

Public Works and Government Services Canada Travaux publics et Services gouvernementaux Canada

Inspection Report and Concept Repair Design

SOLICITATION No.: PEQ754-161278/001/PWL: Burlington Canal Lift Bridge Engineering Services

PROJECT No.: R.081864.001 Urgent Steel Deck Grating Repairs

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April, 2017

MORRISON HERSHFIELD

1160268.03



Public Works and Government Services Canada Travaux publics et Services gouvernementaux Canada

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EXECUTIVE SUMMARY

Morrison Hershfield (MH) has been retained by Public Works and Government Services Canada (PWGSC) to provide professional engineering services associated with an inspection and design related to Urgent Steel Deck Grating Repairs for The Burlington Canal Lift Bridge. The structure is a tower-driven vertical lift bridge constructed in 1958, with an overall lift span of 116 meters. The bridge is located in Hamilton, Ontario and carries 4 lanes of Eastport Drive traffic over the Burlington Canal, with a posted speed limit of 50 km/hr.

The Urgent Steel Deck Grating Repairs pertain to the current deck grating on the lift span. The deck is comprised of total 98 steel grating panels.

A field visual inspection was completed by MH on October 14-15, 2016. The significant findings from MH's visual inspection included the following:

- Cracked welded connections within the grating panels were observed throughout the entire bridge deck surface.
- Cracked welds between the panels and the bridge floorbeams were observed in numerous locations.
- Large areas within the panels with complete failure of joints between grating members were observed throughout the deck with NB outside lane being affected above average. Those Local Grating Failure Areas were classified as critically important.
- The deck grating is subjected to repetitive wheel loading exerted by the heavy volumes of traffic on the bridge.
- Evidence of multiple on-site welding repairs of the cracked welded connections.

Based on the findings of this inspection MH recommends that the repairs to the steel grating panels and to the grating to floorbeam connections need to be completed as soon as possible. Local Grating Failure Areas repairs are classified as of critical importance since they are considered hazardous and present an imminent public safety concern.

The major work items include:

- Pre-Construction Inspection
- Button Weld Crack Repair
- Local Grating Failure Area Repair (Critical Repair)
- M-Beam-to-Stringer Repair

The estimated cost of construction is approximately \$415,000.00, including contingency but excluding HST and Departmental Representative fees.

The recommended repairs constitute a holding strategy until the deck is replaced and upgraded to CHBDC performance levels, which is expected to be within 2 to 3 years. Additional monitoring and regular inspections are also recommended until the deck panels are fully replaced.

1. INTRODUCTION

Morrison Hershfield Limited (MH) has been retained by Public Works and Government Services Canada (PWGSC) to provide engineering services for the design and construction of required short term repairs on the Burlington Canal Lift Bridge under PWGSC Standing Offer No. EQ754-161278/001/PWL: BCLB Bridge Engineering Services, Project Number: R.081864.001 Urgent Steel Deck Grating Repairs.

The detailed scope of services to be provided under this assignment is listed in our proposal dated June 13, 2016. The scope of services consists of:

- 1. Obtain all necessary approvals and all traffic management schemes required during implementation of the emergency repair work;
- 2. Close-up inspection of the steel deck grating and identification of all distressed locations;
 - a. This includes the inspection of the test panel installed in 2014 as part of the emergency repairs and the documentation of its performance since installation
- 3. Design of urgent (short term) repairs to address defects and deficiencies observed in order to allow the bridge to continue safe operation until the long-term deck replacement work is completed;
- 4. Preparation of drawings and documents of the proposed emergency repairs to the defective panels, as well as assistance to PWGSC during the tendering, award process, and implementation;
- 5. Non-resident engineering supervision and support services during construction.

2. BACKGROUND

The Burlington Canal Lift Bridge carries 2 NB and 2 SB lanes of Eastport Drive traffic over the navigation channel linking Hamilton Harbour with Lake Ontario. The bridge carries local traffic and part of the detour route in the event of Burlington Skyway Bridge/QEW closures.

The Burlington Lift Bridge was constructed circa 1958 to carry rail and road traffic. The bridge structure is a tower-driven vertical lift bridge. The lift span is 116 metres long, weighs 1,996 tonnes and has a vertical lift of 33.5 metres. At full lift height, maximum clearance for marine traffic is approximately 36.5 metres during typical water level conditions. When locked in its lowered position, a clearance of approximately 5 metres, during typical water level conditions, allows very small pleasure craft, kayaks and canoes to pass easily beneath the bridge.

Modifications to the lift span were made in 1982 to convert the bridge to only carry car and truck traffic. The current deck grating on the lift span was installed in 2000. The deck is comprised of total 98 steel grating panels spanning across WBL and EBL, 49 steel panels in each direction. The typical panels are 3124mm x 7105mm and 3124mm x 7620mm, 130mm 4-way open grating type, made of grade ASTM A588 steel and welded in accordance with AASHTO/AWS D1.5-95. Each panel consists of 130 mm-deep transverse main bearing beams (M-beams) spaced at 190 mm that connect the grating panel to the longitudinal floorbeams (stringers) below. These M-beams are interspersed with 25 mm-deep supplementary bars (S-bars). 50 mm-deep members (C-bars) spaced at 95 mm are transverse to the M-beams and S-bars to form the grid. The 25 mm-deep diagonal bars connect all members to stiffen the grid. All connections are formed



by seating the members against each other and fused with factory-performed button welds. The drawings for the existing panels are provided in **Appendix A – Reference Structural Drawings.**

The deck grating is in seriously deteriorated condition. It is unknown when the deterioration first manifested itself. As part of an earlier assignment to replace a failed deck panel, MH performed a simplified structural evaluation to select an appropriate replacement grating panel. The evaluation estimated the spacing of the M-beams required to take the highest factored CHBDC wheel load using the section modulus of the existing grating and found the existing spacing is not adequate to carry these wheel loads. This replacement panel (Panel E1) was designed to satisfy CHBDC loading criteria and was installed in March 2014. It was to serve as a Test Panel for the possibility for similar future replacement of the remaining panels, which have been subjected to multiple on-site welding repairs due to structural failures of the welded connections within the grating and between the grating and stringer.

