March 23, 2021

Addendum 1: The following changes/clarifications in the tender documents are effective immediately. This addendum will form part of the contract documents.

REVISIONS TO DRAWINGS:

- 1. Revise drawing S001 per attached clouded revisions
- 2. Revise drawing \$105 per attached clouded revisions
- 3. Revise drawing S203 per attached clouded revisions
- 4. Revise drawing \$204 per attached clouded revisions
- 5. Revise drawing S205 per attached clouded revisions
- 6. Revise drawing S206 per attached clouded revisions
- 7. Revise drawing \$208 per attached clouded revisions
- 8. Revise drawing E100 per attached clouded revisions

Revisions to Specifications

- 1. Section 01 51 01 Temporary Facilities: replace clause 1.8.2.1 with the following:

 "A minimum of 240 sq ft of office space for exclusive use of Departmental Representative's consultant team and other personnel as determined by the Departmental Representative.

 Office space may be one or more buildings containing a minimum of 2 separate lock-up offices and one meeting room. The offices shall be furnished with desks and chairs, and the meeting room shall have a table, six chairs and a white board. The offices shall be equipped with internet access, and a colour printer capable of printing letter size as well as 11x17 size paper, and with connectivity with each office space.-Power, drinking water, heating and maintenance of the buildings are Contractor's responsibility"
- 2. Section 05 12 33 Structural Steel for Bridges, omit specification 05 12 33 Issued for Tender and replace with specification Section 05 12 33 Issued for Tender Addendum 1.
- 3. Section 28 31 00 Fire Detection and Alarm, delete clause 1.7.1.4.

END OF ADDENDUM 1

Part 1 General

1.1 DESCRIPTION

.1 This Section covers the supply, fabrication, and installation of the removable roadway bridge at the east end of the extended dock.

1.2 RELATED REQUIREMENTS

- .1 Section 01 33 00 (Submittal Procedures)
- .2 Section 02 41 16.01 (Structure Demolition)
- .3 Section 03 30 00 (Cast-in-Place Concrete)
- .4 Section 03 39 00 (In-Situ Concrete Structures)
- .5 Section 05 50 00 (Metal Fabrications)
- .6 Section 09 97 19 (Painting Exterior Metal Surfaces)
- .7 Section 26 05 27 (Grounding)

1.3 MEASUREMENT AND PAYMENT PROCEDURES

- .1 Work items related to the removable bridge will not be measured individually. All work items related to removable bridge will be paid for at the Lump Sum price tendered for REMOVABLE BRIDGE. Payment shall be full compensation for all work in connection with the supply, fabrication, and installation of steel removable bridge, splices, bearings, bolts, nuts and washers, survey, levelling, testing, aligning and grouting:
 - .1 Ensure lump sum price includes radiographic examination of optional shop splices and additional field splices.

1.4 REFERENCES

- .1 American Association for State Highway and Transportation Officials (AASHTO)
 - .1 AASHTO Standard Specifications for Highway Bridges-[17th Edition 2002].

.2 ASTM International

- .1 ASTM A325M-[09], Standard Specification for Structural Bolts, Steel, Heat Treated 830 MPa Minimum Tensile Strength [Metric].
- .2 ASTM A490M-[09], Standard Specification for High-Strength Steel Bolts, Classes 10.9 and 10.9.3, for Structural Steel Joints.
- .3 CSA International

- .1 CSA G40.20/G40.21-[04(R2009)], General Requirements for Rolled or Welded Structural Quality Steel/Structural Quality Steel.
- .2 CAN/CSA G164-[M92(R2003)], Hot Dip Galvanizing of Irregularly Shaped Articles.
- .3 CAN/CSA S6-19, Canadian Highway Bridge Design Code.
- .4 CSA S16-14, Design of Steel Structures.
- .5 CSA S269.1-[1975(R2003)], Falsework for Construction Purposes.
- .6 CSA W48-[06], Filler Metals and Allied Materials for Metal Arc Welding.
- .7 CSA W59-[03(R2008)], Welded Steel Construction, (Metal Arc Welding).
- .4 United States Department of Defence
 - .1 MIL-PRF-24667 Performance Specification Coating System, Non-Skid, For Roll, Spray, Or Self-Adhering Application.

1.5 ACTION AND INFORMATIONAL SUBMITTALS

- .1 Submit in accordance with Section 01 33 00 (Submittal Procedures).
- .2 Product Data:
 - .1 Submit manufacturer's instructions, printed product literature and data sheets for structural steel and include product characteristics, performance criteria, physical size, finish and limitations.
 - .2 Submit product data sheets for non-skid coating system.
 - .3 Submit mock-up samples of galvanized and coated bridge deck material of sufficient size to show full surface texture and pattern of coating.

.3 Shop Drawings:

- .1 Submit drawings stamped and signed by professional engineer registered or licensed in the Province of British Columbia, Canada.
- .2 Indicate shop and erection details including shop splices, cuts, copes, connections, holes, bearing plates, threaded fasteners, rivets and welds. Indicate welds by CSA W59, welding symbols.
- .3 Proposed welding procedures to be stamped and approved by Canadian Welding Bureau.
- .4 Submit description of methods, temporary bracing and strengthening, sequence of erection and type of equipment proposed for use in erecting structural steel.

1.6 DELIVERY, STORAGE, AND HANDLING

- .1 Deliver, store and handle in accordance with Section [01 61 00 Common Product Requirements].
- .2 Provide protective blocking for lifting, transportation and storing.

- .1 Exercise care during fabrication, transportation and erection so as not to damage girders and beams.
- .2 Do not notch edges of members.
- .3 Do not cause excessive stresses.
- .3 Mark mass on members weighing more than 3 tonnes.
- .4 Ensure that no portion of steel comes into contact with ground.
- .5 Ensure that no items bear on the coated deck.
- .6 Provide Departmental Representative with delivery schedules minimum 14 days prior to shipping.
- .7 Replace defective or damaged materials with new.

1.7 QUALITY ASSURANCE

- .1 Preconstruction Testing:
 - .1 Provide suitable facilities and cooperate with Departmental Representative in carrying out inspection and tests required.

Part 2 Products

2.1 MATERIALS

- .1 All bridge steel to be hot dip galvanized.
- .2 Traffic surface of bridge deck to be coated with non-skid coating after galvanizing.
- .3 Structural steel: to CSA G40.20/G40.21, grade and types 350WT Category 2.
- .4 High strength bolts, nuts and washers: to ASTM A325M, galvanized.
- .5 Anchor bolts, washers and nuts: to ASTM A193 GRB8 Class 2...
- .6 Bearings: elastomer bearing pads of neoprene, grade 60 to CAN/CSA S6.
- .7 Welding electrodes: to CSA W48 series.
- .8 Hot dip galvanizing: to CAN/CSA G164, minimum zinc coating of [600] g/m².
- .9 Non-skid coating: To MIL-PRF-24667C Type I, V or VI, Composition G.
- .10 Shrinkage compensating grout: premixed compound consisting of non-metallic aggregate, Portland cement, water reducing and plasticizing agents.

