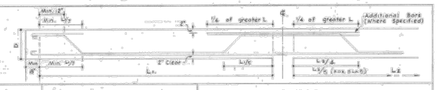
BEAM SCHEDULE

Dimensions are given looking at plan from bottom or right hand side.



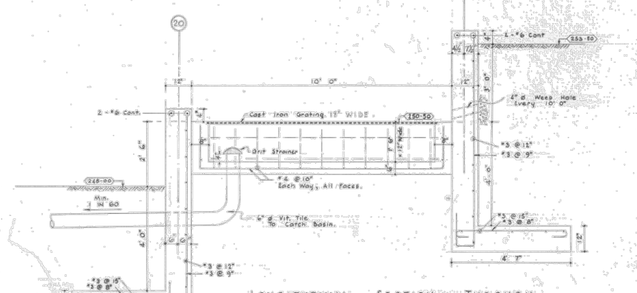
BEAM MARK	SIZE W	SIZE D	BOTTOM BARS BENT	BOTTOM BARS STR.	TOP BARS LEFT	TOP BARS CENTER	TOP BARS RIGHT	REMARKS	STIRRUPS SIZE	STIRRUPS TYPE	STIRRUPS LOCATED AT	STIRRUPS SPACING MARK THUS - 3/4 × 3 @ 4"
BASEMENT FLOOR												
B.B.1.	12"	24"	2-"7	2-"6	2-"9		2-"9		3/8" ϕ	[	Each End	3/9
B.B.2.	10"	16"	2-"5	1-"5					3/8" ϕ	-Do-	Each End.	2/12.
B.B.3.	12"	24"	2-"7	2-"7				3-"8 Extra Top Bars at Supports Between B.B.3. & B.B.3	3/8" ϕ	-Do-	Lines E & I, Line G.	3/12, 2/9, 3/12.
B.B.4.	12"	24"		2-"6 / 2-"5	2-"6		2-"6		3/8" ϕ	-Do-	Each End.	2/12.
B.B.4A.	12"	24"		4-"7	2-"9 / 1-"8		2-"9 / 1-"8	Top Bars Cont into B.B.4a.	3/8" ϕ	-Do-	Each End.	3/12.
B.B.4B.	12"	24"		2-"6 / 2-"5	2-"9 / 1-"8		2-"6	2-"9 / 1-"8 From B.B.4A	3/8" ϕ	-Do-	Lines A & E, Lines A & C	4/12, 3/12.
B.B.5.	24"	36"	5-"9	4-"9				1-"9 Extra Top Bar Over Int. Support. Extend Bottom Bars into B.B.6.	1/2" ϕ	-Do-	Lines 9 & 21, Lines 8 & 20	2/12, 3/12.
B.B.6.	24"	36"	4-"9	3-"9				2-"9 Extra Top Bars Over Supports Between B.B.6 & B.B.5.	1/2" ϕ	-Do-	Each End	2/12.
B.B.7.	12"	36"		6-"8	4-"7		4-"7	Extend Top Bars Over Supports Between B.B.7. & B.B.7.	1/2" ϕ	-Do-	Each End	2/12.
B.B.7A.	12"	36"		4-"7				4-"7 Top Bars At Ext. Support	1/2" ϕ	-Do-	Each End	2/12.
B.B.7B.	12"	36"		4-"7				4-"7 Top Bars Over Supports	3/8" ϕ	-Do-	Each End	3/12.
B.B.7C.	24"	36"		5-"7				5-"7 Top Bars Over Supports	3/8" ϕ	-Do-	Each End	2/12.
B.B.8.												
B.B.8A.	12"	24"		2-"7 / 1-"6				2-"7 / 1-"6 Top Bars Over Support Between B.B.8A & B.B.7A. or B.B.8A	3/8" ϕ	-Do-	Each End	2/12.
B.B.9.	24"	36"	5-"10 (in 2 Rows)	4-"10		2-"7 F.L.		3-"10 Extra Top Bars Over Support Between B.B.9 & B.B.10. 8-"10 Extra Bottom Bars Over Support Between B.B.9. & B.B.10.	1/2" ϕ	-Do-	Lines 9 & 21, Lines 11 & 19	3/12, 1/9, 4/12.
