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PUBLIC SERVICES AND PROCUREMENT CANADA

# CATTLE AND GOAT BARN PROJECT JOYCEVILLE, ONTARIO

JANUARY 10, 2022

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# CATTLE AND GOAT BARN PROJECT JOYCEVILLE, ONTARIO

PUBLIC SERVICES AND PROCUREMENT  
CANADA

STORMWATER MANAGEMENT MEMORANDUM (REVISION 00)  
COPY

PROJECT NO.: OUR REF. NO. 191-08574-02  
CLIENT REF: R.100644.001  
DATE: OCTOBER 15, 2021

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# 1 INTRODUCTION

WSP Canada Inc. (WSP) was retained by Public Services and Procurement Canada (PSPC) to complete a Stormwater Management report for two (2) new livestock barns, one (1) for cattle and one (1) for goats at Joyceville Minimum Institution. The purpose of this report is to summarize the stormwater management measures required for the design and construction of the two (2) new livestock barns.

## 2 EXISTING LAND USE

The proposed site for the new Joyceville Institution Cattle and Goat Barns consists of seven (7) existing buildings and is located approximately 20 kilometers north-east of Kingston, Ontario. The usage of the existing buildings are an Implement Shed, Heavy Equipment Shed, Dairy Cattle Barn, Storage Shed, Farm Manager's House, Old Farm Building, and a Hay Storage Structure. The proposed site for the construction of the two (2) barns is considered a brownfield site (land available for reuse).

The proposed facilities will be designed as industrial occupancy and farm building occupancy. It is bordered by CSC – Joyceville Institution to the north, an existing quarry to the east, open greenspace to the south and Highway 15 to the west.

Access to the site is currently provided by an existing entrance on Provincial Highway 15. The site is located in the Countryside District and does not contain any municipal services (i.e. sanitary sewers or watermains).

## 3 PROPOSED DEVELOPMENT

The proposed development includes the construction of two (2) new livestock barns. The proposed barns are configured to have office spaces, holding areas, parlours, nurseries, and other ancillary spaces. One (1) barn will be a Goat Barn with an area requirement of 6,099 m<sup>2</sup>. The Goat Barn's intended five (5) year occupancy is to maintain a milking goat herd of roughly 1,500 goats. The second barn will be a Cattle Barn with an area requirement of 3,089m<sup>2</sup>. The Cattle Barn will be designed to accommodate approximately 70 Cattle as a milking herd. The Cattle Barn will also incorporate a milking parlour in the barn to allow for the future expansion to the herd.

### 3.1 STORMWATER MANAGEMENT & SITE GRADING

With the construction of two (2) new structures there will be an increase in impervious area for the site post-development. To ensure that post-development flows are equal to or less than pre-development flows, on-site stormwater storage will be included in the development. The overall grading of the site will generally match existing conditions. Localized grading adjacent to new structures will ensure all stormwater is directed away from the finished floor elevations. Swales and road-side ditches will be incorporated to capture and convey stormwater to a dry stormwater management retention area complete with an oil/grit separator. The oil/grit separator operations and maintenance should be as per the supplier's recommendations. The stormwater will ultimately outlet to the existing northeast bound roadside ditch along Highway 15.

HydroCAD 10.00 Stormwater Modelling software was used to calculate the pre and post-development flow conditions for the 5-year (minor) and 100-year (major) return period design storm events. The software uses the Modified Rationale Method and has the ability to solve for critical storm durations taking into consideration peak flow from catchments with differing time of concentrations, as well as retention areas outlet hydraulics.

The following Tables 3-1 and 3-2 detail the pre- and post-development areas and flows for the site. Pre-development time of concentration for the site was calculated to be 91 minutes. Post-development time of concentration was calculated using an initial 15 minutes for runoff to enter the ditching system, followed by an additional 12.2 minutes

of time for the water to route to the stormwater retention area through the ditching network. Refer to Appendix A for detailed calculations.

**Table 3-1 Land Use and Runoff Coefficients**

		Pre-Development		Post-Development	
Lane Use		Runoff Coefficient	Area (m2)	Runoff Coefficient	Area (m2)
<b>Granular</b>		0.80	9,275	0.80	26,264
<b>Asphalt/Concrete</b>		0.90	6,852	0.90	8,197
<b>Manure Tank*</b>		0.00	0	0.00	1,513
<b>Structures</b>		0.90	4,098	0.90	14,237
<b>Grassed</b>		0.25	172,570	0.25	142,656
<b>Weighted Coefficient and Total Area</b>		0.31	192,867	0.40	192,867

\*The manure tanks have been assigned a runoff coefficient of “0” as rainfall which falls within the tanks stays within manure handling process and not directed to the on-site storm system.

**Table 3-2 Pre- and Post-Development Peak Stormwater Flows**

		Pre-Development		Post-Development (Uncontrolled)	
Design Storm		1:5 Year	1:100 Year	1:5 Year	1:100 Year
<b>Weighted Coefficient</b>		0.31	0.39	0.40	0.50
<b>Peak Flow</b>		342 L/s	718 L/s	1,025 L/s	2,141 L/s

The dry stormwater management retention area has been designed with an outlet structure comprised of a 450mm HDPE outlet pipe and a 5.0m wide clay weir lined with Rip-Rap. The release of post-development flows from the retention area have been restricted to the capacity of the new private outlet ditch, as it was found to be the governing factor for the site's allowable stormwater release (less than 100-year pre-development flow rate). Refer to Table 3-3 for comparison of receiving ditch capacities and proposed controlled peak site release rates. Refer to Appendix A for detailed stormwater calculations and Appendix B for HydroCAD model outputs.

**Table 3-3 Ditch Capacity vs. Post-Development Peak Runoff (Controlled)**

Ditch Capacity		Post-Development Peak Runoff (Controlled)		SWM Facility - Peak Ponding	
Private Outlet Ditch	Highway 15 Ditch	1:5 Year*	1:100 Year**	1:5 Year***	1:100 Year****
565 L/s	5,213 L/s	193 L/s	471 L/s	123.48m	123.73m

\* Relative to the outlet's critical storm duration of 93 mins.

\*\* Relative to the outlet's critical storm duration of 132 mins.

\*\*\* Relative to the pond's critical storm duration of 114 mins.

\*\*\*\* Relative to the pond's critical storm duration of 123 mins.

Ditches throughout the development were designed to convey major flows. Culverts throughout the site were designed such that overtopping of the embankment (i.e. laneways) will occur under major events. Spill elevation for

the embankments have been graded such that desired flow routing is maintained and ponding elevations do not encroach up to building envelopes. Refer to Appendix A for detailed ditching calculations and HY-8 model outputs.

The dry stormwater management retention area has been designed to ensure best management practices for quality control. An oil/grit separator device is proposed to provide quality treatment to stormwater released from the retention facility. The device shall be size to provide normal-level protection (70% TSS Removal) in accordance with the Ministry of the Environment, Conservation, and Parks' (MECP) Stormwater Management Guidelines. The device shall also be sized based on Fine Distribution for particle size distribution. As the site is designed with shallow slope ditches, additional water quality will be obtained prior to the quality unit.

## 4 SUMMARY

We trust that this Stormwater Management Memorandum is suitable to support the proposed development of Joyceville Cattle and Goat Barns.

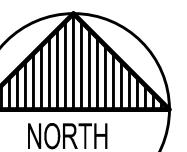
If you require any additional information or have any comments or concerns please do not hesitate to contact our office.

# A STORMWATER CALCULATIONS AND FIGURES

WSP

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CONSULTANT:



SEAL:

JOYCEVILLE INSTITUTION  
CLIENT REF #: 20162022  
PROJECT:

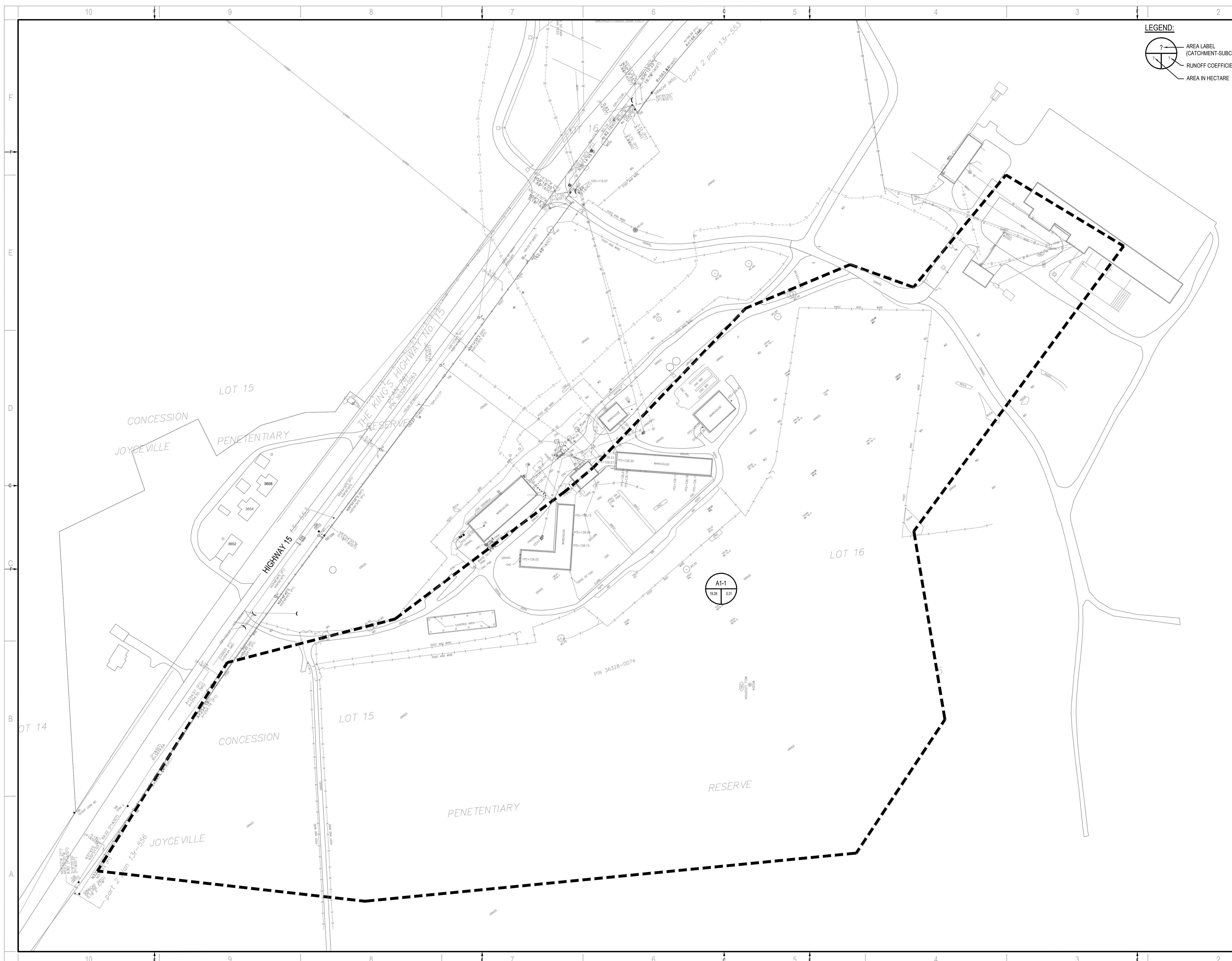
CSC CATTLE AND GOAT BARN

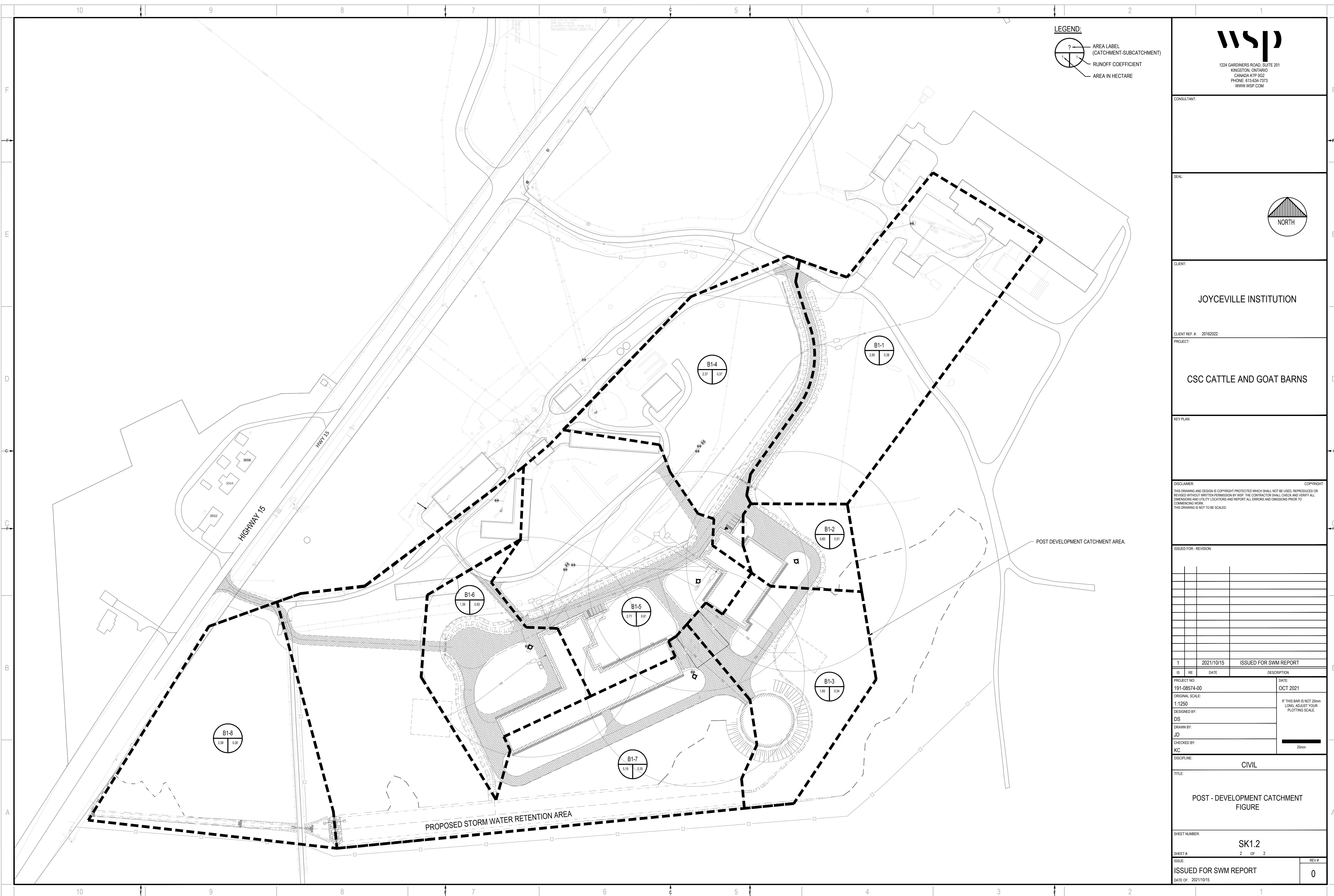
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ISSUED FOR REVISION:

IS	RE	DATE	DESCRIPTION
PROJECT NO:		DATE:	OCT 2021
191-06574-00			
ORIGINAL SCALE:			
1:1250			
DESIGNED BY:			
DS			
DRAWN BY:			
JD			
CHECKED BY:			
KC			
DISCIPLINE:			CIVIL
TITLE:			
PRE - DEVELOPMENT CATCHMENT FIGURE			
SHEET NUMBER:			
SK1.1			
SHEET #:	1	OF	2
ISSUE:			
REV #:			
0			
ISSUED FOR SWM REPORT			
DATE OF:	2021/10/15		





**Table A1 - Land Use**

**PRE-DEVELOPMENT (ENTIRE SITE)**

AREA No.	AREA (m <sup>2</sup> )	RUNOFF COEFFICIENT	
			LAND USE
	9275	0.80	Gravel
	6852	0.90	Asphalt, Concrete
	4098	0.90	Roof Areas
	172642	0.25	Grassed Area
A1-1	<b>192867</b>	<b>0.31</b>	<b>Total</b>

**POST-DEVELOPMENT (ENTIRE SITE)**

AREA No.	AREA (m <sup>2</sup> )	RUNOFF COEFFICIENT	
			LAND USE
	26264	0.80	Gravel
	8197	0.90	Asphalt, Concrete
	1513	0.00	Manure Tank
	14237	0.90	Roof Areas
	142656	0.25	Grassed Area
B	<b>192867</b>	<b>0.40</b>	<b>Total</b>

**Pre-Development Time of Concentration (Tc)**

$$\frac{3.26 \times (1.1 - c) \times L^{0.5}}{S_w^{0.33}}$$

1 - AIRPORT FORMULA  
 (WHERE "C" IS LESS THAN 0.4)

$$\begin{aligned} c &= 0.31 \\ L &= 792 \text{ m} \\ S_w &= 0.5 \% \end{aligned}$$

Pre-Development T<sub>c</sub> = 90.9 mins

### POST-DEVELOPMENT (SUBCATCHMENT BREAKDOWN)

#### POST-DEVELOPMENT (B1-1)

AREA No.	AREA (m <sup>2</sup> )	COEFFICIENT	LAND USE
	5181	0.80	Gravel
	0	0.90	Asphalt, Concrete
	0	0.00	Manure Tank
	1539	0.90	Roof Areas
	22819	0.25	Grassed Area
<b>B1-1</b>	<b>29540</b>	<b>0.38</b>	<b>Total</b>

#### POST-DEVELOPMENT (B1-2)

AREA No.	AREA (m <sup>2</sup> )	COEFFICIENT	LAND USE
	1347	0.80	Gravel
	46	0.90	Asphalt, Concrete
	0	0.00	Manure Tank
	1175	0.90	Roof Areas
	3446	0.25	Grassed Area
<b>B1-2</b>	<b>6014</b>	<b>0.51</b>	<b>Total</b>

#### POST-DEVELOPMENT (B1-3)

AREA No.	AREA (m <sup>2</sup> )	COEFFICIENT	LAND USE
	2254	0.80	Gravel
	70	0.90	Asphalt, Concrete
	1239	0.00	Manure Tank
	809	0.90	Roof Areas
	12577	0.25	Grassed Area
<b>B1-3</b>	<b>16948</b>	<b>0.34</b>	<b>Total</b>

#### POST-DEVELOPMENT (B1-4)

AREA No.	AREA (m <sup>2</sup> )	COEFFICIENT	LAND USE
	3290	0.80	Gravel
	268	0.90	Asphalt, Concrete
	0	0.00	Manure Tank
	1249	0.90	Roof Areas
	18874	0.25	Grassed Area
<b>B1-4</b>	<b>23680</b>	<b>0.37</b>	<b>Total</b>

**POST-DEVELOPMENT (B1-5)**

AREA No.	AREA (m <sup>2</sup> )	COEFFICIENT	LAND USE
	3775	0.80	Gravel
	7307	0.90	Asphalt, Concrete
	0	0.00	Manure Tank
	4356	0.90	Roof Areas
	11650	0.25	Grassed Area
B1-5	<b>27088</b>	<b>0.61</b>	<b>Total</b>

**POST-DEVELOPMENT (B1-6)**

AREA No.	AREA (m <sup>2</sup> )	COEFFICIENT	LAND USE
	4917	0.80	Gravel
	289	0.90	Asphalt, Concrete
	0	0.00	Manure Tank
	1561	0.90	Roof Areas
	7099	0.25	Grassed Area
B1-6	<b>13865</b>	<b>0.53</b>	<b>Total</b>