MH bridge engineering specialists performed an emergency inspection of the deck on April 7th, 2016 and observed that the steel grating was failing under traffic load in several modes including: grating to stringer weld failures, button weld cracks and M-beam cracks. Following the observations, immediate actions were recommended for remediation of the most affected panels. The findings, discussion and recommendations are documented in MH Report No 2.1: 1160268.02 Burlington Lift Bridge – Call up 2 Bridge Deck Cracking.

3. DESIGN REFERENCES

Structural Drawings:	104365 New Deck Grating, Drawing 1, dated October 29, 1999 by National Centre of Expertise Architecture and Engineering Services, Real Property Services Branch.
	Burlington Lift Bridge – New Grid Deck, Drawings 1 to 3, dated January 19, 2000 by IKG Greulich.
	R.012843.055 PWGSC Steel Panel Replacement, Drawings B-0 to B-2, dated January 24, 2014 by Morrison Hershfield
Codes and Standards:	CSA S6-14 – Canadian Highway Bridge Design Code
	CSA S16-14 - Design of Steel Structures
	CSA W59-13 - Welded Steel Construction (metal arc welding

4. CONDITION ASSESSMENT

MH bridge engineers conducted a steel grating deck visual inspection on October 14-15, 2016.

The investigation examined all 98 panels on the bridge. The entire surface of each single panel was inspected for structural and non-structural damages.

This visual inspection was made from the top side of the deck. The panels were checked for structural discontinuity, vibrations and sound response by means of a rubber mallet. MH validated the dimensions of the existing grating and components relevant to the deck repair. Deck deficiencies were marked with temporary spray paint. Each panel was then photographed including close of details of failure modes. (See photos 1 and 2 of **Appendix B – Inspection**



Photographs) During the inspection, the existing roadway and traffic conditions were observed for significance of the relationship between the location of concentrated wheel loads and localized/concentrated deck failures.

The inspection at some locations was obscured by dirt at the level of the M-beams and debris embedded in the grating (see Photos 23 to 26 in **Appendix B**). However, it is estimated that less than 1% of the grating area was obscured.

5. CLASSIFICATION OF GRATING FAILURES WITH CONCLUSIONS

Please refer to **Appendix C: Inspection Summary** and **Appendix D: Inspection Findings Drawings** for specific number and location of the major grating failures described below.

5.1 Button Weld Cracks

See photos 7 and 8 of Appendix B.

Cracks typically occur in the Button Welds that connect the steel grating members to the primary "M-beams" and the transverse "C-bars". The original welds were performed under shop conditions, but several welds have been field-repaired since the original panel installation in 2000.

Cracks can propagate through latent weaknesses in the weld or insufficient weld coverage. The problem is aggravated when the grating is regularly exposed to concentrated wheel loads, particularly from truck traffic that is more than its design capacity. The connecting members in each weld have differing structural properties, which exacerbate induced cyclical strains. The deck panel redistributes the load path around a cracked weld, thus increasing the likelihood of cracks in adjacent welds.

The incidence of button weld cracks is widespread. The frequency of occurrence is greatest within the wheel paths. The quality of the field-repaired welds is likely inferior to that of the factory welds, and thus appear to be cracking at an accelerated rate compared to the original welds. (See photos 21, 22 of **Appendix B**)

Based on highest stress range directly below wheel loads due to local effects, all current button weld cracks are subjected to repetitive fatigue cycles. These failures will eventually progress into connection discontinuities which then lead to appearance of Local Grating Failure areas (see 5.3). The button weld cracks do not necessarily coincide with M-Beam to stringer weld failures (see 5.2) although it can be reasonably assumed that these failures are related. When the stringer welds fail, the flexibility of the panel is increased and the impact loads are magnified. This results in a greater dynamic load and movement range at the button welds that could then result in further cracking.

5.2 M-beam to Stringer Weld Failure

See photos 9 and 10 of Appendix B.

Cracks typically occur in the field fillet welds connecting the 130 mm-deep M-beams to the stringers. It was noted that the welds are often incomplete (e.g. welds on only one side of the flange, discontinuous or inconsistent weld size, etc.). The reduction in connectivity between the M-beam and the stringer due to the cracked welds contribute to the "bounciness" of the panels under heavy traffic load conditions. As the damage progresses more welds are affected leading to instability of the panel due to improper anchorage onto the stringer of the bridge.



5.3 Local Grating Failure

See photos 13 and 20 of Appendix B.

Local Grating Failure (LGF) on single panel is defined as an area of up to 1 m² with complete failure of one or more joints between grating members. It was observed that LGF areas typically did not extend beyond 1 m², but had high concentration of connection damage. Such failures are characterized by the complete failure of the Button Welds and presence of vertical cracks throughout the entire section of the adjoining deck grating member. In addition, such failures are often combined with missing grating members and sharp edges that suggest the members broke after the welds had failed. The LGF areas do not always correspond with M-beam-to-stringer weld cracks, although these cracks could be contributing to the formation of LGF areas.

LGF areas are detected when subjected to impact load, such as a strike of a mallet or high speed rolling vehicle wheel. The response is deflection and vibration of one or more grating members, and is both visibly and audibly detectable.

Several LGF areas coincide with where the Button Welds were field-repaired.

More LGF areas are likely to develop. The existing areas will likely increase due progressive crack development and fatigue because of load redistribution. Presently affected grating members can result in openings of at least the size of a grating unit. Further development of LGF areas is considered hazardous and presents an imminent public safety concern. This is particularly dangerous for motorcyclists and bicyclists who may be more susceptible to their tires punching through the grating openings.

5.4 M-beam Cracks

During MH's emergency inspection of the deck on April 7th, 2016, a single crack was observed in the web of the M-beam supporting deck panel E7 at its connection to the stringer. This damage was subsequently repaired by welding a steel plate to the web as recommended in MH Report No 2.1: 1160268.02 Burlington Lift Bridge – Call up 2 Bridge Deck Cracking. The condition of this repair was good and no further cracking was observed in this location.

All accessible panels were checked for presence of similar M-beam cracks. No occurrences of M-beam cracks were found.

5.5 Other deficiencies

Broken grating members (See photo 12 of Appendix B)

Broken or missing grating members adversely affect panel performance and public safety. They are often associated with LGF areas.