B.B.10.	24"	36"	5-"10	3-"10				2-"10 Extra Top Bars & 4-"10 Extra Bottom Bars Between B.B.10. & B.B.10.	1/2" ϕ	-Do-	Each End	2/12.
B.B.11.	24"	36"	4-"9	3-"9				2-"9 Extra Top Bars Over Support Between B.B.11. & B.B.12.	1/2" ϕ	-Do-	Each End	2/12.
B.B.12.	24"	36"	5-"9	3-"9				5-"8 Extra Top Bars Over Support Between B.B.12. & B.B.12	1/2" ϕ	-Do-	Each End	2/12.
B.B.13.	24"	36"		6-"8	3-"8		7-"8	7-"8 Bars From B.B.12.	1/2" ϕ	-Do-	Each End	2/12.
B.B.14.	24"	36"		4-"8		Y-"8 F.L.		Extend Top Bars into B.B.13.	1/2" ϕ	-Do-	Each End	2/12.
B.B.15.	11"	58"		2-"6		2-"5 F.L.		10-"4 Bars Spaced Between Top & Bottom Bars (See Section).	1/4" ϕ	-Do-	Throughout	At 18" c/c.
B.B.16.	11"	36"	2-"9	2-"5 / 2-"8				Bottom Bars in 2 Rows (See Section D-D, DWG No. S-3)	3/8" ϕ	-Do-	Line B, Line Y.	2/12, 2/9.
FIRST FLOOR												
I.B.1.	12"	20"	3-"9	3-"8				Bottom Bars in 2 Rows	1/2" ϕ	[	Each End	3/9.
I.B.2.	20"	20"	6-"8	3-"8				(See Section) (Dwg No. S-3)	3/4" ϕ	-Do-	Line E, Line G	4/9, 3/3, 3/6, 2/9.
I.B.3.	12"	24"	2-"7	1-"7				1-"7 Extra Top Bar Over Int. Common Support.	3/8" ϕ	-Do-	Each End	3/10.
I.B.4.	12"	24"	2-"8	2-"8				2-"8 Extra Top Bars Over Int. Common Support.	3/8" ϕ	-Do-	Line E, Line G	3/9, 3/9.
I.B.5. / I.B.5A.	8" / 8"	12" / 21"	2-"4	1-"5				1-"4 Extra Top Bar Over Supports Between I.B.5. & I.B.6.	1/4" ϕ	-Do-	Lines 7 & 21, Lines 8 & 20	2/12, 1/6, 1/12.
I.B.6 / I.B.6A	8" / 8"	12" / 21"	2-"4	1-"4					1/4" ϕ	-Do-	Each End	2/12.
I.B.7.	8"	14"	2-"6	1-"6					1/4" ϕ	-Do-	Each End	2/12.
I.B.10.	18 1/2"	19 1/2"		3-"6 / 2-"6				2-"6 Top Bars At Ext. Support. 3-"6 / 2-"5 Top Bars Over Int. Supports	3/8" ϕ	-Do-	Each End	2/12.
I.B.11.	15 3/4"	25"		3-"6 / 2-"6				3-"6 Top Bars Over Supports.	3/8" ϕ	-Do-	Each End	2/12.
I.B.12.	25"	8"	4-"5	4-"5				(See Section) (Dwg No. S-3)	1/4" ϕ	-Do-	Each End	2/12.
I.B.13.	10"	12"	2-"8	1-"8					1/2" ϕ	-Do-	Each End	3/12
I.B.15.	8"	12"	2-"5	1-"5				(See Section) (Dwg No. S-3)	1/4" ϕ	-Do-	Each End.	2/12.

