GENERAL

- THIS IS A METRIC PROJECT, UNLESS OTHERWISE NOTED, ALL DIMENSIONS ARE IN MILLIMETERS.
- PROVIDE ALL MATERIAL AND LABOUR REQUIRED FOR COMPLETION OF THE WORK. 2.
- PRIOR TO CONSTRUCTION, REVIEW STRUCTURAL DRAWINGS IN CONJUNCTION WITH DRAWINGS PROVIDED BY ALL OTHER CONSULTANTS, AND WITH EXISTING CONDITIONS.
- REPORT DISCREPANCIES TO THE DEPARTMENTAL REPRESENTITIVE BEFORE PROCEEDING WITH THE WORK.
- VERIFY EXISTING DIMENSIONS AND CONDITIONS ON SITE PRIOR TO CONSTRUCTION.
- DO NOT SCALE THESE DRAWINGS.
- DRAWINGS SHOW COMPLETED STRUCTURE ONLY. THEY DO NOT SHOW TEMPORARY WORKS FOR WHICH THE CONTRACTOR IS RESPONSIBLE AND WHICH MAY BE REQUIRED FOR EXECUTION OF THE PROJECT. THE CONTRACTOR TO ESTABLISH CONSTRUCTION PROCEDURE AND SEQUENCE TO ENSURE SAFETY OF THE WHOLE STRUCTURE AND ALL ITS COMPONENTS DURING ERECTION.
- MAKE ADEQUATE PROVISIONS FOR ALL LOADS ACTING ON THE STRUCTURE DURING ERECTION. PROVIDE TEMPORARY SHORING AND BRACING TO KEEP THE STRUCTURE PLUMB AND IN TRUE ALIGNMENT DURING CONSTRUCTION.
- CONSTRUCTION LOADS ON COMPLETED STRUCTURE NOT TO EXCEED DESIGN LOADS INDICATED ON DRAWINGS. FULL DESIGN LOADS MAY ONLY BE APPLIED AFTER THE CONCRETE REACHES ITS DESIGN STRENGTH
- NOTIFY THE DEPARMENTAL REPRESENTITIVE 5 DAYS PRIOR TO CONCRETE 10. POURS, BACKFILLING, AND COVERING UP THE STRUCTURE WITH FINISHES. FOR FIELD REVIEWS TO ENSURE THE STRUCTURAL WORKS DETAILED ON THESE DRAWINGS ARE COMPLETED IN GENERAL CONFORMANCE WITH THE CONTRACT DOCUMENTS, THESE REVIEWS DO NOT REPLACE THE CONTRACTOR'S RESPONSIBILITY TO IMPLEMENT AND MAINTIAN A QUALITY CONTROL PROGRAM.

DESIGN CRITERIA

- STRUCTURAL DESIGN IS IN ACCORDANCE WITH THE 2015 NATIONAL BUILDING CODE (NBC) SUPPLEMENTED BY THE 2015 NATIONAL BUILDING CODE OF CANADA STRUCTURAL COMMENTARY.
- THE VALUES FOR CLIMATIC DATA USED IN THE DETERMINATION OF WIND AND 2 SNOW DESIGN LOADS HAVE BEEN OBTAINED FROM THE NATIONAL BUILDING CODE FOR PRINCE RUPERT AND KITIMAT, BC RESPECTIVELY. THE SEISMIC DESIGN DATA WAS COLLECTED FROM THE NATURAL RESOURSES CANADA (NRC) SEISMIC HAZARD CALCULATOR FOR THE PROPOSED NEW BUILDING LOCATION.
- BASED ON THE USE AND OCCUPANCY, THE BUILDING IS DESIGNED TO THE REQUIREMENTS OF A NORMAL IMPORTANCE CATEGORY.
- THE ROOF IS DESIGNED FOR A DEAD LOAD OF 1.0kPa. ALL THE FLOORS ARE DESIGNED FOR A TOTAL DEAD LOAD OF 1.25kPa.
- THE GROUND FLOOR AND STAIRWAY ARE DESIGNED FOR A LIVE LOAD OF 4.8kPa. THE BEDROOMS ON THE GROUND FLOOR AND ALL UPPER FLOORS ARE DESIGNED FOR LIVE LOADING OF 2.4kPa. MINIMUM CONCENTRATED LIVE LOAD OVER WORKSHOP TO BE 4.5kN OVER AN AREA OF 750x750
- SNOW: Ss = 6.5 kPa; Sr = 0.8 kPa; Is (ULS) = 1.0; Is (SLS) = 0.9 6.
- LATERAL LOADS IN THIS STRUCTURE ARE RESISTED BY SHEAR WALLS, AND ARE DETERMINED BASED ON THE WIND AND SEISMIC DATA BELOW.
- WIND: q50 = 0.54 kPa; lw (ULS) = 1.0; lw (SLS) = 0.75 TERRAIN TYPE: OPEN
- **INTERNAL PRESSURE CATEGORY: 2**

CEICIMIC	
Sa (0.2) = 0.226	Rd = 3.0
Sa (0.5) = 0.246	Ro = 1.7
Sa(1.0) = 0.192	le = 1.0
Sa(2.0) = 0.127	SITE CLASSIFICATION = E
PGA = 0.108	

