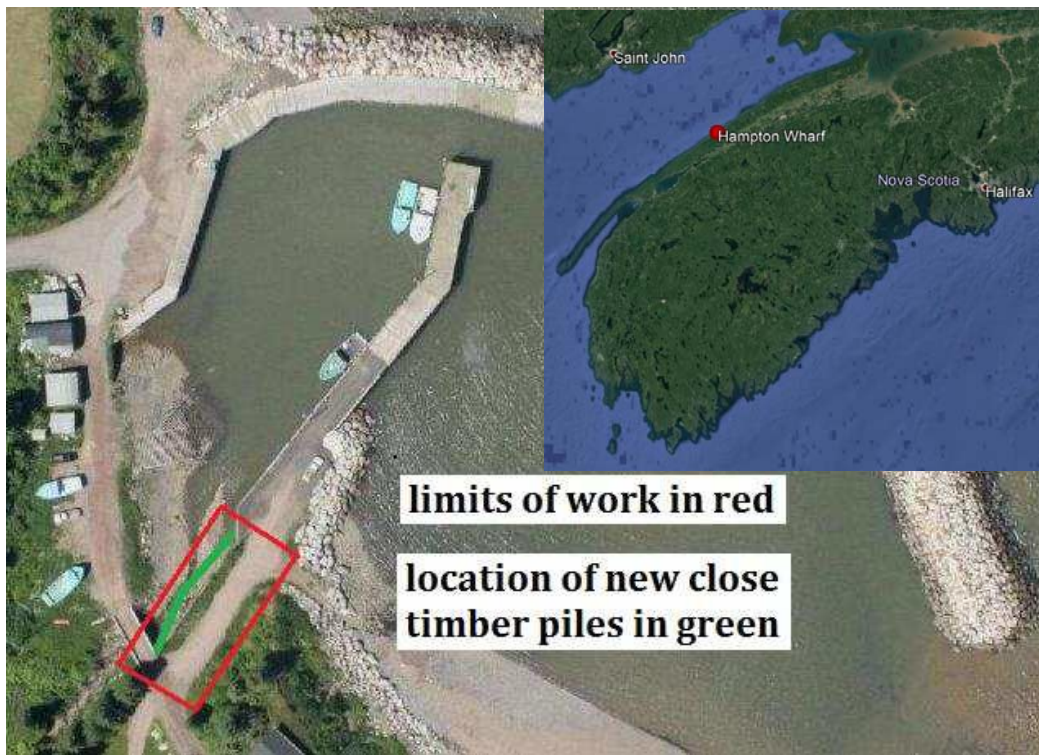


Figure 1: Site Location and Project Layout

During drilling of the borehole the conditions encountered were as follows. The representative samples were placed in moisture-tight containers and taken to our laboratory for classification and testing.

The borehole locations were established in the field by our personnel.

## 4 SUBSURFACE CONDITIONS

Details of the conditions encountered at the borehole locations are provided on the borehole logs in Appendix A. The following section summarizes the conditions and describes them in accordance with the unified soil classification system.

It should be noted that stratigraphic boundaries indicated on the borehole logs do not necessarily represent a transition from one soil type to another and do not necessarily indicate an exact line of geologic change. Subsurface conditions may vary between and beyond the borehole locations.

### 4.1 Silty Sand with Cobbles

A layer of red brown sand with cobbles and small boulders was encountered from ground surface at both the borehole locations. The thickness of this layer ranged from 0 m to 0 m.

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 geotechnical investigation  
 000000 000000000000N  
 000000 0017

