D01-1 GENERAL INFORMATION

- THE INFORMATION PRESENTED ON THESE DRAWINGS HAS BEEN DESIGNED AND ANALYZED IN ACCORDANCE WITH THE 2015 NATIONAL BUILDING CODE OF CANADA. CONSTRUCTION IS TO BE PERFORMED IN ACCORDANCE WITH THIS AND ALL OTHER APPLICABLE CODES. ALL NBC ARTICLES REFERENCED IN THE FOLLOWING SPECIFICATIONS AND DRAWINGS ARE FROM DIVISION B.
- 1.1 CONCRETE STRUCTURE DESIGNED IN ACCORDANCE WITH CSA A23.3:19
- 1.2 STEEL STRUCTURE DESIGNED IN ACCORDANCE WITH CSA-S16:19
- 1.3 WOOD STRUCTURE DESIGNED IN ACCORDANCE WITH CSA-086:19
- ☐ 1.4MASONRY STRUCTURE DESIGNED IN ACCORDANCE WITH CSA S304-1(R2019)
- GUARDRAILS/HANDRAILS SHALL BE DESIGNED AND CERTIFIED BY THE FABRICATOR'S PROFESSIONAL ENGINEER LICENSED IN ONTARIO IN ACCORDANCE WITH LOADS PROVIDED IN (4.1.5.16, 3.4.6.5 AND 3.4.6.6 OF THE 2012 ONTARIO BUILDING CODE/2015NATIONAL BUILDING CODE. STAMPED SHOP DRAWINGS TO BE SUBMITTED. AT LEAST 2 WEEKS PRIOR TO THE PLANNED
- 3. CONTRACTOR IS TO VERIFY/COORDINATE ALL DIMENSIONS/PENETRATIONS WITH ARCHITECTURAL/MECHANICAL/ELECTRICAL DRAWINGS PRIOR TO CONSTRUCTION. REPORT INCONSISTENCIES BEFORE PROCEEDING WITH WORK. ANY OPENINGS NOT
- INDICATED ON STRUCTURAL DRAWINGS ARE TO BE APPROVED BY STRUCTURAL ENGINEER IN WRITING PRIOR TO CONSTRUCTION. 4. CAD VERSIONS OF THE STRUCTURAL DRAWINGS SHALL BE MADE AVAILABLE TO THE CONTRACTOR UPON THE COMPLETION OF A
- RELEASE FORM INDEMNIFYING THE CONSULTANT FROM ANY ERRORS OR OMISSIONS ASSOCIATED WITH THE CAD FILES. 5. LADDERS SHALL BE DESIGNED AT CONTRACTOR'S EXPENSE AND CERTIFIED BY THE FABRICATOR'S PROFESSIONAL ENGINEER LICENSED IN ONTARIO IN ACCORDANCE WITH LOADS PROVIDED IN PART 4 AND PART 3 OF THE 2015 NATIONAL BUILDING CODE.
- STAMPED SHOP DRAWINGS TO BE SUBMITTED AT THE CONTRACTOR'S EXPENSE SEISMIC RESTRAINT OF ARCH/MECH/ELECT ELEMENTS NOT NOTED ON THE DRAWINGS ARE THE RESPONSIBILITY OF THE
- CONTRACTOR'S ENGINEER. RESTRAINT DETAILS ARE TO BE DEVELOPED IN ACCORDANCE WITH THE 2015 NBC. CONTRACTOR'S ENGINEER IS RESPONSIBLE FOR THE DESIGN AND DETAILING OF SEISMIC RESTRAINTS AND ISOLATIONS AS REQUIRED BY SPECIFICATIONS INCLUDING THE VERIFICATION THAT THE EXISTING/NEW STRUCTURE IS CAPABLE OF SAFELY SUPPORTING THE IMPOSED LOADS IN ACCORDANCE WITH THE 2015 NBC. NO ELEMENTS MAY BE CONSTRUCTED WITHOUT WRITTEN CONFIRMATION OF THESE CONDITIONS BY THE DEPARTMENTAL REPRESENTATIVE.
- NO FOUNDATION ELEMENTS ARE TO BE CONSTRUCTED UNTIL WRITTEN APPROVAL OF THE BEARING SURFACES AND PRESSURES IS PROVIDED BY A GEOTECHNICAL ENGINEER THROUGH ON-SITE INVESTIGATION. FAILURE TO COMPLETE THIS WORK COULD RESULT IN THE REMOVAL/REINSTATEMENT OF ANY/ALL FOUNDATION ELEMENTS AT CONTRACTOR'S OWN COST.
- 8. CONTRACTOR TO PROVIDE PRE-ENGINEERED SHORING AS REQUIRED TO ACCOMMODATE THE CONTRACTOR'S CONSTRUCTION ACTIVITIES AND TO PREVENT DAMAGE TO ANY ADJACENT PROPERTY. ALL CONSTRUCTION ACTIVITIES TO BE LIMITED TO THE LIMITS OF THE CONSTRUCTION SITE AND ALL DAMAGE TO EXISTING PROPERTIES MUST BE REINSTATED.
- CONTRACTOR IS REQUIRED TO SUBMIT CONDUIT AND SLEEVING SHOP DRAWINGS FOR ALL FLOORS/ROOFS/WALLS/COLUMNS PRIOR TO THE ERECTION/CONSTRUCTION/FABRICATION OF ANY OF THESE ELEMENTS. THE DRAWINGS ARE TO LOCATE/DIMENSION THE CLEAR SIZES OF OPENINGS/SLEEVES/CONDUITS IN PLAN (FLOORS/ROOFS/COLUMNS) AND ELEVATION (WALLS/BEAMS). THE COORDINATION OF THE VARIOUS DISCIPLINES/SUBTRADES TO ENSURE ALL ITEMS ARE CLEARLY INDICATED IS THE SOLE RÉSPONSIBILITY OF THE CONTRACTOR. DRAWINGS ARE TO BE SUBMITTED A MINIMUM OF 4 WEEKS PRIOR TO THE CONSTRUCTION OF THE AFFECTED ELEMENT.
- 10. THE STRUCTURE OF THE MAIN RESIDENCE AND LINK IS DESIGNED TO THE PART 9 REQUIREMENTS OF 2015 NBC. THE STRUCTURE OF THE GARAGE IS DESIGNED TO THE PART 4 REQUIREMENTS OF 2015 NBC. THE STRUCTURES ARE SEPARATED AS PER 4.1.8.14(1)