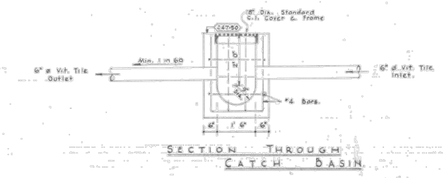
SLAB SCHEDULE

SLAB MARK	THICKNESS	MAIN REINFORCEMENT	TEMP. REINFORCEMENT	REMARKS.
BASEMENT FLOOR				
B.S.1.	6"	#4 @ 15" } #5 @ 15" } Placed Alternatively	#4 @ 16"	
B.S.2.	6"	#4 @ 7 1/2" Alt. Bt. & Str.	#4 @ 16"	
B.S.3.	6"	#4 @ 8" Str. Bottom; #4 @ 15" Str. Top	#3 @ 15" Top & Bottom.	
B.S.4.	6"	#4 @ 9" Alt bent & straight	#3 @ 15"	#4 @ 9" Extra top bars over common support.
FIRST FLOOR				
I.S.1.	4"	#5 @ 17" } #4 @ 17" } Placed Alternatively	#3 @ 15"	#3 @ 8 1/2" Extra Top Bars Over Supports Between: I.S.1. & I.S.2.
I.S.2.	4"	#4 @ 8 1/2" Alt. Bt. & Str.	#3 @ 15"	#3 @ 8 1/2" Extra Top Bars Over Supports Between: I.S.2. & I.S.2.
I.S.3.	4"	#4 @ 10" Str. Bottom, #3 @ 10" Str. Top	#3 @ 15" Top & Bottom.	Extend Top Bars into I.S.6.
I.S.4.	4 1/2"	#4 @ 9" Alt bent & straight	#3 @ 15"	#4 @ 9" Extra top bars over common support
I.S.5.	4 1/2"	#4 @ 4 1/2" Alt bent & straight		

COLUMN SCHEDULE

Column Mk	3E, 6E, 7E.	6G & 7G	5G	2E, 3E, 4E, 5E, 2F, 3F, 4F, 5F, 2G, 3G, 4G, 5G	GB to G2I (INCLUSIVE)	AB to A2I (INCLUSIVE)	A1, A5, B5 & D5	COLS FOR COVERED WALK	B3 & D3	A6	A2 & C
Top of Conc. Roof Slab											
Steel (10" above Fin. 2nd Floor)	6H20	6H20	6H20	6H20	6H20	6H20	8I18-4			8H24-75	6-6H18-9
Steel (13'-0" above Fin. 1st Floor)	8H24-75	6H20	8H24-75	6H20	6H20	6H20	8I18-4	8 STRIPS		8H24-75	6-6H18-9
baseplates Anchor Bolts	11" × 11" × 3/4" 4 - 3/4" ϕ	9" × 9" × 1/2" 4 - 3/4" ϕ	11" × 11" × 3/4" 4 - 3/4" ϕ	8" × 8" × 1/2" 2 - 3/4" ϕ	8" × 8" × 1/2" 2 - 3/4" ϕ	8" × 8" × 1/2" 2 - 3/4" ϕ	8" × 8" × 1/2" 2 - 3/4" ϕ	6" × 6" × 1/2" 2 - 3/4" ϕ	6" × 6" × 1/2" 2-3/4" ϕ	6" × 6" × 1/2" 2- 3/4" ϕ	6" × 6" × 1/2" 2- 3/4" ϕ
Concrete. Piers (13'-0" above Fin. Basement Floor)	12" × 12" 4 - 15 #3 @ 12"	12" × 12" 4 - 11 #3 @ 12"	18" × 36" 4 - 15 #3 @ 12"	12" × 12" 4 - 15 #3 @ 12"	12" × 12" 4 - 15 #3 @ 12" See Section A-A	16 × 12" 4 - 11 #3 @ 12" (See Section A-A)	Wall	Wall	Wall	10" × 12" 4 @ 10" #3 @ 12"	12" × 12" 4 @ 6 #3 @ 12"	WALL
Dowels.	4 - #9	4 - #7	4 - #8	4 - #6	6 - #8	4 - #7				4 @ 6	