2.2 SOURCE QUALITY CONTROL

.1 Steel producer qualifications: certified in accordance with CSA G40.20/G40.21.

- .2 Submit Departmental Representative 2 copies of certified test reports for Charpy V-notch test.
- .3 Provide suitable facilities and co-operate with Departmental Representative in carrying out inspection and tests required.

Part 3 Execution

3.1 EXAMINATION

- .1 Verification of Conditions: verify conditions of substrates previously installed under other Sections or Contracts are acceptable for structural steel installation in accordance with manufacturer's written instructions.
 - .1 Inform Departmental Representative of unacceptable conditions immediately upon discovery.
 - .2 Proceed with installation only after unacceptable conditions have been remedied and after receipt of written approval to proceed from Departmental Representative.

3.2 PREPARATION

- .1 Clean steel surfaces as directed by Departmental Representative when staining or defacing occurs.
- .2 Verify location of substructure units, elevations of bearing seats and location of anchor bolts before erection of structural steel; report discrepancies to Departmental Representative.
- .3 Restrict drifting during assembly to minimum required to bring parts into position without enlarging or distorting holes, and without distorting, kinking or sharply bending metal of any unit.
 - .1 Enlarge holes if necessary, by reaming only after receipt of written approval from Departmental Representative.
 - .2 Ensure reamed holes are 2 mm maximum larger than bolt size used.
- .4 Bridge elements to be trial assembled prior to galvanizing of individual elements.
- .5 Fabricate and install bearings as indicated.
- .6 Coat traffic surface of bridge in full conformance with coating suppliers' requirements.

3.3 INSTALLATION

- .1 Do falsework in accordance to CSA S269.1.
- .2 Do fabrication and erection of structural steel in accordance with CAN/CSA S6, Design of Highway Bridges.

- .3 Do welding in accordance with CSA W59, except where specified otherwise.
 - .1 For CSA G40.20/G40.21, grade 350WT steel, deposited weld metal to have Charpy V-Notch value not lower than that of steel.
 - .2 Do welding in shop unless otherwise permitted by Departmental Representative.
 - .3 Weld only at locations indicated.
- .4 High strength bolting: in accordance with CAN/CSA S6. Use 'turn-of-nut' tightening method.
- .5 Finish: members true to line, free from twists, bends, open joints, sharp corners and sharp edges.
- .6 Allowable tolerance for bolt holes:
 - .1 Matching holes for bolts to line up so that dowel 2 mm less in diameter than hole passes freely through assembled members at right angles to such members.
 - .2 Finish holes not more than 2 mm in diameter larger than diameter of rivet or bolt unless otherwise specified by Departmental Representative.
 - .3 Centre-to-centre distance between any two holes of group to vary by not more than 1 mm from dimensioned distance between such holes.
 - .4 Centre-to-centre distance between any two groups of holes to vary not more than maximum of the following:

Centre-to-Centre distance in metres	Tolerance in plus or minus mm
less than 10	1
10 to 20	2
20 to 30	3

- .5 Correct mis-punched or mis-drilled members only as directed by Departmental Representative
- .7 Span length tolerances:
 - .1 Girders and beams: plus or minus 6 mm
 - .2 Centre-to-centre of bearing stiffeners and bearing plates: plus or minus 3 mm.
- .8 Shop splices:
 - .1 Use complete joint penetration groove welds finished flush.
 - .2 Details of butt joints to CSA W59.
 - .3 Use only as approved by Departmental Representative.
- .9 Camber:
 - .1 Camber tolerances for plate girders to be to CSA W59.

- .2 Record measurements of camber of each girder, at points indicated.
- .3 Submit diagram to Departmental Representative showing camber for each girder fabricated.
- .4 Advise Departmental Representative immediately when camber of fabricated girder is greater than specified tolerances.
- .5 Submit proposal for corrective measures.
- .6 Undertake remedial measures as approved by Departmental Representative.

.10 Shop erection:

- .1 Support each girder on its bearing points and measure and record deflection at same points indicated for measurement of camber.
- .2 Measure deflections in plane of girder web.
- .3 Submit diagram to Departmental Representative showing deflection measurements for each girder before delivery.
- .11 Mark members in accordance with CSA G40.20/G40.21.
 - .1 Do not use die stamping.
 - .2 Place marking at locations hidden when viewed from exterior after erection when steel is to be left in unpainted condition.
- .12 Match marking: shop mark bearing assemblies and splices.
- .13 Protect exposed concrete surfaces of substructures from staining.
 - .1 Use galvanized anchors for anchorage to concrete.
 - .2 Submit details of installation and methods of support to Departmental Representative for review prior to commencing protection work.

3.4 FIELD QUALITY CONTROL

- .1 Manufacturer's Field Services:
 - .1 Obtain written report from manufacturer verifying compliance of Work, in handling, installing, protecting and cleaning of steel.
 - .2 Submit manufacturer's field services consisting of product use recommendations and periodic site visits for inspection of product installation in accordance with manufacturer's instructions.
 - .3 Ensure manufacturer's representative is present before installation, during critical periods of installation and during construction of field joints and testing.
 - .4 Schedule site visits:
 - .1 After delivery and storage of products, and when preparatory Work, or other Work, on which the Work of this Section depends, is complete but before installation begins.

STRUCTURAL STEEL FOR BRIDGES

Page 7 of 7

- .2 Twice during progress of Work at 25% and 60% complete.
- .3 Upon completion of the Work, after cleaning is carried out.

3.5 CLEANING

- .1 Progress Cleaning: clean in accordance with Section 01 74 11 Cleaning.
 - .1 Leave Work area clean at end of each day.
- .2 Final Cleaning: upon completion remove surplus materials, rubbish, tools and equipment in accordance with Section 01 74 11 Cleaning.

END OF SECTION

DESIGN CRITERIA:

- DESIGN BASIS: CAN/CSA S6-19, CANADIAN HIGHWAY BRIDGE DESIGN CODE (CHBDC)
- APRON LIVE LOADS: (ALL LOADS ARE UNFACTORED)
- a) UNIFORMLY DISTRIBUTED LOAD (UDL): 48kPa b) APRON UDL NEED NOT BE CONSIDERED COINCIDENT WITH VEHICLE WHEEL LOADS
- c) AXLE LOADING:

TRUCK LOADING (CL-625 PER S6, LATEST VERSION): FRONT AXLE: 50kN (DISTRIBUTED TO 2 WHEELS)

REAR TANDEM AXLES: 175kN (DISTRIBUTED TO 4 PAIRS OF WHEELS)

MOBILE CRANE LOADING (GROVE GRT880E):

FRONT AXLE: 269kN (DISTRIBUTED TO 2 WHEELS) 247kN (DISTRIBUTED TO 2 WHEELS) REAR AXLE: OUTRIGGER LOAD: 375kN ÒN 775mmø PAD

ESQUIMALT FIRE TRUCK (TYPE 1), 1993 THIBAULT ENGINE:

FRONT AXLE: 75kN (DISTRIBUTED TO 2 WHEELS) REAR AXLE: 100kN (DISTRIBUTED TO 2 PAIRS OF WHEELS)

ESQUIMALT FIRE TRUCK (TYPE 2), 1999 E LADDER TRUCK: FRONT AXLE: 85kN (DISTRIBUTED TO 2 WHEELS)

REAR TANDEM AXLES: 210kN (DISTRIBUTED TO 4 PAIRS OF WHEELS)

ESQUIMALT FIRE TRUCK (TYPE 3), 2008 FORT GARY RESCUE TRUCK: FRONT AXLE: 55kN (DISTRIBUTED TO 2 WHEELS) REAR AXLE: 105kN (DISTRIBUTED TO 2 PAIRS OF WHEELS)

ESQUIMALT FIRE TRUCK (TYPE 4), 2018 ROSENBAUER:

FRONT AXLE: 101kN (DISTRIBUTED TO 2 WHEELS) REAR TANDEM AXLES: 280kN (DISTRIBUTED TO 4 PAIRS OF WHEELS)

DYNAMIC LOAD ALLOWANCE (DLA) SHOULD BE ADDED TO ALL AXLE/WHEEL LOADS.

3. 30T GANTRY CRANE: MAXIMUM WHEEL LOADS (ALL LOADS ARE UNFACTORED)

STATIC LOAD: 299kN OPERATING LOAD: 377kN WORKING WIND LOAD: 464kN

- 4. DRY DOCK FLOOR SLAB LIVE LOADING: (ALL LOADS ARE UNFACTORED) a) KEEL BLOCKS: 2,640kPa
- 5. LIVE LOADING WITHIN ACCESS CHAMBER: (ALL LOADS ARE UNFACTORED) UNIFORMLY DISTRIBUTED LOAD (UDL) = 3.6kPa
- 6. MOORING LOADS: (ALL LOADS ARE UNFACTORED)
 - a) 37.5 TONNE CLEAT: 334kn THE MOORING LINE LOADS CAN BE APPLIED AT UP TO 30° ABOVE
- HORIZONTAL, AND IN ANY OFFSHORE DIRECTION (-90° TO +90°) ON PLAN.
- SEISMIC LOADS: LOADING BASED ON SITE SPECIFIC SEISMIC DATA OBTAINED FROM NATIONAL RESOURCES CANADA, FOR THE A2475 EVENT IN ACCORDANCE WITH NBC 2015 AS FOLLOWS:

	A2475 EVENT
PROBABILITY OF EXCEEDANCE PER ANNUM PROBABILITY OF EXCEEDANCE IN 50 YEARS Sa(0.2) Sa(0.5) Sa(1.0) Sa(2.0) PEAK HORIZONTAL GROUND ACCELERATION,	0.000404 2% 1.310 1.169 0.688 0.407 A 0.586g

GENERAL NOTES:

GENERAL:

- 1. READ STRUCTURAL DRAWINGS IN CONJUNCTION WITH ALL OTHER CONTRACT DRAWINGS AND DOCUMENTS. REPORT ANY CONFLICTS TO THE DEPARTMENTAL REPRESENTATIVE BEFORE
- 2. VERIFY ALL DIMENSIONS AND ELEVATIONS PRIOR TO CONSTRUCTION.
- 3. CONTRACTOR'S RESPONSIBILITY: THESE DRAWINGS SHOW COMPLETED STRUCTURAL COMPONENTS. THE REQUIRED TEMPORARY FALSEWORK, FORMWORK, BRACING AND SHORING TO PERFORM THE WORK SAFELY IS THE RESPONSIBILITY OF THE CONTRACTOR.
- 4. ENVIRONMENTAL WORK PROCEDURES, TIMING, AND SPECIAL PRECAUTIONS SHALL BE IN ACCORDANCE WITH THE SPECIFICATIONS.
- 5. NOTIFY DEPARTMENTAL REPRESENTATIVE 48 HOURS IN ADVANCE FOR REVIEW OF STRUCTURAL COMPONENTS BEFORE COVERING UP.
- 6. UNDER NO CIRCUMSTANCES SHALL DRAWINGS BE SCALED. COMPONENTS MAY BE SHOWN SCHEMATICALLY.
- 7. ELEVATIONS ARE IN METRES AND DIMENSIONS ARE IN MILLIMETRES.
- 8. DATUM: ELEVATIONS ON DRAWINGS ARE GIVEN TO GEODETIC DATUM.

TIDE	ELEVATIONS:	CHART DATUM:	GEODETIC DATUM
	EXTREME HIGH WATER LEVEL (E.H.W.L.)	3.8m	1.929m
	HIGHER HIGH WATER LEVEL (H.H.W.L.)	3.4m	1.529m
	MEAN WATER LEVEL (M.W.L.)	1.9m	0.029m
	LOWER LOW WATER LEVEL (L.L.W.L.)	0.1m	-1.971m
	EXTREME LOW WATER LEVEL (E.L.W.L.)	-0.5m	-2.371m

PWGSC SITE BENCHMARK IS "BOLT". FROM PWGSC PLAN SK4593-1 DATED OCTOBER 1989, "BOLT" IS ELEVATION 4.725m TO PWGSC DATUM, BOLT IS 4.849m ABOVE L.L.W.L.; L.L.W.L. IS 0.124m BELOW PWGSC DATUM 0.0; L.L.W.L. IS 1.871m BELOW GEODETIC ELEVATION.

- TO CONVERT FROM GEODETIC DATUM TO CHART DATUM, ADD 1.871m.
- TO CONVERT FROM GEODETIC DATUM TO PWGSC DATUM, ADD 1.747m.
- TO CONVERT FROM PWGSC DATUM TO CHART DATUM, ADD 0.124m.

GEODETIC CHART **PWGSC** H.H.W.L.: 3.400 —H.H.W.L.: 3.276 —H.H.W.L.: 1.529 —GEODETIC DATUM: 1.871 —GEODETIC DATUM: 1.747 -GEODETIC DATUM: 0.000 PWGSC DATUM: 0.000 PWGSC DATUM: 0.124 **PWGSC DATUM:** −1.747 -CHART DATUM: 0.000 —CHART DATUM: -0.124 -CHART DATUM: -1.871 **└**E.L.W.L.: −0.500 **└**E.L.W.L.: −0.624 **└**E.L.W.L.: −2.371

DATUM CONVERSION SCALE N.T.S.