**POST-DEVELOPMENT (B1-7)**

AREA No.	AREA (m <sup>2</sup> )	COEFFICIENT	LAND USE
	4987	0.80	Gravel
	217	0.90	Asphalt, Concrete
	274	0.00	Manure Tank
	3550	0.90	Roof Areas
	42891	0.25	Grassed Area
B1-7	<b>51919</b>	<b>0.35</b>	<b>Total</b>

**POST-DEVELOPMENT (B1-8)**

AREA No.	AREA (m <sup>2</sup> )	COEFFICIENT	LAND USE
	513	0.80	Gravel
	0	0.90	Asphalt, Concrete
	0	0.00	Manure Tank
	0	0.90	Roof Areas
	23300	0.25	Grassed Area
B1-7	<b>23813</b>	<b>0.26</b>	<b>Total</b>

**STORM DITCH DESIGN SHEET (5-YEAR)**

Q - 2.78(AC) peak flow in litres per second (L/s)

A - area in hectares (ha)

I - rainfall intensity in millimetres per hour (mm/hr)

C - runoff coefficient

$$i = 27.7(Tc/60)^{0.699}$$

$$\text{Peak Flow} = 2.78 \text{ AC} * i$$

$$\text{Capacity} = (1/n)^t A * R^{2/3} * S^{1/1}$$

$$V = (1/n)^t R^{2/3} * S^{1/2}$$

$$\text{Time of Flow} = (L/V)/60$$

LOCATION	AREAS	FROM	TO	AREA (ha)	RUNOFF COEFFICIENT	INDIV. EQUIV. AREA 2.78*(AC)	ACCUM. EQUIV. AREA 2.78*(AC)	TIME OF CONC. (min)	RAINFALL INTENSITY (mm/hr)	PEAK FLOW (l/s)	DITCH DATA													
											BOTTOM WIDTH (m)	SIDE SLOPES		SIDE SLOPES LENGTHS		FLOW DEPTH (m)	WETTED PERIMETER (m)	FLOW AREA (m²)	R (FA/WP)	SLOPE (%)	LENGTH (m)	CAPACITY (l/s) n= 0.035	VELOCITY (m/s)	TIME OF FLOW (min)
												RIGHT	LEFT	RIGHT (m)	LEFT (m)									
Ditching	B1-1			2.954	0.380	3.123	3.123	15.00	72.999	228.00	0.00	3	3	2.34	2.34	0.740	4.68	1.64	0.35	0.75	166.00	2022.69	1.2312	2.25
Ditching	B1-4			2.368	0.368	2.423	2.423	15.00	72.999	176.86	2.00	3	3	0.95	0.95	0.300	3.90	0.87	0.22	0.22	64.00	429.04	0.493	2.16
Ditching	B1-2			0.601	0.505	0.845	6.391	17.25	66.213	423.14	0.00	3	3	2.75	2.75	0.870	5.50	2.27	0.41	0.16	178.00	1438.44	0.6335	4.68
Ditching	B1-3			1.695	0.339	1.595	7.986	21.93	55.978	447.04	0.00	3	3	2.75	2.75	0.870	5.50	2.27	0.41	0.16	200.00	1438.44	0.6335	5.26
Ditching	B1-5			2.709	0.607	4.567	4.567	15.00	72.999	333.41	2.00	3	3	1.52	1.52	0.480	5.04	1.65	0.33	0.20	157.00	1003.23	0.608	4.31
Ditching	B1-6			1.387	0.532	2.050	6.617	19.31	61.192	404.90	2.00	3	3	1.83	1.83	0.580	5.67	2.17	0.38	0.20	192.50	1460.99	0.674	4.76
Pond	B1-7	POND	POND	5.192	0.349	5.033	19.635	27.19	48.165	945.74														
Private Outlet Ditch*	B1-8	POND	OUTLET	2.381	0.262	REFER TO HYDROCAD OUTPUT				193.00	2.50	3	3	1.11	1.11	0.350	4.71	1.24	0.26	0.15	67.00	565.25	0.45	2.45
HWY 15 Ditch**										193.00	0.00	3	3	3.16	3.16	1.000	6.32	3.00	0.47	1.00		5213.32	1.74	

\*TOTAL PEAK FLOW RELATIVE TO COMBINATION OF POND OUTLET AND B1-8 RUNOFF @ SITE OUTLET'S 5-YEAR CRITICAL DURATION (93 mins)

\*\*CAPACITY CHECK OF EXISTING RECEIVING DITCH RELATIVE TO PEAK SITE RELEASE

**STORM DITCH DESIGN SHEET (100-YEAR)**

Q - 2.78(AC) peak flow in litres per second (L/s)

A - area in hectares (ha)

I - rainfall intensity in millimetres per hour (mm/hr)

C - runoff coefficient

$$i = 46.1(Tc/60)^{0.699}$$

$$\text{Peak Flow} = 2.78 \text{ AC} * I^{(1.25)}$$

$$\text{Capacity} = (1/n)^t A * R^{2/3} * S^{1/1}$$

$$V = (1/n)^t R^{2/3} * S^{1/2}$$

$$\text{Time of Flow} = (L/V)/60$$

Additional 25% allocated for the 100-year only.

LOCATION	AREAS	FROM	TO	AREA (ha)	RUNOFF COEFFICIENT	INDIV. EQUIV. AREA 2.78*(AC)	ACCUM. EQUIV. AREA 2.78*(AC)	TIME OF CONC. (min)	RAINFALL INTENSITY (mm/hr)	PEAK FLOW (l/s)	DITCH DATA													
											BOTTOM WIDTH (m)	SIDE SLOPES		SIDE SLOPES LENGTHS		FLOW DEPTH (m)	WETTED PERIMETER (m)	FLOW AREA (m²)	R (FA/WP)	SLOPE (%)	LENGTH (m)	CAPACITY (l/s) n= 0.035	VELOCITY (m/s)	TIME OF FLOW (min)
												RIGHT	LEFT	RIGHT (m)	LEFT (m)									
Ditching	B1-1			2.954	0.380	3.123	3.123	15.00	121.490	474.31	0.00	3	3	2.34	2.34	0.740	4.68	1.64	0.35	0.75	166.00	2022.69	1.2312	2.25
Ditching	B1-4			2.368	0.368	2.423	2.423	15.00	121.490	367.93	2.00	3	3	0.95	0.95	0.300	3.90	0.87	0.22	0.22	64.00	429.04	0.493	2.16
Ditching	B1-2			0.601	0.505	0.845	6.391	17.25	110.196	880.27	0.00	3	3	2.75	2.75	0.870	5.50	2.27	0.41	0.16	178.00	1438.44	0.6335	4.68
Ditching	B1-3			1.695	0.339	1.595	7.986	21.93	93.162	929.98	0.00	3	3	2.75	2.75	0.870	5.50	2.27	0.41	0.16	200.00	1438.44	0.6335	5.26
Ditching	B1-5			2.709	0.607	4.567	4.567	15.00	121.490	693.60	2.00	3	3	1.52	1.52	0.480	5.04	1.65	0.33	0.20	157.00	1003.23	0.608	4.31
Ditching	B1-6			1.387	0.532	2.050	6.617	19.31	101.840	842.33	2.00	3	3	1.83	1.83	0.580	5.67	2.17	0.38	0.20	192.50	1460.99	0.674	4.76
Pond	B1-7	POND	POND	5.192	0.349	5.033	19.635	27.19	80.159	1967.44														
Private Outlet Ditch*	B1-8	POND	OUTLET	2.381	0.262	REFER TO HYDROCAD OUTPUT				474.00	2.50	3	3	1.11	1.11	0.350	4.71	1.24	0.26	0.15	67.00	565.25	0.45	2.45
HWY 15 Ditch**										474.00	0.00	3	3	3.16	3.16	1.000	6.32	3.00	0.47	1.00		5213.32	1.74	

\*TOTAL PEAK FLOW RELATIVE TO COMBINATION OF POND OUTLET AND B1-8 RUNOFF @ SITE OUTLET'S 100-YEAR CRITICAL DURATION (132 mins)

\*\*CAPACITY CHECK OF EXISTING RECEIVING DITCH RELATIVE TO PEAK SITE RELEASE

# **HY-8 Culvert Analysis Report**

## **Project Notes**

Project Title:CSC Cattle & Goat Barns - New Facilities (191-08574-00)

Designer:Daniel Searle, P.Eng.

Project Date:Wednesday, October 13, 2021

Notes:

## **Project Units: SI Units (Metric)**

## **Crossing Discharge Data**

Discharge Selection Method: Recurrence

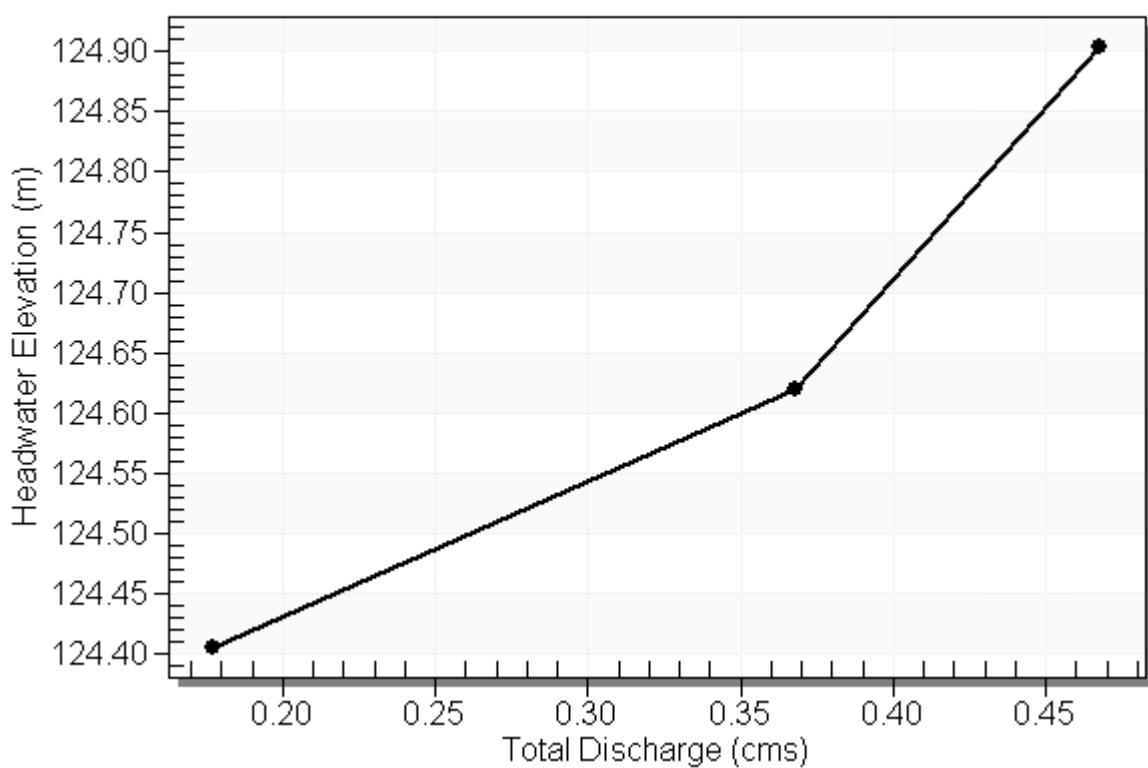
## CULVERT B1-4 (OUTLET)

**Table 1 - Summary of Culvert Flows at Crossing: B1-4 (Outlet)**

Headwater Elevation (m)	Discharge Names	Total Discharge (cms)	2 x 450 Discharge (cms)	Roadway Discharge (cms)	Iterations
124.41	5 year	0.18	0.18	0.00	1
124.62	100 year	0.37	0.37	0.00	1
124.76	Overtopping	0.47	0.47	0.00	Overtopping

**Rating Curve Plot for Crossing: B1-4 (Outlet)**

**Total Rating Curve**  
Crossing: B1-4 (Outlet)



**Table 2 - Culvert Summary Table: 2 x 450**

Discharge Names	Total Discharge (cms)	Culvert Discharge (cms)	Headwater Elevation (m)	Inlet Control Depth (m)	Outlet Control Depth (m)	Flow Type	Normal Depth (m)	Critical Depth (m)	Outlet Depth (m)	Tailwater Depth (m)	Outlet Velocity (m/s)
5 year	0.18	0.18	124.41	0.316	0.229	1-S2n	0.165	0.206	0.165	0.354	1.626
100 year	0.37	0.37	124.62	0.530	0.491	5-JS1f	0.251	0.301	0.450	0.466	1.157

\*\*\*\*\*  
Straight Culvert

Inlet Elevation (invert): 124.09 m, Outlet Elevation (invert): 123.92 m

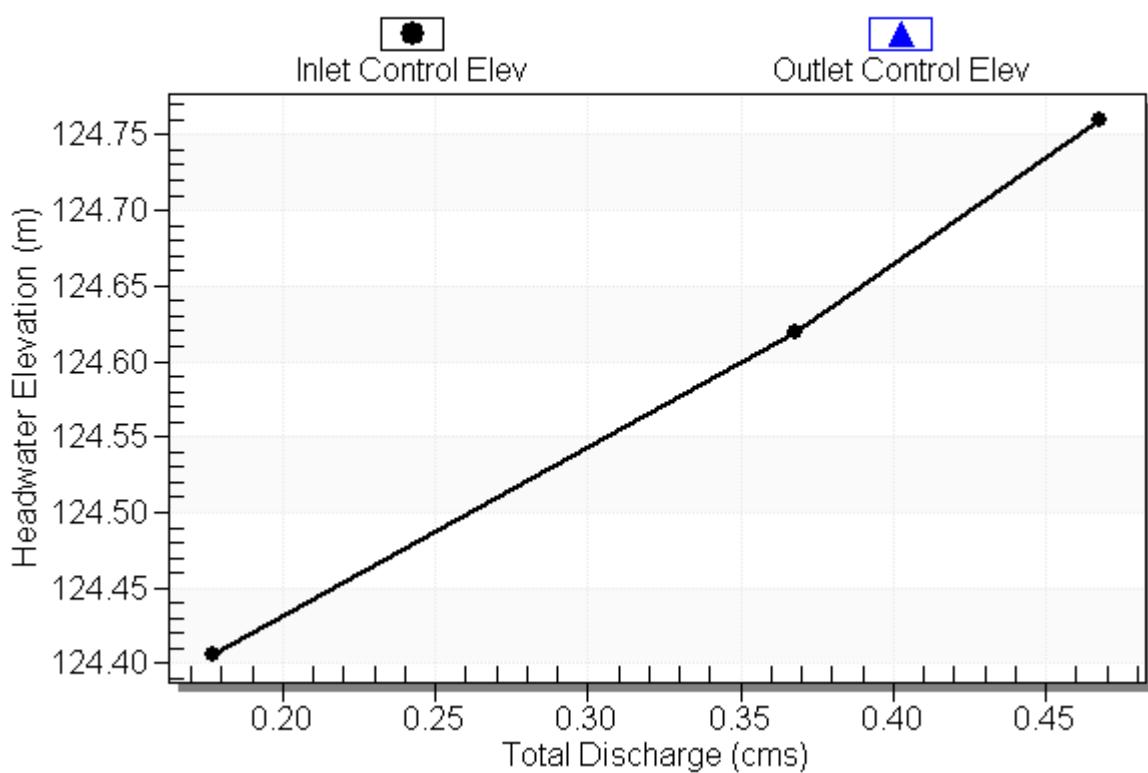
Culvert Length: 18.70 m, Culvert Slope: 0.0091

\*\*\*\*\*

## Culvert Performance Curve Plot: 2 x 450

### Performance Curve

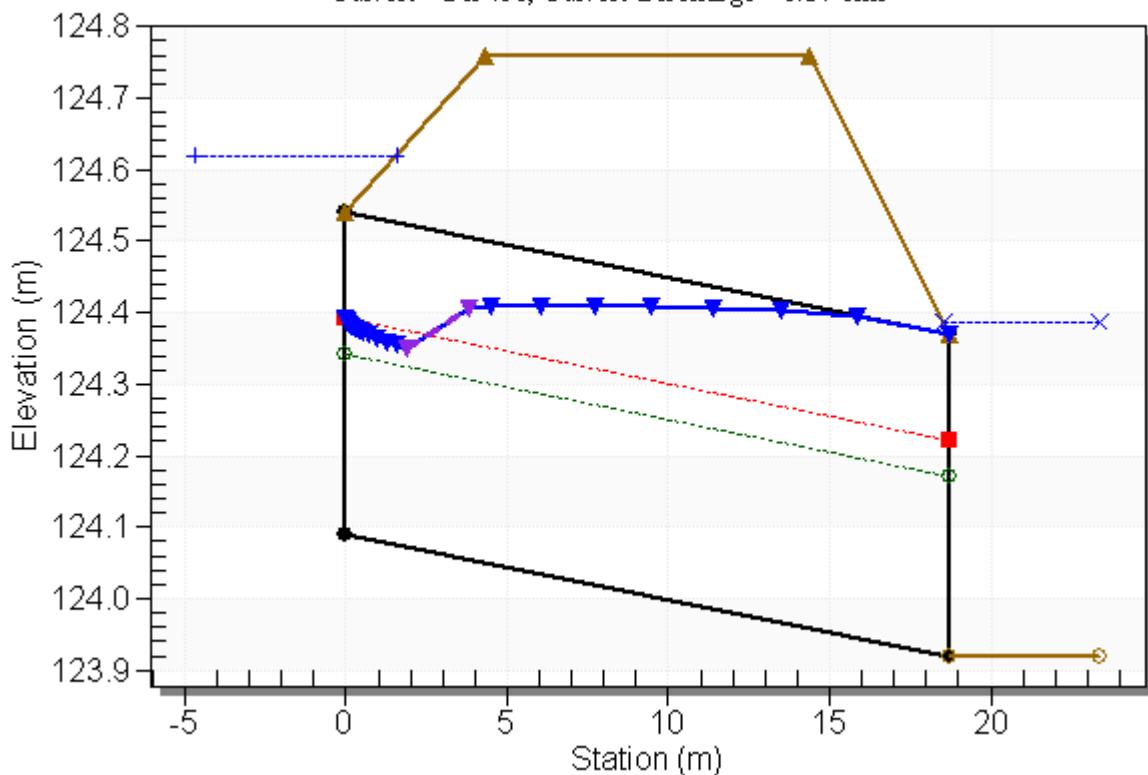
Culvert: 2 x 450



## Water Surface Profile Plot for Culvert: 2 x 450

Crossing - B1-4 (Outlet), Design Discharge - 0.37 cms

Culvert - 2 x 450, Culvert Discharge - 0.37 cms



## Site Data - 2 x 450

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 m

Inlet Elevation: 124.09 m

Outlet Station: 18.70 m

Outlet Elevation: 123.92 m

Number of Barrels: 2

## Culvert Data Summary - 2 x 450

Barrel Shape: Circular

Barrel Diameter: 450.00 mm

Barrel Material: Smooth HDPE

Embedment: 0.00 mm

Barrel Manning's n: 0.0120

Culvert Type: Straight

Inlet Configuration: Thin Edge Projecting

Inlet Depression: None

**Table 3 - Downstream Channel Rating Curve (Crossing: B1-4 (Outlet))**

Flow (cms)	Water Surface Elev (m)	Depth (m)	Velocity (m/s)	Shear (Pa)	Froude Number
0.18	124.27	0.35	0.47	5.20	0.36
0.37	124.39	0.47	0.57	6.84	0.37

### **Tailwater Channel Data - B1-4 (Outlet)**

Tailwater Channel Option: Triangular Channel

Side Slope (H:V): 3.00 (\_:1)

