GENERAL

1. THIS IS A METRIC PROJECT. UNLESS OTHERWISE NOTED, ALL DIMENSIONS ARE IN MILLIMETERS AND ALL FORCES ARE IN METRIC UNITS.
2. PRIOR TO CONSTRUCTION, REVIEW STRUCTURAL DRAWINGS IN CONJUNCTION WITH DRAWINGS PROVIDED BY ALL OTHER DISCIPLINES. CONFIRM ALL DIMENSIONS, ELEVATIONS AND HEADROOM CLEARANCES, AND COORDINATE ALL OPENINGS, SLEEVES AND EMBEDDED ITEMS.
3. REPORT ANY DISCREPANCIES OR CONFLICTS BEFORE PROCEEDING WITH THE WORK.
4. DO NOT CUT OR DRILL ANY OPENINGS IN STRUCTURAL MEMBERS WITHOUT WRITTEN PERMISSION FROM DEPARTMENTAL REPRESENTATIVE.
5. EXISTING STRUCTURAL INFORMATION IS BASED UPON DRAWINGS PREPARED BY ABUGOV AND ASSOCIATED DATED DECEMBER 1987.
6. VERIFY EXISTING DIMENSIONS AND CONDITIONS ON SITE PRIOR TO CONSTRUCTION.
7. USE THESE DRAWINGS ONLY FOR THE PURPOSE IDENTIFIED IN THE REVISIONS COLUMN. DO NOT CONSTRUCT FROM THESE DRAWINGS UNLESS MARKED "ISSUED FOR CONSTRUCTION".
8. DO NOT USE INFORMATION ON THESE DRAWINGS FOR ANY OTHER PROJECT OR WORKS.
9. DO NOT SCALE THESE DRAWINGS.
10. UNLESS OTHERWISE NOTED ON DRAWINGS, FOLLOW TYPICAL DETAILS SHOWN ON S000 DRAWING SERIES. TYPICAL DETAILS SHOW STRUCTURAL INTENT RATHER THAN ACTUAL CONDITIONS FOR THIS PROJECT. IF A TYPICAL DETAIL INCLUDES A CROSS REFERENCE TO ANOTHER TYPICAL DETAIL WHICH IS NOT INCLUDED IN THE DRAWING SET, THE CROSS-REFERENCED DETAIL IS NOT APPLICABLE ON THIS PROJECT.
11. ALL SECTIONS, DETAILS AND STATEMENTS NOTED AS "TYPICAL" APPLY TO LIKE / SIMILAR CONDITIONS IN THE STRUCTURE.
12. REFER TO ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR REQUIRED FIRE RATING, SPRAYED FIREPROOFING, INTUMESCENT PAINTING AND ALL OTHER MEASURES REQUIRED TO ACHIEVE IT.
13. REFER TO ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR WATERPROOFING, SEALERS, ETC.
14. STRUCTURAL DESIGN ASSUMES NON-LOAD RESTRICTED ULC FIRE RATED ASSEMBLIES, AND APPROPRIATE MATERIALS MUST BE USED.
15. DRAWINGS SHOW COMPLETED STRUCTURE ONLY. THEY DO NOT SHOW TEMPORARY WORKS FOR WHICH THE CONTRACTOR IS RESPONSIBLE AND WHICH MAY BE REQUIRED FOR EXECUTION OF THE PROJECT, INCLUDING TEMPORARY SHORING, BRACING, GUYS AND TIE DOWNS. CONTRACTOR TO ESTABLISH CONSTRUCTION PROCEDURE AND SEQUENCE TO ENSURE SAFETY OF THE WHOLE STRUCTURE AND ALL ITS COMPONENTS DURING ERECTION.
16. DESIGN AND CONSTRUCTION REVIEW OF ALL TEMPORARY WORKS TO BE CARRIED OUT BY A PROFESSIONAL ENGINEER RETAINED BY THE CONTRACTOR, LICENSED IN THE PLACE WHERE THE PROJECT IS LOCATED.
17. ANCHOR RODS AND OTHER EMBEDDED ITEMS ARE DESIGNED FOR LOADS ACTING ON THE COMPLETED STRUCTURE ONLY AND ARE NOT TO BE USED OR RELIED UPON FOR TEMPORARY SUPPORT OR BRACING DURING ERECTION UNLESS REVIEWED AND APPROVED BY THE CONTRACTOR'S ENGINEER RESPONSIBLE FOR THE ERECTION PROCEDURES.
18. CONSTRUCTION LOADS ON COMPLETED STRUCTURE NOT TO EXCEED DESIGN LOADS INDICATED ON DRAWINGS. FULL DESIGN LOADS MAY ONLY BE APPLIED TO CONCRETE ELEMENTS AFTER THE CONCRETE HAS REACHED ITS DESIGN STRENGTH.
19. UNLESS SHOWN ON STRUCTURAL DRAWINGS, DESIGN OF NON-STRUCTURAL AND SECONDARY STRUCTURAL ELEMENTS AND THEIR CONNECTIONS TO THE PRIMARY BUILDING STRUCTURE ARE NOT WITHIN THE SCOPE OF SERVICES PROVIDED HEREIN. SUCH ELEMENTS INCLUDE (BUT ARE NOT LIMITED TO) THE FOLLOWING:
    1. MISCELLANEOUS STEEL ELEMENTS: STAIRS, RAILINGS, GUARDRAILS.
    2. PARTITIONS: MASONRY, GLASS, WOOD AND STEEL STUDS, PREFABRICATED PANELS
    3. BULKHEADS, SUSPENDED CEILINGS, INTERIOR AND EXTERIOR SIGNAGE.
    4. EXTERIOR CLADDING: PRECAST PANELS, METAL WALL SYSTEMS, CURTAIN WALLS AND WINDOWS.
    5. MASONRY, STONE OR PRECAST VENEER CONNECTIONS TO BACKUP STRUCTURE.
    6. SKYLIGHTS, SNOW FENCES, GUTTERS, ROOF ANCHORS, WINDOW WASHING SYSTEMS, CHIMNEYS AND STACKS.
    7. SUPPORTS FOR MECHANICAL AND ELECTRICAL EQUIPMENT: HANGERS, BRACES, POSTS, RACKS, SLEEPERS, SEISMIC RESTRAINTS, SUPPORT PLATFORMS AND PADS, SERVICE PLATFORMS.

    DEPARTMENTAL REPRESENTATIVE WILL NOT REVIEW DESIGN, DETAILING AND INSTALLATION OF THESE ELEMENTS, FOR WHICH SUPPLIERS AND / OR SPECIALTY PROFESSIONAL ENGINEERS ARE RESPONSIBLE; THE ONLY REVIEW PROVIDED (WHERE APPLICABLE) WILL BE FOR IMPACT ON THE BASE BUILDING STRUCTURE.
20. MAINTAIN A QUALITY CONTROL PLAN AS PER PROJECT SPECIFICATIONS.
21. FOR INSPECTION AND TESTING REQUIREMENTS, REFER TO SPECIFICATIONS.
22. IN CASE OF DISCREPANCY BETWEEN GENERAL NOTES, DRAWINGS AND SPECIFICATIONS, COMPLY WITH THE MOST STRINGENT REQUIREMENTS.

DESIGN DATA

1. PARTIAL STRUCTURAL UPGRADING OF THE EXISTING BUILDING IS TO THE GENERAL INTENT OF THE NATIONAL BUILDING CODE (NBC) 2015, SUPPLEMENTED BY THE USER'S GUIDE - NBC 2015 STRUCTURAL COMMENTARIES. THE UPGRADING IS LIMITED TO THE AREA(S) SPECIFICALLY SHOWN ON THESE DRAWINGS. FOR THE REMAINDER OF THE EXISTING BUILDING, THE CURRENT PERFORMANCE LEVEL IS MAINTAINED AND SEISMIC OR OTHER STRUCTURAL EVALUATION AND UPGRADING (INCLUDING UPGRADING TO CARRY GRAVITY LOADS) IS NOT INCLUDED IN THE SCOPE OF THE PROJECT. DEPARTMENTAL REPRESENTATIVE IS NOT RESPONSIBLE FOR THE STRUCTURAL ADEQUACY OF THE REMAINDER OF THE EXISTING BUILDING (WHICH REMAINS THE RESPONSIBILITY OF THE ORIGINAL STRUCTURAL ENGINEER), NOR FOR POSSIBLE DETRIMENTAL SEISMIC OR OTHER EFFECTS THE REMAINDER OF THE BUILDING MAY HAVE ON THE RENOVATED AREA(S)
2. CONCRETE ELEMENTS ARE DESIGNED PER CSA A23.3-14 - DESIGN OF CONCRETE STRUCTURES.
3. STEEL ELEMENTS ARE DESIGNED PER CSA S16-14-DESIGN OF STEEL STRUCTURES.
4. THE VALUES FOR CLIMATIC DATA USED IN THE DETERMINATION OF DESIGN LOADS HAVE BEEN OBTAINED FROM THE 2015 NBC FOR THE SPECIFIC LOCATION OF REGINA, SK.
5. BASED ON THE USE AND OCCUPANCY, THE BUILDING IS DESIGNED TO THE REQUIREMENTS OF A NORMAL IMPORTANCE CATEGORY.
6. SELF WEIGHT (SWT) IS DUE TO THE WEIGHT OF THE STRUCTURE ITSELF. IT VARIES WITH THE STRUCTURAL SYSTEM, AND INCLUDES CONCRETE TOPPINGS ON STEEL DECK.
7. SUPERIMPOSED DEAD LOADS (SDL) ARE NON-STRUCTURAL DEAD LOADS DUE TO NON-STRUCTURAL TOPPINGS, FINISHES, PARTITIONS, ROOFING MATERIALS, SUSPENDED EQUIPMENT, PAVERS, SOIL, ETC.
8. DEAD LOAD (DL) IS THE SELF WEIGHT OF THE STRUCTURE PLUS THE SUPERIMPOSED DEAD LOAD.
9. UNLESS OTHERWISE NOTED, DESIGN LOADS SHOWN ON DRAWINGS ARE SPECIFIED (UNFACTORED) LOADS, TO BE USED FOR ULS DESIGN. FOR SLS DESIGN, THESE LOADS CAN BE REDUCED BY MULTIPLYING WITH THE RATIO OF APPROPRIATE IMPORTANCE FACTORS Ix(SLS) / Ix(ULS) GIVEN BELOW.
10. IF ONLY ONE VALUE IS GIVEN FOR A LOAD, CONSIDER IT LIVE LOAD.
11. FOR CONNECTION LOADS, "+" SIGN INDICATES TENSION AND "-" SIGN INDICATES COMPRESSION, EXCEPT FOR COLUMN LOADS WHERE "+" SIGN INDICATES COMPRESSION AND "-" SIGN INDICATES TENSION.
12. SNOW:

    Ss = 1.7 kPa Sr = 0.1 kPa Is (ULS) = 1.0 Is (SLS) = 0.9

    MINIMUM UNFACTORED SNOW LOAD = 1.48 kPa x Is
13. RAIN: 24 HOUR RAINFALL = 103 mm
14. LATERAL LOADS IN THIS STRUCTURE ARE ASSUMED TO BE RESISTED BY SHEAR WALLSAND ARE DETERMINED BASED ON THE WIND AND SEISMIC DATA BELOW.
15. WIND:

    q50 = 0.49 kPa Iw (ULS) = 1.0 Iw (SLS) = 0.75

    TERRAIN TYPE: OPEN
    INTERNAL PRESSURE CATEGORY: 2

    Ce = 0.90

    WIND LOAD AT GRADE LEVEL FOR DESIGN OF OVERALL BUILDING LATERAL LOAD RESISTING SYSTEM: 0.91 kPa.