D01-2 GRAVITY LOADS

REFER TO PLANS FOR LOADS.

SNOW LOADS (PART 4): IMPORTACE FACTORS: 1-IN-50 YEAR GROUND SNOW LOAD:

Is: 1.0(ULS); 0.9(SLS) Ss: 1.2 kPa Sr: 0.4 kPa

1-IN-50 YEAR ASSOCIATED RAIN LOAD: BASIC ROOF SNOW LOAD FACTOR: S: 1.36 kPa (PLUS ACCUMULATION AS SHOWN ON PLAN) SPECIFIED SNOW LOAD:

SNOW LOADS (PART 9): **IMPORTANCE FACTORS: I**s: 1.0(ULS); 0.9(SLS) 1-IN-50 YEAR GROUND SNOW LOAD: Ss: 1.2 kPa 1-IN-50 YEAR ASSOCIATED RAIN LOAD: Sr: 0.4 kPa

BASIC ROOF SNOW LOAD FACTOR: Cb: 0.55 SPECIFIED SNOW LOAD: S: 1.06 kPa (PLUS ACCUMULATION AS SHOWN ON PLAN)

0.30 kPa

6.00 kPa

2.40 kPa

DESIGN LOADS:

	ROOFING	0.20
	13mm PLYWOOD	0.10
	38mm WOOD DECKING	0.20
	STEEL STRUCTURE	0.30
	M. + E. ALLOWANCE	0.30
	TOTAL	1.10
SNOW		1.36
GARAGE MEZZANINE: DEAD LOADS:		
22/12/20/12/3	FINISHES	0.10
	100mm SLAB-ON-DECK	1.85
	PARTITIONS	1.00
	CEILING	0.20

STORAGE

M. + E. ALLOWANCE

LIVE LOAD:

SNOW

<u>:</u>		
	ROOFING 13mm PLYWOOD WOOD TRUSSES CEILING M. + E. ALLOWANCE TOTAL	0.20 kPa 0.10 kPa 0.20 kPa 0.20 kPa 0.30 kPa 1.00 kPa
₹:		
	FINISHES	0.10 kPa 0.11 kPa