Channel Slope: 0.0015

Channel Manning's n: 0.0250

Channel Invert Elevation: 123.92 m

### **Roadway Data for Crossing: B1-4 (Outlet)**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 10.00 m

Crest Elevation: 124.76 m

Roadway Surface: Gravel

Roadway Top Width: 10.00 m

## CULVERT B1-5 (OUTLET)

### Crossing Discharge Data

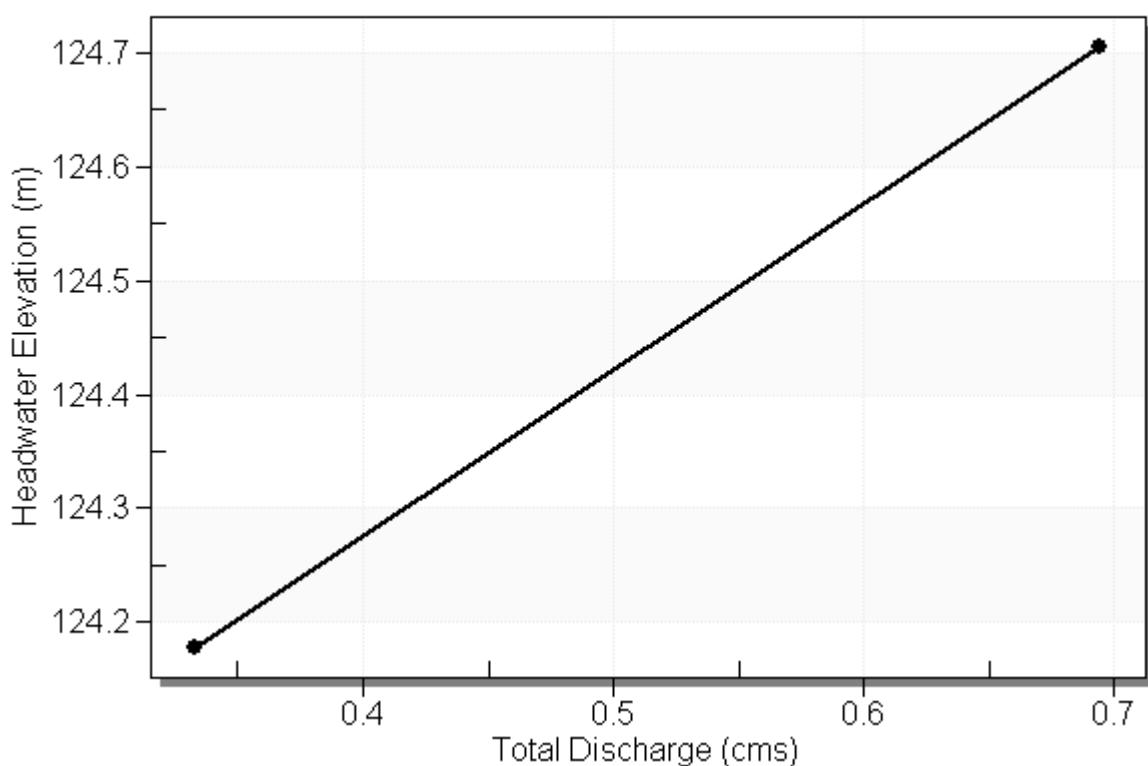
Discharge Selection Method: Recurrence

**Table 4 - Summary of Culvert Flows at Crossing: B1-5 (Outlet)**

Headwater Elevation (m)	Discharge Names	Total Discharge (cms)	2 x 450 Discharge (cms)	Roadway Discharge (cms)	Iterations
124.18	5 year	0.33	0.33	0.00	1
124.71	100 year	0.69	0.60	0.10	11
124.67	Overtopping	0.58	0.58	0.00	Overtopping

**Rating Curve Plot for Crossing: B1-5 (Outlet)**

**Total Rating Curve**  
Crossing: B1-5 (Outlet)



**Table 5 - Culvert Summary Table: 2 x 450**

Discharge Names	Total Discharge (cms)	Culvert Discharge (cms)	Headwater Elevation (m)	Inlet Control Depth (m)	Outlet Control Depth (m)	Flow Type	Normal Depth (m)	Critical Depth (m)	Outlet Depth (m)	Tailwater Depth (m)	Outlet Velocity (m/s)
5 year	0.33	0.33	124.18	0.521	0.529	7-M2c	0.425	0.291	0.291	0.223	1.611
100 year	0.69	0.60	124.71	1.056	1.008	7-M2c	0.425	0.380	0.380	0.332	2.237

\*\*\*\*\*  
Straight Culvert

Inlet Elevation (invert): 123.65 m, Outlet Elevation (invert): 123.62 m

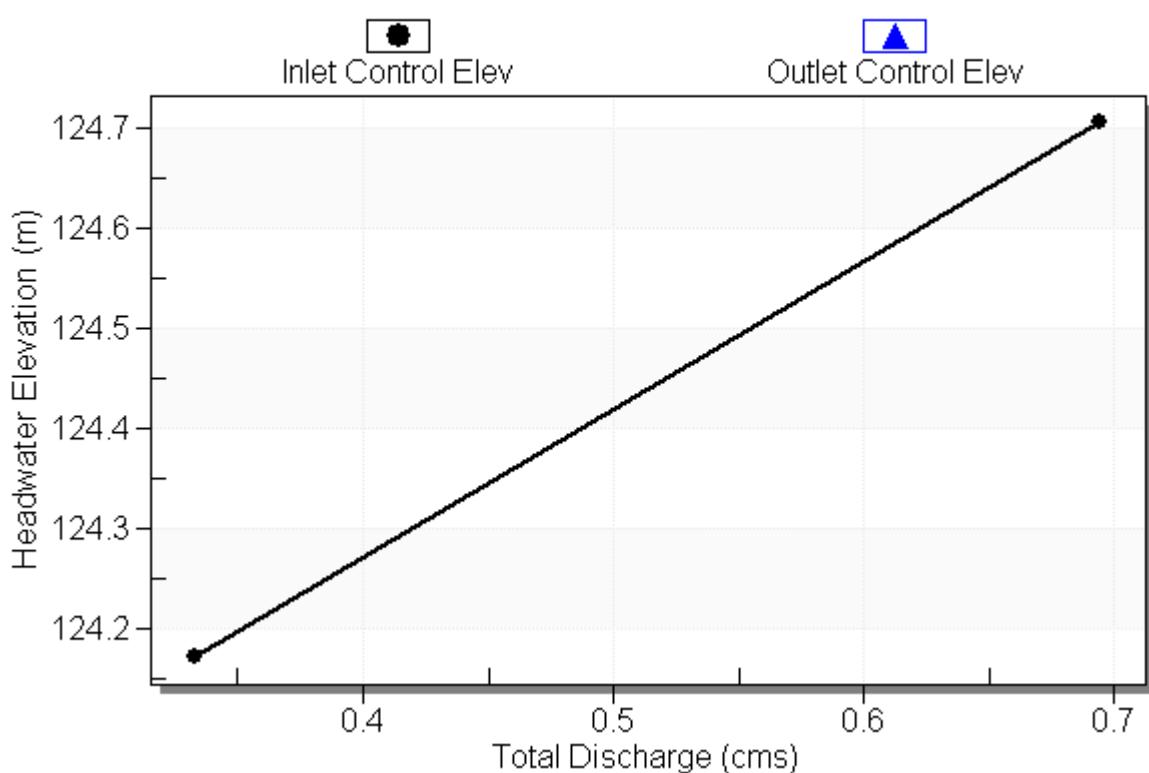
Culvert Length: 16.00 m, Culvert Slope: 0.0019

\*\*\*\*\*

**Culvert Performance Curve Plot: 2 x 450**

**Performance Curve**

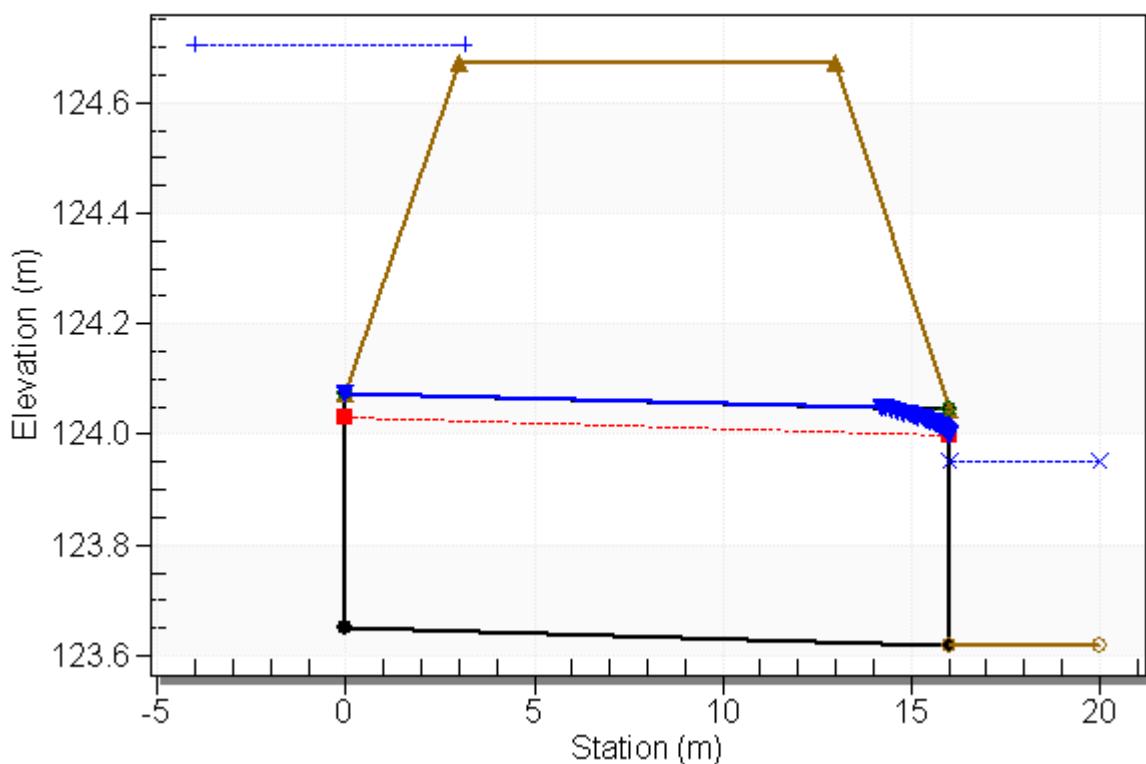
Culvert: 2 x 450



## Water Surface Profile Plot for Culvert: 2 x 450

Crossing - B1-5 (Outlet), Design Discharge - 0.69 cms

Culvert - 2 x 450, Culvert Discharge - 0.60 cms



## Site Data - 2 x 450

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 m

Inlet Elevation: 123.65 m

Outlet Station: 16.00 m

Outlet Elevation: 123.62 m

Number of Barrels: 2

## Culvert Data Summary - 2 x 450

Barrel Shape: Circular

Barrel Diameter: 425.00 mm

Barrel Material: Smooth HDPE

Embedment: 0.00 mm

Barrel Manning's n: 0.0120

Culvert Type: Straight

Inlet Configuration: Thin Edge Projecting

Inlet Depression: None

**Table 6 - Downstream Channel Rating Curve (Crossing: B1-5 (Outlet))**

Flow (cms)	Water Surface Elev (m)	Depth (m)	Velocity (m/s)	Shear (Pa)	Froude Number
0.33	123.84	0.22	0.56	4.38	0.42
0.69	123.95	0.33	0.70	6.52	0.45

### **Tailwater Channel Data - B1-5 (Outlet)**

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 2.00 m

Side Slope (H:V): 3.00 (\_:1)

Channel Slope: 0.0020

Channel Manning's n: 0.0250

Channel Invert Elevation: 123.62 m

### **Roadway Data for Crossing: B1-5 (Outlet)**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 10.00 m

Crest Elevation: 124.67 m

Roadway Surface: Gravel

Roadway Top Width: 10.00 m

## CULVERT B1-6 (MIDPOINT)

### Crossing Discharge Data

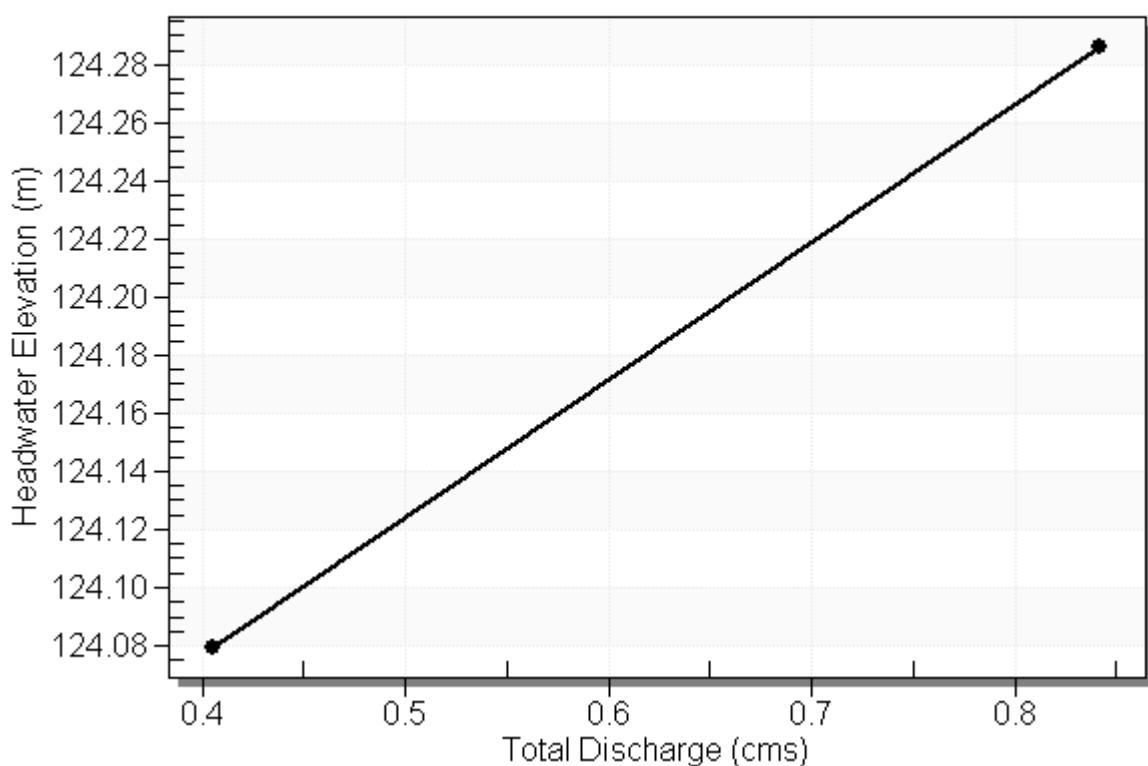
Discharge Selection Method: Recurrence

**Table 7 - Summary of Culvert Flows at Crossing: B1-6 (MidPoint)**

Headwater Elevation (m)	Discharge Names	Total Discharge (cms)	2 X 450 Discharge (cms)	Roadway Discharge (cms)	Iterations
124.08	5 year	0.40	0.40	0.00	1
124.29	100 year	0.84	0.54	0.31	6
124.21	Overtopping	0.49	0.49	0.00	Overtopping

**Rating Curve Plot for Crossing: B1-6 (MidPoint)**

**Total Rating Curve**  
Crossing: B1-6 (MidPoint)



**Table 8 - Culvert Summary Table: 2 X 450**

Discharge Names	Total Discharge (cms)	Culvert Discharge (cms)	Headwater Elevation (m)	Inlet Control Depth (m)	Outlet Control Depth (m)	Flow Type	Normal Depth (m)	Critical Depth (m)	Outlet Depth (m)	Tailwater Depth (m)	Outlet Velocity (m/s)
5 year	0.40	0.40	124.08	0.580	0.579	7-M2c	0.450	0.316	0.316	0.298	1.697
100 year	0.84	0.54	124.29	0.786	0.794	7-M2t	0.450	0.363	0.439	0.439	1.696

\*\*\*\*\*  
Straight Culvert

Inlet Elevation (invert): 123.50 m, Outlet Elevation (invert): 123.47 m

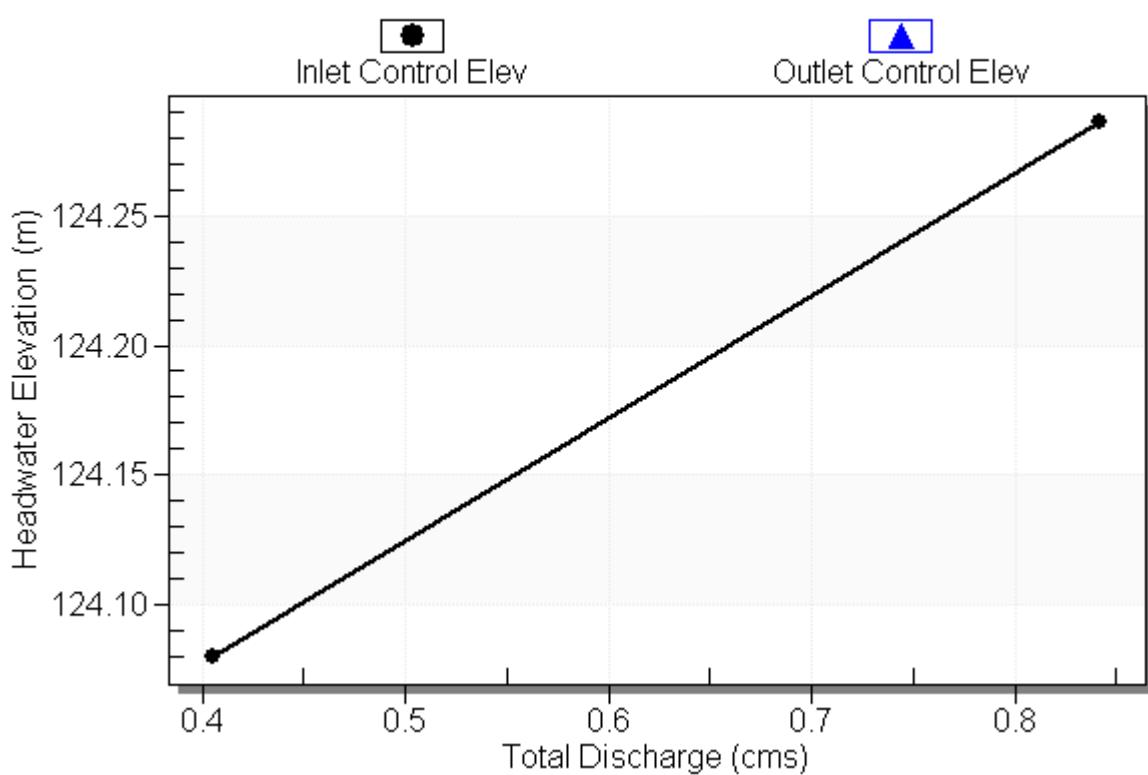
Culvert Length: 14.60 m, Culvert Slope: 0.0021