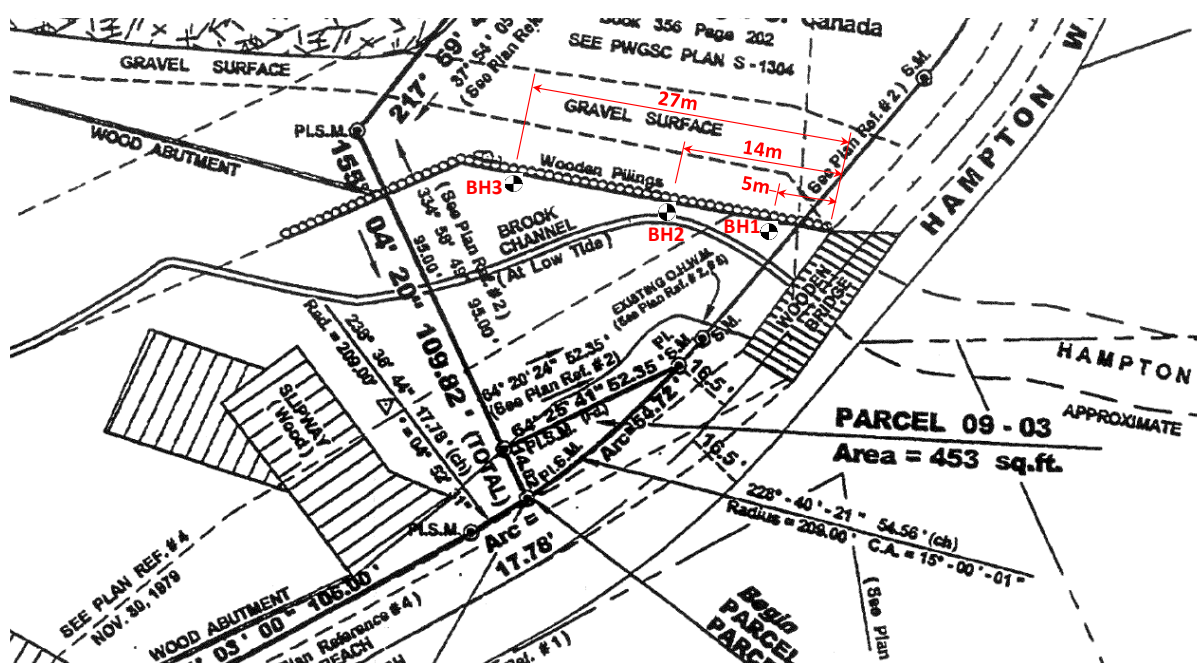


Figure 2: Borehole Location Plan

Measured 'N' values in this layer ranged from 0 to 100 indicating a very loose to compact compressive condition. The 100 N value is attributed to the presence of cobbles.

## 4.2 Sandy Silty Clay

A layer of red brown sand and silt clay was encountered below the sand with cobbles at the borehole. The layer extended to the bottom of the hole. The thickness of the layer ranged from 0.7 m to over 1.0 m.

From the standard curve recorded in Appendix 1 performed on two samples of the layer indicated the material to contain 10 to 20 percent water to 100 percent sand and silt and clay.

An Atterberg limit test performed on one sample of the layer indicated the material to be of low plasticity with a liquid limit of 40 and a plasticity index of 7. The test results are presented on the 100 in Appendix A and on the sieve sheet in Appendix 1.

The in-situ water content from two samples of the layer ranged between 10.1 and 10.0 percent.

Measured 'N' values in this layer ranged from 0 to over 17 indicating firm to very stiff consistency.

## 4.3 Sandy Clay

A layer of red brown sand and clay was encountered below the sand and silt clay in the borehole. The layer extended to the bottom of the hole. The thickness of the layer was 0.1 m.

## 4.4 Inferred Bedrock

Bedrock was inferred below the sandstone in 1 to 2 m depth below ground surface.

## 4.5 Groundwater Conditions

Groundwater was not observed during drilling of the borehole. However, the area is located in the tidal zone.

# 5 DESIGN RECOMMENDATIONS

## 5.1 General

It should be noted that the design recommendations for this project are provided for the guidance of the designer. The contractor bidding on or undertaking the work should make their own assessment of the site and interpretation of the recommendations provided so to effect their construction procedure and schedule.

As mentioned above, the existing closed face timber sheet pile wall on retention wall to support the wharf. The maximum retained depth of the wharf is about 2 m. The top of the sheet pile is tied back with tie rods to a concrete anchor wall. As mentioned above, the existing timber sheet pile has been damaged and tilted outward as a result of foundation settlement.