SEISMIC FORCE RESISTING SYSTEM (SFRS): WOOD SHEAR WALLS

FOUNDATIONS

SEISMI

- STRUCTURAL DESIGN IS BASED ON THE GEOTECHNICAL ASSESSMENT PREPARED BY MCELHANNEY; REPORT NUMBER 2321-22441-01; DATED AUGUST 24, 2020 REFER TO GEOTECHNICAL REPORT FOR ADDITIONAL FOUNDATION AND EARTHWORK INFORMATION.
- SET FOUNDATIONS ON HORIZONTAL ENGINEERED FILL ON TOP OF TILL WHICH IS CAPABLE OF SUPPORTING BEARING PRESSURE OF 75 kPa AT ULS AND 50 kPa AT SLS. REMOVAL OF UP TO 2.3m OF PEAT OVERBURDEN IS REQUIRED TO EXPOSE THE TILL.
- UNLESS OTHERWISE NOTED, CENTRE FOOTINGS, AND PIERS UNDER CENTROID OF COLUMNS. WHERE THERE ARE NO COLUMNS ABOVE, CENTRE UNDER WALLS OR GRADE BEAMS.
- LOCATE ALL EXISTING UNDERGROUND SERVICES PRIOR TO EXCAVATION.
- PROTECT FOOTINGS, PIERS, FOUNDATION WALLS, SLABS-ON-GRADE AND ADJACENT SOIL AGAINST FREEZING AND FROST ACTION AT ALL TIMES DURING CONSTRUCTION. DO NOT POUR CONCRETE AGAINST FROZEN EARTH.
- DO NOT PLACE CONCRETE IN WATER OR ON FROZEN SOIL.
- DO NOT BACKFILL AGAINST WALLS RETAINING EARTH UNTIL ELEMENTS 7 PROVIDING LATERAL SUPPORT, INCLUDING SLABS ON GRADE AND SUSPENDED LEVELS, ARE COMPLETED AND CONCRETE HAS REACHED 75% OF ITS DESIGN STRENGTH.
- FOR ELEMENTS THAT ARE TO BE BACKFILLED ON BOTH SIDES, PLACE BACKFILL SIMULTANEOUSLY ON BOTH SIDES SUCH THAT HEIGHTS DO NOT VARY BY MORE THAN 600 (2') FROM ONE SIDE TO THE OTHER.

CONCRETE

- CONFORM TO CSA A23.1 "CONCRETE MATERIALS AND METHODS OF CONCRETE CONSTRUCTION".
- CONCRETE IS SPECIFIED PER ALTERNATIVE 1 PERFORMANCE SPECIFICATION, 2. AS OUTLINED IN CAN/CSA A23.1. THE CONTRACTOR AND THE CONCRETE SUPPLIER TO MEET ALL CERTIFICATION, DOCUMENTATION, AND QUALITY CONTROL REQUIREMENTS.
- CONCRETE TO BE NORMAL DENSITY (MIN. 2300 kg/m³) UNLESS NOTED OTHERWISE.
- CEMENT TO BE PORTLAND CEMENT TYPE GU, UNLESS NOTED OTHERWISE OR REQUIRED BY EXPOSURE CLASS. CEMENT TO CONFORM TO CSA A3000.
- AGGREGATE TO CONFORM TO CSA A23.1 / A23.2. DO NOT USE RECYCLED CONCRETE AS AGGREGATE.
- CONCRETE ADMIXTURES SHALL NOT CONTAIN CHLORIDES. 6

CONCRETE CONTINUED

- SUBMIT CONCRETE MIX DESIGNS TO DEPARTMENTAL REPRESENTATIVE FOR **REVIEW BEFORE START OF WORK.**
- PERIMETER AND EXTERIOR FOUNDATION WALLS AND FOOTINGS: EXPOSURE CLASS: F2 MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS: 30 MPa
- PROTECT CONCRETE FROM EXCESSIVE HEAT AND DRYING. USE HOT WEATHER 9. CONCRETING METHODS IN ACCORDANCE WITH CAN/CSA-A23.1 WHENEVER THE OUTDOOR TEMPERATURE IS GREATER THAN 27°C.
- PROTECT CONCRETE FROM FREEZING. USE COLD WEATHER CONCRETING 10 METHODS IN ACCORDANCE WITH CAN/CSA-A23.1 WHENEVER OUTDOOR TEMPERATURE IS LESS THAN +5°C. ALL INSULATED COVERS, HEATERS, AND OTHER MATERIALS NEEDED TO PROTECT CONCRETE TO BE ON HAND PRIOR TO POUR. DELIVER CONCRETE AT A TEMPERATURE BETWEEN +15°C AND +27°C. ENSURE A MINIMUM CONCRETE TEMPERATURE OF 10° IS MAINTAINED THROUGHOUT THE CURING PERIOD (MINIMUM 3 DAYS).
- 11 FORMWORK DESIGN, MATERIAL, FABRICATION, AND ERECTION TO CONFORM TO CSA S269.1
- FORMWORK MATERIAL TO BE NEW EXTERIOR PLYWOOD CONFORMING TO CSA 12. O121, EXCEPT FOR ROUGH CONCRETE IN UNEXPOSED LOCATIONS (SUCH AS FOUNDATIONS) WHERE USED MATERIAL IS ACCEPTABLE.