    WIND LOAD AT GRADE LEVEL OUTSIDE THE END ZONES FOR DESIGN OF SECONDARY STRUCTURAL ELEMENTS (GIRTS, WIND COLUMNS, ETC. BUT NOT INCLUDING CLADDING): 1.13 kPa.
16. SEISMIC:

    Sa(0.2) = 0.101 PGA = 0.061 IeFaSa(0.2) =0.166
    Sa(0.5) = 0.060 Rd = 1.5
    Sa(1.0) = 0.030 Ro = 1.3 SITE CLASSIFICATION = E (ASSUMED)
    Sa(2.0) = 0.013 Ie = 1.0

    SEISMIC FORCE RESISTING SYSTEM (SFRS): CONCRETE SHEAR WALLS (ASSUMED)
17. STRUCTURAL MOVEMENTS

    UNLESS NOTED OTHERWISE, MAXIMUM EXPECTED MOVEMENT OF THE BUILDING STRUCTURE (AFTER INSTALLATION OF FINISHES) WILL BE AS FOLLOWS ("L" IS THE CLEAR SPAN OF THE SUPPORTING STRUCTURAL ELEMENT, "H" IS THE STORY HEIGHT):

    - VERTICAL DEFLECTION OF STEEL FRAMED FLOORS AND ROOFS: L/360 u/n

    DESIGN AND DETAIL NON-STRUCTURAL ELEMENTS AND THEIR CONNECTIONS TO BE ABLE TO ACCOMMODATE THE ABOVE MOVEMENTS.

SHOP DRAWINGS

1. REFER TO SPECIFICATIONS FOR SHOP DRAWINGS WHICH NEED TO BE SUBMITTED FOR REVIEW.
2. REVIEW OF SHOP DRAWINGS BY DEPARTMENTAL REPRESENTATIVE IS ON A SAMPLING BASIS, FOR GENERAL CONFORMITY WITH STRUCTURAL CONTRACT DOCUMENTS. IT IS NOT A DETAILED CHECK AND MUST NOT BE CONSTRUED AS RELIEVING THE CONTRACTOR OF THE CONTRACTOR'S RESPONSIBILITY TO MAKE THE WORK ACCURATE AND IN CONFORMITY WITH ALL CONTRACT DOCUMENTS, TO REVIEW SHOP DRAWINGS AND TO COORDINATE WORK OF INTERFACING TRADES AND MANUFACTURE OF INTERFACING PRODUCTS.
3. REVIEW OF SHOP DRAWINGS DOES NOT IMPLY ANY CHANGE IN ANY OTHER DISCIPLINE DEPARTMENTAL REPRESENTATIVES' OR PROFESSIONALS' RESPONSIBILITIES RELATED TO DESIGN OF SPECIFIC ITEMS AS OUTLINED BY THE SPECIFICATIONS.
4. AFTER REVIEW, THE DRAWINGS WILL BE STAMPED AND RETURNED. DO NOT COMMENCE FABRICATION UNTIL RETURNED SHOP DRAWINGS HAVE BEEN EXAMINED. IF FABRICATION BEGINS PRIOR TO EXAMINATION OF RETURNED SHOP DRAWINGS, THE COST ASSOCIATED WITH ANY REQUIRED REPLACEMENT OR REWORK OF FABRICATED ELEMENTS IS THE RESPONSIBILITY OF THE CONTRACTOR.
5. SHOP DRAWINGS MARKED " **REVIEWED**" CAN BE USED FOR FABRICATION. DO NOT MAKE ANY CHANGES OR ADDITIONS TO THESE DRAWINGS WITHOUT NOTIFYING THE DEPARTMENTAL REPRESENTATIVE.
6. SHOP DRAWINGS MARKED " **REVIEWED AS NOTED** " CAN BE USED FOR FABRICATION AFTER THE REVISIONS NOTED ARE IMPLEMENTED. DO NOT MAKE ANY FURTHER CHANGES OR ADDITIONS TO THESE DRAWINGS WITHOUT NOTIFYING THE DEPARTMENTAL REPRESENTATIVE.
7. SHOP DRAWINGS MARKED " **REVISE AND RESUBMIT** " REQUIRE SUBSTANTIAL REVISIONS AND MUST BE RESUBMITTED FOR ADDITIONAL REVIEW PRIOR TO FABRICATION. ALL CHANGES AND ADDITIONS TO THE PREVIOUS SUBMISSION TO BE CLEARLY IDENTIFIED ON THE RESUBMITTED DRAWINGS. ONLY THE IDENTIFIED CHANGES WILL BE REVIEWED ON RE-SUBMISSION.
8. SHOP DRAWINGS MARKED " **REVIEWED FOR IMPACT ON BASE STRUCTURE ONLY** " SHOW WORKS WHICH ARE NOT WITHIN THE SCOPE OF STRUCTURAL CONSULTING SERVICES BUT AFFECT BEHAVIOUR OF THE BASE STRUCTURE. DEPARTMENTAL REPRESENTATIVE WILL NOT REVIEW DESIGN OF THESE WORKS AND ASSUMES THAT THE INDICATED WEIGHTS AND ALL OTHER LOADS IMPOSED ON THE BASE STRUCTURE ARE CORRECTLY IDENTIFIED BY THE DESIGNER / SUPPLIER OF THESE ELEMENTS.
9. DRAWINGS MARKED " **NOT REVIEWED** " SHOW WORKS WHICH ARE NOT WITHIN THE SCOPE OF STRUCTURAL CONSULTING SERVICES AND DO NOT IMPACT THE BASE BUILDING STRUCTURE.
10. DEPARTMENTAL REPRESENTATIVE WILL NOT REVIEW DESIGN AND IMPLEMENTATION OF ANY TEMPORARY WORKS, NOR ASSESS IMPACT OF THESE WORKS ON THE BASE STRUCTURE. THE CONTRACTOR AND / OR THE PROFESSIONAL ENGINEER ENGAGED BY THE CONTRACTOR MUST ENSURE THAT THE BASE STRUCTURE IS NOT ADVERSELY AFFECTED BY THE TEMPORARY WORKS AND CONSTRUCTION PROCESS AND THAT TEMPORARY LOADS DO NOT EXCEED THE DESIGN LOADS INDICATED ON STRUCTURAL DRAWINGS.
11. DO NOT USE SHOP DRAWINGS AS A MEANS TO PROPOSE SUBSTITUTIONS OR ALTERNATIVES TO THE MATERIALS, PRODUCTS OR DETAILS INDICATED IN CONTRACT DOCUMENTS. SUCH SHOP DRAWINGS WILL BE MARKED "REVISE AND RESUBMIT".
12. PROVIDE FINAL RECORD DRAWINGS AFTER ALL CORRECTIONS ARE MADE.

FIELD REVIEW

1. DEPARTMENTAL REPRESENTATIVE WILL PROVIDE PERIODIC FIELD REVIEW OF A REPRESENTATIVE SAMPLE OF THE STRUCTURAL WORKS DETAILED ON THESE DRAWINGS FOR GENERAL CONFORMANCE WITH CONTRACT DOCUMENTS. THESE REVIEWS DO NOT REPLACE THE CONTRACTOR'S RESPONSIBILITY TO IMPLEMENT AND MAINTAIN A QUALITY CONTROL PROGRAM, AND DO NOT MAKE DEPARTMENTAL REPRESENTATIVE A GUARANTOR OF THE CONTRACTOR'S WORK.
2. CONSTRUCTION REVIEW REPORTS WILL OUTLINE ANY DEFICIENCIES FOUND.
3. ASSIST DEPARTMENTAL REPRESENTATIVE DURING FIELD REVIEW AND PROVIDE SAFE ACCESS TO WORK AREAS AS REQUIRED.
4. CHECK THE WORK PRIOR TO FIELD REVIEW TO CONFIRM IT IS COMPLETED AND IN ACCORDANCE WITH CONTRACT DOCUMENTS.
5. BRING TO THE ATTENTION OF DEPARTMENTAL REPRESENTATIVE ANY DEFICIENCIES FOUND IN THE WORK TOGETHER WITH A PROPOSAL FOR REMEDY. DEPARTMENTAL REPRESENTATIVE WILL DECIDE WHAT CORRECTIVE ACTION MAY BE TAKEN AND ISSUE THE NECESSARY INSTRUCTIONS.
6. PROVIDE REASONABLE NOTICE (NOT LESS THAN 48 HOURS) TO ALLOW FOR THE FIELD REVIEW OF THE FOLLOWING:
    a. CONCRETE — BEFORE EACH CONCRETE POUR
    b. STRUCTURAL STEEL — BEFORE COVERING OVER
    c. METAL FLOOR DECK — BEFORE PLACING REINFORCING
7. SCHEDULE REVIEW WORK TO OCCUR DURING NORMAL BUSINESS HOURS.
8. ORGANIZE FOR FIELD REVIEW OF ALL PROPRIETARY PRODUCTS AND OTHER STRUCTURAL WORKS DESIGNED BY SPECIALTY ENGINEERS. THE REVIEW TO BE BY THE ENGINEERS RESPONSIBLE FOR THE DESIGN OR BY OTHER ENGINEERS DESIGNATED BY THE ENGINEERS RESPONSIBLE FOR THE DESIGN AND LICENSED IN THE PLACE WHERE THE PROJECT IS LOCATED. SUBMIT CONSTRUCTION REVIEW REPORTS FOR DEPARTMENTAL REPRESENTATIVE'S RECORD.