LAD LOADO.		
	FINISHES	0.10 kPa
	19mm PLYWOOD	0.11 kPa
	TJ1 JOISTS	0.20 kPa
	PARTITIONS	1.00 kPa
	CEILING	0.20 kPa
	M. + E. ALLOWANCE	0.30 kPa
	TOTAL	1.91 kPa
IVE LOAD:	RESIDENTIAL AREAS	1.90 kPa
	GYM	4.80 kPa

WORK AREAS

D01-3 LATERAL LOADING DATA (GARAGE)

D01.3.1 SEISMIC

SEISMIC FORCE RESISTING SYSTEM (SFRS) SFRS: SYSTEM & CONNECTIONS: (2015 NBC CLAUSE 4.1.8.9/4.1.8.10) LATERAL LOAD RESISTING SYSTEM: CONVENTIONAL CONSTRUCTION (STEEL BRACED FRAME)

CSA STANDARD: CAN/CSA S16:19 APPLICABLE CLAUSE(S): 27.11

SFRS: DIAPHRAGMS & CONNECTIONS: (2015 NBC CLAUSE 4.1.8.15) CSA STANDARD: CAN/CSA S16:19/CAN 086:19

APPLICABLE CLAUSE(S): 27.11

SFRS: SYSTEM FOUNDATIONS: (2015NBC CLAUSE 4.1.8.16) CSA STANDARD: CAN/CSA A23.3-14 ☐ FOR ANCHORED FOOTINGS APPLICABLE CLAUSE(S): ■ FOR UNANCHORED FOOTINGS CONFIRMATION: FOUNDATIONS HAVE BEEN DESIGNED TO RESIST THE LATERAL FORCES APPLIED TO THE SFRS IN ACCORDANCE WITH THE

2015 NBC INCLUDING ALL APPLICABLE AMPLIFICATION FACTORS.

SEISMIC IMPORTANCE FACTOR: (2015 NBC CLAUSE 4.1.8.5)

REFERENCE CITY: COBOURG, ON SITE CLASS: THE NOTED SITE CLASSIFICATION FOR SEISMIC SITE RESPONSE AND SHEAR STRENGTH PARAMETERS INDICATED ARE AS REPORTED IN THE GEOTECHNICAL REPORT 171-07055-00 BY WSP, DATED AUGUST 3, 2017

□A □B □C ■D □E □F

<u>PGA</u>: 0.113 <u>PGV:</u> 0.086

RESPONSE SPECTRUM DATA: 5% DAMPED SPECTRAL RESPONSE NBC - APPENDIX C)

4C(CELERATION	VALUES FOR REFEREN	NCE CITY	<u>′:</u> (2015 NI
	Sa(0.2)	= 0.179	F(0.2)	= 1.22
	Sa(0.5)	=0.106	F(0.5)	= 1.45
	Sa(1.0)	=0.059	F(1.0)	= 1.53
	Sa(2.0)	=0.030	F(2.0)	= 1.55
	Sa(5.0)	=0.0074	F(5.0)	= 1.57
	Sa(10.0)	=0.0031	F(10.0)	= 1.48

DESIGN SPECTRAL RESPONSE ACCELERATION VALUES (DSRAV): (2015 NBC CLAUSE 4.1.8.4)

CLASS 'D':

Sa(0) =0.218 Sa(0.5) =0.154 Sa(1.0) Sa(2.0) =0.047 =0.006 Sa(5.0) Sa(10.0) =0.002

SYSTEM RESTRICTION VALUE: IEFaSa(0.2)=0.22 ≥ 0.35 ☐ YES

PERIOD DATA:

EMPIRICAL PERIOD: (2015NBC CLAUSE 4.1.8.11(3)(a),(b)or(c))

Ta(EMPIRICAL)NS = 0.22 sec Ta(EMPIRICAL)EW = 0.22 sec

MODAL PERIOD: (2015 NBC CLAUSE 4.1.8.11.(3)(d) AND 4.1.8.3.(8))

= 0.22 sec = 0.22 sec Ta(MODAL)EW = 0.22 sec

DESIGN PERIODS/MODE & MOMENT FACTORS: (2012 OBC CLAUSE (4.1.8.11(5))