CONCRETE REINFORCEMENT

- CONFORM TO CSA A23.1 "CONCRETE MATERIALS AND METHODS OF CONCRETE CONSTRUCTION".
- **REINFORCEMENT DEFORMED BAR REINFORCEMENT CONFORMING TO CSA** 2 G30.18 GRADE 400R OR 400W.
- ACCESSORIES, BAR SUPPORTS, AND TIES TO CONFORM TO REINFORCING STEEL INSTITUTE OF CANADA (RSIC) MANUAL OF STANDARD PRACTICE AND CSA A23.1 / A23.2.
- ALL REBAR HOOKS TO BE STANDARD LENGTH 90° OR 180° HOOKS. REBAR 4. LENGTHS LISTED ON DRAWINGS DO NOT INCLUDE THE HOOK LENGTH.
- FIELD BENDING OF BARS IS NOT PERMITTED UNLESS INDICATED OR APPROVED BY DEPARTMENTAL REPRESENTIVE. APPROVED FIELD BENDING TO BE DONE WITHOUT THE USE OF HEAT, THROUGH APPLICATION OF SLOW AND STEADY PRESSURE. REPLACE BARS WITH CRACKS OR SPLITS.
- ALL REINFORCING TO BE CLEAN, FREE OF LOOSE SCALE, OIL, DIRT, RUST, AND ANY OTHER FOREIGN COATING THAT AFFECT BONDING CAPACITY.
- ALL REBAR LAP SPLICES TO BE COMPLETED AS PER THE GENERAL LAP SPLICE 7. TABLE.
- WHERE CONCRETE IS CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH, 8. MINIMUM CONCRETE COVER TO REINFORCING BARS CLOSEST TO THE CONCRETE SURFACE TO BE 75 (3").
- FOR CLASS F-1 AND F-2 CONCRETE, MINIMUM COVER TO BE 40 (1 1/2").
- INCREASE COVER WHERE REQUIRED TO MAINTAIN MINIMUM RATIO OF COVER TO 10. NOMINAL BAR DIAMETER OF 2.
- 11. ENSURE COVER TO REINFORCEMENT IS MAINTAINED DURING CONCRETE POUR.

- STRUCTURAL STEEL CONFORM TO CSA S16 "LIMIT STATES DESIGN OF STEEL STRUCTURES"
- FABRICATOR TO BE CERTIFIED BY THE CANADIAN WELDING BUREAU UNDER 2 REQUIREMENTS OF CSA W47.1, DIVISION 1 OR 2, AND/OR CSA W55.3. WELDERS TO BE CWB CERTIFIED. WELDING TO BE IN ACCORDANCE WITH CSA 3.
- MATERIALS (TO CSA G40.21 UNLESS NOTED OTHERWISE): - WIDE FLANGE SECTIONS AND CHANNELS: GRADE 350W
 - PLATES, BARS AND ANGLES: GRADE 300W - HOLLOW STRUCTURAL SECTIONS (HSS): 350W CLASS "C" OR ASTM A1085
 - GRADE 50 (345 MPa)
 - BOLTS, NUTS AND WASHERS: ASTM F3125, GRADE A325 - ANCHOR RODS: ASTM F1554 GRADE 36
 - SHOP PAINT: CISC/CPMA 1-73A
 - SHOP PRIMER PAINT: CISC/CPMA 2-75 - HOT DIP GALVANIZING: ASTM A123/A123M
- DO NOT CUT HOLES OR OTHERWISE MODIFY STRUCTURAL MEMBERS ON SITE. DO NOT OVERSIZE ANCHOR ROD HOLES FOR SITE TOLERANCES, USE HOLE SIZES SUGGESTED IN THE CISC "HANDBOOK OF STEEL CONSTRUCTION".
- PROTECT COMBUSTIBLE MATERIALS AND FINISHES DURING WELDING OPERATIONS.
- 8. ALL STEEL LOCATED OUTSIDE THE BUILDING ENVELOPE'S VAPOUR BARRIER TO BE HOT DIPPED GALVANIZED.
- PROVIDE VENT HOLES IN HSS SECTIONS WHERE REQUIRED FOR GALVANIZING PROCESS. MAXIMUM SIZE 16 (5/8") DIAMETER.
- SHOP DRAWINGS FOR STRUCTURAL STEEL, STEEL CONNECTIONS, AND STEEL 10. JOISTS TO BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER RESPONSIBLE FOR THEIR DESIGN, RETAINED BY THE CONTRACTOR AND REGISTERED IN THE PLACE THE PROJECT IS LOCATED.
- 11. CONNECT BEAMS FOR THE FORCES SHOWN ON DRAWINGS USING THE CISC "HANDBOOK OF STEEL CONSTRUCTION". IF NO FORCE IS INDICATED, CONNECT NON-COMPOSITE BEAMS FOR THE REACTION DUE TO MAXIMUM UNIFORMLY DISTRIBUTED LOAD CAPACITY OF THE BEAM IN BENDING, AND CONNECT COMPOSITE BEAMS FOR ONE AND A HALF TIMES THE REACTION DUE TO MAXIMUM UNIFORMLY DISTRIBUTED LOAD CAPACITY OF THE NON- COMPOSITE SECTION IN BENDING.
- WHERE SLOTTED CONNECTIONS ARE SHOWN ON STRUCTURAL DRAWINGS. 12. FINGER TIGHTEN BOLTS TO A SNUG FIT AND BURR THREADS TO PREVENT NUTS FROM WORKING LOOSE.
- PREMIXED GROUT: NON-SHRINK, MINIMUM STRENGTH 40 MPa AT 28 DAYS. 13.
- 14. INSTALL GROUT UNDER BASE PLATES AS SOON AS STEEL WORK IS COMPLETE, IN ACCORDANCE WITH MANUFACTURER'S DIRECTIONS, PROVIDE 100% CONTACT OVER GROUTED AREA. DO NOT APPLY ANY LOADS TO THE STEELWORK BEFORE GROUT ACHIEVES SUFFICIENT STRENGTH.

NOMINAL SIZE OF COARSE AGGREGATE: 20 (3/4")

STRUCTURAL WOOD

- CONFORM TO CSA 086 "ENGINEERING DESIGN IN WOOD".
- MATERIALS: 2.