Warped grating members (See photo 11 of Appendix B)

Warped members do not correspond with Button Weld cracks or LGF areas. The cause of the warping is unknown.

Field weld repairs (See photos 21 and 22 of Appendix B)

Re-welded connections appear to be prone to re-cracking.

Surface Rust, Flaking Paint and Debris (See photos 23 and 26 of Appendix B)



The listed items are classified as non-structural deficiencies and require no immediate action. However, those items tend to conceal the grating steel's surface therefore could prevent from accurate monitoring and inspections procedures.

6. PERFORMANCE OF TEST PANEL E1

The Test Panel E1 was checked for any signs of performance deficiencies. All the structural members and connections of the panel were visually checked for, but not limited to, damages listed in point 5.1 of this Report. It was noted that the Test Panel was sturdier and less prone to vibrations than the other panels. There were no structural deficiencies noted for the Test Panel E1. (See photos 3 to 6 of **Appendix B**)

7. OTHER OBSERVATIONS

a. It is reasonable to conclude that NB lanes are subjected to more traffic than SB lanes, in particular heavy load traffic. The conclusion is derived from the fact that the NB lanes on the adjacent Burlington Skyway Bridge have been affected by frequent closures during recent years.

This observation is consistent with the fact that the BCLB NB lanes are damaged to greater extent than the BCLB SB lanes.

- b. Transition between wearing surface at approaches onto the grating is not smooth due to a small change in elevation. Wheels on vehicles travelling northbound impact on the panels upon transition. This observation does not seem to correspond an increase in grating defects; no defects were observed in the end panels.
- c. During our inspection, the speed limit appeared to be frequently exceeded by vehicles on the bridge.
- d. LGF areas pose a significant risk for tire impact or puncture for bicyclists and motorcyclists. Additionally, the vibration noises in these areas may be unsettling.

8. TEMPORARY REPAIR OPTIONS WITH RECOMMENDATIONS

Urgent repair of the steel deck is required. The recommendations below are "holding" repairs for short term. Permanent repair is likely not feasible. It is strongly advised that the deck be replaced within the next 2-3 years horizon. Our recommendation is based on our observations of considerable damage consistent with repeated heavy wheel loading, the high frequency of local failures reported by the bridge operations staff and observed on site, the pre-existing failure of one panel that forced its emergency replacement, and a lack of structural capacity in the existing grating. See **Appendix E: Repair Options** for concept sketches.

8.1 Button Weld Cracks

Repair button weld cracks by welding vertically along the entire depth of the grating members to improve the weld connectivity. This weld is to be performed and inspected according to a rigorous protocol that is based on CSA W59-13.



Routine button weld monitoring and weld repairs as required should continue until the panels are replaced. Cracks with width more than 1 mm should be welded as per the method discussed in point 8.3.

8.2 M-beam to Stringer Weld Failure

Option A – Re-Welding (see Sketch SK-01, Appendix E)

Cracked M-beam to stringer welds will be re-welded on-site. This option is the most cost effective approach but due to the difficulty in accessing the weld location from above the deck, the quality of the field is difficult to control and recracking of the welds is likely.

This option is the least preferred from a technical standpoint. However, it is recommended as an alternative to Option D described below due to its simplicity and cost effectiveness. It is to be applied only if application of Option D is not feasible.

Option B – Clamp Panel Grating to Stringer Flange (see Sketch SK-02 and SK-03, Appendix E)

Mechanical clamp will hold the M-beam in place and transfer the compressive force through the epoxy filler and 4 holding bolts to the stringer flange. This application is possible with an installation crew working from both below and atop the deck. Traffic control is required at all times during the repairs. Access is required to the underside of grating. All access options are costly and logistically challenging.

Option B is a non-invasive method of holding the panels to the stringers.

Option C – Bolt Panel to Stringer using Welded Plates and Studs (see Sketch SK-04 and SK-05, Appendix E)

Steel plate welded to the M-beam web will be held down by threaded studs welded to the top of stringer flange. This application is possible from the top of deck. The method requires minimal removal of grating section. After installation, the missing grating members will be replaced with members having equivalent or better structural properties. Traffic control is required at all times during the repairs.

Option C is considered technically appropriate; it requires structural amendments to existing stringers and grating.

Option D – Weld Hold-Down Plates onto Stringer and C-bars (see Sketch SK-06 and SK-07, Appendix E)

Steel plates welded to C-bars will be welded down to the top of stringer flange. This application is possible from the top of deck. The method requires no removal of grating section and provides an alternate load path from the grating to the top of the stringer by utilizing the C-bars. Traffic control is required at all times during the repairs.

Option D is considered technically appropriate and is the preferred choice due to the simplicity of the components and the effectiveness of the hold-down connection. However, it can be time-consuming to weld.

8.3 Local Grating Failure Areas

The condition of the panels affected by Local Grating Failure Areas warrant immediate remedial action.

The recommended temporary repair is to reinforce the broken grating panels by welding solid round bars to both the broken and the good sections of the panel grating. This provides an alternate load path around the cracks while mitigating the local failed section from bouncing or failing completely. This solution is an intermediate-term holding



measure until the panel is fully replaced. This solution can be applied from the top of the deck with traffic control measures in place. The concept is shown in *Sketch SK-08*, Appendix E.

8.4 M-beam Cracks

The short-term recommendation for the M-beam cracks is monitoring at approximately monthly intervals.

8.5 Other deficiencies

Broken or missing grating members are subject to remedial action. Priority should be given to the deficiencies directly associated with Local Grating Failure areas within the Traffic Lanes. Recommended method of temporary repair is similar to the method proposed in point 8.3 that is application of solid round bars that can be welded to both the broken and the good sections of the panel grating. The missing grating members should be replaced with members of equivalent or better structural properties. Where appropriate, a lap plate can be used to fill in the missing member.