\*\*\*\*\*

## Culvert Performance Curve Plot: 2 X 450

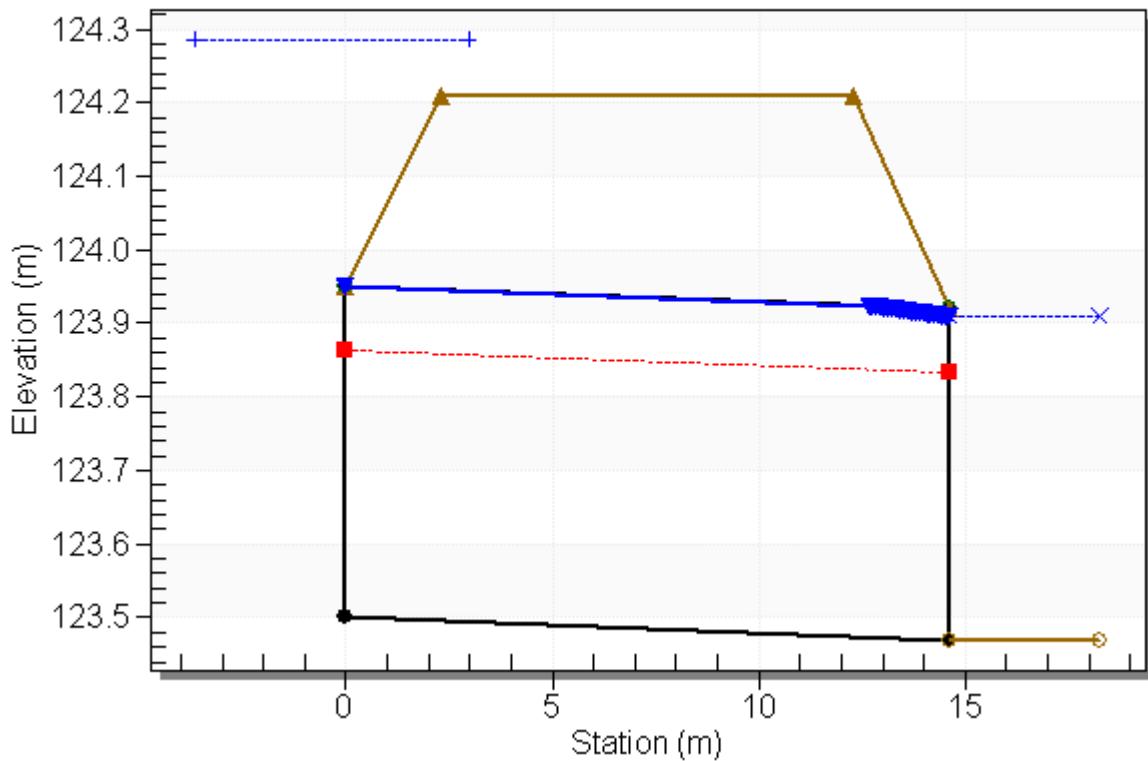
### Performance Curve

Culvert: 2 X 450



## Water Surface Profile Plot for Culvert: 2 X 450

Crossing - B1-6 (MidPoint), Design Discharge - 0.84 cms  
Culvert - 2 X 450, Culvert Discharge - 0.54 cms



## Site Data - 2 X 450

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 m

Inlet Elevation: 123.50 m

Outlet Station: 14.60 m

Outlet Elevation: 123.47 m

Number of Barrels: 2

## Culvert Data Summary - 2 X 450

Barrel Shape: Circular

Barrel Diameter: 450.00 mm

Barrel Material: Smooth HDPE

Embedment: 0.00 mm

Barrel Manning's n: 0.0120

Culvert Type: Straight

Inlet Configuration: Thin Edge Projecting

Inlet Depression: None

**Table 9 - Downstream Channel Rating Curve (Crossing: B1-6 (MidPoint))**

Flow (cms)	Water Surface Elev (m)	Depth (m)	Velocity (m/s)	Shear (Pa)	Froude Number
0.40	123.77	0.30	0.47	5.85	0.31
0.84	123.91	0.44	0.58	8.60	0.33

### **Tailwater Channel Data - B1-6 (MidPoint)**

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 2.00 m

Side Slope (H:V): 3.00 (\_:1)

Channel Slope: 0.0020

Channel Manning's n: 0.0350

Channel Invert Elevation: 123.47 m

### **Roadway Data for Crossing: B1-6 (MidPoint)**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 10.00 m

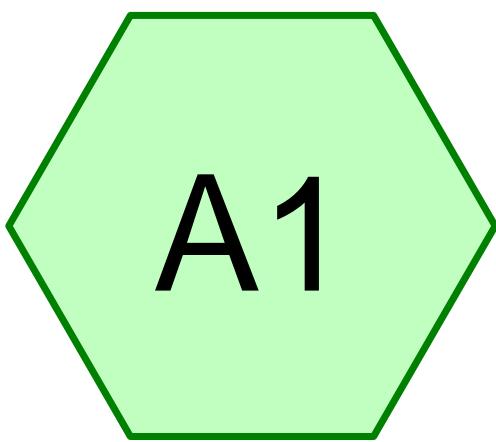
Crest Elevation: 124.21 m

Roadway Surface: Gravel

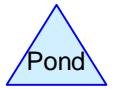
Roadway Top Width: 10.00 m

# B

## HYDROCAD MODEL OUTPUTS



# Pre-Dev (Site)



**Routing Diagram for 191-08574-02 - PreDev**  
Prepared by WSP Canada Inc., Printed 10/13/2021  
HydroCAD® 10.00-24 s/n 10698 © 2018 HydroCAD Software Solutions LLC

**Area Listing (all nodes)**

Area (hectares)	C	Description (subcatchment-numbers)
0.6852	0.90	Asphalt / Concrete (A1)
0.4098	0.90	Building (A1)
17.2570	0.25	Grass (A1)
0.9275	0.80	Gravel (A1)
<b>19.2795</b>	<b>0.31</b>	<b>TOTAL AREA</b>

### Summary for Subcatchment A1: Pre-Dev (Site)

Runoff = 0.3419 m<sup>3</sup>/s @ 1.52 hrs, Volume= 1.870 MI, Depth= 10 mm

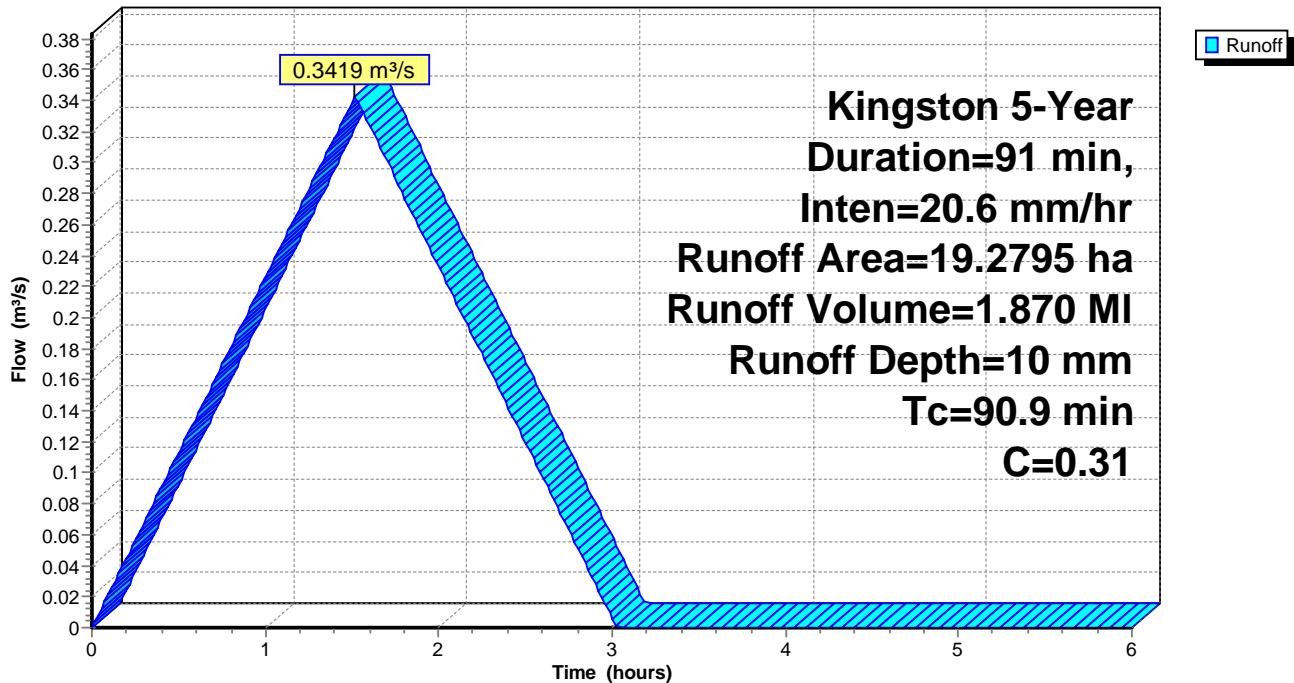
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=91 min, Inten=20.6 mm/hr

Area (ha)	C	Description
0.9275	0.80	Gravel
0.6852	0.90	Asphalt / Concrete
0.4098	0.90	Building
17.2570	0.25	Grass
19.2795	0.31	Weighted Average
19.2795		100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
90.9	<b>Direct Entry, Airport Method</b>				

### Subcatchment A1: Pre-Dev (Site)

**Hydrograph**



### Summary for Subcatchment A1: Pre-Dev (Site)

Runoff = 0.7184 m<sup>3</sup>/s @ 1.52 hrs, Volume= 3.929 MI, Depth= 20 mm

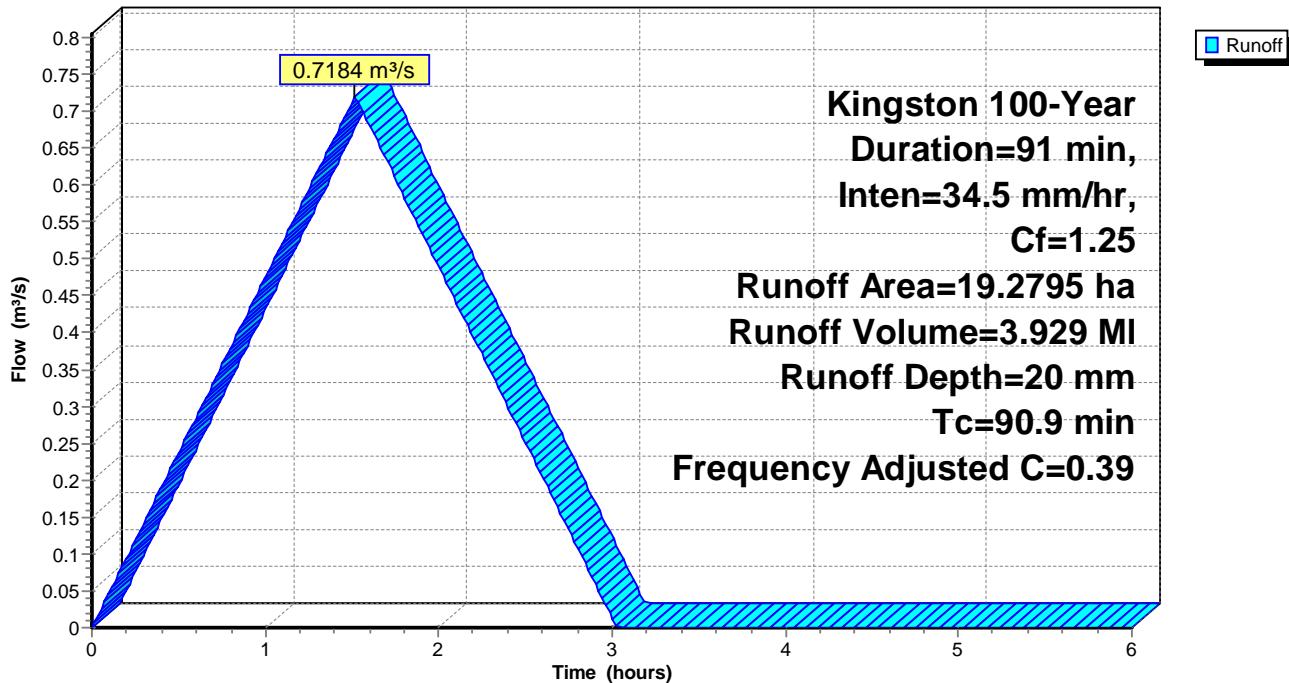
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 100-Year Duration=91 min, Inten=34.5 mm/hr, Cf=1.25

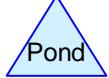
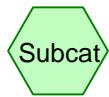
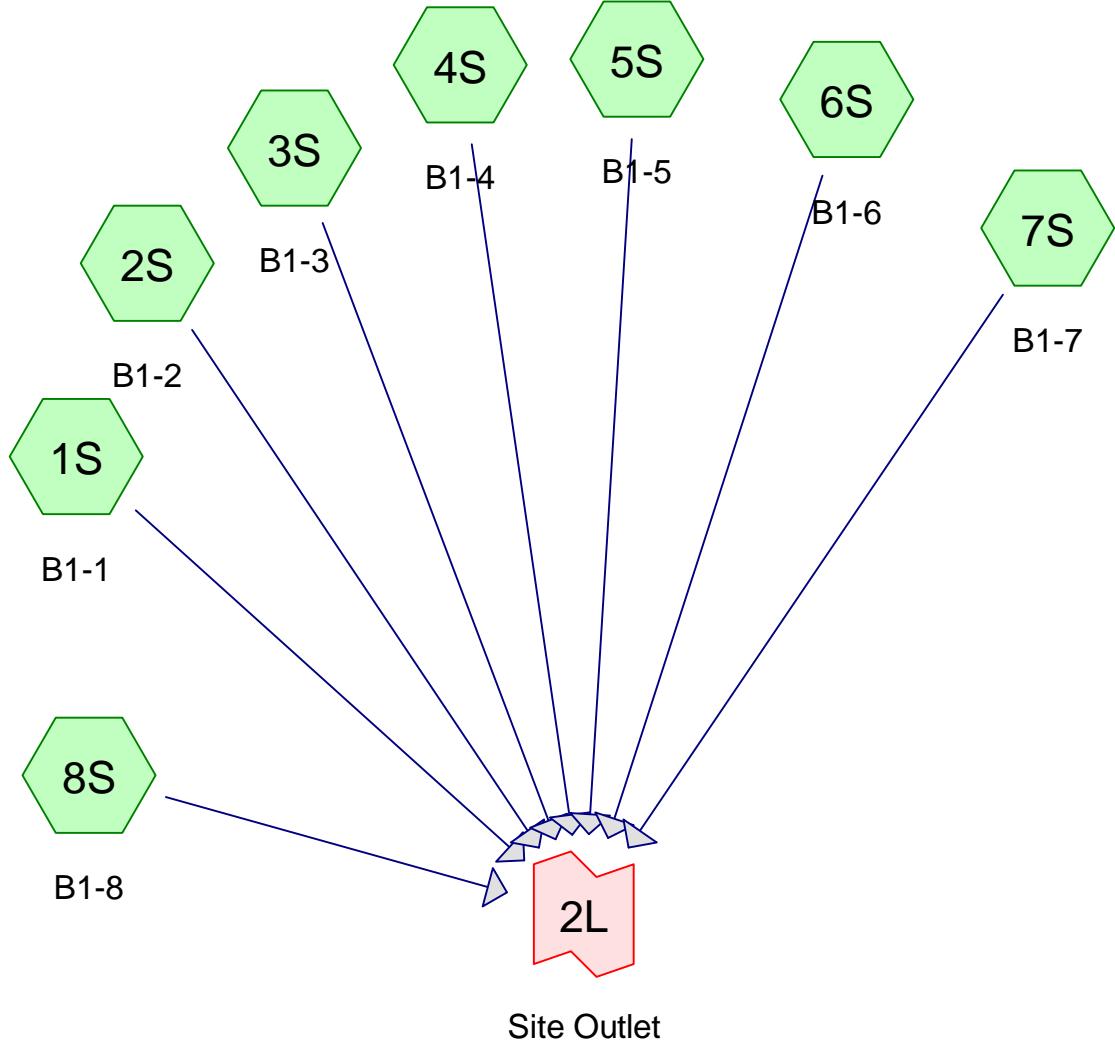
Area (ha)	C	Adj	Description
0.9275	0.80		Gravel
0.6852	0.90		Asphalt / Concrete
0.4098	0.90		Building
17.2570	0.25		Grass
19.2795	0.31	0.39	Weighted Average, Frequency Adjusted
19.2795			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
90.9	<b>Direct Entry, Airport Method</b>				

### Subcatchment A1: Pre-Dev (Site)

**Hydrograph**





Routing Diagram for 191-08574-02 - PostDev\_UNCTLD\_v3  
Prepared by WSP Canada Inc., Printed 10/13/2021  
HydroCAD® 10.00-24 s/n 10698 © 2018 HydroCAD Software Solutions LLC

**Area Listing (all nodes)**

Area (hectares)	C	Description (subcatchment-numbers)
0.8197	0.90	Asphalt / Concrete (2S, 3S, 4S, 5S, 6S, 7S)
14.2656	0.25	Grass (1S, 2S, 3S, 4S, 5S, 6S, 7S, 8S)
2.6264	0.80	Gravel (1S, 2S, 3S, 4S, 5S, 6S, 7S, 8S)
0.1513	0.01	Manure Tank (3S, 7S)
1.4239	0.90	Roof (1S, 2S, 3S, 4S, 5S, 6S, 7S)
<b>19.2869</b>	<b>0.40</b>	<b>TOTAL AREA</b>

### Summary for Subcatchment 1S: B1-1

Runoff = 0.1492 m<sup>3</sup>/s @ 0.45 hrs, Volume= 0.242 MI, Depth= 8 mm

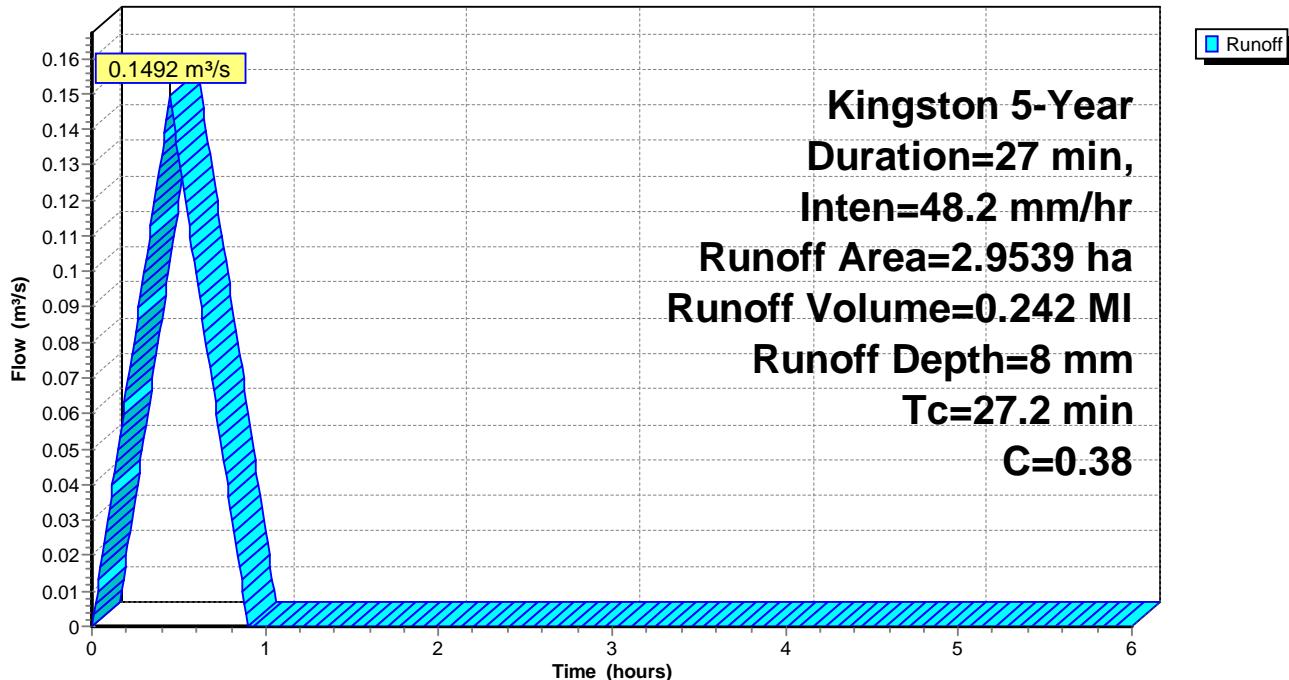
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=27 min, Inten=48.2 mm/hr

