Reference No. 11155201



April 6, 2018

Ms. Annie Dumas, P. Eng.
Project Manager, Energy
Aecom Consultants Inc.
85 Sainte Catherine Street West
Montreal, Quebec H2X 3P4

Dear Ms. Dumas:

Re: Concrete Core Testing – Hogs Back Dam Hogs Back Dam, Ottawa, Ontario

1. Introduction

GHD was retained by Ms. Annie Dumas of Aecom Consultants Inc. to undertake a concrete coring and laboratory testing program for the Hogs Back Dam located in Ottawa, Ontario. Four cores samples were sampled from various structural elements of the Hogs Back Dam and underwent further laboratory analysis. The field work was complete on March 1, 2018 and the laboratory was completed in accordance with clients requirements summarized in GHD's proposal number 11103730LaFlamme-1 dated November 20, 2017.

2. Site and Project Description

The Hogs Back Dam is located in Ottawa, Ontario and is part of the Rideau River. The Dam is on the north side of Hogs Back Road and is adjacent to the Hogs Back Lock Station which is part of the Rideau Canal. See attached Corehole Location Plan attached as Figure 1.

It is our understanding the work is part of a proposed repair program for the Hogs Back Dam and the scope of work was completed in accordance with the requirements provided by Mr. Martin LaFlamme of Aecom.

3. Field and Laboratory Program

3.1 Field

The coring took place on March 1, 2018 and followed the approved sampling program described below as well as the Environmental Protection Plan issued February 22, 2018 and Pedestrian Management Plan issued February 21, 2018.

Aecom selected the core locations and GHD assisted with selection of final position based on site conditions, such as reinforcing steel and bearing pad locations. A GHD subcontractor under GHD's supervision completed the coring and patching of the core holes with an approved grout product.





The coring program consisted of sampling 100mm diameter concrete cores from the following locations and depths:

- (CH1) Pier 2 1100 mm depth cored through the pedestrian deck and into the pier.
- (CH2) Pier 5 850 mm depth cored through the precast pedestrian deck and into the pier.
- (CH3) Pier 7 850 mm depth cored through the precast pedestrian deck and into the pier.
- (CH4) East Abutment 850 mm depth cored through the pedestrian deck and into the abutment.

3.2 Laboratory

Upon completion of the field-coring program the samples were delivered to GHD's laboratory for photo logging, compression strength testing, mass density, carbonation, and alkali silica reaction (ASR). The results are summarized below and included in Attachment B.

4. Summary of Laboratory Results

4.1 Compressive Strength

The two tests for compressive strength of each core were selected from the upper 300 mm and then lower 300 mm from the top of deck. The purpose of the two tests were to collect results on the precast concrete deck that is on top of Pier 5 and 7 as well as the cast in place concrete that is on top of Pier 2 and the East Abutment. The precast deck for Pier 5 and 7 compressive strengths averaged 61.5 MPa, and the results for the upper 300 mm for Pier 2 and the East Abutment were 60.6 MPa and 45.3 MPa. Respectively the average compressive strength for the concrete below 300 mm in all the structures was 41.3 MPa. A full results table is provided in Attachment B.

4.2 Mass Density

The cores selected for compressive strength testing were also measured to determine their mass density. The average for the precast concrete of Pier 5 and 7 was 2371 kg/m³. The average for the cast in place concrete for all remaining cores was 2378 kg/m³. All results can be found in the compressive strength tables below in Attachment B

4.3 Carbonation

The cast in place concrete for each structure was analyzed for the presence of carbonation. There was no observable presence of carbonation found in the cores except for the presence of early onset carbonation observed in the first 1 mm of the East Abutment. The summary of carbonation tables is provided in Attachment B.



4.4 Alkali Silica Reaction (ASR)

The cast in place concrete for each structure was visually examined for the presence of ASR. The full summary tables for the ASR screening tests can be found in Attachment B. In general no observable evidence of ASR was found in the concrete.

Should you have any questions on the above, please do not hesitate to contact us.

Sincerely,

GHD

Eric Bennett, C.E.T.

Gerardo Cardenas, P. Eng., MASc.

EB/vl/3 Encl.