The following is understood:

- The proposed repair of the wharf will include installation of a new retention structure in front of the existing foundation wall to provide required lateral support for the existing wharf.
- The new retention structure will be constructed of precast treated timber sheet pile back to the existing concrete anchor wall.
- The new timber sheet pile will be 150 to 200 mm diameter and 200 mm butt diameter.
- The gap between the existing wall and the new retention structure will be filled with granular material.
- There will be no settlement on the sheet pile other than their own weight.

## 5.2 Timber Piles

### 5.2.1 Structural Design

After the 2004 Indian Ocean Foundation Engineering Manual, the structural design of wood sheet pile must conform with the requirements of Section 4 of the National Building Code of Canada 2004. No special consideration needs to be taken to sand in the sheet pile but special execution must be taken to protect the sheet pile end and head from damage due to drilling the sheet pile.

### 5.2.2 Penetration Depth

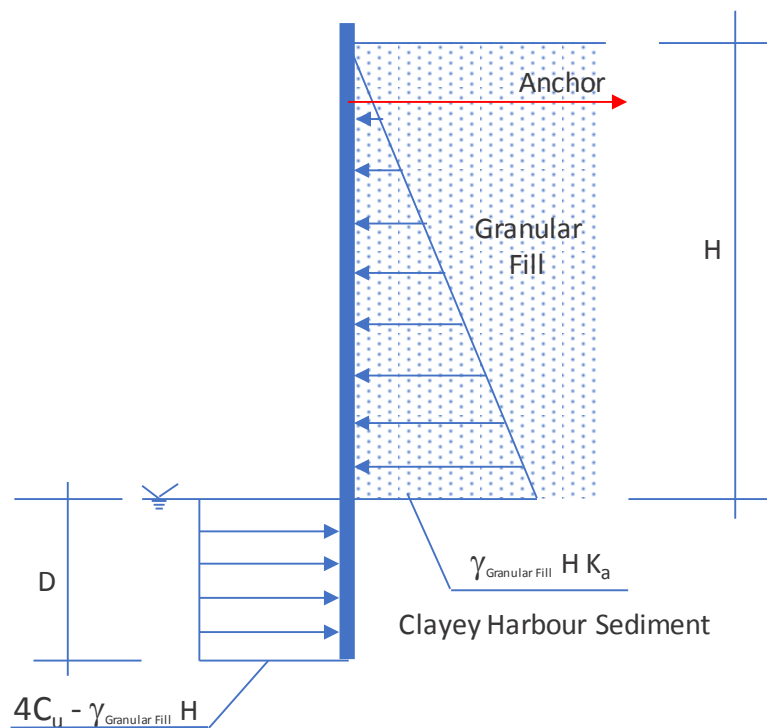
The 2004 Manual provides general guidance for determining penetration depth of flexible sheet pile retention wall similar to the proposed retention structure at location 1.

After 2004 Manual, two different methods can be used for design of precast anchor wall system. Name the "free-earth" and "fixed-earth" method. When the soft ground condition at the site, the



"free-earth" method can be considered more appropriate and suitable for design of single anchor wall system at Port of Mombasa. The "free-earth" method assumes that the wall acts as a beam supported at two points, the base of the wall and the anchor, and the pressure of the earth below the anchor bottom is free to rotate or translate horizontally at its bottom end.