Area (ha)	C	Description
0.5181	0.80	Gravel
0.0000	0.90	Asphalt / Concrete
0.0000	0.01	Manure Tank
0.1539	0.90	Roof
2.2819	0.25	Grass
2.9539	0.38	Weighted Average
2.9539		100.00% Pervious Area

Tc (min)	Length (meters)	Slope (m/m)	Velocity (m/sec)	Capacity (m <sup>3</sup> /s)	Description
27.2					Direct Entry, Per Ditch Design Sheet

### Subcatchment 1S: B1-1

**Hydrograph**



### Summary for Subcatchment 2S: B1-2

Runoff = 0.0408 m<sup>3</sup>/s @ 0.45 hrs, Volume= 0.066 MI, Depth= 11 mm

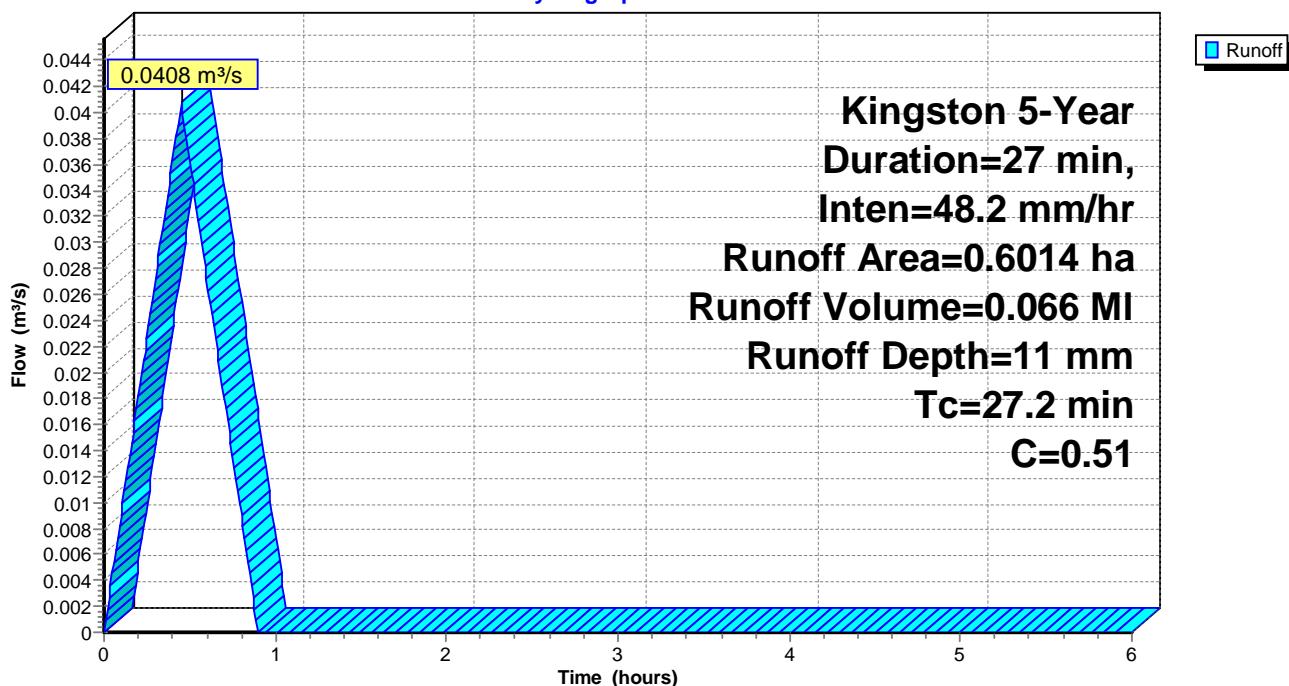
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=27 min, Inten=48.2 mm/hr

Area (ha)	C	Description
0.1347	0.80	Gravel
0.0046	0.90	Asphalt / Concrete
0.0000	0.01	Manure Tank
0.1175	0.90	Roof
0.3446	0.25	Grass
0.6014	0.51	Weighted Average
0.6014		100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 2S: B1-2

**Hydrograph**



### Summary for Subcatchment 3S: B1-3

Runoff = 0.0766 m<sup>3</sup>/s @ 0.45 hrs, Volume= 0.124 MI, Depth= 7 mm

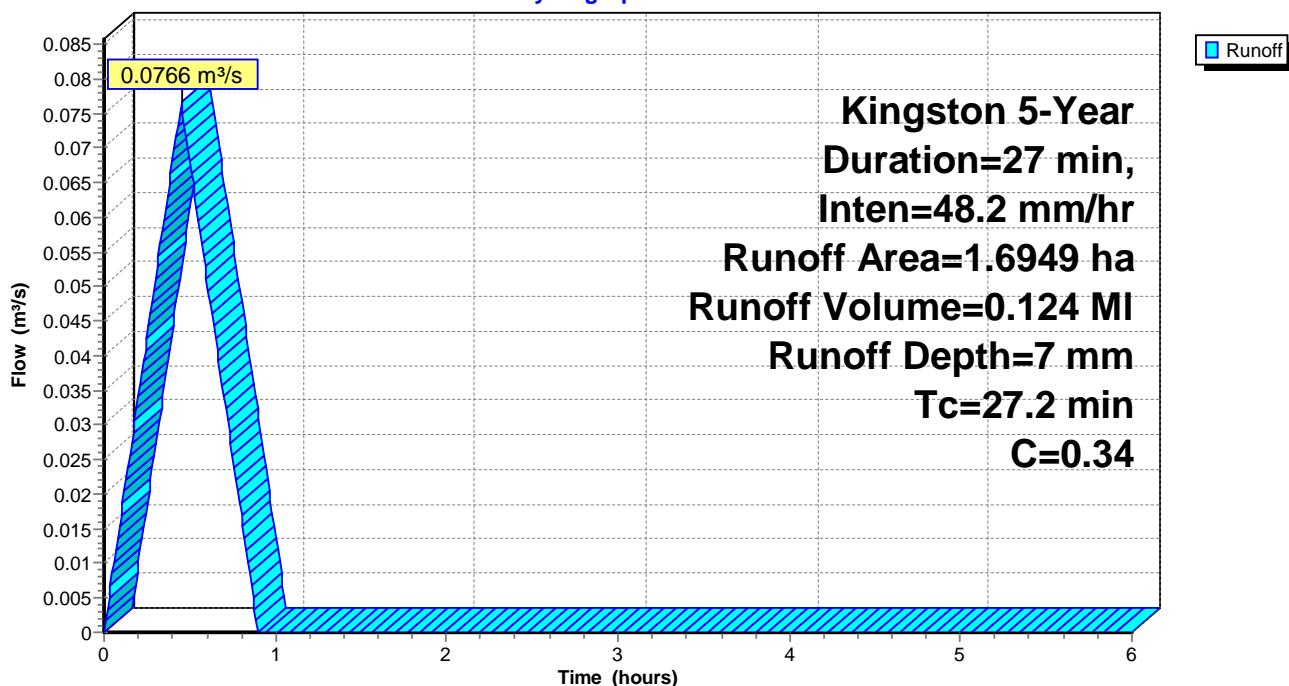
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=27 min, Inten=48.2 mm/hr

Area (ha)	C	Description
0.2254	0.80	Gravel
0.0070	0.90	Asphalt / Concrete
0.1239	0.01	Manure Tank
0.0809	0.90	Roof
1.2577	0.25	Grass
1.6949	0.34	Weighted Average
1.6949		100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 3S: B1-3

**Hydrograph**



### Summary for Subcatchment 4S: B1-4

Runoff = 0.1165 m<sup>3</sup>/s @ 0.45 hrs, Volume= 0.189 MI, Depth= 8 mm

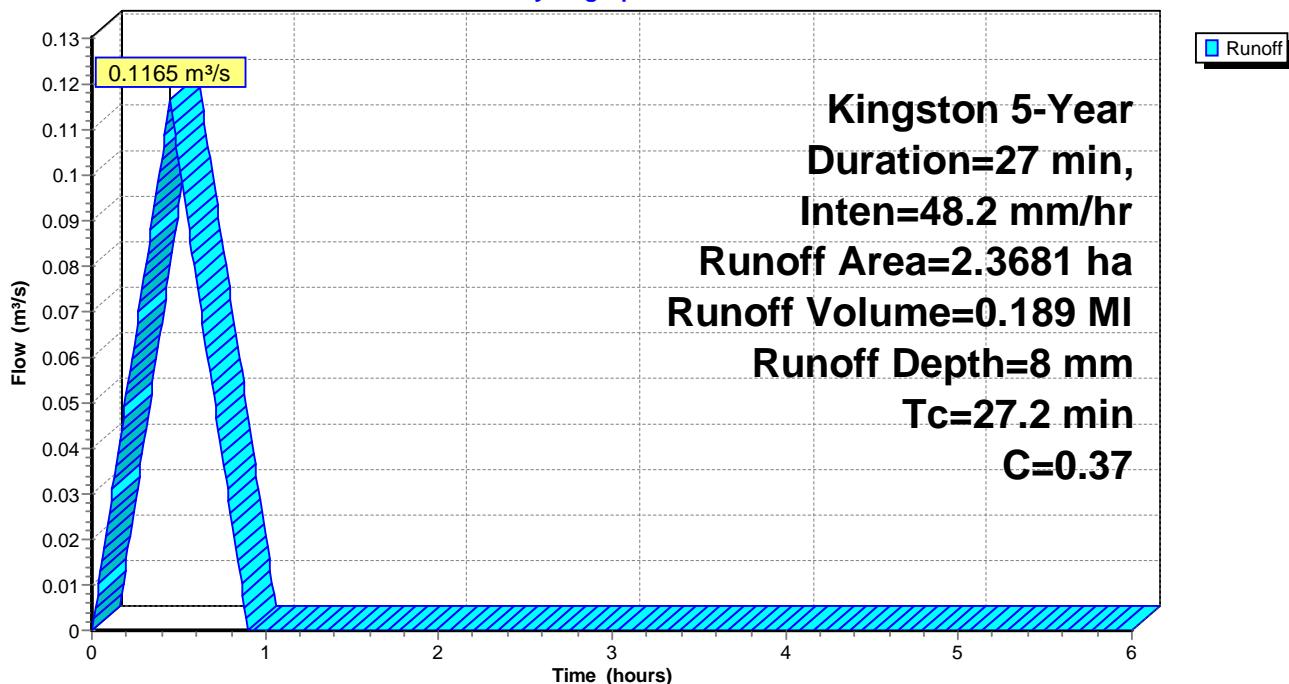
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=27 min, Inten=48.2 mm/hr

Area (ha)	C	Description
0.3290	0.80	Gravel
0.0268	0.90	Asphalt / Concrete
0.0000	0.01	Manure Tank
0.1249	0.90	Roof
1.8874	0.25	Grass
2.3681	0.37	Weighted Average
2.3681		100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 4S: B1-4

**Hydrograph**



### Summary for Subcatchment 5S: B1-5

Runoff = 0.2197 m<sup>3</sup>/s @ 0.45 hrs, Volume= 0.356 MI, Depth= 13 mm

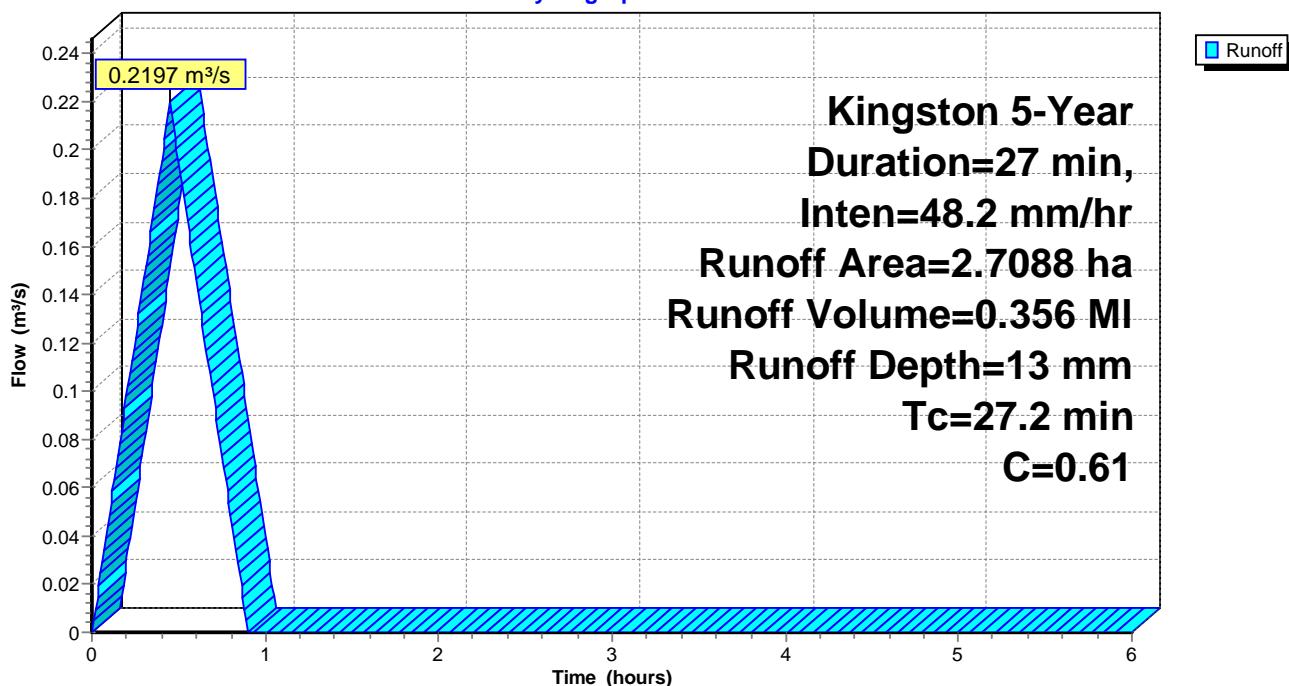
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=27 min, Inten=48.2 mm/hr

Area (ha)	C	Description
0.3775	0.80	Gravel
0.7307	0.90	Asphalt / Concrete
0.0000	0.01	Manure Tank
0.4356	0.90	Roof
1.1650	0.25	Grass
2.7088	0.61	Weighted Average
2.7088		100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 5S: B1-5

**Hydrograph**



### Summary for Subcatchment 6S: B1-6

Runoff = 0.0977 m<sup>3</sup>/s @ 0.45 hrs, Volume= 0.158 MI, Depth= 11 mm

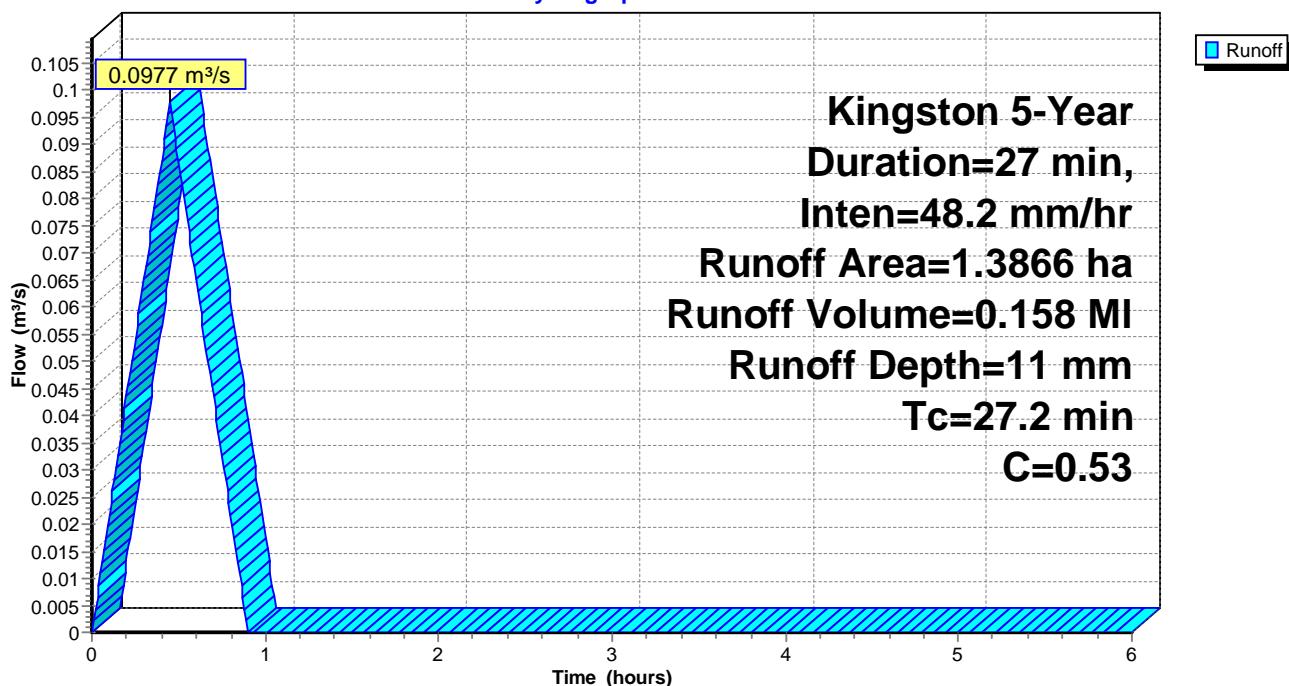
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=27 min, Inten=48.2 mm/hr

Area (ha)	C	Description
0.4917	0.80	Gravel
0.0289	0.90	Asphalt / Concrete
0.0000	0.01	Manure Tank
0.1561	0.90	Roof
0.7099	0.25	Grass
1.3866	0.53	Weighted Average
1.3866		100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 6S: B1-6