9. SUMMARY OF RECOMMENDED REPAIRS

The summary of Recommended Repairs sorted in accordance with the importance factor can be found in the table below:

DEFECT	IMPORTANCE	RECOMMENDATIONS	TIMELINE
Local Grating Failure Areas	Critical	 Welding Repairs with use of Solid Round Bar as a structural connector; Application from top of deck only with Traffic Control; See point 8.3 and sketch SK-08 	Priority, immediate
M-beam to Stringer Weld Failures	Urgent	 PREFERRED: Option D – Weld Hold-Down Plates onto Stringer and C-bars; Application from top of deck only with Traffic Control; See point 8.2 and sketch SK-01 ALTERNATIVE: Option A – Re-Welding of cracked welds; Application from top of deck only with Traffic Control; See point 8.2 and sketch SK-06, SK-07 	Within 0-3 months
Button Weld Cracks	Moderate to Urgent	 Re-Welding of cracked Button Welds; Application from top of deck only with Traffic Control; See point 8.1 	Within 0-3 months
Other deficiencies	Low to Urgent	 Repairs as needed; to be assessed in conjunction with proximity to other defects; Monitoring; See point 8.5 	Urgent repairs to be conducted within 3 months. Monitoring to continue until panel is replaced
M-beam Cracks	Low	Monitoring;See point 8.4	3 month intervals

NOTES:

1. All repairs are intended to act as holding measures until deck is replaced and upgraded to CHBDC performance levels, which is expected to be within 2 to 3 years.

2. Recommended repairs are conceptual designs only and are subject to changes during the detailed design stage.



10. **CLASS 'C' COST ESTIMATE**

The estimated construction cost is shown in the table below.

BCLB BRIDGE ENGINEERING SERVICES URGENT STEEL DECK GRATING REPAIR	COST
Pre-Construction Inspection	\$4,044
Button Weld Crack Repair	\$107,660
M-Beam-to-Stringer Repair (Option D)	\$202,104
Local Grating Failure Area Repair	\$100,870
TOTAL (excluding HST)	\$414,678

The estimated construction cost includes contingency, engineering fees, and Departmental Representative fees. Cost estimate details are provided in Appendix F.

CLOSURE 11.

We trust that this report satisfies your immediate requirements.

Prepared by:

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Reviewed by:

Stouth.

Joe Ostrowski, P.Eng. Project Manager Morrison Hershfield Ltd.

Mida Julie

Michal Szafarski, P.Eng. **Bridge Engineer** Morrison Hershfield Ltd.

Appendix A

REFERENCE STRUCTURAL DRAWINGS





 Image: Image:



E17 E18 E19 E20 E21 E22 E23 E24 $\frac{1}{90}$ $\frac{1}{90}$ $\frac{1}{90}$ $\frac{1}{90}$ $\frac{1}{90}$ $\frac{1}{90}$ 41 B42 B43 B44 B45 B46 B47 C $\frac{1}{90}$ $\frac{1}{90}$ $\frac{1}{90}$ $\frac{1}{90}$ $\frac{1}{90}$ $\frac{1}{90}$ 41 E42 E43 E44 E45 E46 E47 F $\frac{1}{90}$ $\frac{1}{90}$ $\frac{1}{90}$ $\frac{1}{90}$ $\frac{1}{90}$ $\frac{1}{90}$ $\frac{1}{90}$	E17 E18 E19 E20 E21 E22 E23 E24	B17	' B18 B19	B20 B21 B22	B23 B24
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WELDING PROCESS

ALL WELDING TO BE DONE IN ACCORDANCE WITH AASHTO HIGHWAY AND BRIDGE SPEC. A.W.S.D1.5-95.

MATERIAL SPECIFICATIONS

M'BEAMS (7.8 kg/m)TO BE A.S.T.M. 4588 C'BARS (6.35 x 50.80) TO BE A.S.T.M. 4588 SUPP'L BARS (6.35 x 25.40) TO BE A.S.T.M. 4588 DIAG. BARS (6.35 x 25.40) TO BE A.S.T.M. 4588

FINISH SPECIFICATIONS

PREP .:

PAINT:

GALVANIZING SPECIFICATIONS

WHEN REQUIRED, MATERIAL TO BE HOT-DIP GALVANIZED AFTER FABRICATION AS PER A.S.T.M. A123 LATEST REVISION. PANELS NOT TO EXCEED 2337 WIDE WHEN GALVANIZING.

CUSTOMER NOTES

IKG DRAWINGS MUST BE REVIEWED BY THE CONTRACTOR FOR CONFORMITY, WORKABILITY & COMPATIBILITY WITH THE EXISTING STRUCTURE PRIOR TO AUTHORIZING FABRICATION.

SHOP MARK EACH PANEL IN UPPER LEFT HAND CORNER AND FIELD MATCH AT INSTALLATION.

ALL MATERIAL SUBJECT TO SHOP/MILL TOLERANCES.

MANUFACTURER RESERVES THE RIGHT TO MAKE MINOR CHANGES FOR PRODUCT IMPROVEMENT.

WHEN REQUIRED SERRATIONS TO BE APPROXIMATELY 4.76 DEEP \times 9.53 WIDE and SPACED at Random.

WHEN THIS PRODUCT IS USED ON A MOVEABLE BRIDGE OR OTHER WEIGHT CRITICAL APPLICATION, ACTUAL FINISH WEIGHT TO BE DETERMINED BY THE CONTRACTOR PRIOR TO INSTALLATION.

IKG STANDARD PA	ANEL TOLERANCES
PANEL WIDTH	+0mm -3mm
PANEL LENGTH	±6mm
SQUARENESS	±12mm MEASURED DIAGONALLY
TRANSVERSE CAMBER (WIDTH)	.005 x WIDTH
LONGITUDINAL CAMBER (LENGTH)	.003 x LENGTH
SIDE BOW (SWEEP)	±3mm PER 1000 LINEAL mm
MAIN BAR VERTICALITY	±3mm
CROSS BAR VERTICALITY	±2mm



FOR GREULICH USE ONLY **IKE** GREULICH BRIDCE FLOORING SYSTEMS A Division of Harsco Corporation 16 910 P.O. Box 295, Cheswick, Penneyvania (5024-9401 hane: (412) 828-223 Fax: (412) 828-4103 Shop: (412) 828-6444 GRID: 5" 4wdy OPEN (METRIC) CUSTOMER: LOUIS BRAY CONSTRUCTION FOR. PUBLIC WORKS CANADA PUBLIC WORKS CANADA ENG .: AS NOTED FINAL PROL: BURLINGTON LIFT BRIDGE-NEW GRID DECK 1/19/00 LOCATION: HAMILTON, ONTARIC REL TO FABRICATION 1/19/00 TITLE STANDARD (METRIC) DRAWN: SM 12/15/99 CUST/P.O. NO. IKG JOB NO.
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Public Works Government Services Canada