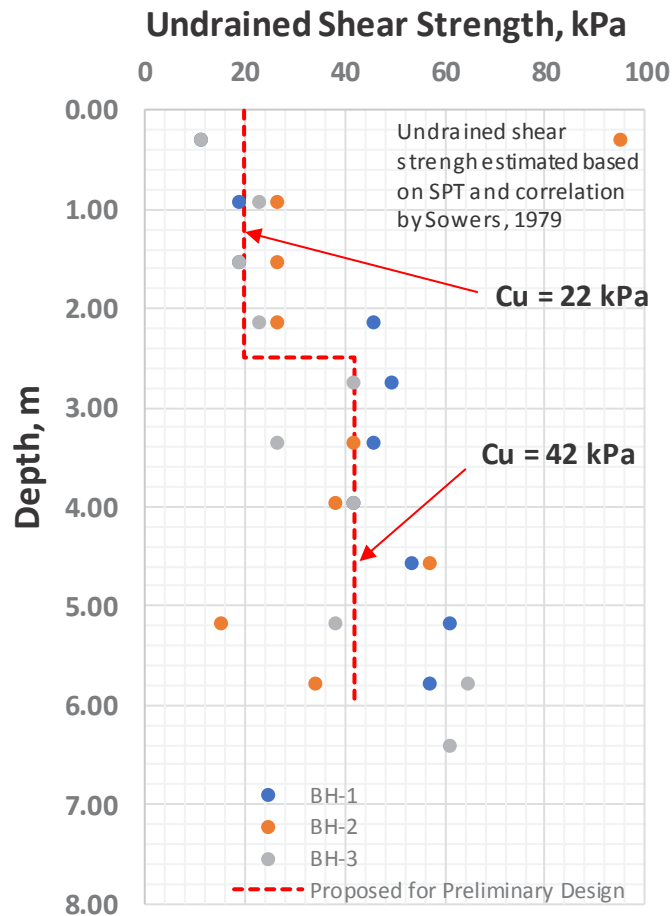
To determine the required depth of penetration, D, the active earth pressure and water pressure on the retaining wall could be estimated from the characteristics of the anchor, the sediment water pressure distribution on the timber pile wall can be modified as shown below in Figure 3.



**Figure 3. Schematic lateral pressure distribution on anchored sheet pile wall in clay**

If the water traffic is present on the wall, the water pressure of that traffic on the retaining wall could be added to the pressure distribution presented above.

As shown on Figure 3, the active pressure coefficient,  $K_a$ , unit weight of granular fill and undrained shear strength,  $c_u$ , of harbour sediment are required to estimate the water pressure on the wall or the purpose of retaining wall. The  $K_a$  and unit weight of granular fill can be assumed 0.3 and 1.0 kN/m<sup>3</sup> respectively. The recommended for retaining design undrained shear strength of the harbour sediment is shown below on Figure 4.



**Figure 4. Proposed undrained shear strength of harbour sediment**

The required depth of penetration is determined from the moment equilibrium about the support contact point. It is recommended to increase the estimated depth by 10% to 20% to provide an adequate factor of safety of 1.0 or more based on our preliminary estimate of penetration depth of the new timber pile. It could be that the

it is possible that the inter-pile resistance induced by the new pile may not fully develop due to the narrow space to be filled between the existing pile and the new timber pile. In the other hand, it is unclear how the existing pile and timber pile will interact with the new pile and the new timber pile will be difficult to model the condition accurately and the model is out of our current scope. Therefore, for the purpose of preliminary design, the full pile resistance on the new pile will be assumed to be acceptable and simplified to more conservative model.

### 5.2.3 Negative Friction (down drag)

After the new pile is installed through the stratum of cohesive soil, the downward movement of the consolidated sediment will cause a drag on the pile. The downward drag may cause settlement and reduce the capacity of the pile. However, based on our

understand there will be no vertical load on the new pile other than their own weight. Therefore potential settlement and reduced capacity of the new timber pile due to down drag will be minimal to no impact on the structural integrity of the wharf.

#### 5.2.4 Installation of Timber Piles

A 100mm M1000 when driven wood pile low-velocity hammer blow should be used for embedding and in the case of termite treatment should be re-treated immediately after incorporation of the soft curing in the case block. The use of the hammer used for the driven depends on experience and on a number of factors. The weight of the pile, the diameter of the head and the length of the pile should be considered. The hammer-rated energy should be about 1000 Joules and should not exceed 1000 Joules equal to 1000 Newton metre (1000 Nm) for the head diameter in metres.

The pile head should be protected with protection in the form of a steel cap and the pile toe should be protected with a steel shoe. Timber piles cannot withstand hard driving and driving will be restricted to the destruction of the pile. To avoid this, driving must be stopped when resistance to penetration is encountered. The set criteria should not exceed 100mm.

Our understanding is that the new retention wall will not be used to tie back and therefore no dredging is required. At the bottom of the trench should be excavated in the harbour bottom to accommodate installation of the new timber pile. Our understanding is that the proposed trench will be offset 1 to 2 m from the base of existing wall and should be 0.5 m deep. The trench is required to remove large cobbles and boulders from the harbour bottom that may obstruct the driving operation. The trench should be backfilled with granular fill following the completion of new pile installation.