SYMBOLS AND ABBREVIATIONS

CD — CENTRELINE CONSTRUCTION JOINT D CLEAR CRB CONCRETE ROAD BARRIER C/W COMPLETE WITH DRAWING EACH FACE ELEVATION

DWG. E.F. EL. EQ. EQUAL FB FLAT BAR GB GRADE BREAK HORZ HORIZONTAL I.F. INSIDE DIAMETER LLH LONG LEG HORIZONTAL LONG LEG VERTICAL LLV METRES m

 MILLIMETRES mm MAX. MAXIMUM MIN. MINIMUM N.T.S. NOT TO SCALE OPP. OPPOSITE PLATE PL _

PROJ. PROJECTION **PWGSC** PUBLIC WORKS AND GOVERNMENT SERVICES CANADA

RADIUS REV. REVISION SIM. SIMILAR SJ SAW CUT CONTROL JOINT

SPACED STAINLESS STEEL S.S. OR SS

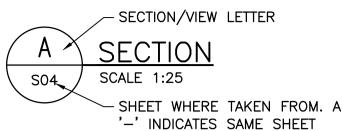
T.S. TYP. TOP OF TYPICAL U/S UNDERSIDE UNLESS NOTED OTHERWISE U.N.O.

 ULTRA HIGH MOLECULAR UHMWPE WEIGHT POLYETHYLENE **VERT** VERTICAL WORK POINT WATER STOP

REINFORCEMENT ABBREVIATIONS

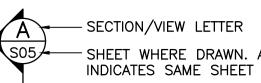
 25M STRAIGHT BAR, 2300 LONG 15M BAR, STANDARD HOOK 1—END 15M H1E 15M H2E - 15M BAR, STANDARD HOOK 2-ENDS 15M H1E500 - 15M BAR, 500 LONG HOOK 1-END 20MSS 20M STAINLESS STEEL BAR

SHEET WHERE DRAWN. A '-' INDICATES SAME SHEET



REVISION LETTER/NUMBER

WORK POINT



Région de Pacifique

Public Works and Government Services

HEL JOB FILE: 3191-017

REAL PROPERTY SERVICES

Pacific Region

SERVICES IMMOBILIERS

Travaux publics et Services gouvernementaux



Client / client		
Revision/ Revision	Description/Description	Date/Date
Α	75% REVIEW	2020.05.20
В	99% REVIEW	2020.07.10
С	TENDER	2021.01.07
D	ADDENDUM	2021.03.23

PUBLIC WORKS AND GOVERNMENT SERVICES CANADA

Project title/Titre du projet **ESQUIMALT GRAVING DOCK** 825 ADMIRALS ROAD, VICTORIA, BC

ESQUIMALT GRAVING DOCK EAST END EXTENSION AND SECTION 3 DOCK FLOOR AND WALL REFURBISHMENT

Consultant Signature Only

Designed by/Concept par KU / MGCS

Drawn by/Dessine par

PWGSC Project Manager/Administrateur de Projets TPSGC **EUGENE YEUNG**

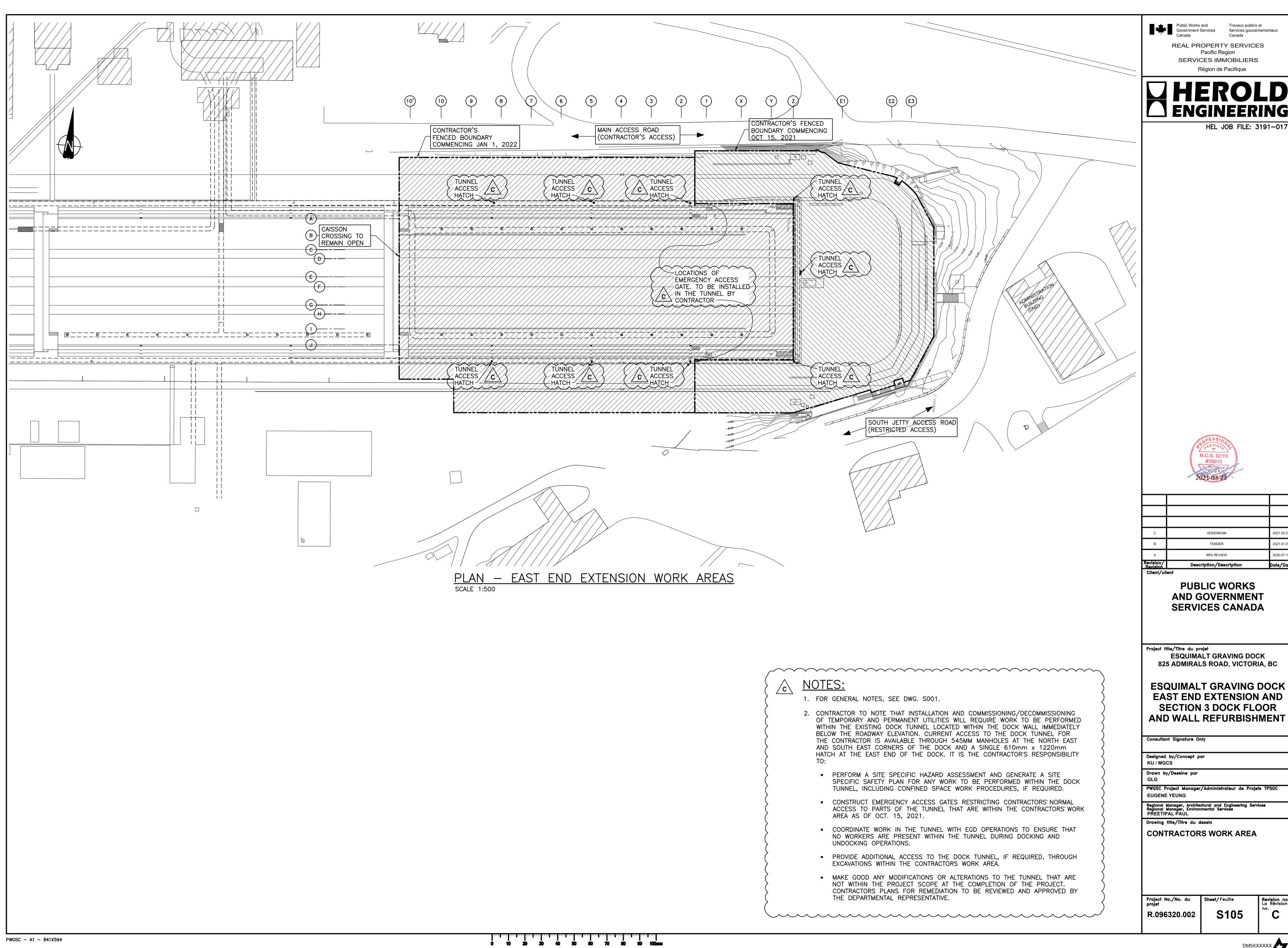
Regional Manager, Architectural and Engineering Services Regional Manager, Environmental Services PREETIPAL PAUL Drawing title/Titre du dessin