Mv = 1.0 J=1.0Ta(DESIGN)NS = 0.22 sec Mv = 1.0 J=1.0 Ta(DESIGN)EW = 0.22 sec

TORSIONAL ECCENTRICITY:

■ ± 0.10 Dnx (4.1.8.11(10a), B ≤ 1.7 EQUIV. STATIC FORCE PROCEDURE) \Box ± 0.10 Dnx (4.1.8.12(4a), B \geq 1.7) □ ± 0.05 Dnx (4.1.8.12(4b), B \leq 1.7, 3-D DYNAMIC ANALYSIS)

STRUCTURAL SEPARATION:

■ THE ADJACENT STRUCTURES HAVE BEEN SEPARATED IN ACCORDANCE WITH 4.1.8.14(1) OF THE 2015 NBC □ N/A

D01.3.1 MAIN BUILDING SEISMIC (CONT.)

BUILDING WEIGHT FOR SEISMIC DESIGN: W=515 kN

STATIC VALUES: NORTH-SOUTH: (♦) (EAST-WEST SIMILAR)

STATIC MAXIMUM/MINIMUM VALUES: NORTH-SOUTH (🐧)(EAST-WEST SIMILAR)

 $V_{MIN} = S(2.0)MvIeW/(RdRo)$

 $VMAX = \frac{2}{3} S (0.2) \mathbf{I} EW/(RdRo)$

 $V_{MIN} = W*0.024 = 12 kN$ $V_{MAX} = W*0.80 = 41 kN$

SEISMIC LOADS		
EQUIVALENT STATIC (ES) FORCE PROCEDURE 2015 NBC CLAUSE 4.1.8.11(1)-(10)	DYNAMIC ANALYSIS (DYI) PROCEDURE (1)(3) (INITIAL SCALING FACTOR) 2015 NBC CLAUSE 4.1.8.12(1)-(5)	DESIGN (D) LOADS (2)
NORTH-SOUTH: (<u>‡</u>)	NORTH-SOUTH: (Ţ)	NORTH-SOUTH: (1)
Vesns = W* 0.086 = 41 kN Mesns = 305 kN•m	N/A	V _{DNS} = 41 kN M _{DNS} = 305 KN•m NON-ORTHOGONAL EFFECTS HAVE BE CONSIDERED IN ACCORDANCE WITH 2 NBC CLAUSE 4.1.8.8 (c) ☐ YES ■ N/A
EAST-WEST: (↔)	EAST-WEST: (↔)	EAST-WEST: (↔)
Vesew = W* 0.080 = 41 kN Mesew = 305 kN•m	N/A	V _{DEW} = 41 kN M _{DEW} = 305 kN•m NON-ORTHOGONAL EFFECTS HAVE BEEN CONSIDERED IN ACCORDANCE V 2015 NBC CLAUSE 4.1.8.8 (c) ☐ YES ■ N/A

NOTES:

- INITIAL DYNAMIC LOAD SCALING FACTOR
- S.F. = g $+\frac{1e}{RdRo}$ = g 0.513
- DYNAMIC ANALYSIS PROCEDURE LOADS ARE BASED ON THE EVALUATION OF THE BUILDING WITH THE INITIAL SCALING FACTOR APPLIED. WHEN USED THESE ARE COMPARED TO THE STATIC FORCE VALUE IN ACCORDANCE WITH 4.1.8.12(6)-(7) TO DETERMINE DESIGN LOAD VALUES.
- DESIGN LOAD SHEAR VALUES ARE BASED ON THE EVALUATION OF VES AND VDYI IN ACCORDANCE WITH 4.1.8.12 (5),(6) AND (7) OF THE 2012 OBC. LOADS INDICATED SHOW THE DESIGN BASE SHEAR AND CORRESPONDING OVERTURNING MOMENT
-) N/A = NOT USED IN THE DESIGN OF THE BUILDING.