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- LUMBER: TO CSA 141; KILN DRIED; SPF NO. 2 OR BETTER; MOISTURE
- CONTENT MAX 19% UNLESS NOTED OTHERWISE ON DRAWINGS - PLYWOOD SHEATHING: TO CSA O121 DOUGLAS FIR
- PARALLAM STRAND LUMBER (PSL): MINIMUM GRADE 2.0E
- LAMINATED VENEER LUMBER (LVL): MINIMUM GRADE 2.0E, 3100Fb OR BETTER
- NAILS: COMMON ROUND STEEL WIRE NAILS - WOOD BOLTS: ASTM A307, 15 DIAMETER UNLESS NOTED OTHERWISE
- LAG SCREWS: ANSI/ASME B18.12.1 MACHINE THREADED
- UNLESS NOTED OTHERWISE, ALL WOOD FRAMING DETAILS TO BE IN
- ACCORDANCE WITH PART 9 OF THE REFERENCED BUILDING CODE.
- PROTECT ALL WOOD PRODUCTS FROM THE ELEMENTS AS REQUIRED TO MAINTAIN THEIR INTEGRITY.
- PROVIDE ALL ERECTION BRACING REQUIRED TO KEEP THE STRUCTURE STABLE AND IN ALIGNMENT DURING CONSTRUCTION.
- SUBSTITUTION OF COMMON NAILS WITH POWER DRIVEN NAILS OF THE SAME LENGTH AND DIAMETER IS ACCEPTABLE. SUBSTITUTION OF POWER DRIVEN NAILS OF SMALLER DIAMETER MUST BE APPROVED IN WRITING BY THE DEPARTMENTAL REPRESENTATIVE PRIOR TO USE. POWER DRIVEN NAILS NOT TO BE OVER-DRIVEN INTO WOOD OR SHEATHING.
- ALL COMPONENTS OF BUILT UP MEMBERS TO BE CONTINUOUS FOR FULL SPAN. DO NOT SPLICE OR USE BUTT JOINTS.
- WHERE STUDS ARE PLACED TOGETHER TO FORM BUILT-UP COLUMNS WITHIN A WALL (FASTENED TO SHEATHING AT MINIMUM 300 (12") CENTRES), BUILT UP COLUMNS MAY BE FASTENED WITH COMMON NAILS. NAIL INDIVIDUAL STUDS TOGETHER WITH ROWS OF 3.25Ø (0.13"Ø) NAILS SPACED AT 225 (9") CENTRES, END NAILS LOCATED 75 (3") FROM BOTH ENDS, AS FOLLOWS
- 38x89 (2x4): STAGGER NAILS 25 (1") FROM ALTERNATE STUD EDGES - 38x140 (2x6) AND 38x184 (2x8): PROVIDE TWO ROWS OF NAILS 50 (2") FROM STUD EDGES
- ALTERNATIVELY, BUILT-UP COLUMNS MAY BE FASTENED WITH 5.6Ø (0.22"Ø) SDW SCREWS BY SIMPSON STRONG-TIE, ARRANGED AS ABOVE. LENGTH OF SCREW TO PENETRATE OUTER PLY MINIMUM 20 (3/4").
- WHERE LVL / SAWN LUMBER MEMBERS ARE PLACED TOGETHER TO FORM BUILT-UP DROPPED BEAMS OR LIGHTLY LOADED FLUSH BEAMS, BEAMS MAY BE FASTENED WITH COMMON NAILS. NAIL INDIVIDUAL PLIES TOGETHER WITH 3.25Ø (0.13"Ø) NAILS SPACED @300 ALONG LENGTH OF BEAM AND 150 (6") FROM EACH END, WITH ROWS CENTRED ON BEAM DEPTH, AS FOLLOWS UNLESS ALTERNATIVE CONNECTION IS SPECIFIED BY SUPPLIER:
- 89 TO 185 (3-1/2" TO 7-1/4") DEEP: 2 ROWS @50 (2") 235 TO 300 (9-1/4" TO 11-3/4") DEEP: 3 ROWS @50 (2")
- 300 TO 400 (11-3/4" TO 15-3/4") DEEP: 4 ROWS @50 (2")
- 400 TO 500 (15-3/4" TO 19-3/4") DEEP: 4 ROWS @75 (3")
- ALTERNATIVELY, BUILT-UP BEAMS MAY BE FASTENED WITH 5.6Ø (0.22"Ø) SDW SCREWS BY SIMPSON STRONG TIE, ARRANGED AS ABOVE. LENGTH OF SCREW TO PENETRATE OUTER PLY MINIMUM 20 (3/4").
- CARRY ALL POSTS DOWN TO FOUNDATION. PROVIDE SOLID VERTICAL BLOCKING OF MATCHING SIZE OR LARGER AND IN LINE WITH POSTS AT FLOOR LEVELS TO ACT AS SQUASH BLOCKS IN THE FLOOR SYSTEM.
- 11. USE JOISTS HANGERS WHERE JOISTS FRAME INTO SIDES OF SUPPORTS.
- FOR ENGINEERED FLOOR SYSTEMS, ALL RIM BOARD AND BLOCKING MATERIAL TO 12 BE LSL/LVL, MINIMUM THICKNESS 44 (1-3/4") UNLESS NOTED OTHERWISE ON DRAWINGS
- 13. UNTREATED WOOD NOT TO BE IN DIRECT CONTACT WITH CONCRETE, PROVIDE FOAM GASKET BETWEEN WOOD AND CONCRETE. OR USE PRESSURE TREATED WOOD. REFER TO PLANS FOR ADDITIONAL REQUIREMENTS.
- 14. PREFABRICATED WOOD JOISTS: DESIGN TO THE REFERENCE BUILDING CODE FOR LOADS AND MAXIMUM DEFLECTIONS GIVEN IN CSA 086. DESIGN TO CONTROL VIBRATION PER CSA 086. SHOP DRAWINGS TO INCLUDE ENGINEERED DESIGNS, MATERIAL GRADES, LAYOUT DRAWINGS, BRACING DETAILS, BEARING DETAILS, ANCHORAGE DETAILS AND CONNECTION DETAILS BETWEEN JOISTS AND TO THEIR SUPPORTS. SHOP DRAWINGS (INCLUDING LAYOUTS) TO BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF BRITISH COLUMBIA, PRIOR TO FABRICATION. SUPPLIER SHALL PROVIDED A SCHEDULE S-B & S-C TO THE ENGINEER OF RECORD IF REQUESTED.
 - PREFABRICATED WOOD TRUSSES: TO COMPLY WITH THE TRUSS PLATE INSTITUTE OF CANADA (TPIC) DESIGN SPECIFICATIONS. FABRICATOR TO BE A MEMBER OF THE CANADIAN WOOD TRUSS ASSOCIATION. DESIGN TO THE REFERENCE BUILDING CODE FOR LOADS AND MAXIMUM DEFLECTIONS GIVEN ON DRAWINGS. SHOP DRAWINGS TO INCLUDE ENGINEERED DESIGNS, MATERIAL GRADES, LAYOUT DRAWINGS, BEARING DETAILS, ANCHORAGE DETAILS AND CONNECTION DETAILS BETWEEN TRUSSES, AND TEMPORARY AND PERMANENT BRACING AND BRIDGING DETAILS AFFECTING THE STRUCTURAL CAPACITY OF THE TRUSSES. DESIGN TRUSSES TO SUPPORT ALL OVERBUILD FRAMING REQUIRED FOR ROOF GEOMETRY, DO NOT INTERRUPT ROOF SHEATHING TO ACCOMMODATE OVERBUILD FRAMING. VAULTED TRUSSES NOT TO RELY ON SUPPORTING STRUCTURE TO RESIST HORIZONTAL SPREADING OF TRUSS. SHOP DRAWINGS (INCLUDING LAYOUTS) TO BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF BRITISH COLUMBIA, PRIOR TO FABRICATION. SUPPLIER SHALL PROVIDED A SCHEDULE S-B & S-C TO THE ENGINEER OF RECORD IF REQUESTED.