Architectural and Engineering Services

Ontario Region

Travaux publics Services gouvernementaux Canada

Services d'architecture et de génie

Région de l'Ontario

LIST OF DRAWINGS

BRIDGES
B-1) - R01 (
B-2)- R01
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HIGHWAYS

G-1

ONTARIO. HAMILTON, PWGSC EMERGENCY STEEL PANEL REPLACEMENT

PWGSC Proj. No.: R.012843.055





COVER SHEET GENERAL ARRANGEMENT PART PLANS GRATING AND RAILING POST DETAILS

TRAFFIC CONTROL PLAN AND PAVEMENT MARKING

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GENERAL NOTES:

- STRUCTURAL LAYOUT BASED ON REFERENCE DRAWINGS PROVIDED BY PWGSC.
 TRAFFIC CONTROL DETAILED ELSEWHERE IN THE CONTRACT DOCUMENTS.
- 3. ALL DIMENSIONS IN mm UNLESS NOTED OTHERWISE.

- SCOPE OF WORK: IMPLEMENT ENVIRONMENTAL ENCLOSURE AND REMOVE BIRD GUANO FROM WORK AREA.
- 2. IMPLEMENT TRAFFIC CONTROL.
- REMOVE AND DISPOSE OF EXISTING GRATING PANEL E1 AS IDENTIFIED ELSEWHERE IN THE CONTRACT DOCUMENTS.
- 4. PREPARE EXISTING STRINGERS TO RECEIVE NEW GRATING PANEL.
- 5. FABRICATE, DELIVER AND INSTALL NEW GRATING PANEL ALONG WITH INSTALLATION HARDWARE AND ALL APPURTENANCES. 6. REINSTATE EXISTING RAILING POST BASE ANCHORAGE.
- 7. COAT TOP FLANGE OF STRINGERS AS SPECIFIED ELSEWHERE IN THE CONTRACT DOCUMENTS.
- CONSTRUCTION NOTES:
- LOUNDIRUCTION TOYLES. 1. EXISTING DIRVISIONS ARE PASED ON AVAILABLE REFERENCE DRAWINGS AND ARE NOT INCESSARILY THE SAME AS AS-BUIL DIMENSIONS. THE CONTRACTOR SHALL VERFY ALL DIMENSIONS, DETAILS AND ELEVATIONS OF THE EXISTING STRUCTURE THAT ARE RELEVANT TO THE WORK SHOWN ON THE DRAWINGS PRIOR TO COMMENCEMENT OF THE WORK.
- THE DRAWINGS PHOR TO COMMERCISATION TO THE WORK. 2. THE CONTRACTOR IS ADVESTO TO TAKE GREAT CARE IN MEASURING FOR THE ARRICATION OF THE REPLACEMENT BEFORE PERFORMING ANY ENVOLUTION THE FARICATED PANEL AND ASSOCIATED ANCHORAGE COMPONENTS CAN BE INSTALLED AS SHOWN ON THE CONTRACT DRAWINGS.









DATE PLOTTED: YYYY-MM-DD

PLOT SCALE

<u>PHOTO - 1</u>

GENERAL VIEW OF GRATING PANEL AT BRIDGE C.L.



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20 20 t

TYPICAL WELD DETAIL D

STRUCTURAL STEEL NOTES:

- 1. GRATING PANEL SHALL BE AS SPECIFIED ELSEWHERE IN THE CONTRACT DOCUMENTS.

- CONTRACT DOCUMENTS. 2. ALL OTHER STRUCTURAL STEEL SHALL CONFORM TO CSA STANDARD CAN/CSA G 40.21-04 GRADE 350 WT CAT II. 3. ALL WELDING SHALL BE IN ACCORDANCE WTH CSA W59 UNLESS NOTED OTHERMSE. 4. BOLTS SHALL BE GALVANZED ASTM A325M TYPE 1. 5. ALL FIELD DRILED HOLES SHALL BE COATED WTH A SINGLE COAT OF ZING RIGH PAINT. 6. TOPS DR EVENDLG TEINAGEDS SHALL BE COATED AST
- TOPS OF EXISTING STRINGERS SHALL BE COATED AS SPECIFIED ELSEWHERE IN THE CONTRACT DOCUMENTS.
 UNLESS SPECIFIED OTHERWISE, THE MINIMUM FILLET WELD SHALL BE AS FOLLOWS:

MATERIAL THICKNESS MINIMUM SIZE OF SINGLE WELD OF THICKER PART JOINED (mm) (mm)

TO 12 INCLUSIVE OVER 12 TO 20

8. TRIM BARS SHALL BE FULLY SHOP WELDED TO ENDS OF BEARING BARS AND ARE REQUIRED ON ALL EDDES WHERE EXPOSED ENDS OF BEARING BARS WOLLD OTHERWISE BE PRESENT. TRIM BARS SHALL BE FLUSH WITH THE TOP OF MAIN BEARING BARS.



PHOTO - 3 130x50 COPED PANEL CORNER



<u>PHOTO - 5</u> SOUTH EAST GRATING CORNER



Public Works and Government Services Co Travaux publics et Services gouvernementa	unada ux Canada						
	PLAN						
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ADDED DIMENSIONS	2013-12-31						
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Appendix B

INSPECTION PHOTOGRAPHS



PWGSC PEQ754-161278/001/PWL/R.081864.001



Photo 1: Typical BCLB Grating view with Traffic Control during inspection



Photo 2: Typical picture of grating with spray-on markups taken during inspection



Photo 3: Test Panel E1 (left) and regular panel D (right)



Photo 4: Test Panel detail



Photo 5: Test Panel E1 (left) and regular panel D (right)



Photo 6: Test Panel – Typical M-beam to stringer connection

PWGSC PEQ754-161278/001/PWL/R.081864.001

Inspection Photographs



Photo7: Button Weld crack – typical example



Photo 8: BW crack – typical example



Photo 9: M-beam to stringer fillet weld deterioration and damage to stringer's flange