GENERAL NOTES:—

- ALL CONCRETE TO HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3,000 P.S.I. AT THE END OF 28 DAYS, UNLESS OTHERWISE SPECIFIED.
- ALL REINFORCING STEEL TO BE INTERMEDIATE OR HARD GRADE STEEL DEFORMED BARS.
- HORIZONTAL BARS IN WALLS TO BE LAPPED 24" AT ALL CORNERS AND SPLICES.
- CONCRETE LINTELS OVER ALL OPENINGS IN INTERIOR PARTITIONS TO BE EQUAL TO THE WALL THICKNESS. DEPTH SHALL BE 8" FOR SPANS UP TO 6'-0", REINFORCED WITH 1-#4 BAR TOP AND BOTTOM FOR EACH 4" OF THICKNESS.
- STEEL COVER:— SLABS INTERIOR FACE OF BASEMENT WALLS 3/4" BEAMS & COLUMNS 1 1/2" EXTERIOR FACE OF BASEMENT WALLS 2" FOOTINGS AS INDICATED AND SPECIFIED 3"
- MEMBRANE WATERPROOFING AS INDICATED AND SPECIFIED.
- ALL SLABS ON GRADE TO BE REINFORCED AS SHOWN ON PLAN OR WITH EQUIVALENT WELDED WIRE MESH.
- PROVIDE RUBBER WATER STOPPER AS INDICATED AND SPECIFIED.
- GROUT UNDER ALL BASEPLATES OF STEEL COLUMNS TO PROVIDE FULL BEARING.
- CONTRACTOR TO PAY SPECIAL ATTENTION TO CORRECTLY POSITIONING ALL PIPE SLEEVES IN R.C. BEAMS. CONTRACTOR SHALL WORK IN CLOSE COORDINATION WITH THE MECHANICAL CONTRACTOR WITH RESPECT TO LEAVING & PREREQUISITE NUMBER OF OPENINGS FOR PIPES, VENTILATION DUCTS, ETC., PASSING THROUGH R.C. WALLS. THESE OPENINGS SHALL BE REINFORCED AS SPECIFIED IN DETAIL ON DRAWING No. S.2.
- REINFORCING FOR R.C. WALLS
  - 12" WALL #4 @ 12" c/c EACH WAY, EACH FACE.
  - 10" WALL #4 @ 12" c/c " " " "
  - 8" WALL #4 @ 10" c/c " " AT MID SECTION

REVISIONS DATE	RECORD

DEPARTMENT OF PUBLIC WORKS OF CANADA
BUILDING CONSTRUCTION BRANCH — E. A. GARDNER, CHIEF ARCHITECT

OTTAWA, ONTARIO.
RIVERSIDE DRIVE DEVELOPMENT
OFFICE BUILDING FOR DEPARTMENT of INSURANCE.

DRAWN BY: A.D.L.	CHECKED BY: J.D.V.	APPROVED BY: J.D.V.	SCALE: AS NOTED	DRWG. NO. S. 6 OF 6
DATE:	DATE:	DATE: 31-5-18	PLAN FILE NO.: 676	

Job No. CA 58 - 5-12

14-A

Canada

Public Works and Government Services Canada / Travaux publics et services gouvernementaux Canada

Project Delivery & Professional/Technical Services
Architecture & Interior Design
Real Property Operations Solutions

Exécution de Projets et Services Professionnels/ Techniques
Architecture et Design d'intérieur
Solutions des Opérations Immobilière

Adjeleian Allen Rubeli
Consulting Engineers
75 Albert Street, Ottawa, Ontario
5255 Yonge Street, Toronto, Ontario

AAR PROJECT 5806-00

N

LICENSED PROFESSIONAL ENGINEER — J. D. VIENNEAU — 100176742 — JULY 11 2018 — PROVINCE OF ONTARIO

0	ISSUED FOR TENDER DOCUMENT DE SOUMISSION	JULY 11 2018
B	ISSUED FOR 99% REVIEW DOCUMENT À 99%, À FAIRE RÉVISER.	MAY 17 2018
A	ISSUED FOR 66% REVIEW DOCUMENT À 66%, À FAIRE RÉVISER.	APRIL 17 2018
revisions	description	date

A detail no. no. du détail
B location drawing no. sur dessin no.
C drawing no. dessin no.

project / projet

INSURANCE BUILDING DECONSTRUCTION / DÉCONSTRUCTION DE L'ÉDIFICE DES ASSURANCES

770 HERON ROAD / 770 CHEMIN HERON
CONFEDERATION HEIGHTS CAMPUS / CAMPUS DE LA CONFÉDÉRATION
OTTAWA, ONTARIO CANADA

drawing / dessin

ORIGINAL STRUCTURAL DRAWING S6 (NOT TO SCALE) / DESSIN STRUCTUREL ORIGINAL S6 (PAS À L'ÉCHELLE)

Designed By / Conçu par	J. VIENNEAU
Date	(yyyy/mm/dd)
Drawn By / Dessiné par	M. KACHI
Date	(yyyy/mm/dd)
Reviewed By / Examiné par	J. VIENNEAU
Date	(yyyy/mm/dd)
Approved By / Approuvé par	J. VIENNEAU
Date	(yyyy/mm/dd)
Tender / Soumission	RICHARD CZUBA
Project Manager / Administrateur de projets	
Project no. / No. du projet	R.083733.100
Drawing no. / No. du dessin	S505

A1 594 x 841

0 10 20mm 40 60 80 100 120 140 160 180 200mm

C