REINFORCING LAP SPLICES										
BAR SIZE VERTICAL LAP HORIZONTAL LAP HOOK LENGTH										
10M 16" [430mm] 20" [500mm] 7" [180mm]										
15M 24" [600mm] 32" [800mm] 10" [250mm]										
20M	20M 30" [750mm] 40" [1000mm] 12" [300mm]									
25M	25M 48" [1200mm] 60" [1550mm] 16" [400mm]									
30M 57" [1450mm] 72" [1850mm] 24" [600mm]										
HORIZONTAL WALL REINFORCING SHALL BE CONTINUOUS AROUND CORNERS AND										
HOOKED AT WALL INTERSECTIONS.										
ADD 2-15M CONT AT TOPS AND ENDS OF WALLS. UNLESS OTHERWISE NOTED, HOOK										
AND LAP LENGTHS AS FOLLOWS:										

POST-INSTALLED ANCH

- WHERE DRILLED CONCRETE ANCHORS HILTI KWIK BOLT - TZ EXPANSION ANCH FOLLOWS: 12 (1/2") DIAMETER - 83 (3-1/4") EMBEDME
- 16 (5/8") DIAMETER 102 (4") EMBEDMEN 19 (3/4") DIAMETER - 121 (4-3/4") EMBEDM WHERE ADHESIVE CONCRETE ANCHOR
- HILTI HIT-HY200 ADHESIVE ANCHORING **EFFECTIVE EMBEDMENT LENGTHS AS F** 12 (1/2") DIAMETER - 114 (4-1/2") EMBEDM 16 (5/8") DIAMETER - 143 (5-5/8") EMBEDM 19 (3/4") DIAMETER - 171 (6-3/4") EMBEDM
- WHERE REBAR DOWELS ARE NOTED ON ADHESIVE ANCHORING SYSTEM INSTAL BIT TECHNOLOGY. EFFECTIVE EMBEDM 12 (1/2") DIAMETER - 114 (4-1/2") EMBEDM 16 (5/8") DIAMETER - 143 (5-5/8") EMBEDM 19 (3/4") DIAMETER - 171 (6-3/4") EMBEDM
- WHERE ANCHORS ARE INSTALLED OUTS STAINLESS STEEL ANCHORS.
- CONCRETE TO BE MINIMUM 28 DAYS OL USE DRILLING AND INSTALLATION TOOL
- MANUFACTURERS' RECOMMENDATIONS
- DO NOT CUT REINFORCEMENT TO ACCO DOWELS.
- A WHEN OBSTRUCTIONS PREVENT DRIL THE REQUIRED DEPTH, RELOCATE AT N OBTAIN DEPARTMENTAL REPRESENTAT BEFORE DRILLING HOLES. FILL ALL ABA DO NOT TIGHTEN ANCHORS UNTIL GROU REACHES 75% fc'.