Photo 10: M-beam to stringer fillet weld – typical crack, full length and depth

PWGSC PEQ754-161278/001/PWL/R.081864.001



Photo 11: Typical warped grating members



Photo 12: Typical broken grating members



Photo 14: Typical Local Grating Failure area



Photo 16: All members within LGF marked with white are affected

PWGSC PEQ754-161278/001/PWL/R.081864.001



Photo 17: LGF – visible grating structural discontinuity along diagonal members



Photo 18: LGF detail – BW crack, full depth, complete connection failure



Photo 19: LGF detail – BW crack, full depth, complete connection failure



Photo 20: LGF area – all members and welds marked with white are affected

PWGSC PEQ754-161278/001/PWL/R.081864.001



Photo 21: Previous Welding Repairs – typical example of failure at Repaired Button Weld



Photo 22: Previous Welding Repairs – typical example of failure between panels



Photo 23: Surface rust on panels – mostly outside of traffic load zone



Photo 24: Surface rust on panels – paint facilitates corrosion of Button Welds



Photo 25: Flaking paint – typical for all lane markings



Photo 26: Debris – typical - mostly at end panels (A, C, D, F)

Appendix C

INSPECTION SUMMARY


PWGSC PEQ754-161278/001/PWL/R.081864.001

Inspection Summary

38

133

SB Lanes							NB Lanes							
	Inner Lane		Outer Lane					Inner Lane		Outer Lane				
					M-beam to	BW crack failures						M-beam to	BW crack failures	
Panel	вw	LGF	BW	LGF	stringer weld	per panel	Panel	вw	LGF	ВW	LGF	stringer weld	per panel	
	cracks	Areas	cracks	Areas	cracks	P - P		cracks	Areas	cracks	Areas	cracks	P - P	
	er dents	/	0.0010	/ eus				er a ento	/ eus	0.0010	/ eus			
D	20	1	18	1		4.40%	Δ	٩					1 04%	
E01	20	-	10	-		0.00%	R01	38		18	1		5.67%	
E01 F02	1		29			3.04%	B01	Q		13			2 23%	
E02	1		25			0.61%	B02	46		21	2		6 70%	
E03	2		15			1 92%		40		21	2		6.49%	
	3		17		1	2 129/		43		21			2,62%	
E05	5		7		3	1 22%	B05	40	1	21	1		7 19%	
E00	4		, 19		8	2 33%	B07			5		1	3.04%	
E07			13		0	1 32%	B08	16	1	36	1	1	5.04%	
E00	11	1	21	1	2	3 24%	B00	30	-	30	1		4.26%	
E03	11	1	33	1	1	3.24%	B10	22		20	2	1	4.20%	
E10 E11	4		1/	1	1	1 / 2%	B10	51	1	20	1		9.02%	
F12	3		17		1	2 03%	B12	23	-	27	-	2	5.02%	
F13	4		34		1	3 85%	B12	12	1	27		2	3.44%	
E13 F14	4		23		2	2 53%	B14	11	1	22		3	3.44%	
E14	1		20		2	2.53%		36		21	1	3	1.46%	
E15	1		20		2	2.13%	B16	30		5	1	۷	4.40%	
E10			50		2	5.93%	B17	12		2	-	2	2.03%	
E17 E18	9 10	1	53	1	5	6 38%		57		0	2	3	6.08%	
E10	10	1	13	1	3	2 9/1%	B10	42		3	1	1	5 17%	
E19 E20	10		10		4	1 22%	B20	42		13	1	2	1.56%	
E20	2		10		1	1.22%	B20	12		20	-	2	4.30%	
E21	2		25		1	2 74%		25		15	2	5	1.05%	
E22	2		23	1	2	2.74%	B22	23		15	2		4.05%	
E24	2		32		2	2 55%	B24	32		20	2	1	5 78%	
E24	2		11		1	1 32%	B25	56		20		2	9.22%	
E25	1		62		2	6 38%	B26	18		7		2	2 53%	
F27			8			0.81%	B27	24		, 27		1	5.17%	
E27	1		26			2.74%	B28	18		13		1	3.14%	
E20	- 8		34	1	6	4.26%	B29					2	1.11%	
E30	3		12	-	3	1.52%	B30	17		8	1	2	2.53%	
E31	2		9			1.11%	B31	36		21		2	5.78%	
E32	2		38			4.05%	B32	55		13		1	6.89%	
E33			44		4	4.46%	B33	55				1	5.57%	
E34	12		41		1	5.37%	B34	13		6			1.93%	
E35	5		44			4.96%	B35	29		8			3.75%	
E36	4		1		4	0.51%	B36	2				2	0.20%	
E37	1		14		3	1.52%	B37	20		7			2.74%	
E38			20	1	1	2.03%	B38	43		10			5.37%	
E39	3		22		2	2.53%	B39	16		11			2.74%	
E40	8		32			4.05%	B40	2		6			0.81%	
E41	14	1	43	1		5.78%	B41	9					0.91%	
E42	11		41		1	5.27%	B42	29		3	1		3.24%	
E43	1		9		1	1.01%	B43	17					1.72%	
E44	2		25			2.74%	B44	2		1		2	0.30%	
E45			1			0.10%	B45	1		1		2	0.20%	
E46	1		5		2	0.61%	B46	6		1		3	0.71%	
E47	3		4		3	0.71%	B47	15		25	1	2	4.05%	
F			5			0.58%	С						0.00%	
Totals	193	4	1104	8	77	2.70%	Totals	1221	4	604	22	56	3.79%	
Total BW cracks SB lanes: 1297								Fotal BW cracks NB lanes: 1825						
Total LO	GF areas	SB lane	s:			12	Total LO	Fotal LGF areas NB lanes: 26						
Total D	M arack	~ .											2122	

Total BW cracks: Total LGF areas:

Total M-beam to stringer weld cracks:

XX - indicates panels requiring immediate repairs due to the worst combination of damages