GENERAL NOTES

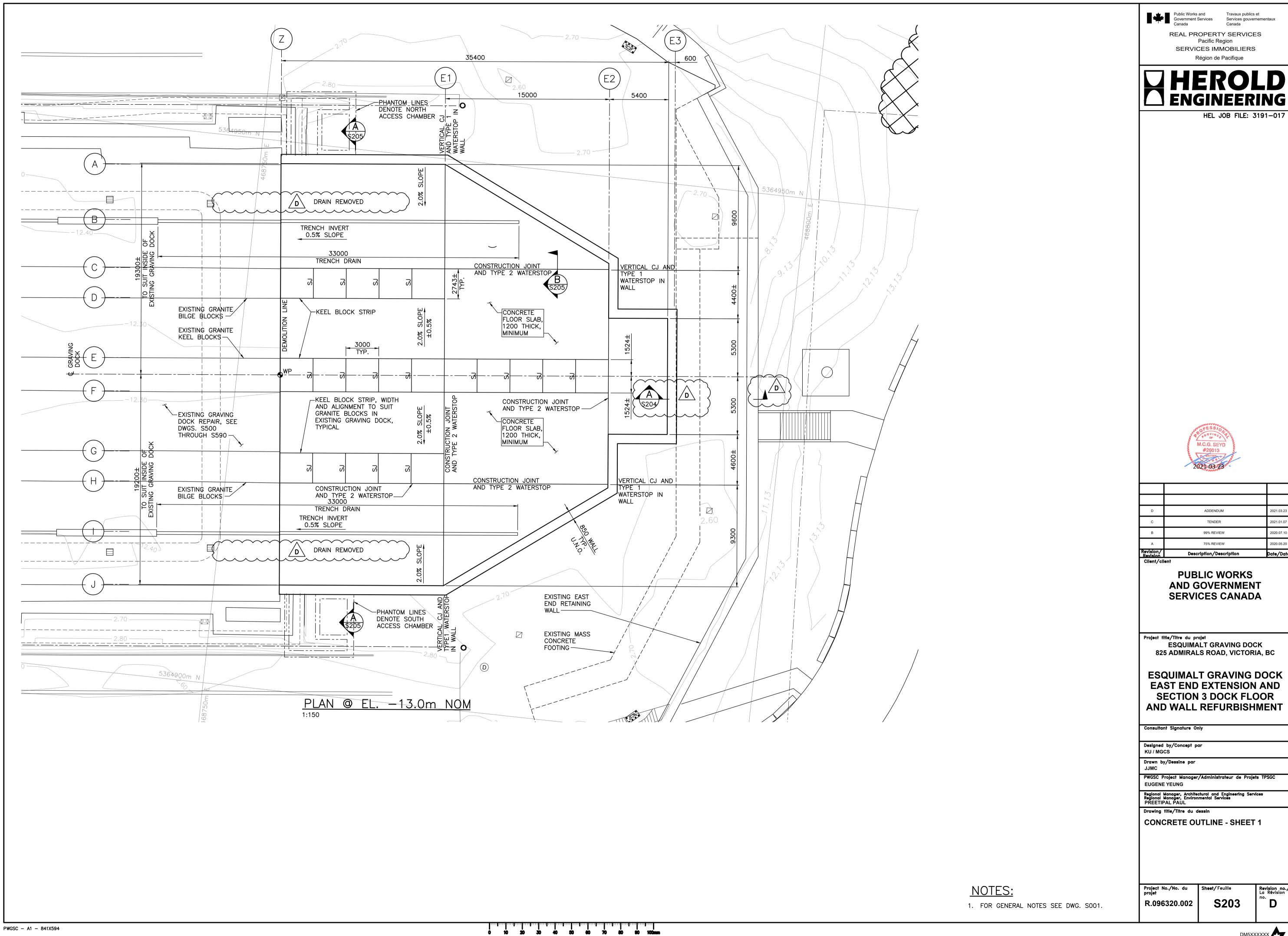
Project No./No. du projet Sheet / Feuille **S001** R.096320.002