ABBREVIATIONS

7

ALT -	ALTERNATE
A-ROD -	ANCHOR ROD
BOT -	BOTTOM
BLDG -	BUILDING
BTWN -	BETWEEN
CLR -	CLEAR
CL -	CENTER LINE
CONC -	CONCRETE
CONT -	CONTINUOUS
CP -	COMPLETE PENETRATION W
C/W -	COMPLETE WITH
DN -	DOWN
DP -	DEEP
DWG -	DRAWING
EA -	EACH
EE -	EACH END
EF -	
EL -	ELEVATION
EMBED -	EMBEDMENT
ES -	EACH SIDE
EW -	EACH WAY
EXI -	EXTERIOR
FL -	FLOOR
FND -	FOUNDATION
FIG -	
GALV -	
NIC .	
NTS -	NOT TO SCALE
00.	
OPP .	OPPOSITE
PI .	PLATE
PT -	PRESSURE TREATED
REINF -	REINFORCEMENT
REQ'D -	REQUIRED
REV -	REVISION
R/W -	REINFORCE WITH
SIM -	SIMILAR
SOG -	SLAB ON GRADE
SS -	STAINLESS STEEL
SST -	SIMPSON STRONG TIE
STAGG -	STAGGERED
STD -	STANDARD
STIFF -	STIFFENER
STL -	STEEL
T&B -	TOP AND BOTTOM
T&G -	TONGUE AND GROOVE
т/О -	TOP OF
TYP -	TYPICAL
U-BAR -	"U" SHAPED BAR
U/N, UNO	UNLESS NOTED OTHERWISE
U/S -	UNDERSIDE
VERT -	VERTICAL
WSP-S -	WSP STRUCTURAL

ORS AND DOWELS	Public Works and Government Services Canada Canada Canada
	Pacific Region
DCA) ARE NOTED ON DRAWINGS, PROVIDE DRS. EFFECTIVE EMBEDMENT LENGTHS AS	SERVICES IMMOBILIERS
INT	Region de Pacifique
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S ARE NOTED ON DRAWINGS, PROVIDE SYSTEM WITH HILTI HIT-Z ANCHOR RODS. OLLOWS: IENT IENT	JOB No. 191-03502-00
ENT	
DRAWINGS, PROVIDE HILTI HIT-HY200 LED USING HILTI SAFESET HOLLOW DRILL ENT LENGTHS AS FOLLOWS: IENT IENT IENT	G. LIANG PROFESSIONAL ENGINEER # 27199
IDE OF VAPOUR BARRIER, PROVIDE	
O AT THE TIME OF ANCHOR INSTALLATION. S AND PROCEDURES PER	
MMODATE DRILLED ANCHORS AND	2021-02-02
LING HOLES IN SPECIFIED LOCATIONS TO O EXTRA COST TO THE CONTRACT. IVE APPROVAL OF NEW LOCATIONS NDONED HOLES WITH MIN. 30MPa GROUT. JT IN ADJACENT ABANDONED HOLES	
D	
	3 ISSUED FOR TENDER 2021/02/
	2 ISSUED FOR 100% REVIEW 2021/01/
	1 ISSUED FOR 95% REVIEW 2020/10/
	Revision/ Description /Description Date /D/
	Client/client
	FISHERIES AND OCEANS, REAL PROPERTY, SAFETY AND SECURITY
	VANCOUVER, BC 200-401 BURRARD ST.
	Project title/Titre du projet
	HARTLEY BAY SAR RESIDENCE & SAR FLOAT SERVICES
	HARTLEY BAY, BC
	 Designed by/Concept par GL
	Drawn by/Dessine par GM
	PWGSC Project Manager/Administrateur de Projets TPSGC Regional Manager, Architectural and Engineering Services
	Gestionnaire regionale, Services d'architectural et de genie, IPSGC Drawing title/Titre du dessin GENERAL NOTES
	Project No./No. du Sheet/Feuille Revision no
	2018552 S1.01 1 OF 9
	DM5XXXXX



	HOLD DOWN SCHEDULE									
MARK	HOLD-DOWN	HD STUDS	ANCHORS							
HD1	CONT 19Ø ROD W/ TAKE-UP DEVICE, 45kN OR SST ATS-SR6	2-PLY ES	SIMPSON 19Ø PAB (EMBED 500)							
HD2	CONT 25Ø ROD W/ TAKE-UP DEVICE, 105kN OR SST ATS-SR8	3-PLY ES	SIMPSON 25Ø PAB (EMBED 800)							
HD STUDS ARE IN ADDITION TO BEARING STUDS ON PLANS HD STUD SIZE TO MATCH PLATE SIZE FOR STAGGER STUD STUD WALLS.										

FOOTING SCHEDULE								
MARK	SIZE	REINFORCEMENT						
SF1	600x235 DP	2-15M BOT CONT						
SF2	800x235 DP	2-15M BOT CONT						
SF3	1000x235 DP	2-15M BOT CONT						
F1	900x900x250 DP	4-15M BOT EW						



	WOOD BEAM
⊢ ^{H1} — —	BEAM HANGERS: H1 - SST HUS412, Vf=14kN UPLIFT AND DOWN H2 - SST HGUS7.25/12, Vf=60kN DOWN
× PB	A = NO. OF PLYS IN BUILT-UP COL B = NOMINAL STUD DEPTH (6 = 2x6)
× ^{NB} ^t	A = NO. OF KING STUDS B = NOMINAL STUD DEPTH (6 = 2x6) XX = NO. OF CRIPPLES: SC = SINGLE CRIPPLE DC = DOUBLE CRIPPLE TC = TRIPLE CRIPPLE
<u> </u>	JOISTS, DESIGNED BY SUPPLIER
W3	BEARING WALLS: W1 - 38x89 @ 305 W2 - 38x89 STAGGER STUDS @ 406 ON 38x140 PLATES W3 - 38x140 @ 406 (ALL EXTERIOR WALLS)
	SHEAR WALLS, SHEATH BOTH SIDES WHERE SW SYMBOL IS SHOWN ON BOTH SIDES OF WALL (SEE SCHEDULE)
H	HOLD DOWN (SEE SCHEDULE)