Appendix D

INSPECTION FINDINGS DRAWINGS







DRWING PATH AND NAME: K:/Proj/1160268/03 Bridge Deck Grating- Urgent Repai/O8 Design and Development/Structura/Working Drawings/1160268-Inspection-1-KEY PLM.dwg DRWMNG Structura/Working - LAST UPDATED: Oct 28, 2016-09:19 FRMTED: oct 28, 2016-09:31

GSC-A4A1 DATE PLOTTED:

PLOT SCALE:



DRAWING PATH AND MAME: Kr\Proj\1160268\03 Bridge Deck Grating- Urgent Repair\08 Design and Development\Structural\Working Drawing*\1160268-Inspection-1-01 to S-32.dwg Maw G. MONI: 2016-15.07 PRINED: OF 277. 2016-15.07



DRAWING PATH AND NAME: K/Pro//160288/03 Bridge Deck Grating- Urgent Repair/OB Design and Development/Structural/Working Drawings/1160268-Inspection-1-01 to S-32.dwg NamMio Ekynol. 2. PRINED: det 27. 2016–1507



DRWING PATH AND NAME: K/Proj/1160288/03 Bridge Deck Grating- Urgent Repair/OB Design and Development/Structural/Working Drawings/1160268-inspection-1-01 to S-32.dwg PRINED: dot 27. 2016-1507 PRINED: dot 27. 2016-1507



DRAWING PATH AND NAME: K:/Pro//1160268/03 Bridge Deck Grating- Ungent Repair/08 Design and Development/Structura/Working Drawings/1160268-Inspection-1-01 to 5-32.dwg NamMis Livrol. 2016-15:07 PRINED: dot 27. 72.06-15:07



DAWNUK PATH AND MAME: Kr\Proj\1160268\03 Bridge Deck Grating- Urgent Repair\08 Design and Development\Structural\Working Drawing*\1160268-Inspection-1-01 to S-32.dwg Mawnee UrX011: 2016-15.07 PRINTED: or 277: 2016-15.07



DRWING PATH AND NAME: K/Proj/1160268/03 Bridge Deck Grating- Urgent Repair/08 Design and Development/Structural/Working Drawings/1160268-Inspection-1-01 to S-32.dwg Namio Livoliz, 2-66 UST Vierbarte: det 27, 2016-15:06 PRINED: det 27, 2016-15:07

GSC-A4A1 DATE PLOTTED:



DRWING PATH AND NAME: K/Proj/1160288/03 Bridge Deck Grating- Urgent Repair/OB Design and Development/Structural/Working Drawings/1160268-inspection-1-01 to S-32.dwg NamMio LAVOID: 2016-1327 UST UST UPDATED: 0ct 27, 2016-13-06 PRINED: 0ct 27, 2016-1327



DRWING PATH AND NAME: K/Proj/1160288/03 Bridge Deck Grating- Urgent Repair/OB Design and Development/Structural/Working Drawings/1160268-inspection-1-01 to S-32.dwg NamMio LAVOID: 2-08 UST UPDATED: 0ct 27, 2016-15:06 PRINED: 0ct 27, 2016-15:07



DRWING PATH AND NAME: K/Proj/1160288/03 Bridge Deck Grating- Urgent Repair/OB Design and Development/Structura/Working Drawings/1160268-Inspection-1-01 to S-32.dwg NamMio LAVOID: 2016-15:05 UST UPDMTED: Get 27, 2016-15:06 PRINED: Get 27, 2016-15:07 PRINED: Get 27, 2016-15:07



DRAWNG PXTH AND NAME: K:/Pno//1160288/03 Bridge Deck Grating- Urgent Repair/08 Design and Development/Structural/Working Drawings/1160268-Inspection-1-01 to 5-33.0wg Names LXVD: 2:-10 PRINED: dd: ZX, 2:016-15:07



DRWING PATH AND NAME: K/Proj/1160268/03 Bridge Deck Grating- Urgent Repair/OB Design and Development/Structura/Working Drawings/1160268-Inspection-1-01 to S-32.dwg NamMio LAVOID: 25-17 UST UPDMTED: 64: 27: 2016-15:06 PRIMED: 64: 27: 2016-15:07



DRAWING PATH AND NAME: K/Proj/1160268/03 Bridge Deck Grating- Urgent Repair/OB Design and Development/Structura/Working Drawings/1160268-Inspection-1-01 to S-32.dwg NamMio LAVOID: 25-12 UST UPDATED: 0ct 27, 2016-15:06 PRINED: dot 27, 2016-15:06





DRAWING PATH AND NAME: K/Proj/1160268/03 Bridge Deck Grating- Urgent Repair/OB Design and Development/Structura/Working Drawings/1160268-Inspection-1-01 to S-32.dwg NamMio LAVOID: 25-14 UST UPDATED: 6ct 27, 2016-15:06 PRINED: 6dt 27, 2016-15:04



DRWING PATH AND NAME: K/Proj/1160268/03 Bridge Deck Grating- Urgent Repair/OB Design and Development/Structura/Working Drawings/1160268-Inspection-1-01 to S-32.dwg NamMio LAVOID: 2-15 UST UPDMTED: 0ct 27, 2016-15:06 PRIMED: 0ct 27, 2016-15:05



DRAWING PATH AND NAME: K/Pro//160288/03 Bridge Deck Grating- Urgent Repair/OB Design and Development/Structural/Working Drawings/1160268-Inspection-1-01 to S-32.dwg Markow LAVOID: 2-16 UST UPDATED: 0ct 27, 2016-15:06 PRINED: dot 27, 2016-15:06



DRAWING PATH AND NAME. K:/Proj/1160288/03 Bridge Deck Grating- Ungent Repair/08 Design and Development/Structural/Working Drawings/1160268-Inspection-1-01 to 5-32.dwg Names LAVID: Act 27, 2016-15:06 PRINED: dot 27, 2016-15:08



CAD FILE:

DRAWING PATH AND NAME: K/Pro//1160268/03 Bridge Deck Grating- Ungent Repair/08 Design and Development/Structura/Working Drawings/1160268-Inspection-1-01 to 5-32.dwg NamMis LAVID: 2-18 UST UPDATED: Get 27, 2016-15:06 PRINED: Get 27, 2016-15:08