### 5.3 New Granular fill

#### 5.3.1 Fill Gradation

The new granular fill to be placed between the new and the existing pile should consist of rock fill between 20 and 100 mm in size.

#### 5.3.2 Consolidation Settlement under the New Fill

The new fill placed between two walls, existing and new, will introduce additional weight on the soft harbour sediment causing it to settle. The settlement will consist of three components: immediate settlement, consolidation settlement and secondary compression creep. The magnitude of each component will depend on the nature and properties of the soil. Consolidation settlement dominates in saturated or near-saturated fine grain soils.

To accurately estimate the consolidation settlement under the new fill, a field vane test or a program combined with advanced consolidation testing is required. However, a program and testing is outside of the current scope.

Based on our preliminary estimate of the consolidation settlement under the new fill, not expected to exceed 20 cm with consolidation to be completed within four years or less. The estimate was made based on the following assumptions:

- The thickness of the new fill is 1 m
- Offset distance between the new and the existing timber pile wall is 0.5 m or less

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- The thickness of the soft clastic sediment range between 0 and 10 m and the clastic sediment underlain by low permeability bedrock and
- The confining conditions of the soft clastic sediment were controlled based on subsided emersion

## 6 CLOSURE

A geotechnical investigation is required on the limited amount of site. The recommendation contained in this report is based upon the condition encountered at the borehole location. It could in condition be encountered which differ from those at the borehole location. We request that we be notified immediately in order to permit revision of our recommendation.

This work has been undertaken in accordance with normal accepted electronic engineering practice. No other warrantable error is identified or implied. The limitation of this report is referred to in Appendix A. Advice was given to third parties of this information or information on or decision made based on the true reliability of such third parties. Amec is not a dealer or acceptor of no responsibility for damage suffered by third parties as a result of decision made or action based on this report.

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**AMEC Foster Wheeler Environment & Infrastructure**  
A division of AMEC Foster Wheeler Americas Limited

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## **APPENDIX A**

### **BOREHOLE LOGS**

## GENERAL REPORT NOTES

### STANDARD PENETRATION TEST—SPT

The standard penetration values are recorded on the Borehole Records as N values. The N values are the number of blows required to advance a standard, 50 mm diameter, split spoon sampler a distance of 305 mm into the soil using a 63.5 kg hammer freely falling a distance of 760 mm.

### DYNAMIC CONE PENETRATION TEST----DCPT

This is a similar procedure to that used in driving a standard 50 mm split spoon sampler except that a cone is driven rather than a soil sampler. A variety of cones can be used. Often the cones are 51 mm diameter with a 60 degree taper from the tip.

### SAMPLE TYPE ABBREVIATION USED ON BOREHOLE LOGS

S.S.	Split spoon	S. H.	Shelby tube	W.S.	Wash sample
A.S.	Auger sample	R. C.	Rock Core	P.	Sample pushed

### SOIL DESCRIPTION

The standard terminology to describe cohesionless soils includes the compactness condition as generally determined by the SPT.

The standard terminology to describe cohesive soils includes the consistency, which is based on various methods of determining undrained shear strength, and by SPT


Cohesionless Soils.		Cohesive Soils		
<u>Condition</u>	<u>N Values</u>	<u>Consistency</u>	<u>N Values</u>	<u>Undrained Shear Strength, kPa</u>
Very loose	0 – 4	Very soft	0 – 2	< 12.5
Loose	4 – 10	Soft	2 – 4	12.5 - 25
Compact	10 – 30	Firm	4 – 8	25 – 50
Dense	30 – 50	Stiff	8 – 15	50 - 100
Very Dense	> 50	Very stiff	15 – 30	100 – 200
		Hard	>30	>200

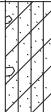
### NOTE

The soil conditions, profiles, comments, conclusions and recommendations found in this report are based upon samples recovered during the field work. Soils are heterogeneous materials, and, consequently, variations may be encountered at site locations away from where the samples were obtained. During construction, competent, qualified personnel should verify that no significant variations exist from those described in the report.