Source: ESRI Basemap Imagery, Date Unknown





AECOM CONSULTANTS INC. HOGS BACK DAM OTTAWA, ONTARIO

COREHOLE LOCATION PLAN

11155201-B1 Mar 26, 2018

FIGURE 1

Attachment A Core Log Photographs



CH1-1 Pier 2 – 0 mm to 300 mm



CH1-2 Pier 2 – 300 mm to 430 mm



Core Log Photographs



CH1-3 Pier 2 - 430 mm to 890 mm



CH1-3 Pier 2 – 890 mm to 1100 mm



Core Log Photographs



CH2-1 Pier 5 - 0 mm to 300 mm



CH2-2 Pier 5 - 300 mm to 850 mm



Core Log Photographs



CH3-1 - Pier 7 - 0 mm to 300 mm



CH3-2 - Pier 7 - 300 mm to 850 mm



Core Log Photographs



CH4-1 - East Abutment - 0 mm to 300 mm



CH4-2 - East Abutment - 300 mm to 850 mm



Core Log Photographs

Attachment B Laboratory Results



March 13, 2018

Re: Hogs Back Dam Our Project No. 11155201-B1

Summary of Concrete Core Test Results					
	Client Core #:	CH1	CH1	CH2	CH2
Core Data	GHD Core #:	CST-18-20A	CST-18-20B	CST-18-21A	CST-18-21B
	Core Location (depth of test)	Pier 2 15 to 220mm	Pier 2 520 to 725mm	Pier 5 15 to 220mm	Pier 5 685 to 890mm
ry Test Its	Compressive Strength of Concrete Core				
ato esu	MPa	60.6	36.6	59.0	38.2
Re	L/D Ratio	2.00	2.00	1.92	2.00
Lat	Concrete Core Density kg/m ³	2387	2343	2367	2343
Remarks	Testing Information: Compressive Strength of Concrete Cores as per LS-410 Depths from surface of core.				

21. 2000 \sim

Matt Rawlings, Laboratory Manager



March 13, 2018

Re:	Hogs Back Dam
	Our Project No. 11155201-B1

	Summary of Concrete Core Test Results					
	Client Core #:	CH3	CH3	CH4	CH4	
ore ata	GHD Core #:	CST-18-22A	CST-18-22B	CST-18-23A	CST-18-23B	
0 1	Core Location (depth of test)	Pier 7 20 to 225mm	Pier 7 530 to 735mm	E. Abutment 90 to 230mm	E. Abutment 320 to 530mm	
ry Test Ilts	Compressive Strength of Concrete Core					
ato esu	MPa	64.0	41.8	45.3	48.7	
n n n	L/D Ratio	1.97	2.00	1.27	2.00	
Lat	Concrete Core Density kg/m3	2375	2385	2386	2424	
Remarks	Testing Information: Compressive Strength of Concrete Cores as per LS-410 Depths from surface of core.					

W1. R.

Matt Rawlings, Laboratory Manager



Attention:

Re: Hogs Back Dam Our Project No. 11155201-B1

Summary of Carbonation Testing				
Core Data	Client Core #:	CH1		
	GHD Core #:	CST-18-20		
	Core Location	Pier 2		
	Date Tested	Mar 12, 2018		
Laboratory Test Results	Diameter of Sample (mm)	95mm		
	Depth of Sample (mm)	280 to 520mm		
	Results of Carbonation Testing	With the addition of phenolphthalein a dark pink colour change was observed throughout the full depth of the test specimen.		
	Conclusion	Based on our analysis, evidenced by the dark pink colour change, it is concluded that there is no observable evidence that carbonation has occurred in this test specimen.		
ıarks	Testing Information: Carbonation determined by a	pplication of phenolphthalein solution.		
Ren	No Surface membrane present			

21. C.____

Matt Rawlings, Laboratory Manager



Attention:

Re:

Hogs Back Dam Our Project No. 11155201-B1

Summary of Carbonation Testing				
	Client Core #:	CH2		
Core Data	GHD Core #:	CST-18-21		
	Core Location	Pier 5		
	Date Tested	Mar 12, 2018		
sults	Diameter of Sample (mm)	95mm		
Laboratory Test Re	Depth of Sample (mm)	280 to 430mm		
	Results of Carbonation Testing	With the addition of phenolphthalein a dark pink colour change was observed throughout the full depth of the test specimen.		
	Conclusion	Based on our analysis, evidenced by the dark pink colour change, it is concluded that there is no observable evidence that carbonation has occurred in this test specimen.		
ıarks	Testing Information: Carbonation determined by a	oplication of phenolphthalein solution.		
Ren	No Surface membrane preser	nt		

Matt Rawlings, Laboratory Manager



Attention:

Re: Hogs Back Dam Our Project No. 11155201-B1

Summary of Carbonation Testing				
Core Data	Client Core #:	CH3		
	GHD Core #:	CST-18-22		
	Core Location	Pier 7		
Laboratory Test Results	Date Tested	Mar 12, 2018		
	Diameter of Sample (mm)	95mm		
	Depth of Sample (mm)	380 to 530mm		
	Results of Carbonation Testing	With the addition of phenolphthalein a dark pink colour change was observed throughout the full depth of the test specimen.		
	Conclusion	Based on our analysis, evidenced by the dark pink colour change, it is concluded that there is no observable evidence that carbonation has occurred in this test specimen.		
narks	Testing Information: Carbonation determined by a	pplication of phenolphthalein solution.		
Ren	No Surface membrane present			

W1. R. _____

Matt Rawlings, Laboratory Manager



Attention:

Re: Hogs Back Dam Our Project No. 11155201-B1

Summary of Carbonation Testing				
Core Data	Client Core #:	CH4		
	GHD Core #:	CST-18-23		
	Core Location	East Abutment		
	Date Tested	Mar 12, 2018		
sults	Diameter of Sample (mm)	100mm		
Res	Depth of Sample (mm)	0 to 70mm		
Laboratory Test	Results of Carbonation Testing	With the addition of phenolphthalein a lighter pink colour change occurred at 0 to ~1mm from the surface of the test specimen, and a dark pink colour change was observed throughout the remaining depth.		
	Conclusion	Based on our analysis, the observation of the lighter pink colouration indicates the early onset of carbonation at 0 to ~1mm. However, there is no observable evidence that carbonation has occurred beyond the ~1mm depth.		
ıarks	Testing Information: Carbonation determined by a	oplication of phenolphthalein solution.		
Ren	No Surface membrane preser	nt		

W1. R._____

Matt Rawlings, Laboratory Manager



March 13, 2018

Re: Hogs Back Dam Our Project No. 11155201-B1

Summary of ASR Screening				
re Ita	Client Core #:	CH1		
	GHD Core #:	CST-18-20		
പ് പ്	Core Location	Pier 2		
	Sample Depth:	280 to 520mm from surface		
	General Concrete Co	mments		
	The concrete test face was visually examined without magnification and the following comments were noted:			
	 The largest observable coarse aggregate within the concrete was 16 mm, consisting predominately of crushed particles. 			
	 I he concrete paste was grey in colour The paste was noted visually to contain several small to medium air voids, indicating the presence of air-entrainment 			
ts	• Complete, uneven horizontal fracture at a depth of ~405 to 440mm from surface of core			
sul	 No visible micro-cracks travelling through or extending between aggregate particles No visible ASR gel or reaction rims present in test sample 			
t Re	ASR Screening Test (SHRP-C-315 Comments)			
Test	 After the addition of the yellow reagent there was no visible yellow colour change to the surface of the test face. 			
atory	• With the addition of the red reagent, areas around/within air voids located in the cement paste turned a bright yellowish-orange, however no bright red colouration was observed.			
ora	Summary			
Labo	The sample tested was comprised predominately of crushed aggregates with the largest observable size being 16 mm. There was no observable evidence of ASR prior to the conditioning of the test surface. Observation under UV light after the addition of the yellow reagent did not produce the bright yellow colouration that is indicative of ASR. The addition of the red reagent caused the areas around and within some of the air voids located in the cement paste to present as a bright yellowy-orange glow but did not express the bright red colouration under UV lighting indicative of advanced ASR. In accordance with SHRP-C-315 the above results show that there is no observable evidence that alkali silica reactivity has occurred in the concrete. Coupled with the test results there were no common identifiers to suggest ASR has/is occurring, such as: micro-cracks which extend through or between aggregates, observable reaction rims or ASR gel product			

W.R.C.

Matt Rawlings, Laboratory Manager



March 13, 2018

Re: Hogs Back Dam Our Project No. 11155201-B1

Summary of ASR Screening				
re ita	Client Core #:	CH2		
	GHD Core #:	CST-18-21		
õँ	Core Location	Pier 5		
	Sample Depth:	280 to 430mm from surface		
	General Concrete Co	mments		
ts	The concrete test face wa noted:	as visually examined without magnification and the following comments were		
	 The largest observable coarse aggregate within the concrete was 17 mm, consisting predominately of crushed particles. The concrete paste was grey in colour The paste was noted visually to contain several small to medium air voids, indicating the presence of air-entrainment No visible micro-cracks travelling through or extending between aggregate particles 			
nsə	No visible ASR gel or reaction rims present in test sample			
Ŗ	ASR Screening Test (SHRP-C-315 Comments)			
est	• After the addition of a of the test face.	the yellow reagent there was no visible yellow colour change to the surface		
tory 1	 With the addition of t turned a bright yellow 	he red reagent, areas around/within air voids located in the cement paste vish-orange; however, no bright red colouration was observed.		
orat	Summary			
Labo	The sample tested was comprised predominately of crushed aggregates with the largest observable size being 17 mm. There was no observable evidence of ASR prior to the conditioning of the test surface. Observation under UV light after the addition of the yellow reagent did not produce the bright yellow colouration that is indicative of ASR. The addition of the red reagent caused the areas around and within some of the air voids located in the cement paste to present as a bright yellowy-orange glow but did not express the bright red colouration under UV lighting indicative of advanced ASR. In accordance with SHRP-C-315 the above results show that there is no observable evidence that alkali silica reactivity has occurred in the concrete. Coupled with the test results there were no common identifiers to suggest ASR has/is occurring, such as: micro-cracks which extend through or between aggregates, observable reaction rims or ASR gel product			