DRAWING PATH AND NAME. K:/Proj/1160288/03 Bridge Deck Grating- Ungent Repair/08 Design and Development/Structural/Working Drawings/1160268-Inspection-1-01 to 5-32.dwg Marko Evrol. 2-19 UST UPDATED: Get 27, 2016-15:06 PRINED: Get 27, 2016-15:08



DRAWING PATH AND NAME: K:/Proj/1160288/03 Bridge Deck Grating- Ungent Repair/08 Design and Development/Structural/Working Drawings/1160268-Inspection-1-01 to 5-32.dwg Names LAYOUS - UST UPDATED: Get 27, 2016-15.06 PRINED: Get 27, 2016-15.08



DRAWING PATH AND NAME: K:/Proj/1160268/03 Bridge Deck Grating- Ungent Repair/08 Design and Development/Structural/Working Drawings/1160268-Inspection-1-01 to 5-32.dwg Marking Livrol. 2016-15:06 PRINED: dat 27. 2016-15:08







DRAWNG PXTH AND NAME: KYProjV1160208/03 Bridge Deck Grating- Ungent Repair/08 Design and Development/Structura/Working Drawings/1160268-Inspection-1-01 to 5-32.dwg Name LXVD1: 2-24 PRINED: 64 ZX7. 2016-15:09



DRAWNG PXTH AND NAME: Kr)ProjV1160288/03 Bridge Deck Grating- Ungent Repair/08 Design and Development\Structura\Warking Drawings\1160268-Inspection-1-01 to 5-32.dwg Names LXV012: 2-25: UST UPDATED: Oct 27, 2016-15:06 PRINED: dat 27, 2016-15:09



DRAWING PXTH AND NAME: K:/Pno//1160288/03 Bridge Deck Grating- Urgent Repair/08 Design and Development/Structural/Working Drawings/1160268-Inspection-1-01 to 5-33.0wg Names (XXVD: 2-26 PRINED: dd XX7. 2016-1503







DRAWING PATH AND NAME: K:/Pro//1160268/03 Bridge Deck Grating- Ungent Repair/08 Design and Development/Structural/Working Drawings/1160268-Inspection-1-01 to 5-32.dwg Names LAYOU: 2016-1529 PRINED: dot 27, 2016-1539



DRAWING PATH AND NAME: K/Pro//160288/03 Bridge Deck Grating- Urgent Repair/OB Design and Development/Structural/Working Drawings/1160268-Inspection-1-01 to S-32.dwg Markow LAVOID: 25:00 UST UPDATED: 0ct 27, 2016-15:06 PRINED: dot 27, 2016-15:09



DRAWNG PXTH AND NAME: Kr/Proj/1160288/03 Bridge Deck Grating- Ungent Repair/08 Design and Development/Structural/Working Drawings/1160268-Inspection-1-01 to 5-32.dwg Marko EXVD1: 25-31 UST UPDATED: Oct 27, 2016-15:06 PRINED: dat 27, 2016-15:09



DRAWING PXTH AND NAME: K:/Proj/1160288/03 Bridge Deck Grating- Ungent Repair/08 Design and Development/Structura/Working Drawings/1160268-Inspection-1-01 to 5-32.dwg Name: Dx016-5-22 PRINED: 6d: 27. 2016-15:08
Appendix E

REPAIR OPTIONS





GSC-A4A1 DATE PLOTTED:

PLOT SCALE:

CAD FILE:













GSC-A4A1 DATE PLOTTED:

PLOT SCALE:



Appendix F

COST ESTIMATE



BCLB: Urgent Steel Deck Grating Repairs

Recommended Repairs

Item No.	Item Description	Unit	Quantity	Estimated Unit Price		Total
Pre-Construction Inspection						
1	Traffic Control	hour	17	\$ 110.00	\$	1,870.00
2	Site Inspection by Contractor's Representative	hour	15	\$ 100.00	\$	1,500.00
				Subtotal	\$	3,370.00
				20% Contingency	\$	674.00
			1	Total with Contingency	\$	4,044.00
Repair Item 8.1 Button Welds Cracks - Re-Welding (estimated amount of weld repairs - 4000)						
3	Traffic Control	hour	150	\$ 110.00	\$	16,500.00
4	Site supervision	hour	100	\$ 130.00	\$	13,000.00
5	Weld Repairs	hour	400	\$ 70.00	\$	28,000.00
6	Preparation (locating, grinding, cleaning)	hour	100	\$ 50.00	\$	5,000.00
7	Equipment (pick up + welding arc)	hour	400	\$ 36.00	\$	14,400.00
				Subtotal	\$	76,900.00
				40% Contingency	\$	30,760.00
			٦	Total with Contingency	\$	107,660.00
Repair Item 8.2 M-beam to Stringer Failure - Repair OPTION D - Hold Down						
8	Traffic Control	hour	260	\$ 110.00	\$	28,600.00
9	Site supervision	hour	180	\$ 130.00	\$	23,400.00
10	Deck Surface preparation (locating, cleaning, clamping)	hour	350	\$ 50.00	\$	17,500.00
11	Weld Repairs	hour	700	\$ 70.00	\$	49,000.00
12	Material (4 No steel plates per 1 cracked weld)	kg	330	\$ 2.00	\$	660.00
13	Equipment (pick up + welding arc)	hour	700	\$ 36.00	\$	25,200.00
				Subtotal	\$	144,360.00
40% Contingency					\$	57,744.00
			1	Total with Contingency	\$	202,104.00
Repair Item 8.3 Local Grating Failure Areas - Re-Welding with Solid Round Bars (each - approx. 1m2)						
14	Traffic Control	hour	130	\$ 110.00	\$	14,300.00
15	Site supervision	hour	90	\$ 130.00	\$	11,700.00
16	Surface Preparation (locating, grinding, cleaning, clamping)	hour	175	\$ 50.00	\$	8,750.00
17	Weld Repairs	hour	350	\$ 70.00	\$	24,500.00
18	Material (round steel bars)	kg	100	\$ 2.00	\$	200.00
19	Equipment (pick up + welding arc)	hour	350	\$ 36.00	\$	12,600.00
Subtotal					\$	72,050.00
40% Contingency					\$	28,820.00
Total with Contingency					\$	100,870.00
Grand Total with Contingency					\$	414,678.00

Class 'C' Cost Estimate

April 2017