D01.3.2 MAIN BUILDING: WIND

E IVIII WILL BOILD II TO. TTII TD	
<u>WIND</u> :	NORTH-SOUTH: () ↑
q= 0.49 KPa	Vbase = 125 kN
(1 IN 50 YEARS)	Mbase = 725 kN∙m
Iw = 1.00 (ULS)	EAST-WEST: ()↔
Iw = 0.75 (SLS)	Vbase = 85 kN
C _P C _g = 0.95 (1.43 AT ENDS)	Mbase = 495 kN•m

D01-4 DEFINITIONS

THE FOLLOW	ING ABBREVIATIONS HAVE BEEN USED IN TH	IESE NOTES A	ND DRAWINGS:
@ ARCH. B BLL BUL c/c C CONT. CW EA. EE EF EL. ES EW FF H HD MR IF LL m	AT (SPACING c/c) ARCHITECTURAL BOTTOM BOTTOM LOWER LAYER BOTTOM UPPER LAYER CENTRE TO CENTRE CENTRE LINE CONTINUOUS CORE WALL EACH EACH END EACH FACE ELEVATION EACH SIDE EACH WAY FAR FACE HORIZONTAL HEAVY DUTY GALVANIZED TRUSS TYPE MASONRY REINFORCEMENT INSIDE FACE LOWER LAYER METRES	mm MAX. MECH MIN. NF NTS OF PCO % S MR SW T TLL TOPC TYP. U/N UL U/S V	MILLIMETRES MAXIMUM MECHANICAL MINIMUM NEAR FACE NOT TO SCALE OUTSIDE FACE PILE CUT-OFF PLATE STANDARD GALVANIZED LADD MASONRY REINFORCEMENTS SHEARWALL TOP TOP LOWER LAYER TOP UPPER LAYER TOP OF PILE CAP TYPICAL UNLESS OTHERWISE NOTED UPPER LAYER UNDERSIDE VERTICAL

D01-5 SHOP DRAWINGS

c) STRUCTURAL STEEL/JOISTS.

- SUBMIT SHOP DRAWINGS FOR ALL STRUCTURAL WORK AND ANY WORK AFFECTING THE STRUCTURE TO THE THE DEPARTMENTAL REPRESENTATIVE. OBTAIN ARCHITECT'S & ENGINEER'S APPROVAL BEFORE PROCEEDING WITH THE
- EACH OF THE FOLLOWING SHOP DRAWINGS MUST BEAR THE SIGNATURE AND STAMP OF A QUALIFIED PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE (PLUS OTHER DRAWINGS AS NOTED). ALL SHOP DRAWINGS TO BE PREPARED AT THE EXPENSE OF THE CONTRACTOR.
- a) DRAWINGS FOR ANY TEMPORARY WORK. b) DRAWINGS FOR ANY STRUCTURAL PARTS DESIGNED BY THE CONTRACTOR'S FORCES INCLUDING EXTERIOR BUILDING ENVELOPE.
- 3. SHOP DRAWINGS MUST BE REVIEWED AND STAMPED REVIEWED BY THE CONTRACTOR BEFORE ISSUING TO THE DEPARTMENTAL REPRESENTATIVE.. SHOP DRAWINGS NOT STAMPED BY THE CONTRACTOR WILL BE REJECTED. ANY DELAYS IN THE CONSTRUCTION SCHEDULE DUE TO NONCOMPLIANCE WITH THIS REQUIREMENT SHALL BE THE
- RESPONSIBILITY OF THE CONTRACTOR. 4. SUBMIT STRUCTURAL STEEL, STEEL JOIST AND STEEL DECK SHOP DRAWINGS FOR STRUCTURAL ENGINEER'S REVIEW
- 5. SHOP DRAWINGS ARE REVIEWED FOR CONFORMANCE WITH THE GENERAL DESIGN CONCEPT. THIS REVIEW DOES NOT IMPLY APPROVAL OF THE DETAILED DESIGN OR QUANTITIES DESCRIBED IN THE SHOP DRAWINGS. THE RESPONSIBILITY FOR THE QUANTITIES AND DETAILED DESIGN OF THE MATERIALS AND COMPONENTS AS REQUIRED TO PROVIDE THE COMPLETE AND SATISFACTORY JOB DESCRIBED IN THE DESIGN DOCUMENTS REMAINS WITH THE CONTRACTOR.