DM5XXXXXX

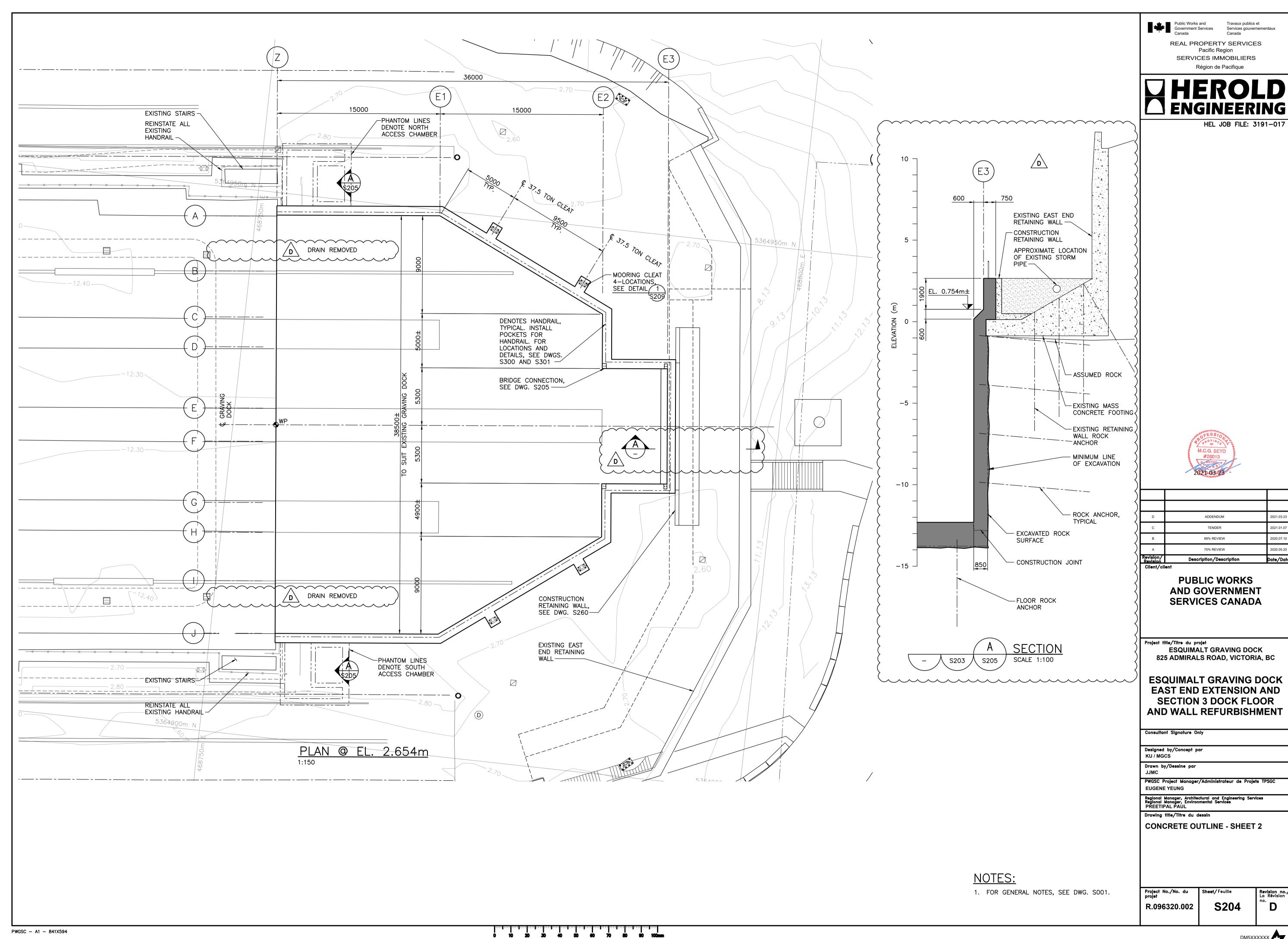
PWGSC - A1 - 841X594

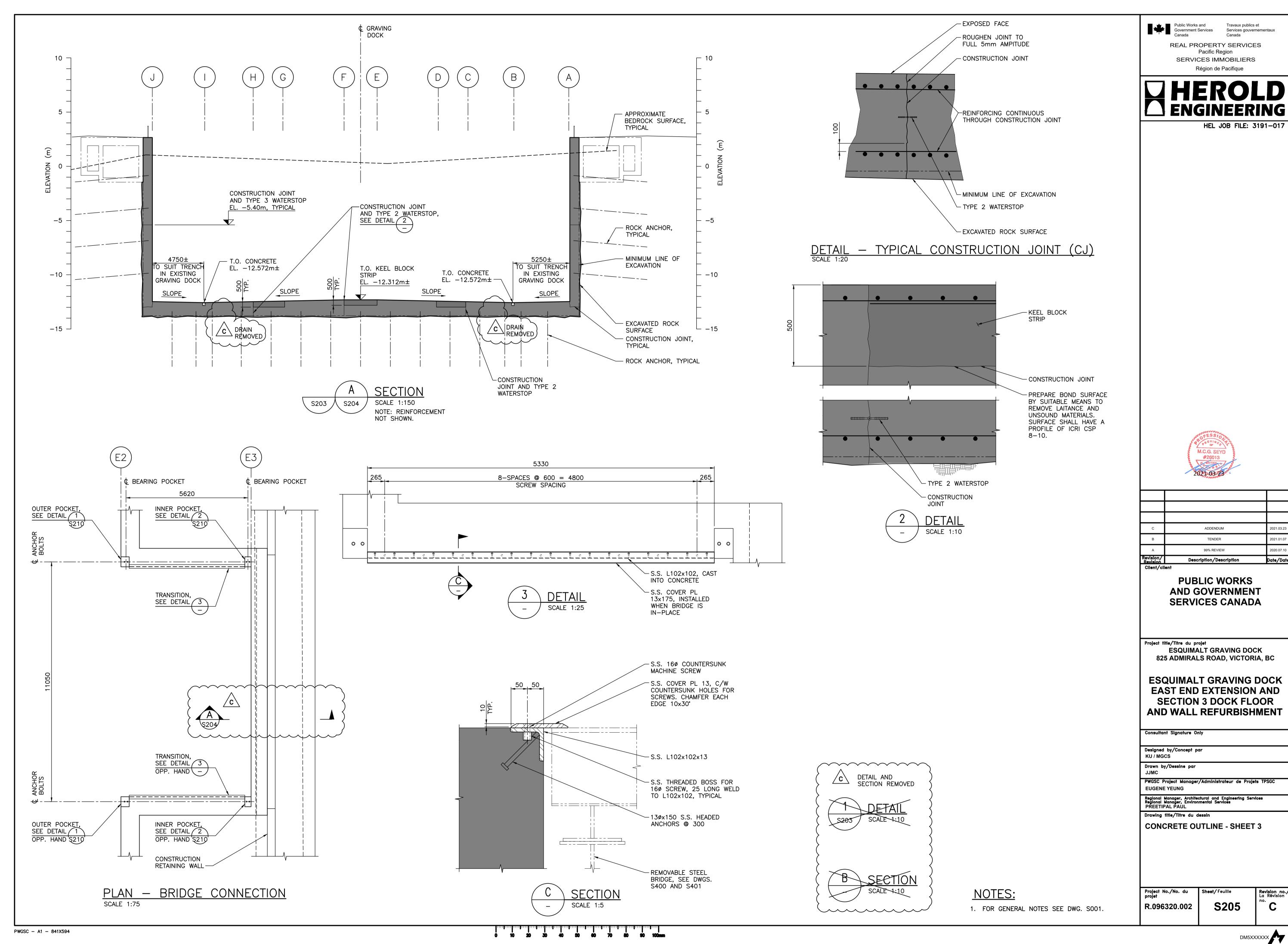


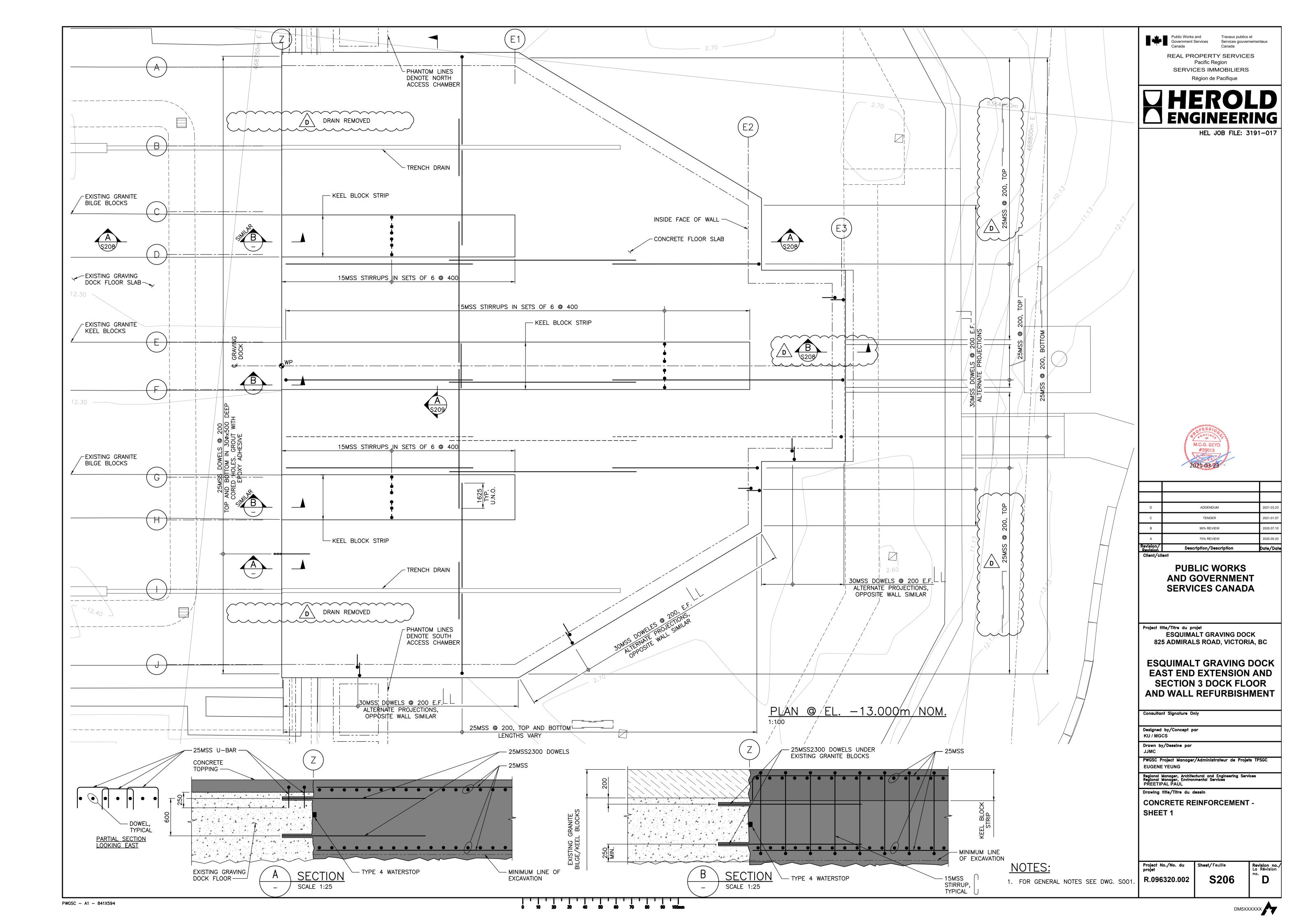
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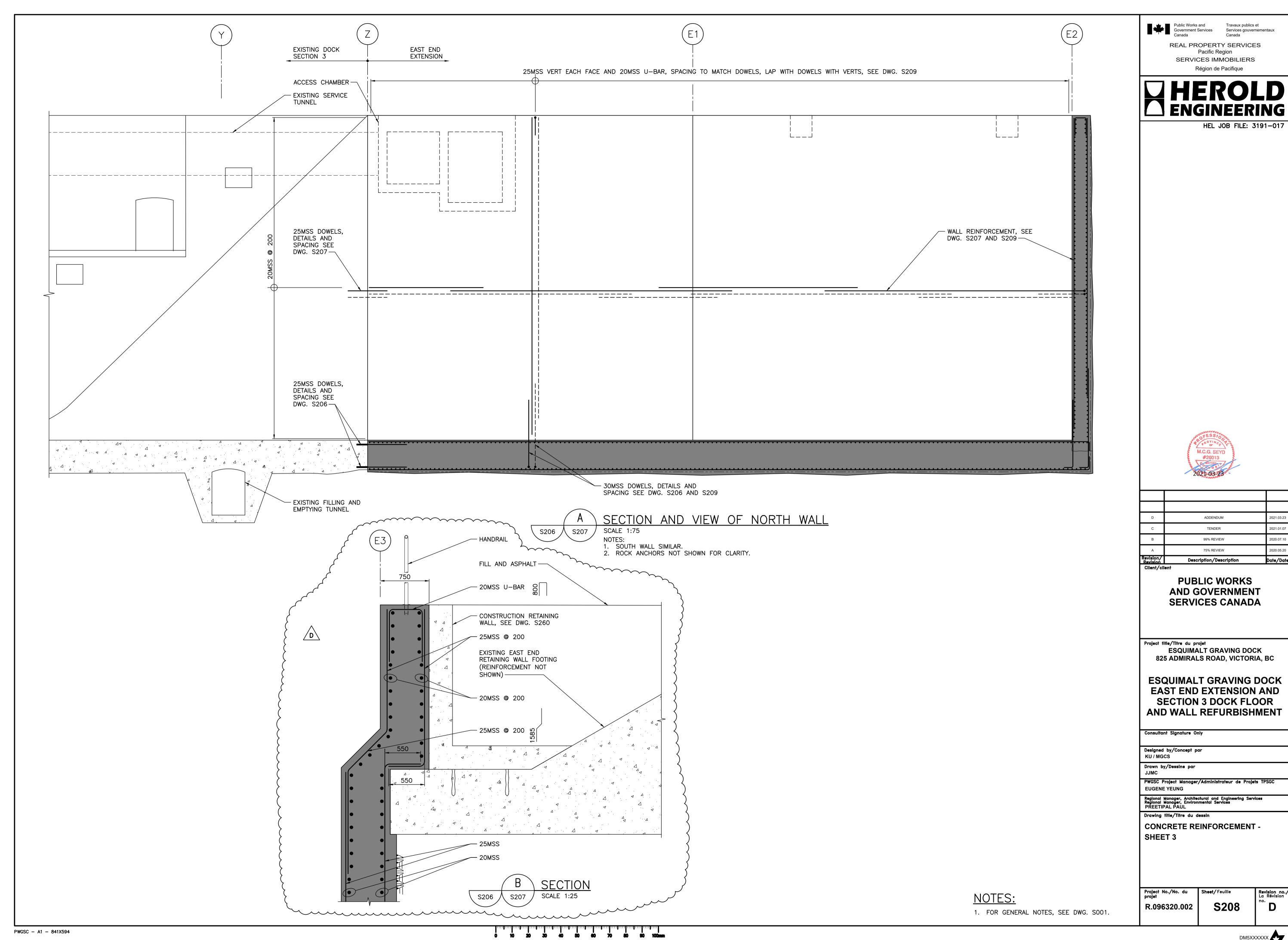


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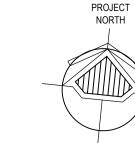






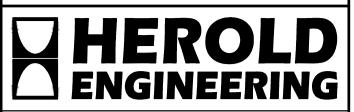


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REAL PROPERTY SERVICES Pacific Region SERVICES IMMOBILIERS Région de Pacifique



HEL JOB FILE: 3191-017

Services gouvernementaux



AES PROJECT #1-19-062



Client/client		
Revision/ Revision	Description/Description	Date/Date
1	ISSUED FOR 75% REVIEW	20/05/20
2	ISSUED FOR 99% REVIEW	10/07/20
3	ISSUED FOR TENDER	07/01/21
4	ISSUED FOR ADDENDUM#1	04/02/21

PUBLIC WORKS AND GOVERNMENT SERVICES CANADA

Project title/Titre du projet **ESQUIMALT GRAVING DOCK** 825 ADMIRALS ROAD, VICTORIA, BC

ESQUIMALT GRAVING DOCK EAST END EXTENSION AND SECTION 3 DOCK FLOOR AND WALL REFURBISHMENT

Consultant Signature Only

Drawn by/Dessine par GINA LECLAIR, RACHEL SHIN

PWGSC Project Manager/Administrateur de Projets TPSGC

Regional Manager, Architectural and Engineering Services Regional Manager, Environmental Services PREETIPAL PAUL