# LOG OF BOREHOLE BH1

SHEET 1 OF 1


PROJECT No.: <b>TV177001</b>	ELEVATION: ---	
CLIENT: <b>PWGSC</b>	DATUM:	
PROJECT NAME: <b>PRO Delhaven &amp; Hampton</b>	METHOD: <b>SS / Auger</b>	
LOCATION: <b>Hampton Warf</b>	DIAMETER: <b>100 mm</b>	
DATE DRILLED: <b>9-14-17</b>	WATER LEVEL:	
LOGGED BY: <b>A. Gale</b>	CONTRACTOR: <b>Nova Drilling</b>	

DEPTH (m)	ELEVATION (m)	STRATIGRAPHIC DESCRIPTION	STRATA PLOT	WATER LEVEL	SAMPLES					UNDRAINED SHEAR STRENGTH					PLASTIC LIMIT W <sub>P</sub>	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W <sub>L</sub>
					TYPE	NUMBER	RECOVERY (mm)	N-VALUE or RQD%	OTHER TESTS	△ Peak (kPa)    ▲ Residual							
										STANDARD PENETRATION TEST Blows/0.3m							
										20	40	60	80	●			
0		Red / Brown, silty SAND with cobbles, small boulders and clay			SS	1	25	3		●							
1  <																	

GEOTECHNICAL BOREHOLE TV177001\_HAMPTON WHARF.GPJ AMEC HALIFAX.GDT 10/18/17

# LOG OF BOREHOLE BH2

SHEET 1 OF 1

PROJECT No.: <b>TV177001</b>	ELEVATION: ---	
CLIENT: <b>PWGSC</b>	DATUM:	
PROJECT NAME: <b>PRO Delhaven &amp; Hampton</b>	METHOD: <b>SS / Auger</b>	
LOCATION: <b>Hampton Warf</b>	DIAMETER: <b>100 mm</b>	
DATE DRILLED: <b>9-14-17</b>	WATER LEVEL:	
LOGGED BY: <b>A. Gale</b>	CONTRACTOR: <b>Nova Drilling</b>	


DEPTH (m)	ELEVATION (m)	STRATIGRAPHIC DESCRIPTION	STRATA PLOT	WATER LEVEL	SAMPLES					UNDRAINED SHEAR STRENGTH					PLASTIC LIMIT	NATURAL MOISTURE CONTENT	LIQUID LIMIT		
					TYPE	NUMBER	RECOVERY (mm)	N-VALUE or RQD%	OTHER TESTS	△ Peak		▲ Residual							
										(kPa)									
										20	40	60	80						
STANDARD PENETRATION TEST										Blows/0.3m					WATER CONTENT (%)				
										20	40	60	80	●	W <sub>P</sub>	W	W <sub>L</sub>		
0		Red / Brown, silty SAND with cobbles, small boulders and clay			SS	1	100	25			●								
1		Red / Brown, sandy silty CLAY (CL-ML)			SS	2	325	7		●									
					SS	3	325	7		●									
2					SS	4	500	7		●									
					AU														
3					SS	5	25	11		●									
					SS		0	10		●									
4					SS	6	350	15		●									
					SS	7	300	4		●									
5					SS	8	450	9		●									
6																			
7		Red / Brown, sandy CLAY with occasional cobbles			AU														
8																			
		End of Borehole @ 8.8 m																	

GEOTECHNICAL BOREHOLE TV177001\_HAMPTON WHARF.GPJ AMEC HALIFAX.GDT 10/18/17



# LOG OF BOREHOLE BH3

SHEET 1 OF 1

PROJECT No.: <b>TV177001</b>	ELEVATION: ---	
CLIENT: <b>PWGSC</b>	DATUM:	
PROJECT NAME: <b>PRO Delhaven &amp; Hampton</b>	METHOD: <b>SS / Auger</b>	
LOCATION: <b>Hampton Warf</b>	DIAMETER: <b>100 mm</b>	
DATE DRILLED: <b>9-14-17</b>	WATER LEVEL:	
LOGGED BY: <b>A. Gale</b>	CONTRACTOR: <b>Nova Drilling</b>	