**Hydrograph**



### Summary for Subcatchment 7S: B1-7

Runoff = 0.2416 m<sup>3</sup>/s @ 0.45 hrs, Volume= 0.391 MI, Depth= 8 mm

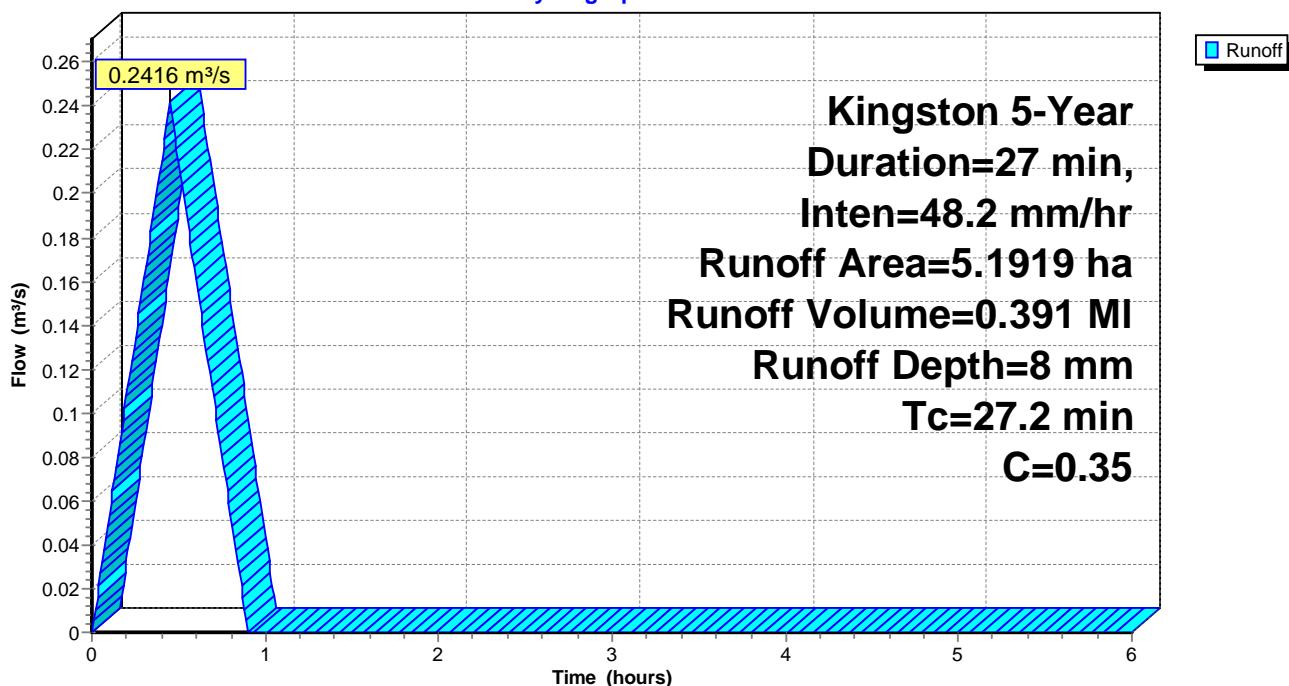
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=27 min, Inten=48.2 mm/hr

Area (ha)	C	Description
0.4987	0.80	Gravel
0.0217	0.90	Asphalt / Concrete
0.0274	0.01	Manure Tank
0.3550	0.90	Roof
4.2891	0.25	Grass
5.1919	0.35	Weighted Average
5.1919		100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 7S: B1-7

**Hydrograph**



### Summary for Subcatchment 8S: B1-8

Runoff = 0.0829 m<sup>3</sup>/s @ 0.25 hrs, Volume= 0.134 MI, Depth= 6 mm

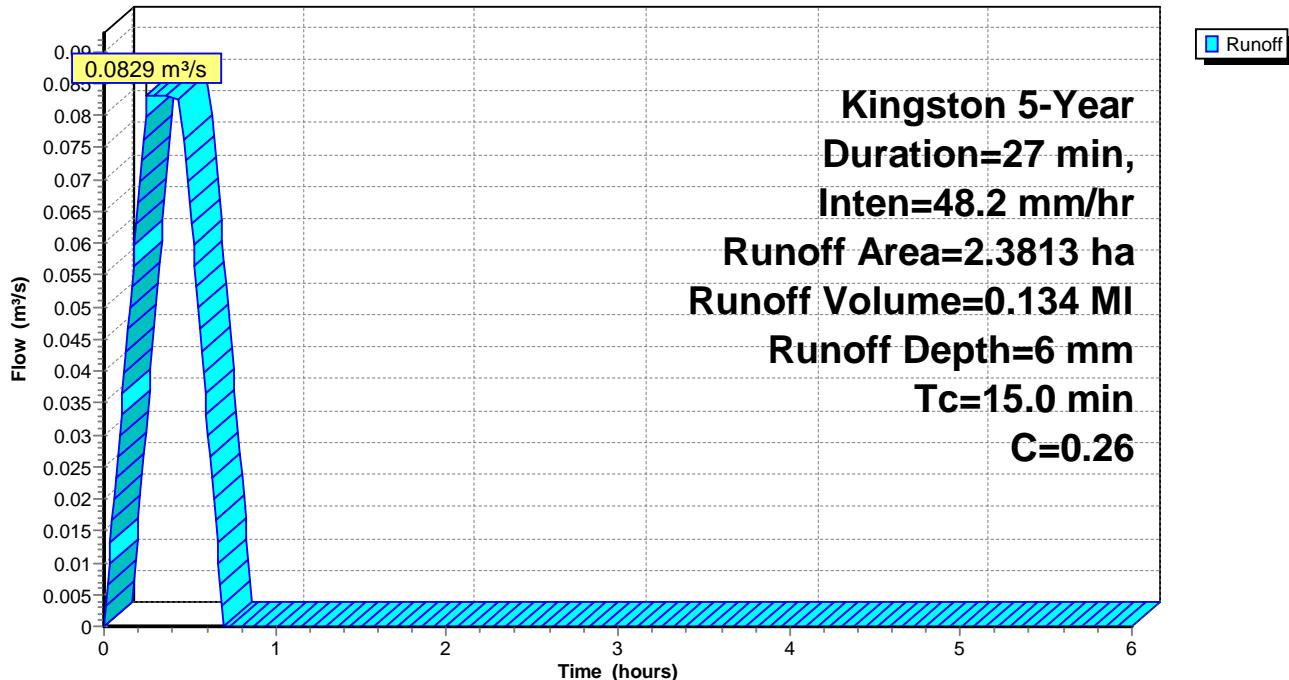
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=27 min, Inten=48.2 mm/hr

Area (ha)	C	Description
0.0513	0.80	Gravel
0.0000	0.90	Asphalt/Concrete
0.0000	0.01	Manure Tank
0.0000	0.90	Roof
2.3300	0.25	Grass
2.3813	0.26	Weighted Average
2.3813		100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
15.0	Direct Entry, Minimum				

### Subcatchment 8S: B1-8

**Hydrograph**



### Summary for Link 2L: Site Outlet

Inflow Area = 19.2869 ha, 0.00% Impervious, Inflow Depth = 9 mm for 5-Year event

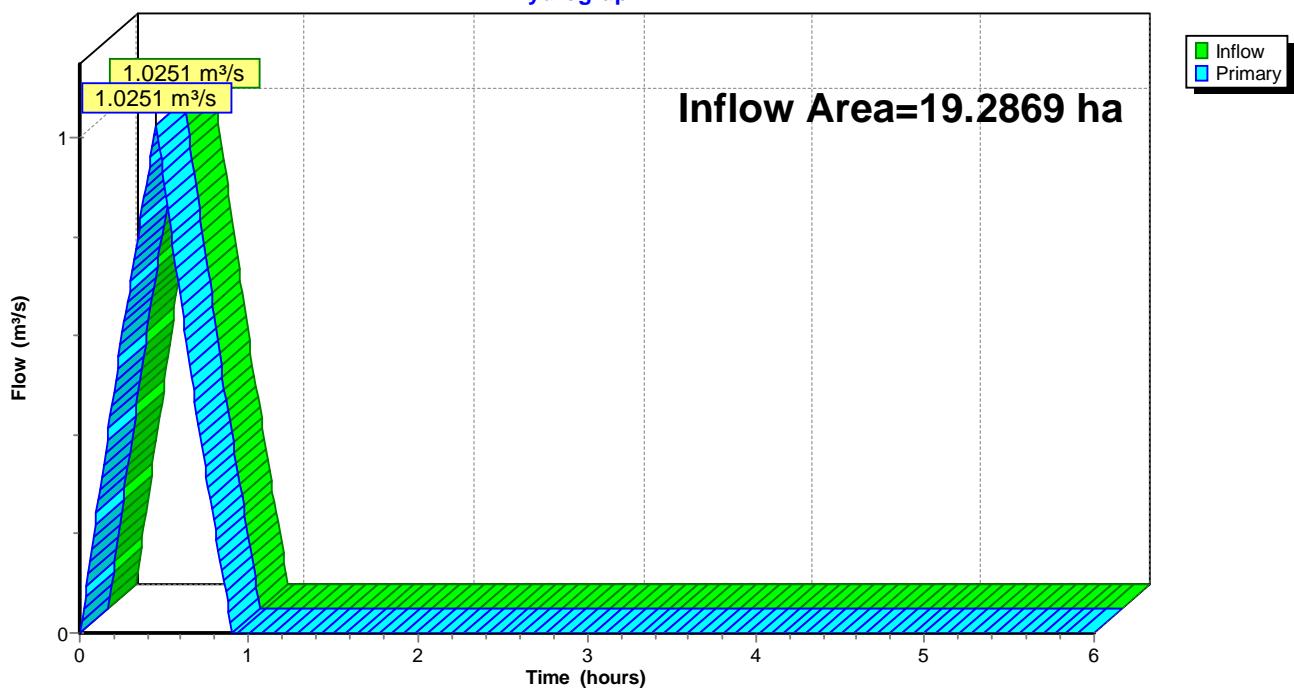
Inflow = 1.0251 m<sup>3</sup>/s @ 0.45 hrs, Volume= 1.661 MI

Primary = 1.0251 m<sup>3</sup>/s @ 0.45 hrs, Volume= 1.661 MI, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs

#### Link 2L: Site Outlet

**Hydrograph**



### Summary for Subcatchment 1S: B1-1

Runoff = 0.3083 m<sup>3</sup>/s @ 0.45 hrs, Volume= 0.499 MI, Depth= 17 mm

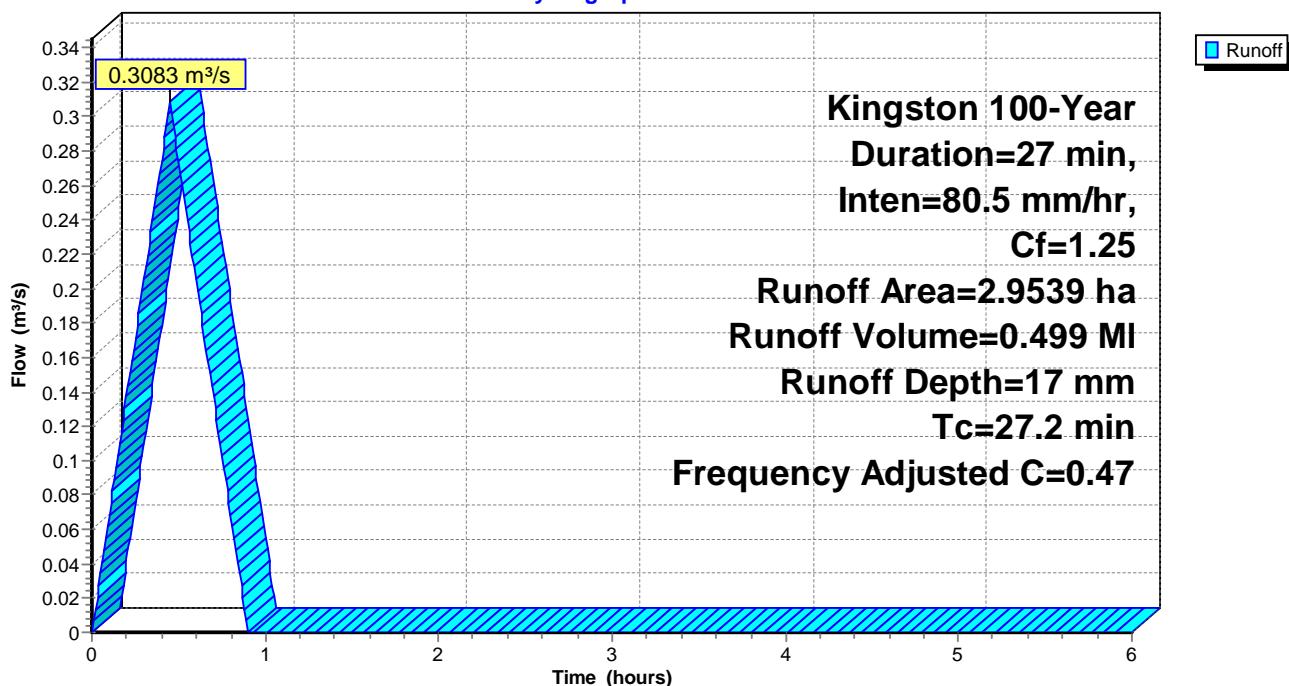
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
Kingston 100-Year Duration=27 min, Inten=80.5 mm/hr, Cf=1.25

Area (ha)	C	Adj	Description
0.5181	0.80		Gravel
0.0000	0.90		Asphalt / Concrete
0.0000	0.01		Manure Tank
0.1539	0.90		Roof
2.2819	0.25		Grass
2.9539	0.38	0.47	Weighted Average, Frequency Adjusted
2.9539			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Per Ditch Design Sheet

### Subcatchment 1S: B1-1

Hydrograph



### Summary for Subcatchment 2S: B1-2

Runoff = 0.0855 m<sup>3</sup>/s @ 0.45 hrs, Volume= 0.138 MI, Depth= 23 mm

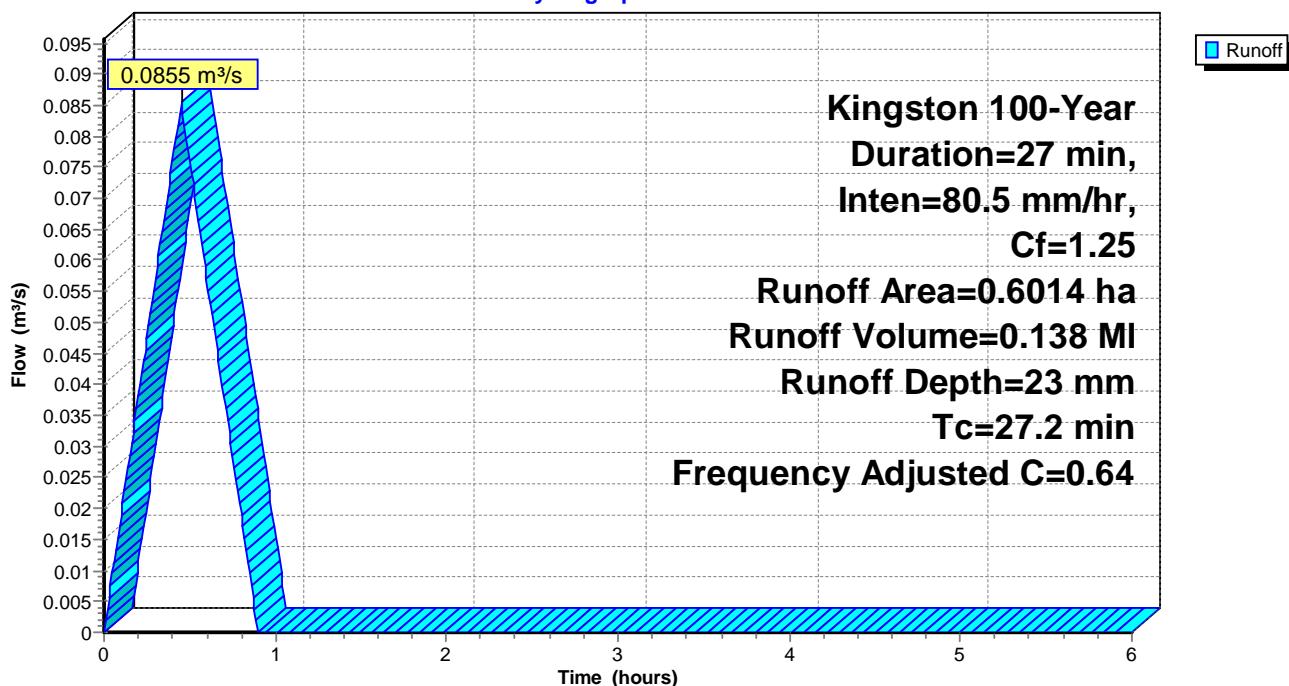
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
Kingston 100-Year Duration=27 min, Inten=80.5 mm/hr, Cf=1.25

Area (ha)	C	Adj	Description
0.1347	0.80		Gravel
0.0046	0.90		Asphalt / Concrete
0.0000	0.01		Manure Tank
0.1175	0.90		Roof
0.3446	0.25		Grass
0.6014	0.51	0.64	Weighted Average, Frequency Adjusted
0.6014			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 2S: B1-2

Hydrograph



### Summary for Subcatchment 3S: B1-3

Runoff = 0.1618 m<sup>3</sup>/s @ 0.45 hrs, Volume= 0.262 MI, Depth= 15 mm

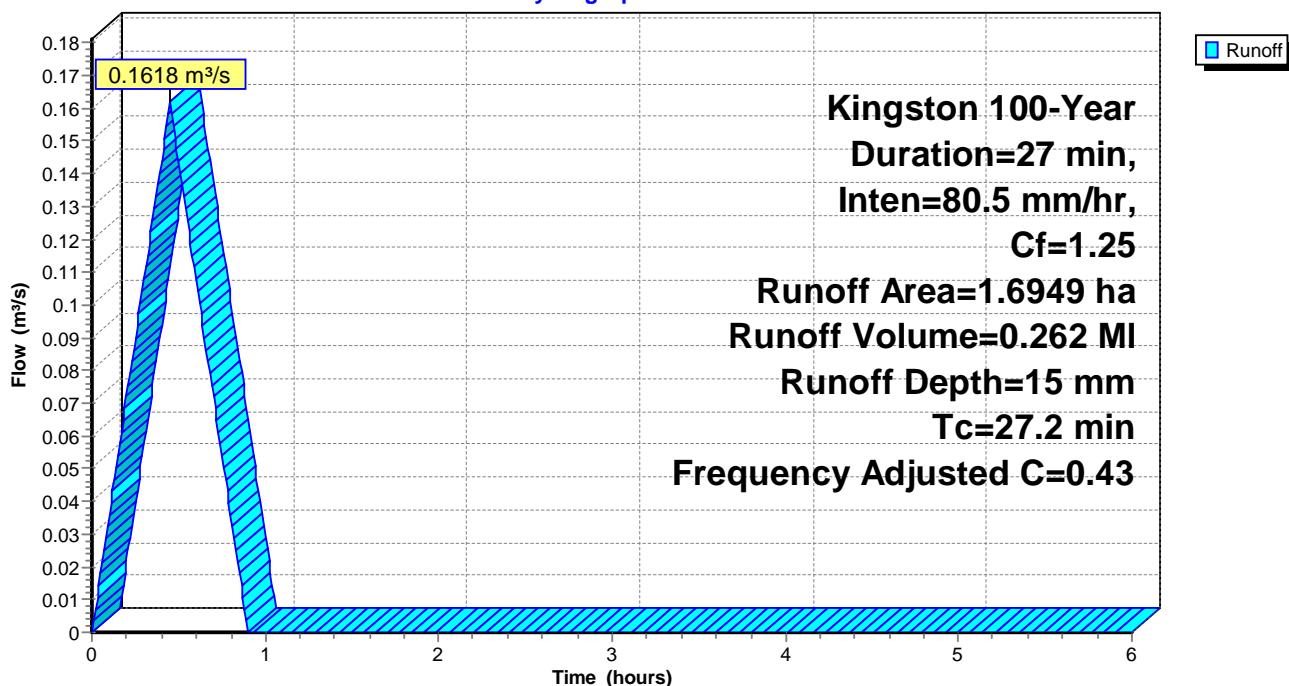
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
Kingston 100-Year Duration=27 min, Inten=80.5 mm/hr, Cf=1.25

Area (ha)	C	Adj	Description
0.2254	0.80		Gravel
0.0070	0.90		Asphalt / Concrete
0.1239	0.01		Manure Tank
0.0809	0.90		Roof
1.2577	0.25		Grass
1.6949	0.34	0.43	Weighted Average, Frequency Adjusted
1.6949			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 3S: B1-3