BEFORE FABRICATION. ALL SHOP DRAWINGS SHALL BEAR THE SEAL OF A REGISTERED PROFESSIONAL ENGINEER IN THE

D01-6 TYPICAL DETAILS

- 1. TYPICAL DETAILS NOTED IN FOLLOWING SECTIONS TO BE USED WHERE SPECIFIC DETAILS HAVE NOT BEEN PROVIDED ON
- 2. WHERE MORE THAN ONE DETAILS APPLIES MORE EXPENSIVE VERSION WILL GOVERN AS PER THE DECISION OF THE DESIGN
- 3. DETAILS ARE TO BE READ IN CONJUNCTION WITH PLANS/SPECIFICATIONS.

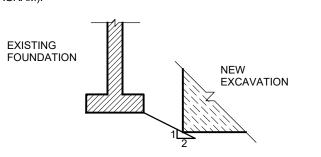
D31) FOUNDATIONS

D31-1 FOOTINGS ALL FOOTINGS TO BEAR ON COMPACTED ENGINEERED FILL WITH MINIMUM ALLOWABLE BEARING STRENGTHS OF 250 kPa (ULS) AND 150KPA (SLS) AND AS APPROVED BY GEOTECHNICAL ENGINEER ON SITE. REFERENCE GEOTECHNICAL REPORT: 171-07055-00

REPORT AUTHOR: WSP REPORT DATE: AUGUST 3, 2017

D31-2 PROTECT LATERAL STABILITY OF BEARING STRATA UNLESS NOTED

UNLESS OTHERWISE OUTLINED IN GEOTECTNICAL REPORT DO NOT EXCAVATE BELOW A LINE EXTENDING DOWNWARD FROM ANY BEARING STRATA AT A SLOPE OF MORE THAN 1 VERTICAL TO 2 HORIZONTAL. ADJUST FOOTING AND TRENCH ELEVATIONS TO MEET THIS REQUIREMENT (SEE DIAGRAM).



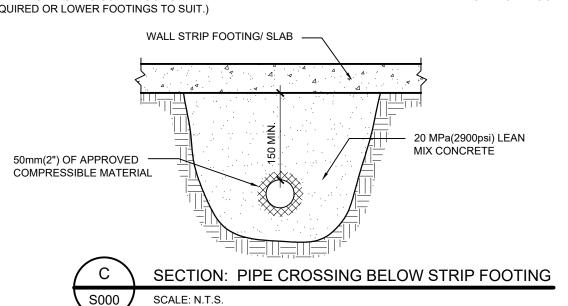
D31-4 MINIMUM FROST COVER REQUIREMENTS (NOTE: INCREASE DEPTHS AS REQUIRED BY GEOTECHNICAL REPORT/ENGINEER)

1500mm (5'-0") @ HEATED BUILDING (SNOW CLEARED): 1800mm (6'-0") @ ISOLATED AREAS: 2100mm (7'-0")

NOTE: DOES NOT APPLY TO FOUNDATIONS ADEQUATELY PROTECTED WITH EPS INSULATION, AS INDICATED ON DRAWINGS

D31-6 PIPE CROSSING BELOW STRIP FOOTING/SLAB

(NOTE: LOCATIONS WHERE PIPES CROSS BELOW FOOTINGS ARE TO BE APPROVED BY DEPARTMENTAL REPRESENTATIVE IN WRITING PRIOR TO CONSTRUCTION. DEPARTMENTAL REPRESENTATIVE RESERVES THE RIGHT TO RELOCATE PIPES AS REQUIRED OR LOWER FOOTINGS TO SUIT.)



D03) CONCRETE

D03-1 CONCRETE COVER (CLEAR TO REINFORCING): (UNLESS NOTED)

U/S FOOTINGS, PILE CAPS, GRADE BEAMS (AGAINST SOIL) FOOTINGS, PILE CAPS, GRADE BEAMS (SIDES & TOP) 50mm (2") WALLS 40mm (1½") SLABS 25mm (1") U/N 40mm (1½")(TO STIRRUPS) **PIERS** 40mm (1½")(TO TIES) U/N

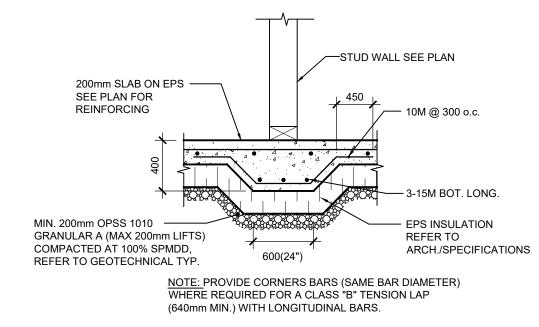
PROVIDE 32mm (11/4") COVER FOR BOTTOM STEEL FOR SLAB ABOVE 3HR, FIRE RATED AREAS. PROVIDE 50mm (2") COVER FOR COLUMN TIES IN 3HR, FIRE RATED AREAS.