Public Works and Government Services Canada Canada Canada

REAL PROPERTY SERVICES Pacific Region SERVICES IMMOBILIERS Région de Pacifique

JOB No. 191-03502-00



(E) RAMP BEAMS

	WOOD BEAM
⊢ ^{H1} — —	BEAM HANGERS: H1 - SST HUS412, Vf=14kN UPLIFT AND DOWN H2 - SST HGUS7.25/12, Vf=60kN DOWN
× ^{p/8}	A = NO. OF PLYS IN BUILT-UP COL B = NOMINAL STUD DEPTH (6 = 2x6)
×net	A = NO. OF KING STUDS B = NOMINAL STUD DEPTH (6 = 2x6) XX = NO. OF CRIPPLES: SC = SINGLE CRIPPLE DC = DOUBLE CRIPPLE TC = TRIPLE CRIPPLE
	JOISTS, DESIGNED BY SUPPLIER
W3	BEARING WALLS: W1 - 38x89 @ 305 W2 - 38x89 STAGGER STUDS @ 406 ON 38x140 PLATES W3 - 38x140 @ 406 (ALL EXTERIOR WALLS)
	SHEAR WALLS, SHEATH BOTH SIDES WHERE SW SYMBOL IS SHOWN ON BOTH SIDES OF WALL (SEE SCHEDULE)
HU	HOLD DOWN (SEE SCHEDULE)



				WOOD SHEAR WALL SCHEDULE									
				MADIZ			NAIL SPACIN	G		ANCH	OR RODS	FACTORED SHEAR	SIL
				MARK	SHEATHING	EDGE	INTERIOR	PLATE	SHEAR CONNECTORS	SIZE	SPACING	(kN/m)	FAS
				SW2.5	12.5 SHEATHING	65	300	65	A35 @ 400	16Ø	600	10.5	6.40 SCREW
	HOLD DO	WN SCHEDULE		SW3 (SHOWN BOTH SIDES)	12.5 SHEATHING BOTH SIDES	75	300	75	A35 @ 400 + LTP4 @ 400	19Ø	600	16.3	6.40 SCREW
MARK	HOLD-DOWN	HD STUDS	ANCHORS	014/2		75	200	75	A35 @ 400	100	600	0.5	6.40
HD1	CONT 19Ø ROD W/ TAKE-UP	2-PLY ES	SIMPSON 19Ø PAB (EMBED 500)	5003	12.5 SHEATHING	75	300	/5	A35 @ 400	00	600	9.5	SCREV
	DEVICE, 45kN OR SST ATS-SR6 CONT 25Ø ROD W/ TAKE-UP	3-PLV ES	SIMPSON 25Ø PAB (EMBED 800)	SW4	12.5 SHEATHING	100	300	100	A35 @ 400	16Ø	900	6.3	6.40 SCREW
HD STUDS A	DEVICE, 105kN OR SST ATS-SR8 ARE IN ADDITION TO BEARING STUDS ON	I PLANS		SW6	12.5 SHEATHING	150	300	150	A35 @ 400	16Ø	1200	4.6	6.40 SCREW
HD STUD SI	ZE TO MATCH PLATE SIZE FOR STAGGE	R STUD STUD WALLS.		SW MARK SHOWN PROVIDE BLOCKIN	ON BOTH SIDES OF WAL G TO ALL PLYWOOD ED(L ON PLAN GES	INDICATES T	HE WALL IS	SHEATHED ON BOTH SIDES	•			

1 GROUND FLOOR PLAN SHOWING SECOND FLOOR FRAMING \$2.03 SCALE: 1:50





	WOOD BEAM
⊢ ^{H1} — —	BEAM HANGERS: H1 - SST HUS412, Vf=14kN UPLIFT AND DOWN H2 - SST HGUS7.25/12, Vf=60kN DOWN
× ^{p®}	A = NO. OF PLYS IN BUILT-UP COL B = NOMINAL STUD DEPTH (6 = 2x6)
×Ret	A = NO. OF KING STUDS B = NOMINAL STUD DEPTH (6 = 2x6) XX = NO. OF CRIPPLES: SC = SINGLE CRIPPLE DC = DOUBLE CRIPPLE TC = TRIPLE CRIPPLE
<u> </u>	JOISTS, DESIGNED BY SUPPLIER
W3	BEARING WALLS: W1 - 38x89 @ 305 W2 - 38x89 STAGGER STUDS @ 406 ON 38x140 PLATES W3 - 38x140 @ 406 (ALL EXTERIOR WALLS)
<u>SW</u>	SHEAR WALLS, SHEATH BOTH SIDES WHERE SW SYMBOL IS SHOWN ON BOTH SIDES OF WALL (SEE SCHEDULE)
HU	HOLD DOWN (SEE SCHEDULE)