Hydrograph



### Summary for Subcatchment 4S: B1-4

Runoff = 0.2419 m<sup>3</sup>/s @ 0.45 hrs, Volume= 0.392 MI, Depth= 17 mm

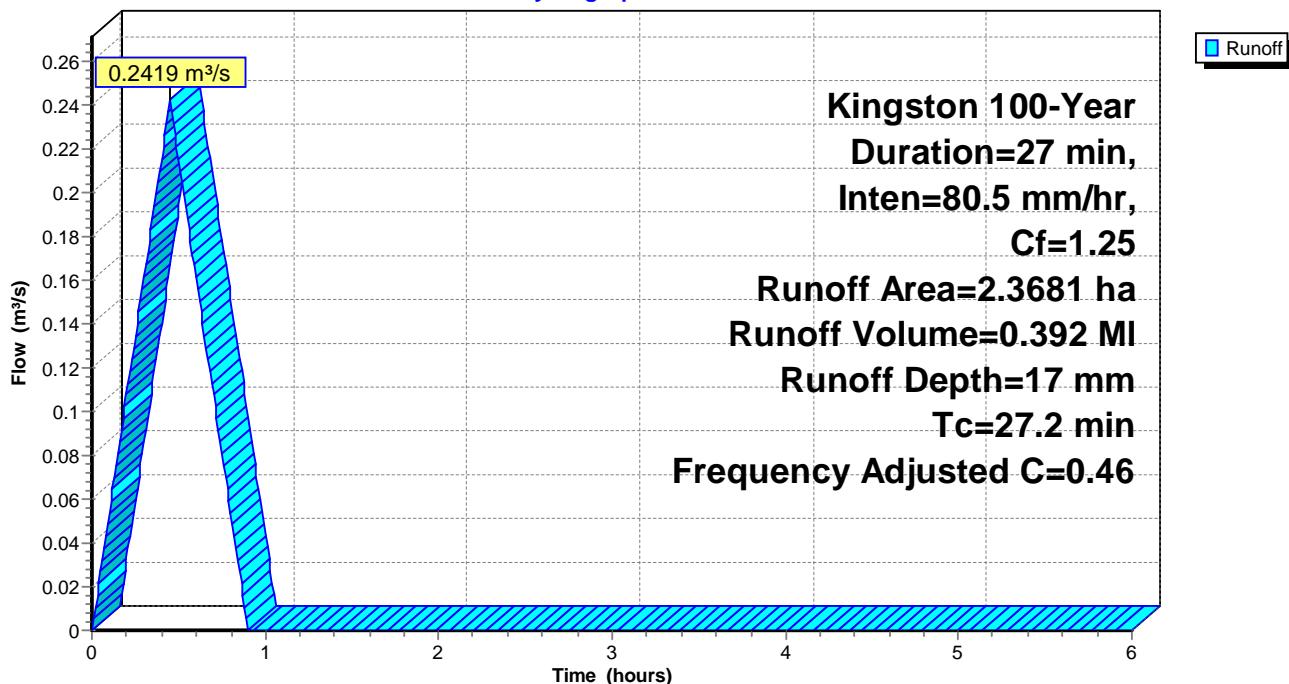
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 100-Year Duration=27 min, Inten=80.5 mm/hr, Cf=1.25

Area (ha)	C	Adj	Description
0.3290	0.80		Gravel
0.0268	0.90		Asphalt / Concrete
0.0000	0.01		Manure Tank
0.1249	0.90		Roof
1.8874	0.25		Grass
2.3681	0.37	0.46	Weighted Average, Frequency Adjusted
2.3681			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 4S: B1-4

Hydrograph



### Summary for Subcatchment 5S: B1-5

Runoff = 0.4571 m<sup>3</sup>/s @ 0.45 hrs, Volume= 0.740 MI, Depth= 27 mm

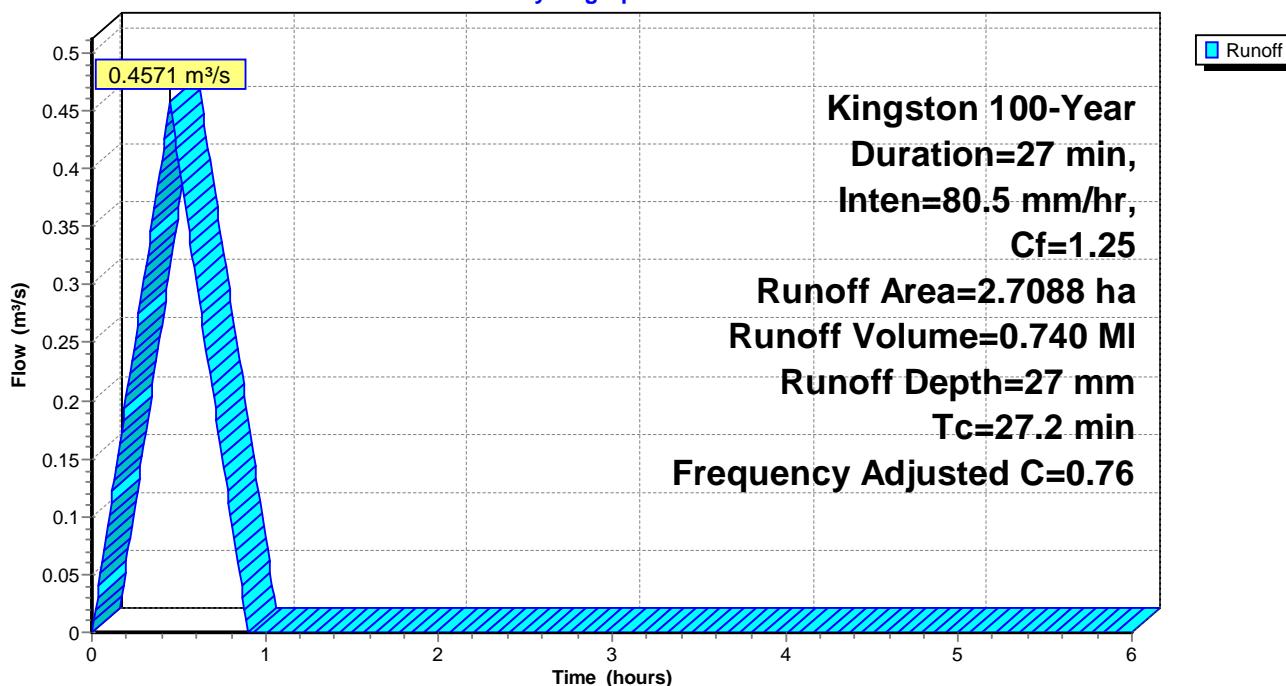
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
Kingston 100-Year Duration=27 min, Inten=80.5 mm/hr, Cf=1.25

Area (ha)	C	Adj	Description
0.3775	0.80		Gravel
0.7307	0.90		Asphalt / Concrete
0.0000	0.01		Manure Tank
0.4356	0.90		Roof
1.1650	0.25		Grass
2.7088	0.61	0.76	Weighted Average, Frequency Adjusted
2.7088			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 5S: B1-5

Hydrograph



### Summary for Subcatchment 6S: B1-6

Runoff = 0.2032 m<sup>3</sup>/s @ 0.45 hrs, Volume= 0.329 MI, Depth= 24 mm

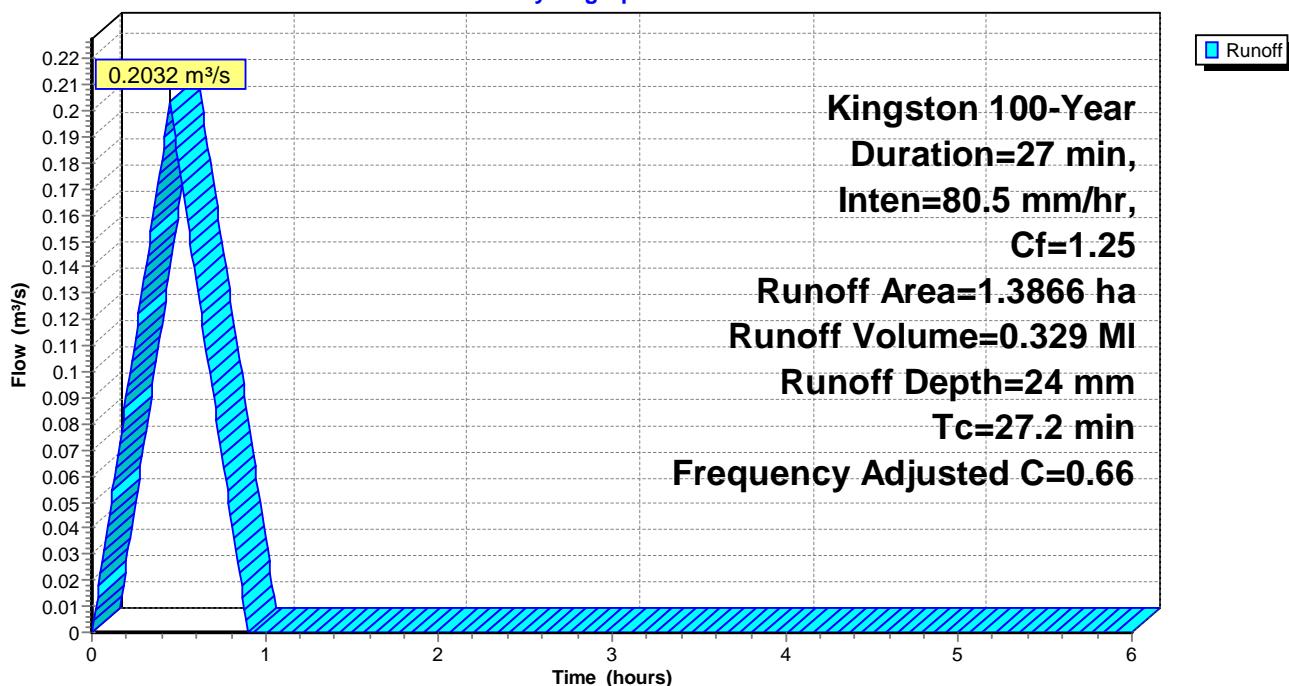
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
Kingston 100-Year Duration=27 min, Inten=80.5 mm/hr, Cf=1.25

Area (ha)	C	Adj	Description
0.4917	0.80		Gravel
0.0289	0.90		Asphalt / Concrete
0.0000	0.01		Manure Tank
0.1561	0.90		Roof
0.7099	0.25		Grass
1.3866	0.53	0.66	Weighted Average, Frequency Adjusted
1.3866			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 6S: B1-6

Hydrograph



### Summary for Subcatchment 7S: B1-7

Runoff = 0.5072 m<sup>3</sup>/s @ 0.45 hrs, Volume= 0.822 MI, Depth= 16 mm

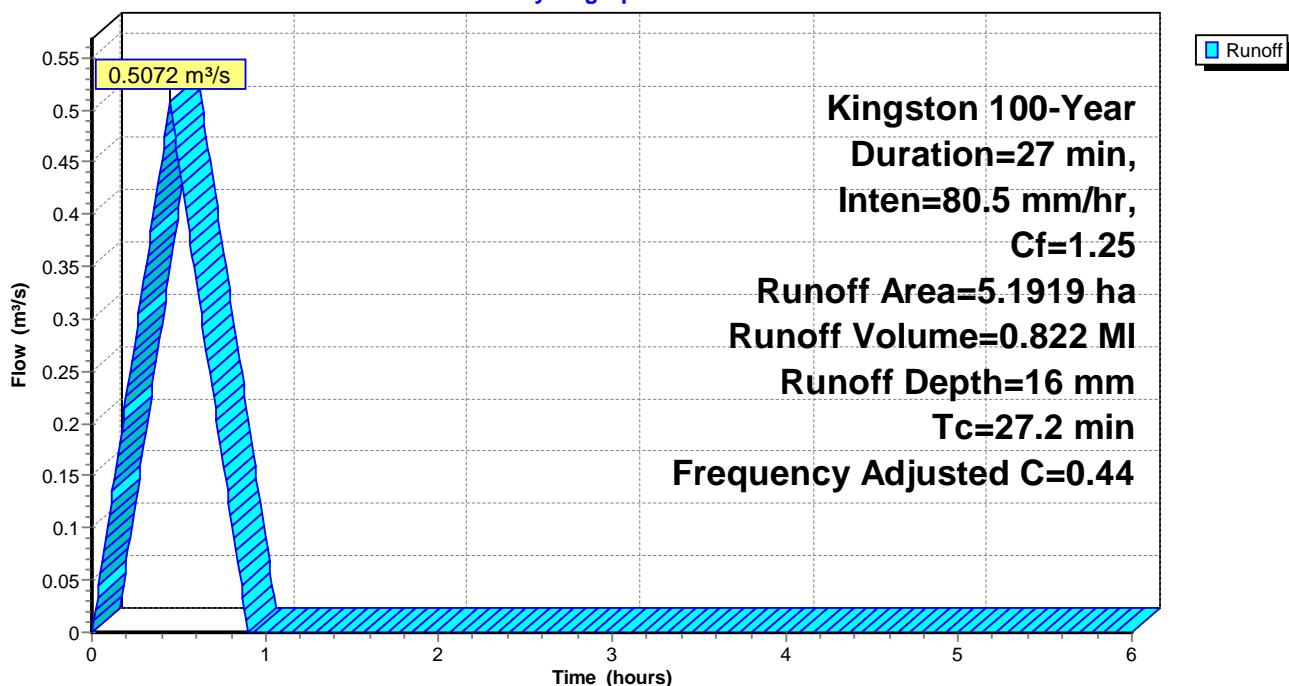
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
Kingston 100-Year Duration=27 min, Inten=80.5 mm/hr, Cf=1.25

Area (ha)	C	Adj	Description
0.4987	0.80		Gravel
0.0217	0.90		Asphalt / Concrete
0.0274	0.01		Manure Tank
0.3550	0.90		Roof
4.2891	0.25		Grass
5.1919	0.35	0.44	Weighted Average, Frequency Adjusted
5.1919			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 7S: B1-7

Hydrograph



### Summary for Subcatchment 8S: B1-8

Runoff = 0.1758 m<sup>3</sup>/s @ 0.25 hrs, Volume= 0.285 MI, Depth= 12 mm

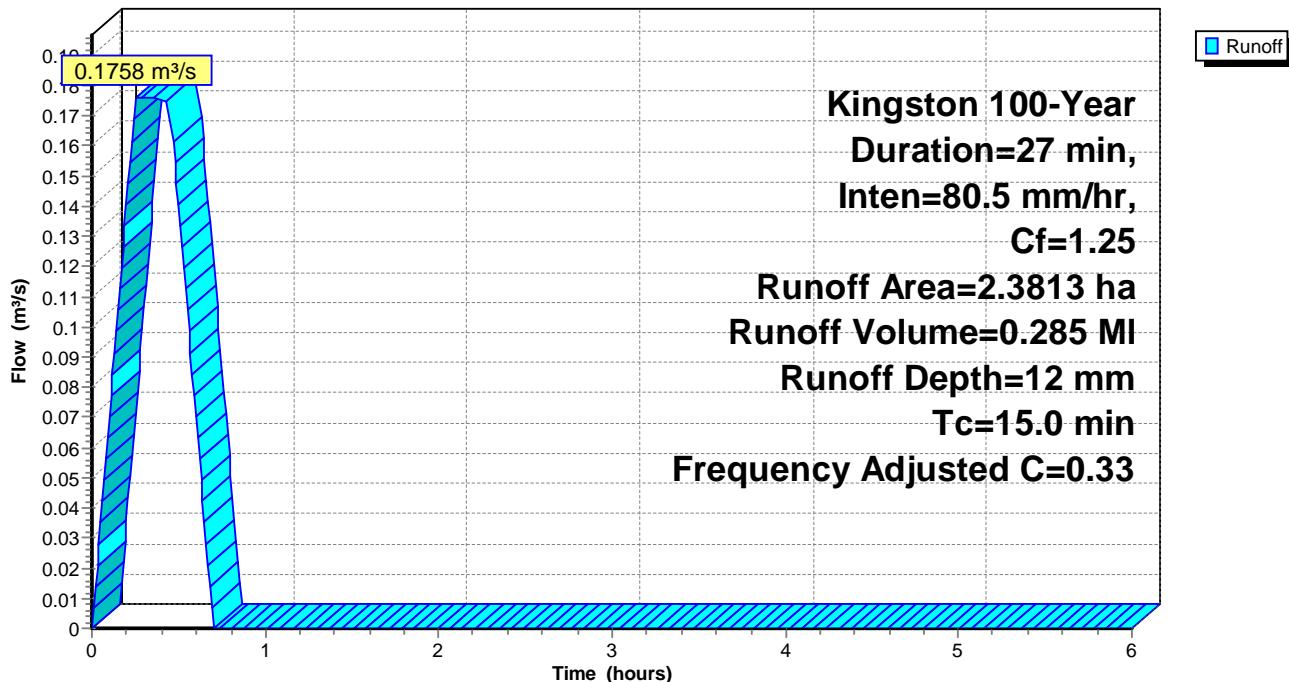
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
Kingston 100-Year Duration=27 min, Inten=80.5 mm/hr, Cf=1.25

Area (ha)	C	Adj	Description
0.0513	0.80		Gravel
0.0000	0.90		Asphalt/Concrete
0.0000	0.01		Manure Tank
0.0000	0.90		Roof
2.3300	0.25		Grass
2.3813	0.26	0.33	Weighted Average, Frequency Adjusted
2.3813			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
15.0					Direct Entry, Minimum