DEPTH (m)	ELEVATION (m)	STRATIGRAPHIC DESCRIPTION	STRATA PLOT	WATER LEVEL	SAMPLES					UNDRAINED SHEAR STRENGTH △ Peak (kPa)    ▲ Residual				PLASTIC LIMIT	NATURAL MOISTURE CONTENT	LIQUID LIMIT
					TYPE	NUMBER	RECOVERY (mm)	N-VALUE or RQD%	OTHER TESTS	STANDARD PENETRATION TEST Blows/0.3m				W <sub>p</sub>	W	W <sub>L</sub>
										20	40	60	80	10	20	30
0		Red / Brown, silty SAND with cobbles, small boulders and clay			SS	1	100	3		•						
1		Red / Brown, sandy silty CLAY (CL-ML)			SS	2	125	6		•						
					SS	3	600	5		•						
2					SS	4	375	6		•						
					SS	5	225	11		•						
3					SS	6	125	7		•						
					SS	7	125	11		•						
4					AU	8										
					SS	9	250	10		•						
5					SS	10	500	17	S, M	•				○		
6					SS	11	400	16		•						
		End of Borehole @ 6.7 m														

GEOTECHNICAL BOREHOLE TV177001\_HAMPTON WHARF.GPJ AMEC HALIFAX.GDT 10/18/17

## **APPENDIX B**

### **LAB TEST RESULTS**



## Appendix B. Summary of Laboratory Results

Sheet 1 of 1

Project No.: TV177001

Client: PWGSC

Project Name: PRO Delhaven & Hampton

Location: Hampton Warf

### GENERAL INFORMATION:

Number of BH/TP: 3  
Total Length of Drilling: 22.3 m

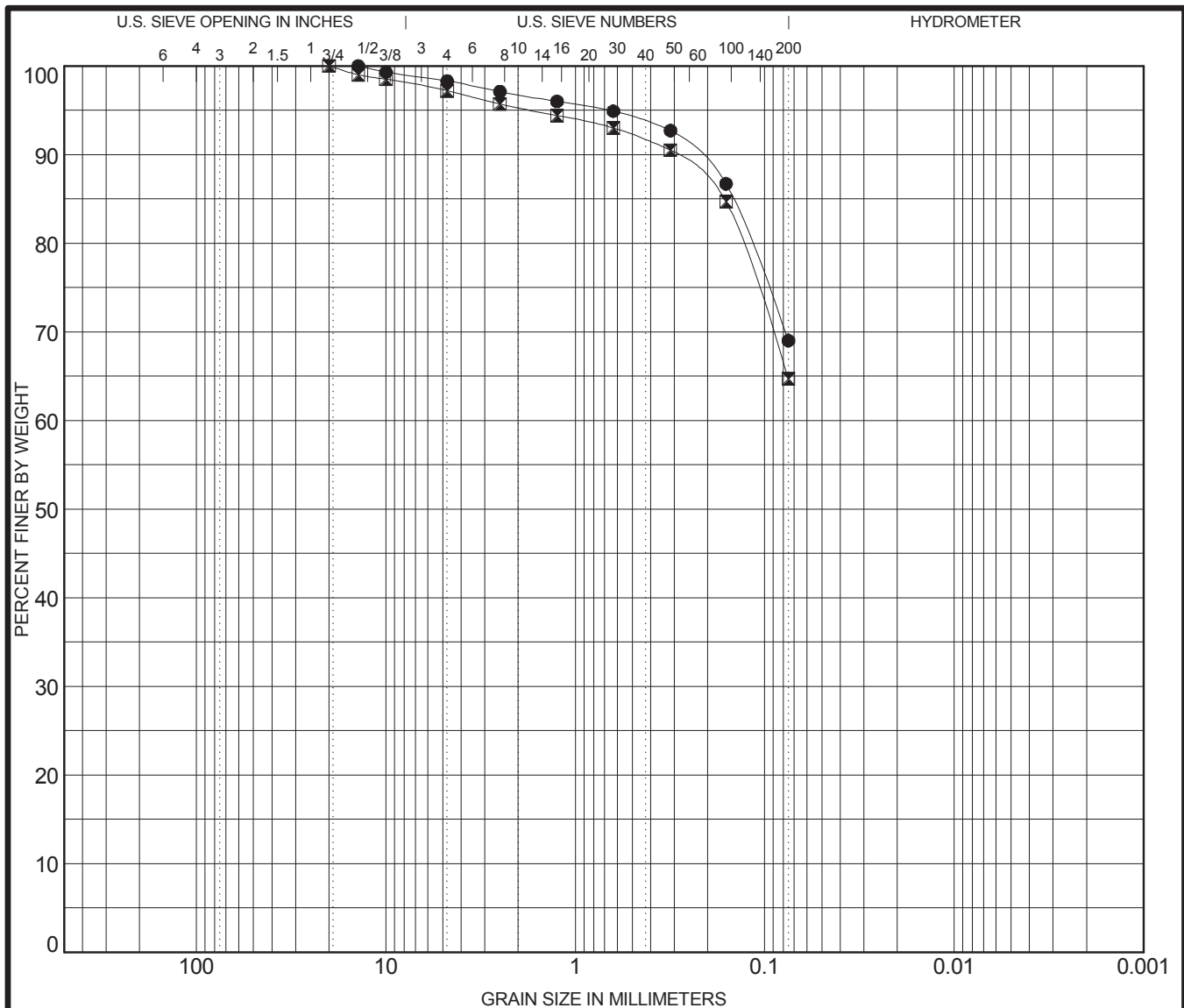
### SAMPLING:

Auger Cuttings 4  
Split Spoon 30

### LAB TEST STATISTICS:

Moisture Content: 2  
Atterberg Limits: 1  
Sieve Analysis: 2  
Hydrometer Analysis: 0

Borehole	Depth (m)	Liquid Limit	Plastic Limit	Plasticity Index	% Gravel	% Sand	% Silt and Clay	Class-ification	Water Content (%)	Dry Density (Mg/m <sup>3</sup> )	Void Ratio
BH1	2.13	20	13	7	1.7	29.3	69.0	CL-ML	20.0		
BH3	5.79				2.8	32.5	64.7	CL-ML	18.1		



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification		Classification				LL	PL	PI	Cc	Cu
●	BH1	2.13 m	SANDY SILTY CLAY(CL-ML)				20	13	7	
✕	BH3	5.79 m	SANDY SILTY CLAY(CL-ML)							
Specimen Identification		D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay	
●	BH1	2.13 m	14			1.7	29.3	69.0		
✕	BH3	5.79 m	20			2.8	32.5	64.7		

## GRAIN SIZE DISTRIBUTION

Project No.: TV177001

Client: PWGSC

Project Name: PRO Delhaven & Hampton

Location: Hampton Warf



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## **APPENDIX C**

### **REPORT LIMITATIONS**

## **REPORT LIMITATIONS**

The conclusions and recommendations given in this report are based on information determined at the test hole location. The information contained herein in no way reflects on the environment or effect of the project unless otherwise stated. Subsurface and groundwater conditions between and beyond the test hole may differ from those encountered at the test hole location and conditions may become different during construction which could not be detected or anticipated at the time of the site investigation. It is recommended practice that the geotechnical engineer be retained during the construction to confirm that the subsurface conditions across the site do not deviate materially from those encountered in the test hole.

The design recommendations given in this report are applicable only to the project described in the test and then only if constructed substantially in accordance with the details stated in this report. Since a detailed view of the design may not be shown, we recommend that we be retained during the final design stage to verify that the design is consistent with our recommendations and that assumptions made in our analysis are valid.

The comments made in this report relating to potential construction problems and possible methods of construction are intended only for the guidance of the designer. The number of test holes may not be sufficient to determine the effect of construction methods and cost for estimating the price of surficial or foundation work or for estimating and understanding the construction. The contractor bidding on this project or undertaking the construction should therefore make their own interpretation of the factual information presented and draw their own conclusions as to how the subsurface conditions may affect their work. This work has been undertaken in accordance with normally accepted geotechnical engineering practice. No other warranties are made or implied.

Anyone who is third party made of this report or in reliance on or decisions to be made based on the reasonable basis of such third parties. Amec Foster Wheeler, Environment or Infrastructure, accepts no responsibility for damages or losses suffered by a third party as a result of decisions made or actions based on this report.