EXISTING STRUCTURES

1. EXISTING CONDITIONS ARE ASSUMED. SURVEY THE EXISTING STRUCTURE AFTER REMOVING FINISHES AND REPORT ANY VARIATIONS TO DEPARTMENTAL REPRESENTATIVE BEFORE PROCEEDING WITH THE WORK.
2. DESIGN OF STRUCTURAL WORKS RELATED TO THE EXISTING BUILDING HAS BEEN CARRIED OUT AS FAR AS PRACTICAL, GIVEN LIMITED AVAILABILITY OF THE EXISTING DRAWINGS AND LIMITED RECORDS OF THE STRUCTURAL MODIFICATIONS LIKELY TO HAVE BEEN MADE THROUGH THE LIFE OF THE BUILDING. MODIFICATIONS TO THE PROPOSED STRUCTURAL FRAMING AND / OR DETAILS MAY BE REQUIRED IF EXISTING CONDITIONS ARE FOUND TO BE DIFFERENT FROM THOSE ASSUMED AND SHOWN ON DRAWINGS.
3. TAKE ALL PRECAUTIONS NECESSARY TO PROTECT EXISTING STRUCTURES DURING DEMOLITION AND NEW CONSTRUCTION.
4. DISCONNECT ALL SERVICES IN THE AREAS AFFECTED BY DEMOLITION AND NEW CONSTRUCTION. REROUTE SERVICES AS REQUIRED TO KEEP THE REMAINDER OF THE BUILDING OPERATIONAL.
5. SAFELY STORE ALL STRUCTURAL ELEMENTS AND OTHER PRODUCTS WHICH ARE TO BE RE-USED.
6. REMOVE FROM SITE ALL OTHER STRUCTURAL ELEMENTS AND PRODUCTS WHICH ARE NOT INDICATED TO BE HANDED OVER TO THE OWNER.
7. SCHEDULE WORK TO MINIMIZE EFFECT ON THE EXISTING BUILDING OPERATION. USE EQUIPMENT AND PROCEDURES TO MINIMIZE NOISE, DUST AND VIBRATIONS. SUBMIT PROPOSED SCHEDULE FOR REVIEW BY THE DEPARTMENTAL REPRESENTATIVE AND THE OWNER.
8. ALL DEMOLITION, SHORING AND OTHER TEMPORARY WORKS TO BE DESIGNED BY A PROFESSIONAL ENGINEER RETAINED BY THE CONTRACTOR, LICENSED IN THE PLACE WHERE THE PROJECT IS LOCATED. PREPARE DRAWINGS SIGNED AND SEALED BY THAT ENGINEER SHOWING DEMOLITION PROCEDURE AND SEQUENCE AND ALL THE NECESSARY SHORING.
9. INSTALL AND AFTERWARDS REMOVE ALL TEMPORARY SHORING AND BRACING REQUIRED TO ENSURE THE INTEGRITY OF THE EXISTING STRUCTURE DURING CONSTRUCTION.
10. DO NOT ALTER MATERIAL PROPERTIES OF THE STRUCTURAL STEEL WHICH IS TO REMAIN BY CUTTING AND DEMOLITION PROCEDURE.
11. ASSESS CAPACITY OF THE EXISTING STRUCTURE AND CONSTRUCTION LOADS APPLIED TO IT. PROVIDE ADEQUATE SHORING IF THE LOADS EXCEED THE EXISTING STRUCTURAL CAPACITY.
12. MAKE GOOD ALL EXISTING WORK DISTURBED BY THE SHORING OPERATIONS, DEMOLITION, EXCAVATION AND OTHER CONSTRUCTION PROCEDURES.

CAST-IN-PLACE CONCRETE

1. CONCRETE IS SPECIFIED PER ALTERNATIVE 1 - PERFORMANCE SPECIFICATION, AS OUTLINED IN CSA A23.1. THE CONTRACTOR AND THE CONCRETE SUPPLIER TO MEET ALL CERTIFICATION, DOCUMENTATION, AND QUALITY CONTROL REQUIREMENTS.
2. CONTRACTOR AND CONCRETE SUPPLIER TO ENSURE THAT PLASTIC AND HARDENED MIX PROPERTIES MEET SITE REQUIREMENTS FOR PLACING, FINISHING AND THE SPECIFIED PERFORMANCE REQUIREMENTS.
3. CEMENT TO BE PORTLAND CEMENT TYPE GU UNLESS NOTED OTHERWISE OR REQUIRED BY EXPOSURE CLASS.
4. CONCRETE TO BE NORMAL DENSITY (MIN. 2300 kg/m^3) UNLESS NOTED OTHERWISE.
5. UNLESS NOTED OTHERWISE, CONCRETE TO BE IN ACCORDANCE WITH THE FOLLOWING SCHEDULE:

ELEMENT	COMPRESSIVE STRENGTH (MPa) AT 28 DAYS (SEE NOTE #3 BELOW)	EXPOSURE CLASS
SLABS ON STEEL DECK	25 MPa	N-CF

6. REFER TO CSA A23.1 FOR THE MAXIMUM WATER/CEMENT RATIO, MINIMUM COMPRESSIVE STRENGTH, AIR CONTENT, CURING REQUIREMENTS, CHLORIDE ION PENETRABILITY AND ALTERNATE CEMENT TYPES TO MEET THE REQUIREMENTS FOR THE NOTED EXPOSURE CLASS.
7. DO NOT ADD WATER TO CONCRETE ON SITE.
8. CONVEY CONCRETE FROM TRUCK TO FINAL LOCATION BY METHODS WHICH WILL PREVENT SEPARATION OR LOSS OF MATERIAL. MAXIMUM FREE FALL NOT TO EXCEED 1.5m (5'-0"). CONSOLIDATE CONCRETE USING MECHANICAL VIBRATORS.
9. PLACE CONCRETE AS CLOSE AS POSSIBLE TO FINAL LOCATION TO AVOID SEGREGATION. VIBRATE ALL CONCRETE.
10. SLABS
    1. SUBMIT COMPOSITE LAYOUT DRAWINGS SHOWING CONDUITS, SLEEVES AND OPENINGS REQUIRED BY ALL TRADES FOR DEPARTMENTAL REPRESENTATIVE REVIEW. DO NOT INSTALL ANY SLEEVES OR OPENINGS WHICH ARE NOT SHOWN ON STRUCTURAL DRAWINGS WITHOUT DEPARTMENTAL REPRESENTATIVE REVIEW AND ACCEPTANCE. MAXIMUM OUTSIDE DIAMETER OF ANY CONDUIT OR PIPE EMBEDDED IN SLAB NOT TO EXCEED ONE THIRD OF THE SLAB THICKNESS.

CONCRETE REINFORCEMENT

1. REINFORCEMENT TO CONFORM TO THE FOLLOWING STANDARDS:

    DEFORMED BARS - CSA G30.18, GRADE400R OR 400W. WHERE REBAR ARE SHOWN TO BE WELDED AND WHERE INDICATED ON PLANS / NOTES USE ONLY GRADE 400W.

    WELDED WIRE FABRIC - ASTM A1064/A1064M, YIELD STRENGTH 450 MPA, SUPPLIED IN FLAT SHEETS ONLY.
2. ALL REINFORCING BAR SIZES ARE METRIC; "M" IS NOT NECESSARILY MARKED AFTER A BAR SIZE. FOR EXAMPLE, 10-15B NOTED ON PLAN INDICATES 10 BARS OF 15M DIAMETER, PLACED AT BOTTOM.
3. ALL REBAR HOOKS TO BE STANDARD LENGTH 90° OR 180° HOOKS. REBAR LENGTHS LISTED ON DRAWINGS DO NOT INCLUDE THE HOOK LENGTH.
4. LAP WELDED WIRE FABRIC SHEETS BY ONE SPACING OF CROSS WIRES + 50 (2"), MEASURED BETWEEN THE OUTERMOST CROSS WIRES IN EACH SHEET.
5. PROVIDE ADDITIONAL SUPPORT BARS AS REQUIRED TO ADEQUATELY SUPPORT AND SECURE ALL REINFORCEMENT AND PREVENT MOVEMENT WHEN PLACING CONCRETE.
6. PROVIDE SUFFICIENT CHAIRS TO REINFORCING TO MAINTAIN SPECIFIED CONCRETE COVER.
7. ALL REINFORCING TO BE CLEAN, FREE OF LOOSE SCALE, OIL, DIRT, RUST, AND ANY OTHER FOREIGN COATING THAT AFFECT BONDING CAPACITY.
8. MINIMUM CLEAR SPACING BETWEEN ADJACENT BARS TO BE AT LEAST 1.4 TIMES THE BAR DIAMETER OR 1.4 TIMES THE NOMINAL MAXIMUM SIZE OF THE COARSE AGGREGATE, WHICHEVER IS MORE.
9. MINIMUM CONCRETE COVER TO PRINCIPAL REINFORCEMENT TO BE 25 mm FOR REINFORCEMENT IN SLAB ON STEEL DECK.

POST-INSTALLED ANCHORS AND DOWELS

1. UNLESS OTHERWISE NOTED, ANCHORAGE TO CONCRETE TO BE:

    WHERE ADHESIVE CONCRETE ANCHORS (ACA) ARE NOTED ON DRAWINGS, PROVIDE HILTI HIT-HY 200 ADHESIVE ANCHORING SYSTEM WITH HILTI HIT-Z ANCHOR RODS OR APPROVED EQUIVALENT.

    WHERE REBAR DOWEL ANCHORS (RDA) ARE NOTED ON DRAWINGS, PROVIDE HILTI HIT-RE 500 V3 ADHESIVE ANCHORING SYSTEM INSTALLED USING HILTI SAFESET HOLLOW DRILL BIT TECHNOLOGY OR APPROVED EQUIVALENT .
2. UNLESS OTHERWISE NOTED ON DRAWINGS, ANCHORAGE TO SOLID OR GROUTED MASONRY TO BE:
    1. WHERE DRILLED MASONRY ANCHORS (DMA) ARE NOTED ON DRAWINGS, PROVIDE HILTI KB-3 EXPANSION ANCHORS OR APPROVED EQUIVALENT. LOCATE MIN. 35 (1-3/8") FROM ANY VERTICAL MORTAR JOINT.
    2. WHERE ADHESIVE MASONRY (AMA) ANCHORS ARE NOTED ON DRAWINGS, PROVIDE HILTI HIT-HY 270 ADHESIVE ANCHORING SYSTEM WITH HAS-E THREADED RODS OR APPROVED EQUIVALENT.
3. UNLESS OTHERWISE NOTED ON DRAWINGS, ANCHORAGE TO HOLLOW MASONRY TO BE HILTI HIT-HY 270 ADHESIVE ANCHORING SYSTEM WITH HIT-SC MESH SLEEVE (WHEN INSTALLING IN UNGROUTED CELLS) AND HAS-E THREADED RODS OR APPROVED EQUIVALENT.
4. IN ORDER TO BE ACCEPTED, ANY ALTERNATIVES TO THE HILTI PRODUCTS SPECIFIED IN NOTES 1, 2 AND 3 MUST BE ACCOMPANIED BY TESTING DATA AND ICC-ES REPORTS DEMONSTRATING THAT THEIR PERFORMANCE (INCLUDING CAPACITY IN CRACKED CONCRETE AND CAPACITY REDUCTIONS DUE TO SPACING AND EDGE DISTANCE) IS EQUIVALENT TO THE PERFORMANCE OF HILTI PRODUCTS. IN ADDITION, THAT PERFORMANCE MUST BE ACHIEVED USING INSTALLATION TOOLS AND PROCEDURES WHICH DO NOT REQUIRE DRILLED HOLES TO BE CLEANED PRIOR TO ANCHOR INSTALLATION.
5. CONCRETE TO BE MINIMUM 28 DAYS OLD AT THE TIME OF ANCHOR INSTALLATION.
6. USE DRILLING AND INSTALLATION TOOLS AND PROCEDURES PER MANUFACTURER'S RECOMMENDATIONS. DO NOT CORE DRILL UNLESS SPECIFICALLY NOTED ON DRAWINGS. HOLE DIAMETERS NOT TO EXCEED THOSE REQUIRED BY MANUFACTURER.
7. WHERE CORE DRILLING IS SPECIFIED, CLEAN AND ROUGHEN HOLES PER MANUFACTURER'S RECOMMENDATION.
8. ARRANGE FOR THE ANCHOR MANUFACTURER TO CONDUCT ON SITE TRAINING FOR INSTALLATION OF ALL THE PRODUCTS SPECIFIED, AND FOR ALL CONDITIONS ENCOUNTERED (E.G. HORIZONTAL, INCLINED, OVERHEAD). ALL INSTALLERS MUST COMPLETE THE SUPPLIER CERTIFIED INSTALLER TRAINING PROGRAM. SUBMIT COPIES OF COMPLETION CERTIFICATES FOR DEPARTMENTAL REPRESENTATIVE RECORD.
9. ARRANGE FOR A MANUFACTURER'S TECHNICAL REPRESENTATIVE TO BE PRESENT DURING INSTALLATION OF FIRST FEW ANCHORS OF EACH SIZE AND TYPE. SUBMIT SITE REPORTS INDICATING ANCHOR TYPES AND SIZES INSTALLED, LOCATIONS AND INSTALLERS' NAMES.
10. ANCHOR AND DOWEL CAPACITY IS DEPENDENT UPON SPACING BETWEEN ADJACENT ANCHORS AND THEIR PROXIMITY TO CONCRETE AND MASONRY EDGES; THEREFORE, ALL ANCHORS MUST BE INSTALLED WITH CLEARANCES AND EDGE DISTANCES INDICATED ON DRAWINGS
11. UNLESS CORE DRILLING IS SPECIFIED ON DRAWINGS, DO NOT CUT REINFORCEMENT TO ACCOMMODATE DRILLED ANCHORS AND DOWELS. SCAN THE STRUCTURE TO LOCATE REINFORCEMENT PRIOR TO FABRICATING STRUCTURAL STEEL FASTENED BY DRILLED ANCHORS.
12. WHEN OBSTRUCTIONS PREVENT DRILLING HOLES IN SPECIFIED LOCATIONS TO THE REQUIRED DEPTH, RELOCATE AT NO EXTRA COST TO THE CONTRACT. OBTAIN DEPARTMENTAL REPRESENTATIVE APPROVAL OF NEW LOCATIONS BEFORE DRILLING; MODIFICATIONS TO CONNECTED MEMBERS AND ADDITIONAL ANCHORS / DOWELS MAY BE REQUIRED. FILL ABANDONED HOLES WHICH ARE CLOSER THAN 3 TIMES THE HOLE DIAMETER FROM THE RELOCATED ANCHORS WITH HILTI HY-200 ADHESIVE. DO NOT TIGHTEN ANCHORS UNTIL THE ADHESIVE HAS FULLY CURED.
13. UNLESS OTHERWISE NOTED ON DRAWINGS, EMBEDMENT LENGTHS FOR POST-INSTALLED HILTI ANCHORS TO BE:

ANCHOR SIZE	ADHESIVE ANCHORS INTO CONCRETE AND SOLID OR GROUTED CONCRETE MASONRY (HILTI HIT HY-200 AND HIT HY-270)	ADHESIVE ANCHORS INTO HOLLOW CONCRETE MASONRY (HILTI HIT HY-270)	ADHESIVE ANCHORS INTO HOLLOW BRICK MASONRY (HILTI HIT HY-270)
12 (1/2")	114 (4-1/2")	50 (2")	80 (3-1/8")
16 (5/8")	143 (5-5/8")	-	-
19 (3/4")	171 (6-3/4")	-	-

NOTES:

1. ALL EMBEDMENT LENGTHS SHOWN ARE EFFECTIVE EMBEDMENT LENGTHS; FOR REQUIRED HOLE DEPTHS FOLLOW HILTI RECOMMENDATIONS.
2. SEE DRAWINGS FOR EMBEDMENT LENGTHS OF REBAR DOWELS.

IF ANCHORS OTHER THAN THE HILTI PRODUCTS SPECIFIED IN NOTES 1, 2, AND 3 ARE APPROVED TO BE USED, ANCHOR SUPPLIER TO ESTABLISH EMBEDMENT LENGTHS REQURED TO ACHIEVE PERFORMANCE EQUIVALENT TO THE HILTI PRODUCTED EMBEDDED AS INDICATED IN THE TABLE ABOVE.

14. DO NOT BEND POST INSTALLED DOWELS AND RODS AFTER INSTALLATION.
15. DO NOT WELD TO PLATES FASTENED WITH ADHESIVE ANCHORS AFTER THE ADHESIVE IS PLACED.

STRUCTURAL STEEL

1. CONFORM TO CSA S16.
2. MATERIALS: TO CAS G40.21 UNLESS NOTED OTHERWISE, WITH THE FOLLOWING GRADES:
    W SECTIONS, CHANNELS, AND ANGLES: 350W
    PLATES, BARS: 300W
    HOLLOW STRUCTURAL SECTIONS: 350W CLASS C OR H
3. DETAILS ON STRUCTURAL DRAWINGS SHOW DESIGN INTENT. REFER TO SPECIFICATIONS FOR CONNECTION DESIGN, DETAILING, FABRICATION, AND ERECTION REQUIREMENTS.
4. CONNECT BEAMS FOR THE FORCES SHOWN; IF NO FORCE IS INDICATED, CONNECT NON COMPOSITE BEAMS FOR THE REACTION DUE TO MAXIMUM UNIFORMLY DISTRIBUTED LOAD CAPACITY OF THE BEAM IN BENDING, AND CONNECT COMPOSITE BEAMS FOR ONE AND A HALF TIMES THE REACTION DUE TO MAXIMUM UNIFORMLY DISTRIBUTED LOAD CAPACITY OF THE NON COMPOSITE SECTION IN BENDING.
5. DO NOT CUT HOLES OR OTHERWISE MODIFY STRUCTURAL MEMBERS ON SITE.
6. CLEAN SURFACES DOWN TO BARE METAL AND APPLY TWO COATS OF ZINC-RICH TOUCH-UP PAINT TO ANY GALVANIZED SURFACE THAT HAS BEEN DAMAGED OR FIELD WELDED.
7. PROVIDE ALL ERECTION BRACING REQUIRED TO KEEP THE STRUCTURE STABLE AND IN ALIGNMENT DURING CONSTRUCTION.
8. PROVIDE 40 MPa NON SHRINK GROUT UNDER BASE PLATES. DO NOT APPLY ANY LOADS TO THE STEELWORK BEFORE GROUT ACHIEVES SUFFICIENT STRENGTH.
9. DISTRIBUTE HANGER LOADS FROM MECHANICAL AND HEAVY ELECTRICAL SERVICES SUSPENDED FROM STEELWORK UNIFORMLY ALONG MEMBERS. ALTERNATE HANGER POSITION ON EITHER SIDE OF MEMBERS.
10. DO NOT APPLY LATERAL LOADS TO MEMBERS UNLESS APPROVED BY THE DEPARTMENTAL REPRESENTATIVE.

FLOOR METAL DECK

1. CONFORM TO CSA S136 FOR STEEL DECKING, AND TO CAST IN PLACE CONCRETE AND CONCRETE REINFORCEMENT NOTES.
2. STEEL DECK MATERIAL: TO ASTM A653/653M OR ASTM A792/792M, GRADE 230
3. REQUIRED DECK DEPTH AND CORE NOMINAL THICKNESS ARE SHOWN ON DRAWINGS; PROVIDE DECK PROFILE TO MEET THE LOADING AND PERFORMANCE REQUIREMENTS OUTLINED IN THE SPECIFICATIONS.
4. MINIMUM DECK FASTENING REQUIREMENTS ARE AS FOLLOWS:

    TRANSVERSE (FRAME) FASTENERS
    19 (3/4") DIAMETER ARC SPOT WELDS IN EVERY SECOND FLUTE (FOR DECKS WITH FLUTE SPACING OF 200 (8") OR LESS) OR IN EVERY FLUTE (FOR DECKS WITH FLUTE SPACING OF OVER 200 (8")). IN ADDITION, PLACE TWO FASTENERS IN FLUTES WHERE SIDE LAPS ARE MADE; AT INTERLOCKING JOINTS, LOCATE ONE FASTENER AT EACH SIDE OF LAP. FOR GALVANIZED AND PREFINISHED DECK, USE #10 STAINLESS STEEL SCREWS INSTEAD OF WELDING AND PAFs.

    LONGITUDINAL (PERIMETER) FASTENERS
    TYPE TO MATCH TRANSVERSE FASTENERS, SPACED AT 450 (18") ON CENTRE

    SIDELAPS
    MECHANICALLY CLINCHED (BUTTON PUNCHED), WELDED OR FASTENED WITH #10 SCREWS, SPACED AT 600 (24") ON CENTRE. FOR GALVANIZED AND PREFINISHED DECK, USE ONLY #10 STAINLESS STEEL SCREWS.
5. STEEL DECK IS DESIGNED TO SUPPORT UNIFORMLY DISTRIBUTED LOADS INDICATED ON DRAWINGS AND MAY NOT BE ABLE TO RESIST CONCENTRATED LOADS. IF CEILING IS PROPOSED TO BE HUNG DIRECTLY FROM STEEL ROOF DECK (OR FROM STEEL FLOOR DECK BEFORE CONCRETE IS POURED AND GAINED THE SPECIFIED STRENGTH), HANGER LOADS, LAYOUTS AND PROPOSED FASTENERS TO BE REVIEWED AND APPROVED BY A PROFESSIONAL ENGINEER RETAINED BY THE CONTRACTOR PRIOR TO INSTALLATION. DO NOT HANG ANY OTHER CONCENTRATED LOADS FROM STEEL ROOF DECK, ATTACH TO STRUCTURAL STEEL FRAMING INSTEAD.
6. DECK SUPPLIER TO DESIGN AND PROVIDE REINFORCING FOR ALL DECK OPENINGS BETWEEN 150 AND 450 WIDE. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATIONS. CLEAR SPACING BETWEEN ADJACENT OPENINGS TO BE MIN. 3 TIMES THE WIDTH OF THE LARGER OPENING.
7. UNLESS OTHERWISE NOTED, DECK WITH CONCRETE TOPPING TO BE A COMPOSITE PROFILE.
8. REFER TO PLANS FOR COVER SLAB THICKNESS.
9. DO NOT INCREASE OR REDUCE SPECIFIED SLAB THICKNESS WHILE PLACING CONCRETE; TOP OF FINISHED CONCRETE WILL NOT NECESSARILY BE LEVEL DUE TO BEAM DEFLECTION OR CAMBER.
10. PROVIDE DECK SHORING WHERE SHOWN ON DRAWINGS. DO NOT REMOVE UNTIL CONCRETE HAS REACHED 75% OF ITS SPECIFIED 28 DAY COMPRESSION STRENGTH.
11. SEE CONCRETE REINFORCEMENT NOTES FOR WELDED WIRE FABRIC LAP SPLICES.
12. HIGH CHAIRS TO BE CONTINUOUS, SEATED IN BOTTOM OF DECK FLUTES.
13. LOW CHAIRS TO BE CUT FROM REBAR AND PLACED ACROSS DECK FLUTES; LENGTH EQUAL TO FLUTE WIDTH.
14. PRIOR TO CONCRETE PLACEMENT, STEEL DECK TO BE FREE OF SOIL, DEBRIS, STANDING WATER, LOOSE MIL SCALE, AND OTHER FOREIGN MATTER.