D03-2 REINFORCING STEEL

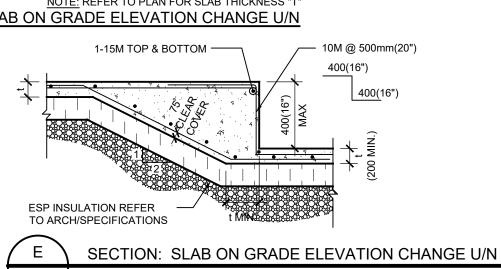
- DO NOT ELIMINATE OR DISPLACE REINFORCEMENT TO ACCOMMODATE HARDWARE. IF INSERTS CANNOT BE LOCATED AS SPECIFIED, OBTAIN APPROVAL OF ALL MODIFICATIONS FROM DEPARTMENTAL REPRESENTATIVE BEFORE THE PLACING OF
- WHERE TENSION LAPS ARE SPECIFIED, LAP REINFORCING STEEL IN ACCORDANCE WITH THE REQUIREMENTS OF CSA A23.3-19, LATEST EDITION. ALL OTHER LAPS AND EMBEDMENT OF DOWELS SHALL BE 24 BAR DIAMETERS, BUT NOT LESS THAN 600mm IF NOT SPECIFIED OTHERWISE. WIRE MESH LAPS SHALL BE 150mm MINIMUM. TYPICAL REBAR REQUIREMENTS:

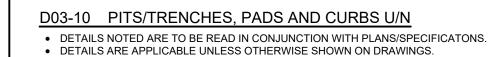
LAPS: AS ON DRAWINGS \geq 36 BAR DIA. \geq 1.5 Id \geq 600mm(24")

D03-3 LOADBEARING STUD WALL @ SLAB-ON-GRADE

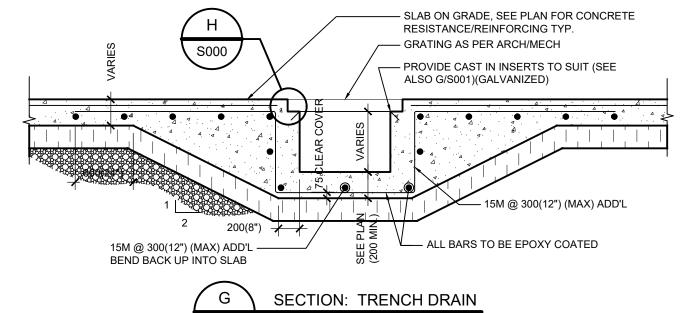


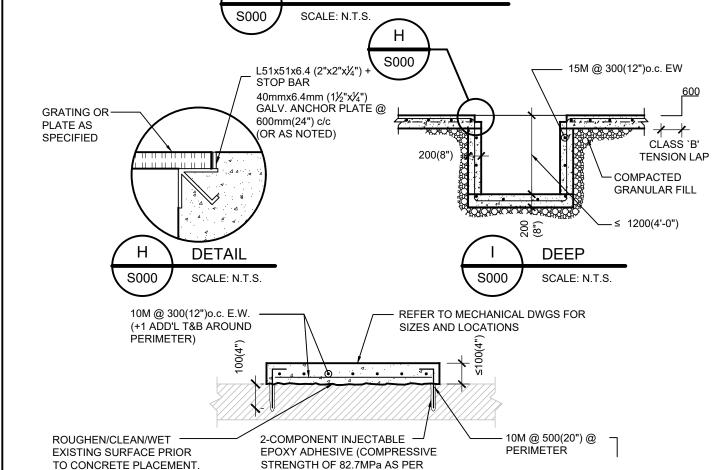
D03-7 HORIZONTAL STEEL DETAIL AT CORNERS U/N (NON-SHEAR WALL LOCATIONS) TYPICAL CORNER ≥ 36 BAR DIA. ≥ 500mm(20") — STANDARD HOOK -4-20M VFRT 4-20M VERT. -CORNER BARS SAME SIZE AND SPACING AS HORIZONTAL WALL BARS. SECTION: HORIZONTAL STEEL DETAIL AT CORNERS U/N S000 NOTE: REFER TO PLAN FOR SLAB THICKNESS "T" D03-8 SLAB ON GRADE ELEVATION CHANGE U/N