Drawing title/Titre du dessin

R.096320.002

LEGEND, AND EXISTING

SITE PLAN NEAR DOCK END

E100

ELECTRICAL SYMBOL LEGEND **ABBREVIATIONS** F/A FIRE ALARM SSM STAND SINGLE MODE OPTICAL FIBER SMM STRAND MULTI MODE OPTICAL FIBER WP DENOTES WEATHER PROOF DEVICE JUNCTION BOX PULLBOX, NEW OR EXISTING. NUMBERS REFER TO EGD 53 EXISTING OR UPDATED NUMBERING SCHEME. E LABELS ARE UUSED WHEN PULLBOX IS NOT NAMED ● ► PILASTER EQUIPMENT CONTROL PANEL LINE TYPES EXISTING DEVICES AND INFRASTRUCTURE − − − NEW UNDERGROUND CONDUITS OR DUCT BANKS NEW FLEXIBLE ARMORED CABLE CONNECTIONS NEW CABLE/CONDUIT, TYPE NOT SPECIFIED

1 EXISTING SITE PLAN E100

0 🗆

_FA JB'S 🕄

F/A PS

GENERAL NOTES

-50C-(CAPSTAN #4S)

- EXISTING BUMPER LIGHT

SERVICE TO BE DEMOLISHED,

WITH PYROTENAX FEEDER

BACK TO SOUTH SIDE SUB

REMOVED FROM TUNNELS

1X50mm 4C#6 FROM 6PHS-SP-A

UPPER WALL SERVICE CONNECTION -

- 12SMM FIBER FROM VSL OFFICES TO DOCK END

-3c#1/0 & 10c#16 - CAPSTAN #1S 3c#1/0 & 10c#16 - CAPSTAN #2S

3c#1/0 & 10c#16 - CAPSTAN #3S

3c#1/0 & 10c#16 - CAPSTAN #4S

EXISTING BUMPER LIGHT SERVICE TO BE DEMOLISHED,

WITH PYROTENAX FEEDER

BACK TO SOUTH SIDE SUB

REMOVED FROM TUNNELS

1x50mm SPARE

- 1. ALL CABLES SHOWN AS EXISTING ON THIS SHEET ARE TO BE DISCONNECTED AND DEMOLISHED UNLESS OTHERWISE INDICATED. ALL COSTS FOR DISMOUNTING, REMOVAL AND DISPOSAL OF ALL CABLES IS TO BE INCLUDED IN BASE BID.
- 2. ALL REPLACEMENT CABLES ARE TO BE INSTALLED IN NEW END OF DOCK DUCT BANK AND PULLED AND INSTALLED TO THEIR APPROPRIATE LOCATIONS AFTER EAST END EXTENSION DUCT BANK IS INSTALLED THIS WILL PUT THE EXISTING SOUTH SIDE CAPSTANS OFFLINE UNTIL THE NEW DUCT BANK IS INSTALLED.
- 3. DUE TO THE LENGTH OF THIS OUTAGE, ALL EXISTING COMMUNICATIONS CABLES ARE TO BE RE-ROUTED AROUND THE SOUTH JETTY SERVICE DUCT BANK WHICH IS LOCATED OUTSIDE THE PROJECT FOOTPRINT, WILL NOT BE DISTURBED BY THE DEMOLITION UNDER THIS PROJECT, AND IS ALREADY IN PLACE.
- 4. ALL SERVICE INTERRUPTIONS ARE TO BE COORDINATED THROUGH EGD FOR ALL EGD FACILITY AND TENANT INTERRUPTIONS. CONTRACTOR WILL BE RESPONSIBLE FOR COORDINATING ALL THESE SHUT DOWNS, AND SHALL ASSUME ALL FINAL SWITCHOVER SHALL BE PERFORMED DURING OFF HOURS.
- 5. EXISTING TUNNEL LIGHTING CIRCUITS AND LIGHTS INSIDE EAST END TUNNEL PORTION AFFECTED BY THE EXTENSION ARE TO BE DEMOLISHED. ALL EQUIPMENT IN TUNNEL PORTIONS NOT AFFECT BY THE WORK
- 6. EXISTING CIRCUIT FROM PH TO UPPER WALL PANEL TO BE DEMOLISHED, NEW SERVICE TO BE INSTALLED TO THIS PANEL FROM SES, COORDINATE WITH EGD FOR OUTAGE AND SERVICE SWITCHOVER.
- 7. REFER TO SHEET E300 FOR RECONNECTION DETAILS.

0 10 20 30 40 50 60 70 80 90 100mm

PWGSC - A1 - 841X594

REFER TO E200 FOR SITE

REFER TO E200 FOR SITE

CONNECTION DETAILS

CONNECTION DETAILS

REFER TO E200 FOR SITE

CONNECTION DETAILS

EXISTING DUCT BANK CABLES

EXISTING TUNNEL MOUNTED CABLES:

100 PAIR CAT#3 DEMARC TO OLD SSS

100 PAIR CAT#3 DEMARC TO SSSR

3c#12 TECK TUNNEL LIGHTING

2c#14 TECK F/A CONNECTIONS, EAST END CENTER 2c#14 TECK F/A CONNECTIONS, EAST END CENTER

2c#14 TECK F/A CONNECTIONS, EAST END CENTER

12SMM FIBER FROM VSL OFFICES TO DOCK END

2c#18 TECK GRAY - ABANDONED F/A CABLES

24SMM FIBER FROM DEMARC TO OLD SSS

3C#1/0 & 10c#16 - CAPSTAN-CENTRE TO BE DEMOLISHED DURING DOC EXTENSION AND NOT REBUILT. EXISTING FEEDERS ARE TO BE REMOVED

AND MCC CELL TO BE MARKED AS SPARE

EXISTING TUNNEL LIGHT FIXTURES, TECK

CABLES, SPLICE BOXES AND PYROTENEX

IN DOCK END TUNNEL ONLY-

50C-(CAPSTAN #3S)

50C-(CAPSTAN #2S) 50C-(CAPSTAN #1S) -

FEEDERS TO BE REMOVED AND DEMOLISHED

EXISTING TUNNEL MOUNTED BASKET CABLE TRAY TO BE DISMOUNTED AND TURNED OVER

24SMM BETWEEN SSS WATER METERS AND PH PATCH PANELS

2c#12 TECK FROM PH TO EAST END EMERGENCY STROBE

12SMM FIBER PUMPHOUSE TO OLD SSS P/A & CAMERAS-

2c#18 TECK GRAY - TECK GRAY SCADA RS485 WATER METER LOOP

EXISTING TUNNEL MOUNTED SERVICES TO BE DEMOLISHED. EXISTING COMMUNICATION JB AND RECS ARE DISCONNECTED, AND NEED TO BE

SYSTEMS UPDATED TO ENSURE SYSTEM OPERATES CORRECTLY.

EXISITNG E/A AND F/A PULL STATION AND JUNCTION BOXES ARE INS SERVICE AND WILL NEED TO BE REMOVED FROM SERVICE AND THE F/A AND E/A

50C-(CAPSTAN-CENTRE) 50C-(CAPSTAN #1S) 50C-(CAPSTAN #2S)

50C-(CAPSTAN #3S)

50C-(CAPSTAN #4S)

50C-F/A PULL STATION

CONNECTION DETAILS