DRAWING LIST	
DRAWING No	DRAWING NAME
S000	GENERAL NOTES
S001	TYPICAL DETAILS
S100	PARTIAL MAIN FLOOR PLAN, SECTION & DETAIL
S200	ELEVATION

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Permission to Consult held by:

Discipline	Sk. Reg. No.	Signature
Structure	23426	

DO NOT SCALE DRAWINGS

Revision / Revision	Description/Description	Date/Date
2	ISSUED FOR TENDER	2021-03-17
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Drawn by/Dessine par: **P.JUNG**

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GENERAL NOTES

Project no./No. du projet	Sheet/Feuille	Revision no./La Révision no.
201-08499-00	S000 (1 OF 4)	2

201-08499-00

DRAWING ABBREVIATIONS — TG-ABBR-01

ABUT	ABUTMENT	DP	DEEP	ℓd	TENSION DEVELOPMENT LENGTH OF REBAR	SPEC	SPECIFICATIONS
ACA	ADHESIVE CONCRETE ANCHORS	DWG	DRAWING	ℓdc	COMPRESSION DEVELOPMENT LENGTH OF REBAR	SPF	SPRUCE PINE FIR
ADD'L	ADDITIONAL	DWL	DOWEL	ℓdh	TENSION EMBEDMENT LENGTH WITH STANDARD HOOK	SR	STUD RAIL
AEC	ARCHITECTURALLY EXPOSED CONCRETE	EA	EACH	LE	LEFT END	SS	STAINLESS STEEL
AESS	ARCHITECTURALLY EXPOSED STRUCTURAL STEEL	ECR	EPOXY COATED REINFORCEMENT	LG	LONG	ST	STRAIGHT
AIFB	ASPHALT IMPREGNATED FIBERBOARD	EBF	ECCENTRICALLY BRACED FRAME	LL	LOWER LEVEL	STD	STANDARD
ALT	ALTERNATE	EE	EACH END	LLH	LONG LEG HORIZONTAL	STE	SHEAR TRANSFER ELEMENTS
AMA	ADHESIVE MASONRY ANCHORS	EF	EACH FACE	LLV	LONG LEG VERTICAL	STG	STAGGERED
ARCH	ARCHITECTURAL	EJ, EXP JT	EXPANSION JOINT	LONG	LONGITUDINAL	STIR	STIRRUP
A-ROD	ANCHOR ROD	ELECT	ELECTRICAL	LSH	LONG SIDE HORIZONTAL	STIFF	STIFFENER
ASPH	ASPHALT	EL	ELEVATION	LP	LOW POINT	STL	STEEL
AVG	AVERAGE	ELEV	ELEVATOR	LWT	LIGHT WEIGHT	STR	SEISMIC STRAP
B, BOT	BOTTOM	EMBED	EMBEDMENT	MAX	MAXIMUM	STRUCT	STRUCTURAL
BOF	BOTTOM OF FOOTING	ENG	ENGINEER	MC	MOMENT CONNECTION (─◀ ▶─)	SWT	SELF WEIGHT
BOP	BOTTOM OF PILE	EOD	EDGE OF DECK	MECH	MECHANICAL	SYMM	SYMMETRICAL
BCE	BOTTOM CHORD EXTENSION	EOS	EDGE OF SLAB	MEZZ	MEZZANINE	t	THICKNESS
BCP	BORED CONCRETE PILE	ES	EACH SIDE	MF	MOMENT FRAME	TB	TRANSFER BEAM
BEW	BOTTOM EACH WAY	EQ	EQUAL	MIN	MINIMUM	TBB	TOP BASIC BARS
BLL	BOTTOM LOWER LAYER	EW	EACH WAY	MISC	MISCELLANEOUS	T	TOP
BP	BASE PLATE	EX, EXIST, (E)	EXISTING	MJ	MOVEMENT JOINT	TDL	TENSION DEVELOPMENT LENGTH
BRG	BEARING	EXT	EXTERIOR	ML	MIDDLE LAYER	TEW	TOP EACH WAY
BRP	BEARING PLATE	FC	FUTURE COLUMN	NF	NEAR FACE	T&G	TONGUE AND GROOVE
BSMT	BASEMENT	FD	FLOOR DRAIN	NIC	NOT IN CONTRACT	TJ	TIE JOIST
BUL	BOTTOM UPPER LAYER	FF	FAR FACE	NOM	NOMINAL	TLL	TOP LOWER LAYER
BUP	BOTTOM OF UNDERPINNING	FIN	FINISHED	NTS	NOT TO SCALE	T/O	TOP OF
C	CAMBER	FL	FLOOR	O/C	ON CENTER	TOB	TOP OF (GRADE) BEAM
CA	COLUMN ABOVE ONLY (NO COLUMN BELOW)	FMC	FULL MOMENT CONNECTION (FOR FULL MOMENT CAPACITY)	OD	OUTSIDE DIAMETER	TOC	TOP OF CONCRETE
CANT	CANTILEVER	FND	FOUNDATION	OF	OUTSIDE FACE	TOF	TOP OF FOOTING
CAT	CATEGORY (FOR AESS)	FTG	FOOTING	OPP	OPPOSITE	TOS	TOP OF STEEL
CB	COLUMN BELOW ONLY (NO COLUMN ABOVE)	GA	GAUGE	OWSJ	OPEN WEB STEEL JOIST	TOP	TOP OF PILE
CDL	COMPRESSION DEVELOPMENT LENGTH	GALV	GALVANIZED	PAF	POWDER ACTUATED FASTENERS	TOW	TOP OF WALL
CEL	CUT OFF ELEVATION FOR PILES	GB	GRADE BEAM	PC	PILE CAP	TPC	TOP OF PILE CAP
CIP	CAST-IN PLACE	GEN	GENERAL	PL	PLATE	TRANS	TRANSVERSE
CJ	CONTROL JOINT	GL	GRIDLINE	PROJ	PROJECT, PROJECTION	TSA	TENSION SPLICE "A"
CLR	CLEAR	GRD	GROUND	PS	PIPE SUPPORT	TSB	TENSION SPLICE "B"
CL	CENTRELINE	h	TOTAL THICKNESS, SLAB THICKNESS AWAY FROM DROP PANEL	PT	POST TENSIONED	TUL	TOP UPPER LAYER
CMU	CONCRETE MASONRY UNITS	hd	SLAB OVERALL THICKNESS AT DROP PANEL	PTL	PRESSURE TREATED LUMBER	TYP	TYPICAL
CNT	STEEL DECK CORE NOMINAL THICKNESS	H, HORIZ	HORIZONTAL	R	RADIUS	U-BAR	"U" SHAPED BAR
COMP	COMPOSITE	(H)	HIGH BEAM	RA	ROOF ANCHOR	UDB	UNIFORMLY DISTRIBUTED BARS
COL	COLUMN	HC	HOLLOWCORE	RD	ROOF DRAIN	U/F	UNDERSIDE OF FOOTING
CONC	CONCRETE	HD	HOLD DOWN	RE	RIGHT END	UL	UPPER LEVEL
CONT	CONTINUOUS	HDG	HOT DIPPED GALVANIZED	REINF	REINFORCEMENT	ULS	ULTIMATE LIMIT STATE
CONT'D	CONTINUED	HEF	HORIZONTAL EACH FACE	REM	REMAINDER	U/S	UNDERSIDE
CONST.J.	CONSTRUCTION JOINT	HIF	HORIZONTAL INSIDE FACE	REQ'D	REQUIRED	U/N, UNO	UNLESS NOTED OTHERWISE
CP	CONNECTION PLATE	HH	HOOK EACH END	REV	REVISION	UPT	UPTURNED
CPL	CAP PLATE	HIC	HORIZONTAL IN CENTRE	RF	RIGID FRAME	V, VERT	VERTICAL, VERTICALS
CS	COMPRESSION LAP SPLICE	HOF	HORIZONTAL OUTSIDE FACE	RL	REFERENCE LINE	VB	VERTICAL BRACING
COV	CLEAR COVER	HP	HIGH POINT	RSS	RETAINED SOIL SYSTEM	VEF	VERTICAL EACH FACE
C/W	COMPLETE WITH, CONNECT WITH	HSC	HORIZONTAL SLOTTED CONNECTION	RTU	ROOF TOP UNIT	VIF	VERTICAL INSIDE FACE
CWS	(SEE TO GENERAL NOTES)	IBI	INTEGRITY BARS INTERIOR	RET. WALL	RETAINING WALL	VIC	VERTICAL IN CENTRE
CLS	(SEE TO GENERAL NOTES)	IBE	INTEGRITY BARS EXTERIOR	R/W	REINFORCE WITH	VOF	VERTICAL OUTSIDE FACE
DCA	DRILLED CONCRETE ANCHOR, SEE GENERAL NOTES	IBA	INTEGRITY BARS ADDED	r.w.	REQUIRED WITH	VSC	VERTICALLY SLOTTED CONNECTION
DEMO	DEMOLITION	IBB	INTEGRITY BOTTOM BARS (THROUGHOUT)	SDF	STEP DOWN FOOTING (IN DIRECTION OF ARROW)	WB	WALL BELOW
DET	DETAIL	ID	INSIDE DIAMETER	SEC	SECTION	WC	WIND COLUMN
D.FIR-L	DOUGLAS FIR-LARCH	INT	INTERIOR	SIM	SIMILAR	w/o	WITHOUT
DIA, Ø	DIAMETER	IF	INSIDE FACE	SJ	STEEL JOIST	WP	WORK POINT
DIV	DIVIDER BEAM	JG	JOIST GIRDER	SL	SLAB, SHELF ANGLE	WSP-S	WSP STRUCTURAL
DMA	DRILLED MASONRY ANCHOR, SEE GENERAL NOTES	KB	KNEE BRACING	SLBB	SHORT LEG BACK TO BACK	WWF	WELDED WIRE FABRIC
DN	DOWN	(L)	LOW BEAM	SLS	SERVICEABILITY LIMIT STATE	ZRP	ZINC RICH PAINT
DNW	DOUBLE NUT AND WASHER	2-L	BACK TO BACK ANGLES	SOG	SLAB-ON-GRADE	Yc	CONCRETE DENSITY