M.R.C.

Matt Rawlings, Laboratory Manager



March 13, 2018

Re: Hogs Back Dam Our Project No. 11155201-B1

Summary of ASR Screening					
re Ita	Client Core #:	CH3			
	GHD Core #:	CST-18-22			
õँ	Core Location	Pier 7			
	Sample Depth:	380 to 530mm from surface			
	General Concrete Co	mments			
	The concrete test face wa noted:	The concrete test face was visually examined without magnification and the following comments were noted:			
	 The largest observation predominately of cruit 	 The largest observable coarse aggregate within the concrete was 19 mm, consisting predominately of crushed particles. 			
	 ~20mm dia. re-enforcing steel present at ~520 to 540mm from surface of core 				
	 The paste was noted visually to contain several small to medium air voids, indicating the 				
ts	presence of air-entrainment				
sul	 No visible micro-cracks travelling through or extending between aggregate particles No visible ASR gel or reaction rims present in the test sample 				
t Re	ASR Screening Test (SHRP-C-315 Comments)				
Test	 After the addition of the yellow reagent there was no visible yellow colour change to the surface of the test face. 				
atory	• With the addition of the red reagent, areas around/within air voids located in the cement paste turned a bright yellowish-orange, however no bright red colouration was observed.				
ors	Summary				
Lab	The sample tested was comprised predominately of crushed aggregates with the largest observable size being 19.0mm. There was no observable evidence of ASR prior to the conditioning of the test surface. Observation under UV light after the addition of the yellow reagent did not produce the bright yellow colouration that is indicative of ASR. The addition of the red reagent caused the areas around and within some of the air voids located in the cement paste to present as a bright yellowy-orange glow but did not express the bright red colouration under UV lighting indicative of advanced ASR. In accordance with SHRP-C-315 the above results show that there is no observable evidence that alkali silica reactivity has occurred in the concrete. Coupled with the test results there were no common identifiers to suggest ASR has/is occurring, such as: micro-cracks which extend through or between aggregates, observable reaction rims or ASR gel product				

W.R. \leq

Matt Rawlings, Laboratory Manager

GHD Limited



March 13, 2018

Re: Hogs Back Dam Our Project No. 11155201-B1

Summary of ASR Screening				
re ita	Client Core #:	CH4		
	GHD Core #:	CST-18-23		
őö	Core Location	East Abutment		
	Sample Depth:	0 to 70mm from surface		
	General Concrete Co	mments		
Laboratory Test Results	 The concrete test face was visually examined without magnification and the following comments were noted: The largest observable coarse aggregate within the concrete was 16 mm, consisting predominately of crushed particles. The concrete paste was grey in colour The paste was noted visually to contain several small to medium air voids, indicating the presence of air-entrainment No visible micro-cracks travelling through or extending between aggregate particles ASR Screening Test (SHRP-C-315 Comments) After the addition of the yellow reagent there was no visible yellow colour change to the surface of the test face. With the addition of the red reagent, areas around/within air voids located in the cement paste 			
	Summary The sample tested was considered by the sample tested was considered within the surface. Observation und yellow colouration that is and within some of the air glow but did not express accordance with SHRP-Consilica reactivity has occur identifiers to suggest ASF aggregates, observable of the surface of the surfa	omprised predominately of crushed aggregates with the largest observable was no observable evidence of ASR prior to the conditioning of the test er UV light after the addition of the yellow reagent did not produce the bright indicative of ASR. The addition of the red reagent caused the areas around r voids located in the cement paste to present as a bright yellowy-orange the bright red colouration under UV lighting indicative of advanced ASR. In C-315 the above results show that there is no observable evidence that alkali red in the concrete. Coupled with the test results there were no common R has/is occurring, such as: micro-cracks which extend through or between reaction rims or ASR gel product		

M. Rome \sim

Matt Rawlings, Laboratory Manager