				WOOD SHEAR WALL SCHEDULE										
				MADIZ		NAIL SPACING		G		ANCHOR RODS		FACTORED SHEAR	SILL PLATE	DEMADKO
				WARA	SHEATHING	EDGE	INTERIOR	PLATE	SHEAR CONNECTORS	SIZE	SPACING	(kN/m)	FASTENERS	REMARNO
				SW2.5	12.5 SHEATHING	65	300	65	A35 @ 400	16Ø	600	10.5	6.4Øx100LG SCREWS @ 125 OC	3.66Øx75LG NAILS
HOLD DOWN SCHEDULE				SW3 (SHOWN BOTH SIDES)	12.5 SHEATHING BOTH SIDES	75	300	75	A35 @ 400 + LTP4 @ 400	19Ø	600	16.3	6.4Øx100LG SCREWS @ 125 OC	3.66Øx75LG NAILS
MARK	HOLD-DOWN	HD STUDS	ANCHORS	S/W/3		75	300	75	A35 @ 400	160	600	9.5	6.4Øx100LG	3 660 v751 C NAILS
HD1	CONT 19Ø ROD W/ TAKE-UP	2-PLY ES	SIMPSON 19Ø PAB (EMBED 500)		12.5 SHEATHING	15	500		700 @ 400	100	000	9.0	SCREWS @ 75 OC	J.000X/JEG NAILS
DEVICE, 45kN OR SST ATS-SR6		SW4	12.5 SHEATHING	100	300	100	A35 @ 400	16Ø	900	6.3	6.4Øx100LG	3.25Øx64LG NAILS		
HD2 CONT 250 ROD W/ TAKE-UP 3-PLY ES SIMPSON 250 PAB (EMBED 800)													SCREWS @ 200 OC	
HD STUDS ARE IN ADDITION TO BEARING STUDS ON PLANS				SW6	12.5 SHEATHING	150	300	150	A35 @ 400	16Ø	1200	4.6	6.4Øx100LG SCREWS @ 200 OC	3.25Øx64LG NAILS
HD STUD SI	ZE TO MATCH PLATE SIZE FOR STAGGEF	R STUD STUD WALLS.		SW MARK SHOWN	ON BOTH SIDES OF WAL	L ON PLAN I	NDICATES TH	HE WALL IS S	SHEATHED ON BOTH SIDES			•		
				PROVIDE BLOCKING	G TO ALL PLYWOOD FDO	FS								

1 SECOND FLOOR PLAN SHOWING THIRD FLOOR FRAMING \$2.04 SCALE: 1:50



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	WOOD BEAM
⊢ ^{H1} — —	BEAM HANGERS: H1 - SST HUS412, Vf=14kN UPLIFT AND DOWN H2 - SST HGUS7.25/12, Vf=60kN DOWN
× ^{p18}	A = NO. OF PLYS IN BUILT-UP COL B = NOMINAL STUD DEPTH (6 = 2x6)
×eet	A = NO. OF KING STUDS B = NOMINAL STUD DEPTH (6 = 2x6) XX = NO. OF CRIPPLES: SC = SINGLE CRIPPLE DC = DOUBLE CRIPPLE TC = TRIPLE CRIPPLE
<i></i>	JOISTS, DESIGNED BY SUPPLIER
W3	BEARING WALLS: W1 - 38x89 @ 305 W2 - 38x89 STAGGER STUDS @ 406 ON 38x140 PLATES W3 - 38x140 @ 406 (ALL EXTERIOR WALLS)
<u>SW</u>	SHEAR WALLS, SHEATH BOTH SIDES WHERE SW SYMBOL IS SHOWN ON BOTH SIDES OF WALL (SEE SCHEDULE)
HU	HOLD DOWN (SEE SCHEDULE)



									WOOD SHEAR WA	LL SCHE	DULE			
					NA	AIL SPACING			ANCHOR RODS		FACTORED SHEAR	SILL PLATE		
				MARK	SHEATHING	EDGE	INTERIOR	PLATE	SHEAR CONNECTORS	SIZE	SPACING	(kN/m)	FASTENERS	KEWARKS
				SW2.5	12.5 SHEATHING	65	300	65	A35 @ 400	16Ø	600	10.5	6.4Øx100LG SCREWS @ 125 OC	3.66Øx75LG NAILS
				SW3									6 40×1001 C	
HOLD DOWN SCHEDULE			(SHOWN BOTH SIDES)	BOTH SIDES	75	300	75	A35 @ 400 + LTP4 @ 400	19Ø	600	16.3	SCREWS @ 125 OC	3.66Øx75LG NAILS	
MARK	HOLD-DOWN	HD STUDS	ANCHORS	0)///2		75	200	75	A 25 @ 400	400	C00	0.5	6.4Øx100LG	
HD1	CONT 19Ø ROD W/ TAKE-UP	2-PLY ES	SIMPSON 19Ø PAB (EMBED 500)	5773	12.5 SHEATHING	/5	300	75	A35 @ 400	160	600	9.5	SCREWS @ 75 OC	3.00/0X/5LG NAILS
	DEVICE, 45kN OR SST ATS-SR6			SW4	12.5 SHEATHING	100	300	100	A35 @ 400	16Ø	900	6.3	6.4Øx100LG	3.25Øx64LG NAILS
HD2	DEVICE 105kN OR SST ATS-SR8	3-PLY ES	SIMPSON 25Ø PAB (EMBED 800)						Ű				SCREWS @ 200 OC	
HD STUDS ARE IN ADDITION TO BEARING STUDS ON PLANS			SW6	12.5 SHEATHING	150	300	150	A35 @ 400	16Ø	1200	4.6	SCREWS @ 200 OC	3.25Øx64LG NAILS	
HD STUD SIZE TO MATCH PLATE SIZE FOR STAGGER STUD STUD WALLS.			SW MARK SHOWN ON BOTH SIDES OF WALL ON PLAN INDICATES THE WALL IS SHEATHED ON BOTH SIDES											
				PROVIDE BLOCKING	TO ALL PLYWOOD FDG	FS								

THIRD FLOOR PLAN SHOWING ROOF FRAMING S2.05 SCALE: 1:50

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TYPICAL REINF AT HOLD DOWN

SCALE: 1:20



PLAN DETAIL

4 PLAN DE \$4.02 SCALE: 1:20





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S4.02 SCALE: 1:10



ROOF SECTION AT OUTRIGGERS