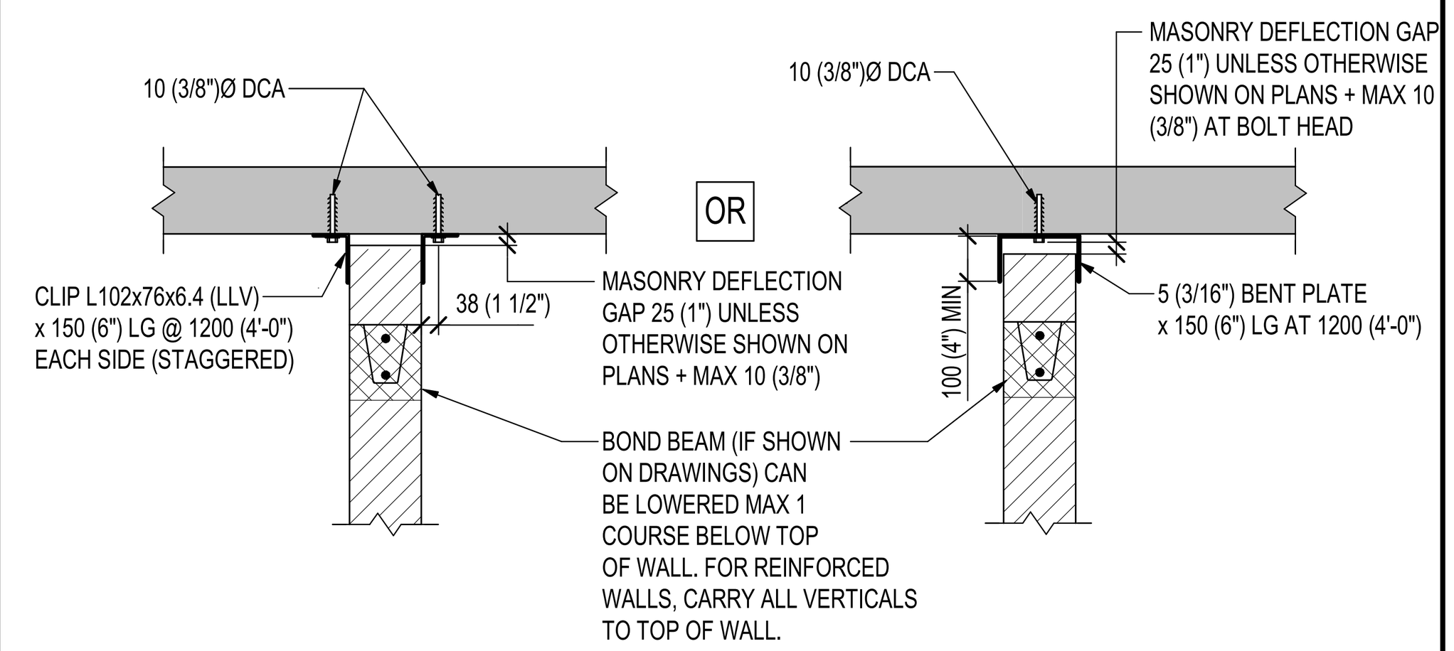
DEC 2017

LOADING ABBREVIATIONS — TG-ABBR-02

Af	FACTORED AXIAL LOAD IN kN (+ INDICATES TENSION, - INDICATES COMPRESSION)
Cf	FACTORED COMPRESSION IN kN
f'c	COMPRESSIVE STRENGTH OF CONCRETE, IN MPa
fy	YIELD STRENGTH IN MPa
Mf	FACTORED MOMENT IN kN.m
Mfx	FACTORED MOMENT ABOUT X-X (STRONG) AXES IN kN.m
Mfy	FACTORED MOMENT ABOUT Y-Y (WEAK) AXES IN kN.m
MPL	MASONRY PARTITION LOAD IN kN/m
MTf	FACTORED TORSION IN KN.m
Rf	FACTORED VERTICAL REACTION IN kN
RHf	FACTORED HORIZONTAL REACTION IN kN
P	SPECIFIED (UNFACTORED) POINT LOAD IN kN
Pf	FACTORED POINT LOAD IN kN
Vf	FACTORED SHEAR IN kN
Tf	FACTORED TENSION IN kN

JUNE 2017

LATERAL SUPPORT OF MASONRY PARTITION AT FORMED CONCRETE STRUCTURE — TM-LATS-11

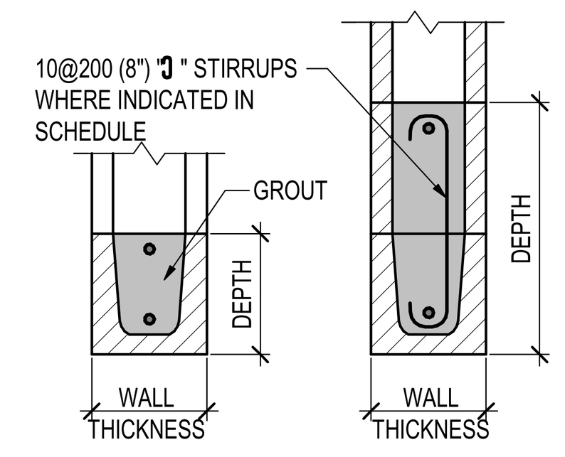


OCT 2018

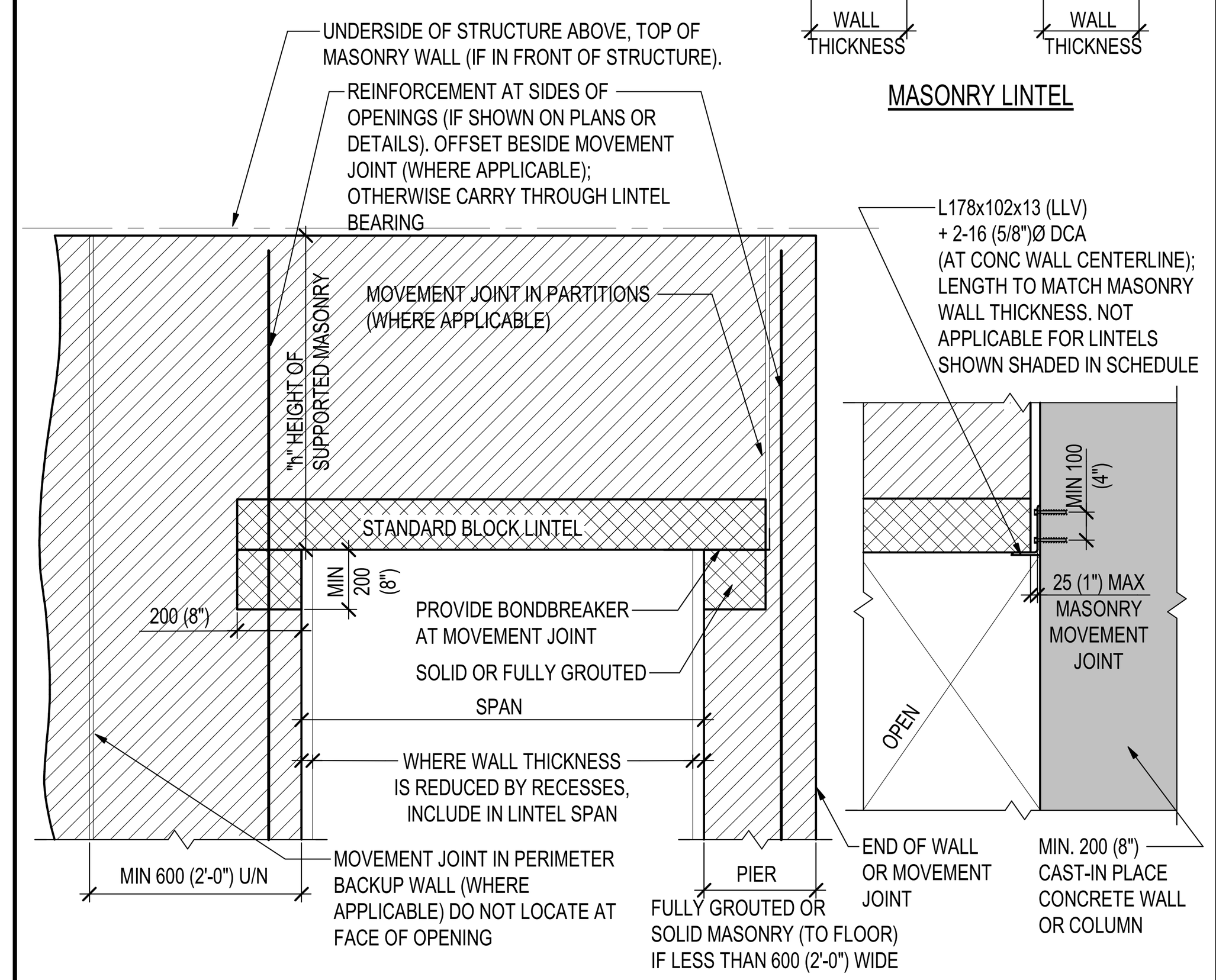
STANDARD MASONRY LINTELS IN NON LOAD BEARING MASONRY WALLS — TM-WALL-12

NOTES:

1. THIS DETAIL APPLIES FOR HOLLOW MASONRY WALLS AND FOR MASONRY WALLS WITH GROUTED CORES SPACED NOT CLOSER THAN 800 (2'-8").
2. STANDARD LINTELS ARE NOT NECESSARILY SHOWN ON STRUCTURAL DRAWINGS, REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR OPENING LOCATIONS.
3. SEE PLANS FOR SPECIAL LINTELS.
4. LINTEL BLOCKS TO HAVE SOLID BOTTOM.
5. FILL LINTEL BLOCKS WITH FINE GROUT. IT IS NOT ACCEPTABLE TO FILL LINTELS WITH MORTAR OR COARSE GROUT.
6. SUPPORT LINTELS UNTIL GROUT REACHES SUFFICIENT STRENGTH, BUT NOT LESS THAN 7 DAYS.
7. FOR LINTELS WHICH ARE SHADED IN SCHEDULE, SPECIAL DETAIL FOR CONNECTION TO CONCRETE WALL IS REQUIRED.