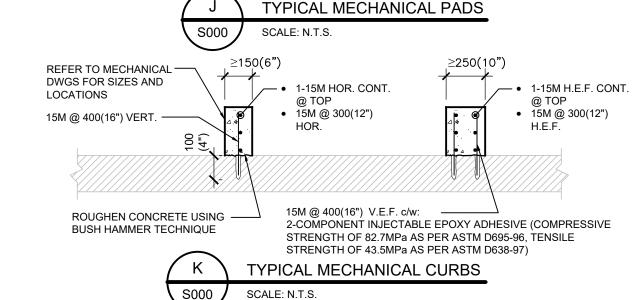
SCALE: N.T.S

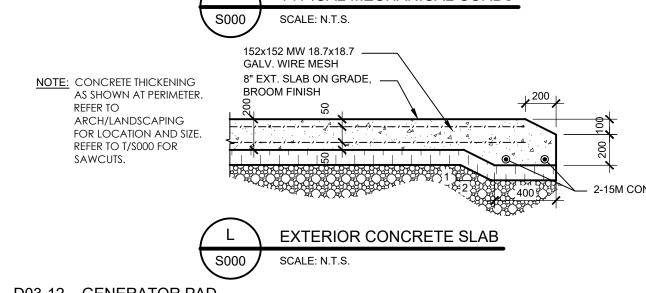


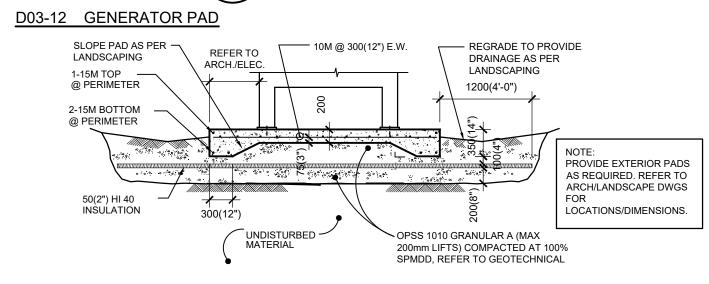


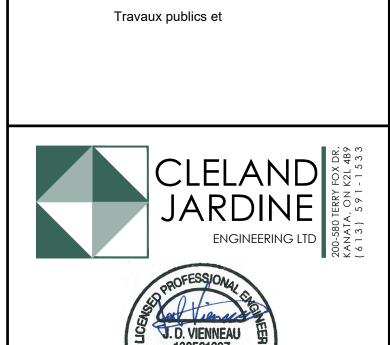
ASTM D695-96, TENSILE STRENGTH OF 43.5MPa AS PER

ASTM D638-97)





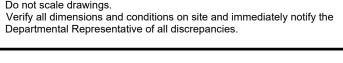


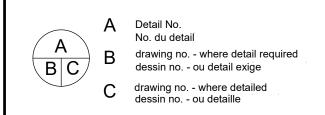


Public Works and

Government Services Canada

05	ISSUED FOR TENDER	17 DEC 2020
04	ISSUED FOR 100% COORDINATION	07 OCT 2020
03	RE-ISSUED FOR 99% COORDINATION	30 JUNE 2020
02	ISSUED FOR 99% COORDINATION	30 January 2020
01	ISSUED FOR 66% COORDINATION	29 March 2019
revision		date
D t	L. dansdam	





COBOURG CITY COBOURG SEARCH AND

project title

titre du projet

COBOURG

RESCUE STATION GENERAL NOTES AND DETAILS

drawn by dessine par	WL	
designed by conc par	JV	
approved by approuve par	JV	
bid offre	DFO PROJECT MANAGER	project manager administratet de proje
project date date du projet	05-03-2019	

no. du projet R.084112.005

dessine no.

CJE JOB #: 17-0273AA

ON