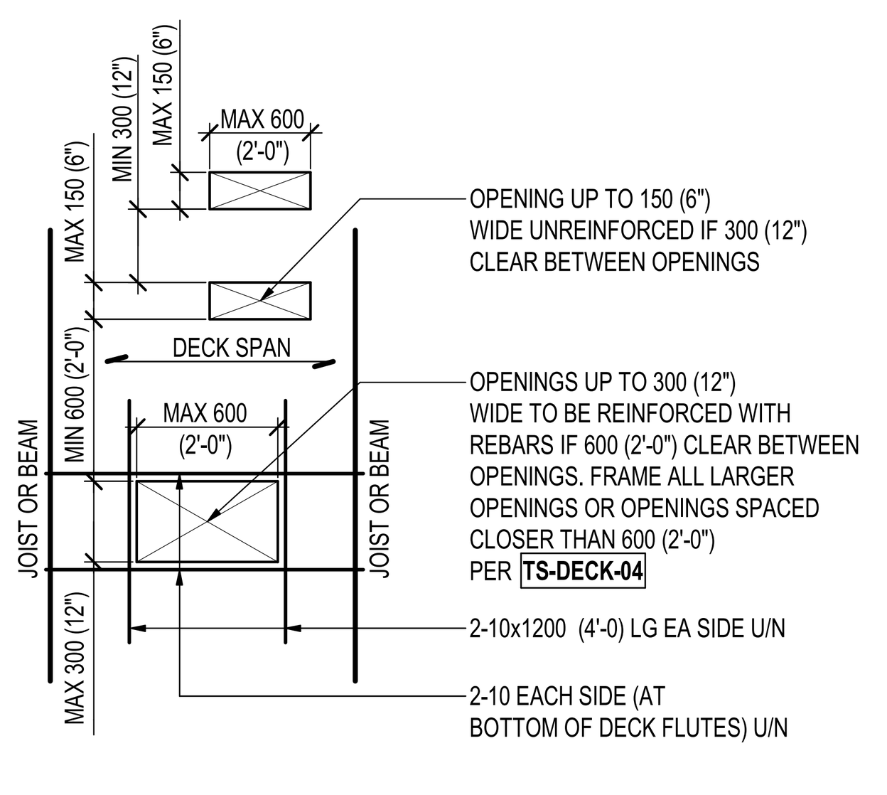
MASONRY LINTEL



WALL THICKNESS	SPAN	HEIGHT OF SUPPORTED MASONRY "h"					
		h≤1200 (4'-0")		1200 (4'-0") < h ≤ 2600 (8'-4")		2600 (8'-4") ≤ h ≤ 4800 (16'-0")	
		BLOCK LINTEL DEPTH	REINF: T&B	BLOCK LINTEL DEPTH	REINF: T&B	BLOCK LINTEL DEPTH	REINF: T&B
140 (6")	UP TO 1200 (4'-0")	190 (7 1/2")	1-10	190 (7 1/2")	1-10	390 (15 1/2")	1-10
	1200(4'-0") TO 1800(6'-0")	190 (7 1/2")	1-10	390 (15 1/2")	1-10	390 (15 1/2")	1-10+STIRRUPS
	1800(6'-0") TO 2400(8'-0")	390 (15 1/2")	1-10	390 (15 1/2")	1-10+STIRRUPS	590 (23 1/2")	1-15+STIRRUPS
	2400(8'-0") TO 3000(10'-0")	390 (15 1/2")	1-10	390 (15 1/2")	1-10+STIRRUPS	590 (23 1/2")	1-15+STIRRUPS
	3000(10'-0") TO 3600(12'-0")	390 (15 1/2")	1-10	590 (23 1/2")	1-15+STIRRUPS	590 (23 1/2")	1-15+STIRRUPS
190 (8")	UP TO 1200 (4'-0")	190 (7 1/2")	1-10	190 (7 1/2")	1-10	390 (15 1/2")	1-15+STIRRUPS
	1200(4'-0") TO 1800(6'-0")	190 (7 1/2")	1-10	390 (15 1/2")	1-15+STIRRUPS	390 (15 1/2")	1-15+STIRRUPS
	1800(6'-0") TO 2400(8'-0")	390 (15 1/2")	1-15+STIRRUPS	390 (15 1/2")	1-15+STIRRUPS	590 (23 1/2")	1-15+STIRRUPS
	2400(8'-0") TO 3000(10'-0")	390 (15 1/2")	1-15+STIRRUPS	390 (15 1/2")	1-15+STIRRUPS	590 (23 1/2")	1-15+STIRRUPS
	3000(10'-0") TO 3600(12'-0")	390 (15 1/2")	1-15+STIRRUPS	590 (23 1/2")	1-15+STIRRUPS	590 (23 1/2")	1-20+STIRRUPS
240 (10")	UP TO 1200 (4'-0")	190 (7 1/2")	1-10	190 (7 1/2")	1-10	390 (15 1/2")	1-15+STIRRUPS
	1200(4'-0") TO 1800(6'-0")	190 (7 1/2")	1-10	390 (15 1/2")	1-15+STIRRUPS	390 (15 1/2")	1-15+STIRRUPS
	1800(6'-0") TO 2400(8'-0")	390 (15 1/2")	1-15+STIRRUPS	390 (15 1/2")	1-15+STIRRUPS	590 (23 1/2")	1-20+STIRRUPS
	2400(8'-0") TO 3000(10'-0")	390 (15 1/2")	1-15+STIRRUPS	390 (15 1/2")	1-15+STIRRUPS	590 (23 1/2")	1-20+STIRRUPS
	3000(10'-0") TO 3600(12'-0")	390 (15 1/2")	1-15+STIRRUPS	590 (23 1/2")	1-20+STIRRUPS	590 (23 1/2")	1-20+STIRRUPS
290 (12")	UP TO 1200 (4'-0")	190 (7 1/2")	1-15	190 (7 1/2")	1-15	390 (15 1/2")	1-15+STIRRUPS
	1200(4'-0") TO 1800(6'-0")	190 (7 1/2")	1-15	390 (15 1/2")	1-15+STIRRUPS	390 (15 1/2")	1-15+STIRRUPS
	1800(6'-0") TO 2400(8'-0")	390 (15 1/2")	1-15+STIRRUPS	390 (15 1/2")	1-15+STIRRUPS	590 (23 1/2")	1-20+STIRRUPS
	2400(8'-0") TO 3000(10'-0")	390 (15 1/2")	1-15+STIRRUPS	390 (15 1/2")	1-20+STIRRUPS	590 (23 1/2")	1-20+STIRRUPS
	3000(10'-0") TO 3600(12'-0")	390 (15 1/2")	1-15+STIRRUPS	590 (23 1/2")	1-20+STIRRUPS	590 (23 1/2")	1-25+STIRRUPS

JUNE 2019

REINFORCEMENT AT OPENINGS IN SLAB ON DECK — TS-DECK-03

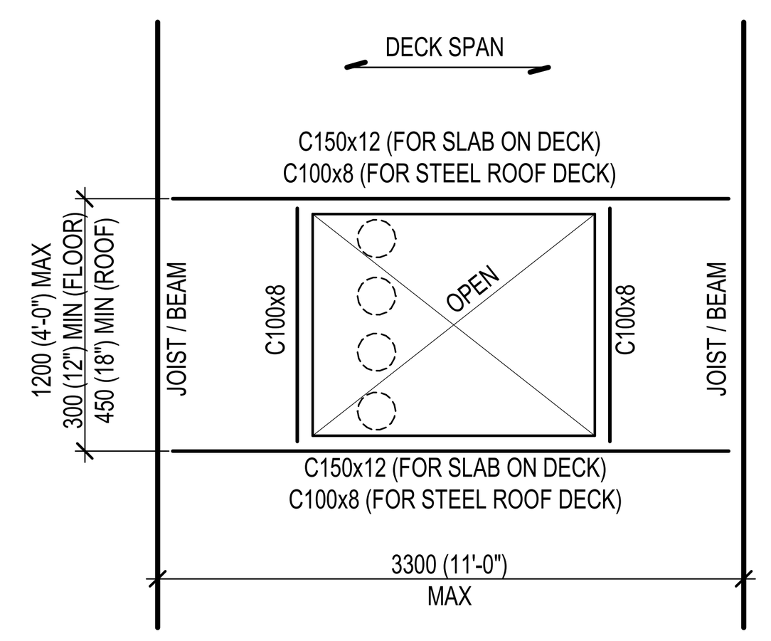


NOTE:

1. CUT STEEL DECK FOR THE OPENINGS ONLY AFTER CONCRETE REACHES MINIMUM 75% OF SPECIFIED CONCRETE STRENGTH

DEC 2017

ADDITIONAL FRAMING AT DECK OPENINGS — TS-DECK-04



NOTES:

1. DECK SUPPLIER TO DESIGN AND PROVIDE REINFORCEMENT FOR STEEL ROOF DECK OPENINGS LESS THAN 450 (18")
2. SEE TS-DECK-03 FOR REINFORCING OF OPENINGS IN SLAB ON DECK LESS THAN 300 (12")
3. MULTIPLE SLEEVES PLACED IN LINE PERPENDICULAR TO DECK SPAN WITH LESS THAN ONE WHOLE DECK FLUTE (COMPLETE WITH TWO WEBS) REMAINING BETWEEN THEM SHOULD BE TRATED AS A SINGLE OPENING

JUNE 2019

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Discipline	Sk. Reg. No.	Signature
Structure	23426	[illegible]

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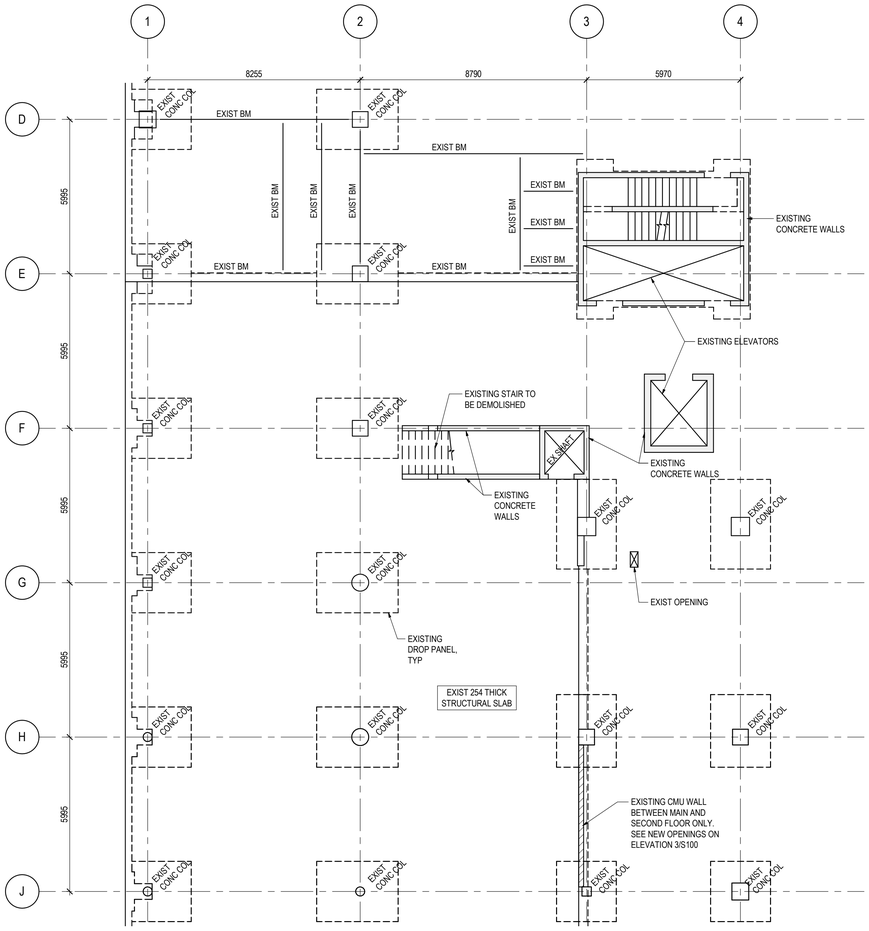
201-08499-00	S001 2 OF 4	2

201-08499-00

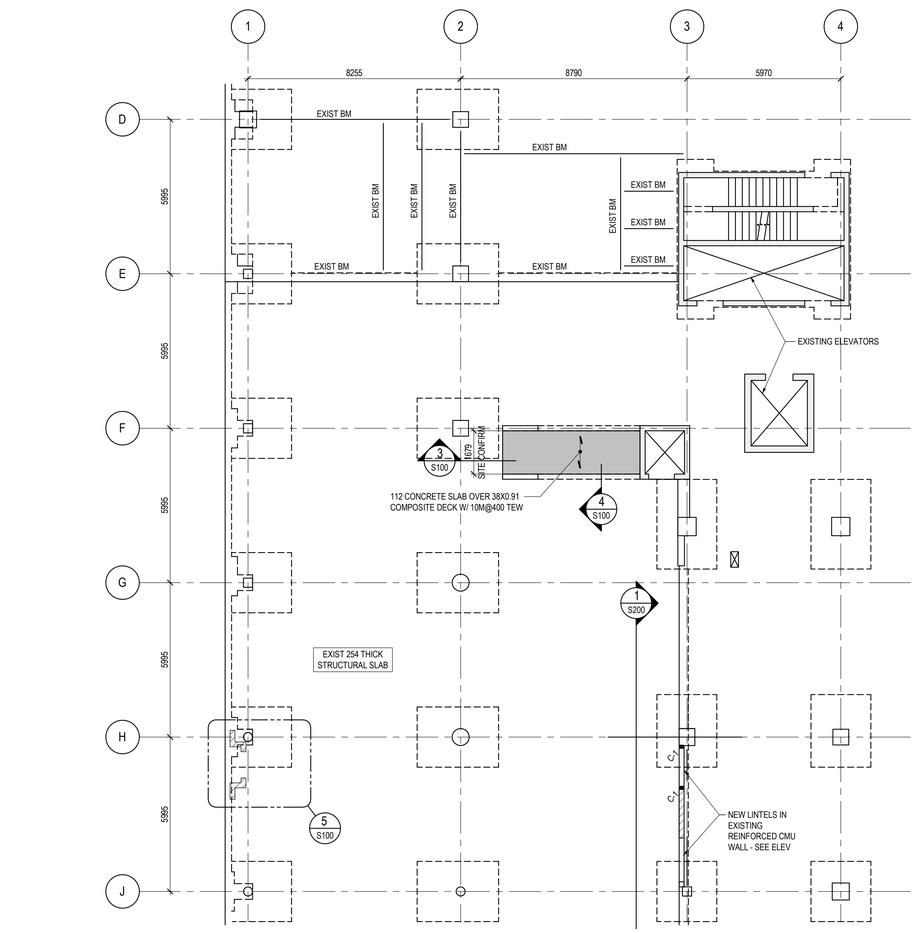


1 S100 PARTIAL MAIN FLOOR - DEMO PLAN

1 : 100

MAIN FLOOR DEMO PLAN NOTES:

1. SEE GENERAL NOTES AND TYPICAL DETAILS ON S000 SERIES DRAWINGS.



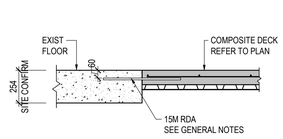
2 S100 PARTIAL MAIN FLOOR - NEW FRAMING PLAN

1 : 100

MAIN FLOOR FRAMING PLAN NOTES:

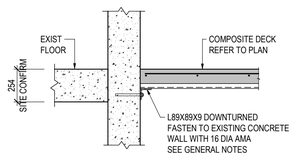
1. SEE GENERAL NOTES AND TYPICAL DETAILS ON S000 SERIES DRAWINGS.
2. MAIN FLOOR ELEVATION ASSUMED 10 000.
3. UNLESS NOTED OTHERWISE ON PLAN, DESIGN LOADS ARE:
   LIVE LOAD (LL) = 4.8 kN/m²
   SUPERIMPOSED DEAD LOAD (SDL) = 1.25 kN/m²
   PARTITION LOAD IS INCLUDED IN THE SDL
4. UNLESS OTHERWISE NOTED ON PLANS OR DETAILS, THE FOLLOWING DATA APPLIES:
   4.1.TOP OF SLAB IS ±0 FROM FLOOR DATUM ELEVATION.
   4.2.TOPS OF STEEL ANGLES ARE AT UNDERSIDE OF STEEL DECK.
   4.3.REFER TO TYPICAL DETAIL TS-DECK-03 FOR MAXIMUM SIZE OF OPENINGS IN SLAB ON DECK AND FOR ADDITIONAL REINFORCING AROUND THEM.
   4.4.TRIM SIDES OF DECK OPENINGS AS PER TYPICAL DETAIL TS-DECK-04.

COLUMN SCHEDULE				
MARK	SIZE	BASE PLATE	ANCHORAGE	REMARKS
C-1	HSS 89x89x8.0	SEE DETAIL A	2-13 DIA ACA SEE GENERAL NOTES	SCAN FLOOR SLAB TO ENSURE ANCHOR PLACEMENT DOES NOT COMPROMISE EXISTING SLAB REINFORCING



3 S100 SECTION

1 : 20



4 S100 SECTION

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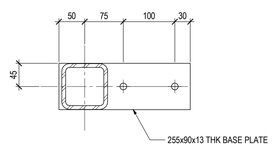

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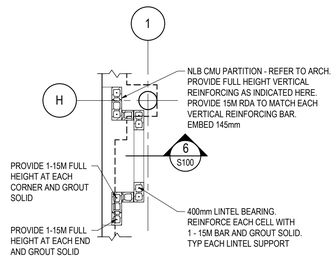
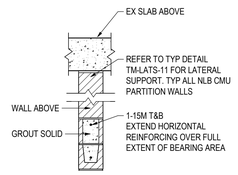


5 S100 PARTIAL PLAN

1 : 50



6 S100 SECTION

1 : 20

Public Works and Government Services Canada
Travaux publics et Services gouvernementaux Canada

REAL PROPERTY SERVICES
Western Region
SERVICES IMMOBILIERS
Région de l'ouest

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Association of Professional Engineers & Geoscientists of Saskatchewan
CERTIFICATE OF AUTHORIZATION
WSP Canada Inc.
Number C0868
Permission to Consult held by:

Discipline	Sk. Reg. No.	Signature
Structure	23426	

DO NOT SCALE DRAWINGS

Revision / Revision	Description/Description	Date/Date
2	ISSUED FOR TENDER	2021-03-17
1	99% SUBMISSION	2021-02-05
0	66% SUBMISSION	2020-12-07

Client/client:

PUBLIC SERVICES AND PROCUREMENT CANADA

Project title/Titre du projet:

1783 HAMILTON STREET
REGINA, SASKATCHEWAN

ESDC - PPT REGINA AMALGAMATION

Approved by/Approuve par:
R.KELLN

Designed by/Concept par:
R.KELLN

Drawn by/Dessine par:
P.JUNG

PWGSC Project Manager/Administrateur de Projets TPSGC:
JEAN-PHILIPPE BLOUIN

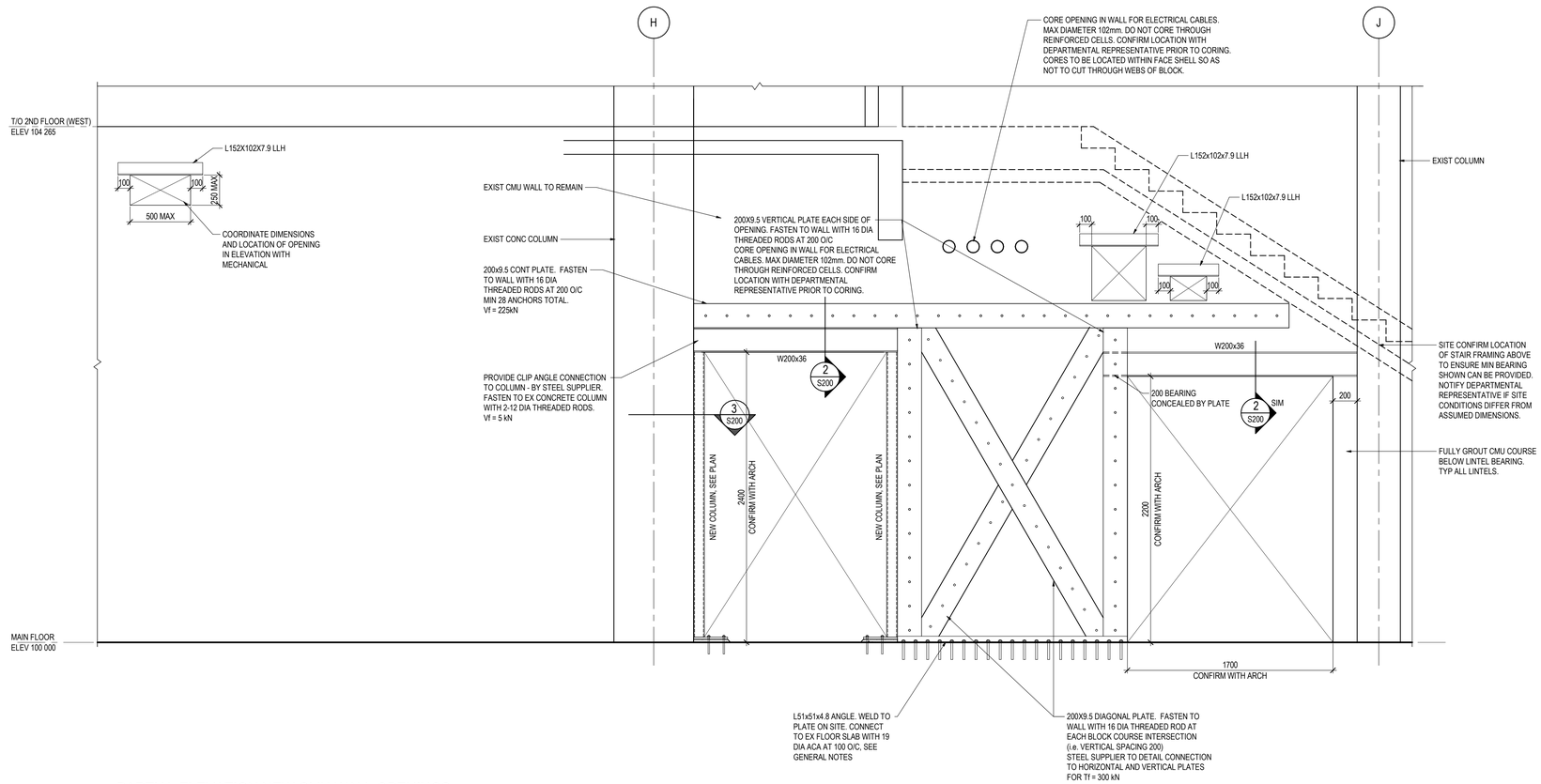
PWGSC, Architectural and Engineering Resources Manager/ Ressources Architectural et de Directeur d'Ingénierie, TPSGC:

Client/Client:
PUBLIC SERVICES AND PROCUREMENT CANADA

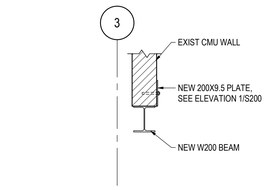
Drawing title/Titre du dessin:

PARTIAL MAIN FLOOR PLAN, SECTION & DETAIL

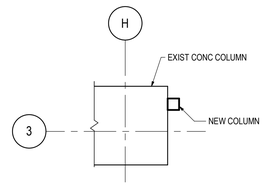
Project no./No. du projet	Sheet/Feuille	Revision no./ Le Révision no.
201-08499-00	S100 3 OF 4	2



1 S100 PARTIAL ELEVATION NEW CMU WALL OPENINGS
1 : 20



2 S200 SECTION
1 : 20



3 S200 SECTION
1 : 20

PARTIAL ELEVATION NOTES:

1. CONFIRM ALL DIMENSIONS ON SITE ONCE EXISTING FINISHES ARE REMOVED BY PRIOR TO COMMENCEMENT OF STRUCTURAL WORK. NOTIFY DEPARTMENTAL REPRESENTATIVE OF ANY CONFLICTS WITH EXISTING CONDITIONS.
2. SCAN EXISTING WALL FOR REINFORCING PRIOR TO COMMENCING WORK. PROVIDE REPORT TO DEPARTMETAL REPRESENTATIVEF FOR REVIEW SHOWING RESULTS OF SCANNING.
3. SCAN EXISTING FLOOR SLAB AT ANCHOR LOCATIONS FOR REINFORCING – SEE GENERAL NOTES FOR REQUIREMENTS.
4. AT ALL PLATE LOCATIONS, GROUT FILL CMU COURSES WHERE ANCHORS WILL BE INSTALLED PRIOR TO PLATE INSTALLATION. REPAIR FACE SHELL AND MAKE GOOD PRIOR TO PLATE INSTALLATION.
5. RETAIN PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF SASKATCHEWAN TO DESIGN TEMPORARY SHORING FOR CREATION OF NEW OPENINGS IN EXISTING WALL.

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ELEVATION

Project no./No. du projet	Sheet/Feuille	Revision no./ La Révision no.
201-08499-00	S200 4 OF 4	1