### Subcatchment 8S: B1-8

Hydrograph



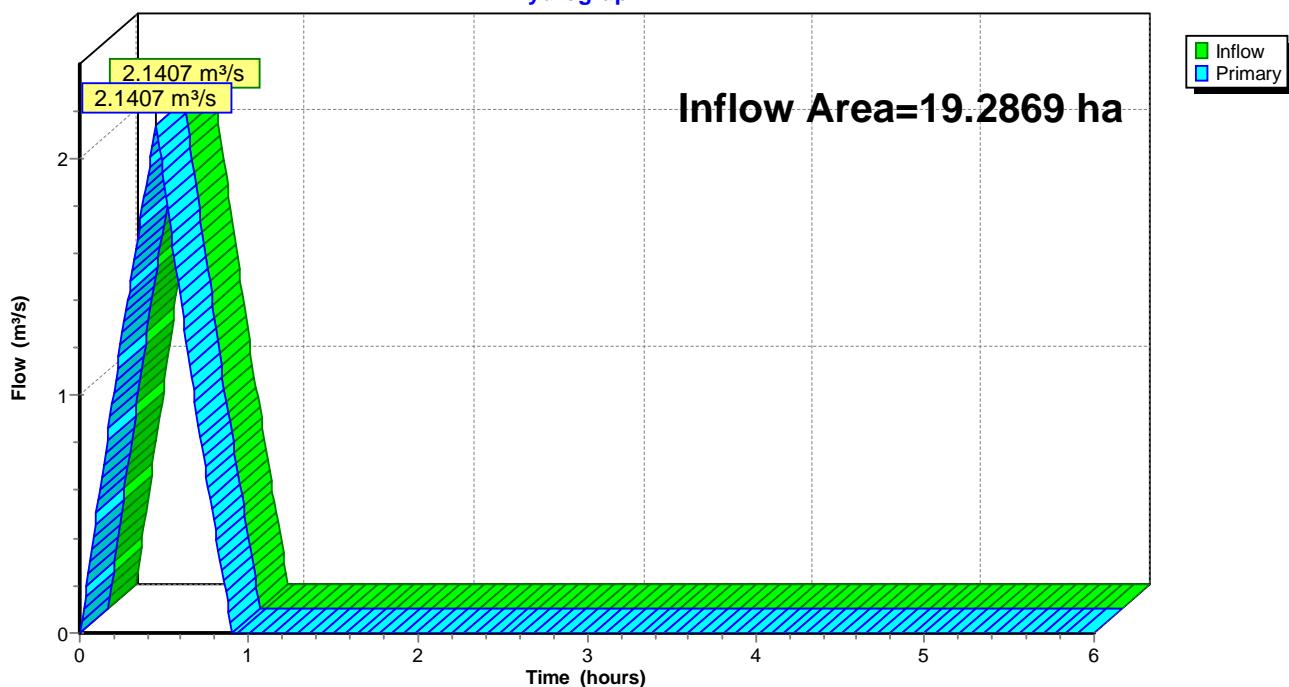
### Summary for Link 2L: Site Outlet

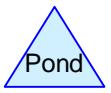
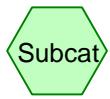
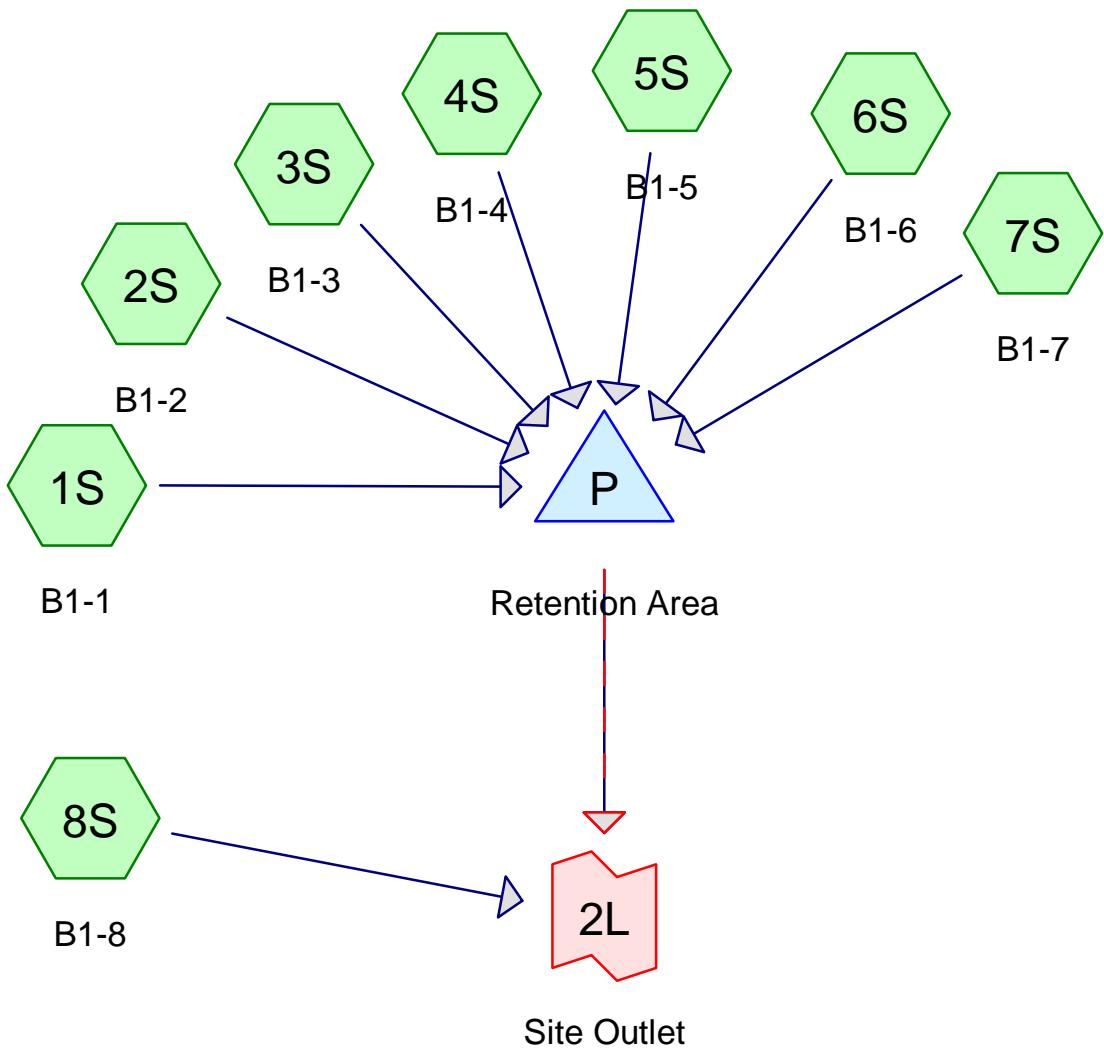
Inflow Area = 19.2869 ha, 0.00% Impervious, Inflow Depth = 18 mm for 100-Year event  
Inflow = 2.1407 m<sup>3</sup>/s @ 0.45 hrs, Volume= 3.468 MI  
Primary = 2.1407 m<sup>3</sup>/s @ 0.45 hrs, Volume= 3.468 MI, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs

### Link 2L: Site Outlet

Hydrograph





**Routing Diagram for 191-08574-02 - PostDev\_CTL\_D\_v3**  
 Prepared by WSP Canada Inc., Printed 10/13/2021  
 HydroCAD® 10.00-24 s/n 10698 © 2018 HydroCAD Software Solutions LLC

**Area Listing (all nodes)**

Area (hectares)	C	Description (subcatchment-numbers)
0.8197	0.90	Asphalt / Concrete (2S, 3S, 4S, 5S, 6S, 7S)
14.2656	0.25	Grass (1S, 2S, 3S, 4S, 5S, 6S, 7S, 8S)
2.6264	0.80	Gravel (1S, 2S, 3S, 4S, 5S, 6S, 7S, 8S)
0.1513	0.01	Manure Tank (3S, 7S)
1.4239	0.90	Roof (1S, 2S, 3S, 4S, 5S, 6S, 7S)
<b>19.2869</b>	<b>0.40</b>	<b>TOTAL AREA</b>

### Summary for Subcatchment 1S: B1-1

Runoff = 0.0633 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.353 MI, Depth= 12 mm

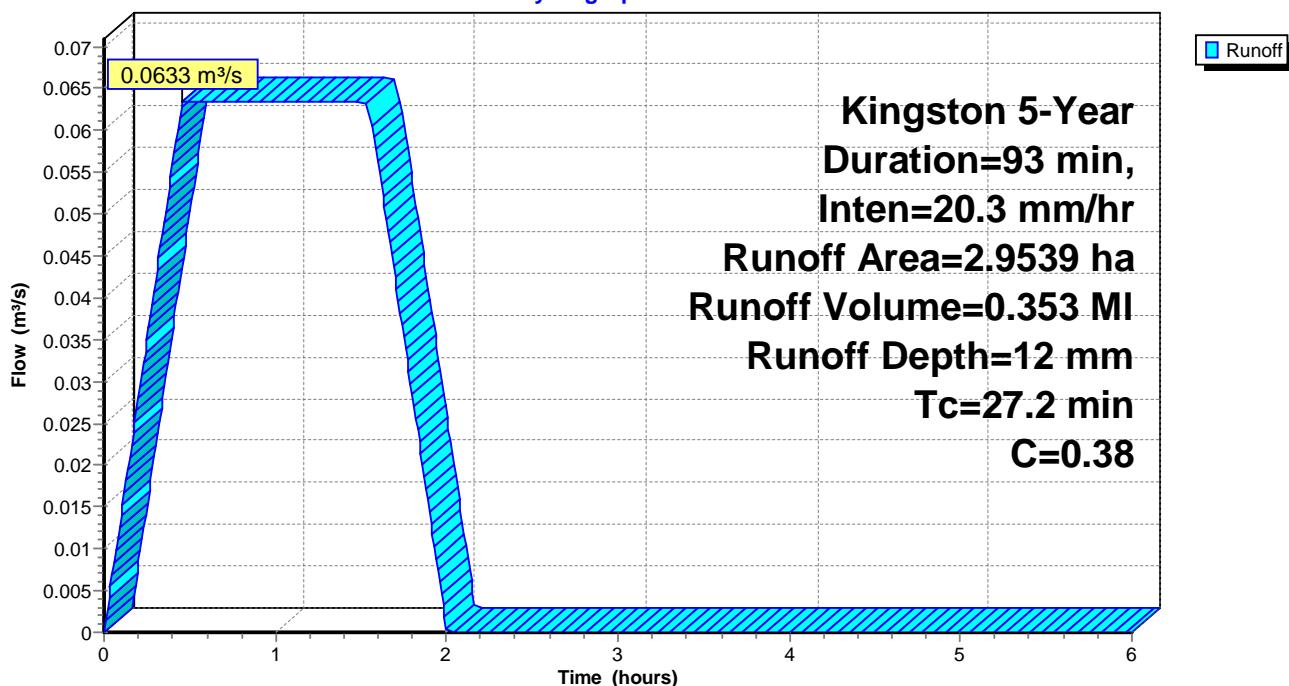
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=93 min, Inten=20.3 mm/hr

Area (ha)	C	Description
0.5181	0.80	Gravel
0.0000	0.90	Asphalt / Concrete
0.0000	0.01	Manure Tank
0.1539	0.90	Roof
2.2819	0.25	Grass
2.9539	0.38	Weighted Average
2.9539		100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2	Direct Entry, Per Ditch Design Sheet				

### Subcatchment 1S: B1-1

**Hydrograph**



### Summary for Subcatchment 2S: B1-2

Runoff = 0.0173 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.097 MI, Depth= 16 mm

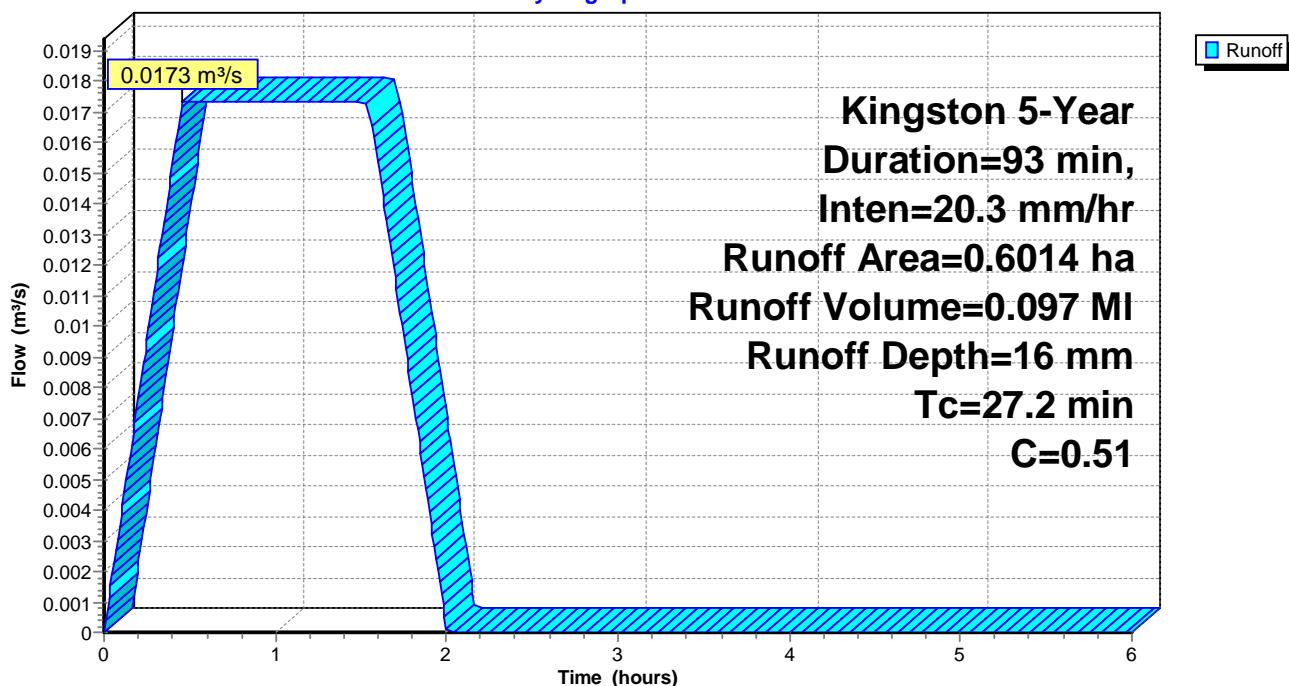
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=93 min, Inten=20.3 mm/hr

Area (ha)	C	Description
0.1347	0.80	Gravel
0.0046	0.90	Asphalt / Concrete
0.0000	0.01	Manure Tank
0.1175	0.90	Roof
0.3446	0.25	Grass
0.6014	0.51	Weighted Average
0.6014		100.00% Pervious Area

Tc (min)	Length (meters)	Slope (m/m)	Velocity (m/sec)	Capacity (m <sup>3</sup> /s)	Description
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 2S: B1-2

**Hydrograph**



### Summary for Subcatchment 3S: B1-3

Runoff = 0.0325 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.181 MI, Depth= 11 mm

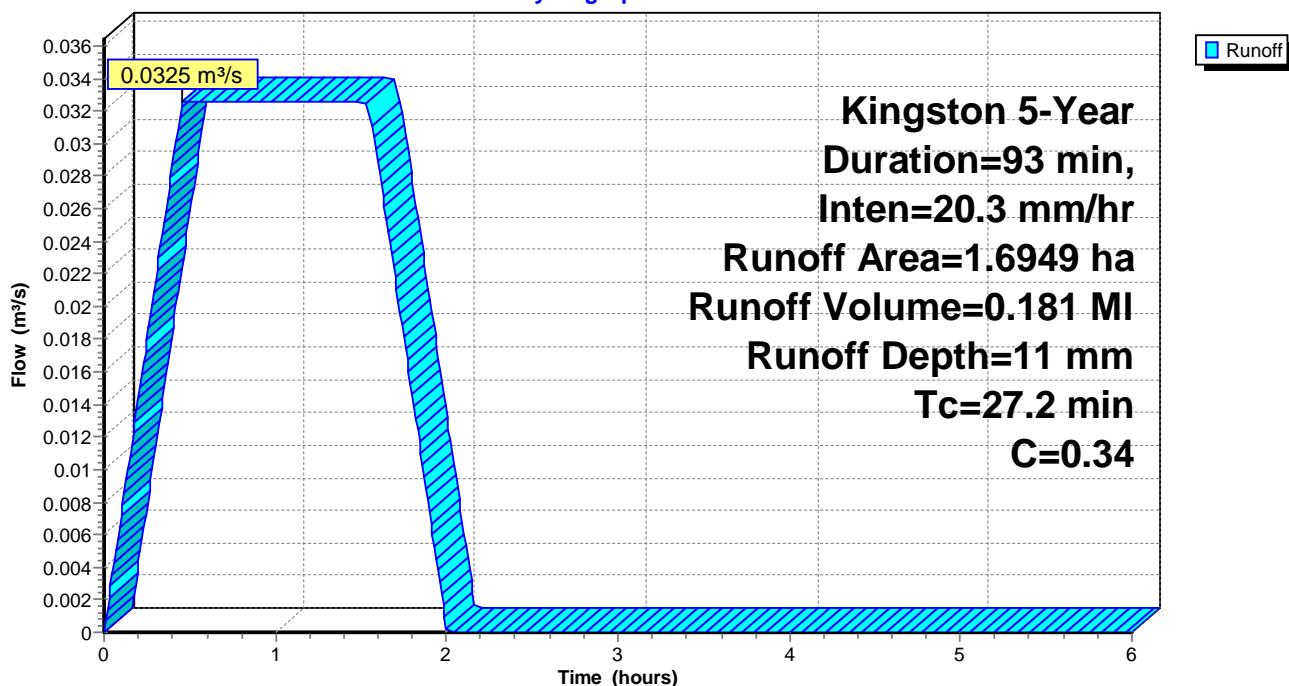
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=93 min, Inten=20.3 mm/hr

Area (ha)	C	Description
0.2254	0.80	Gravel
0.0070	0.90	Asphalt / Concrete
0.1239	0.01	Manure Tank
0.0809	0.90	Roof
1.2577	0.25	Grass
1.6949	0.34	Weighted Average
1.6949		100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 3S: B1-3

**Hydrograph**



### Summary for Subcatchment 4S: B1-4

Runoff = 0.0494 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.276 MI, Depth= 12 mm

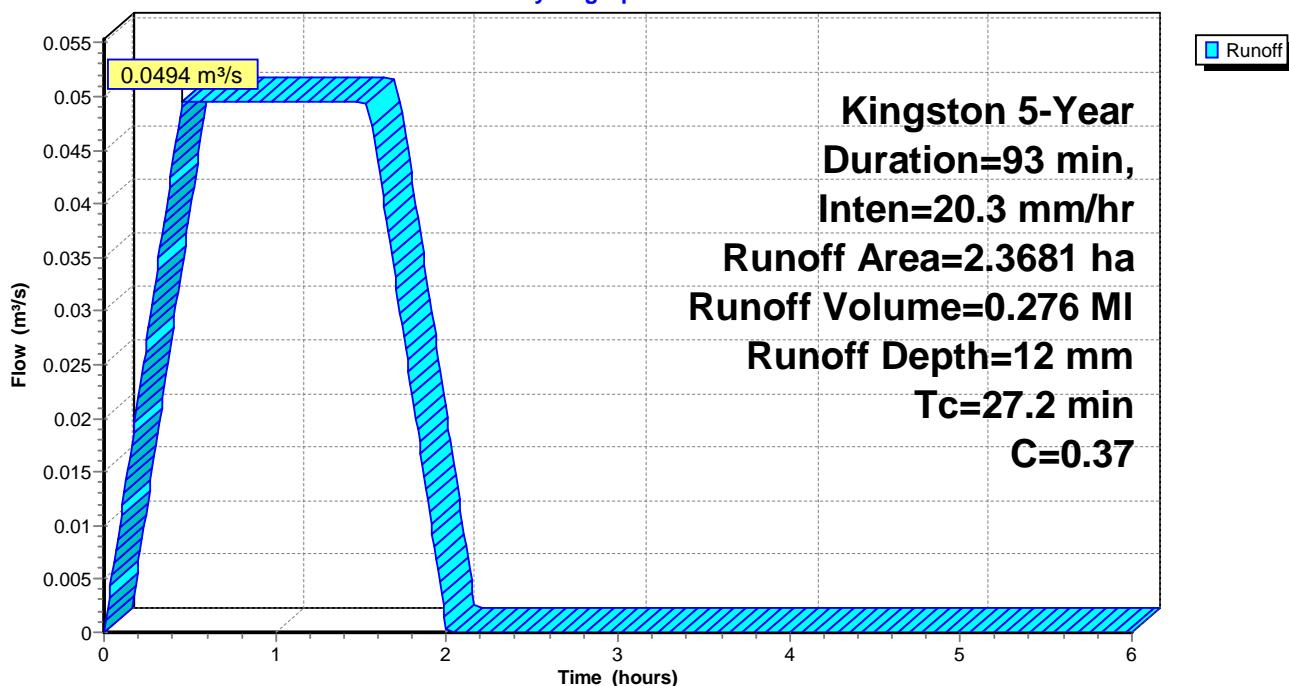
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=93 min, Inten=20.3 mm/hr

Area (ha)	C	Description
0.3290	0.80	Gravel
0.0268	0.90	Asphalt / Concrete
0.0000	0.01	Manure Tank
0.1249	0.90	Roof
1.8874	0.25	Grass
2.3681	0.37	Weighted Average
2.3681		100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 4S: B1-4

**Hydrograph**



### Summary for Subcatchment 5S: B1-5

Runoff = 0.0932 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.520 MI, Depth= 19 mm

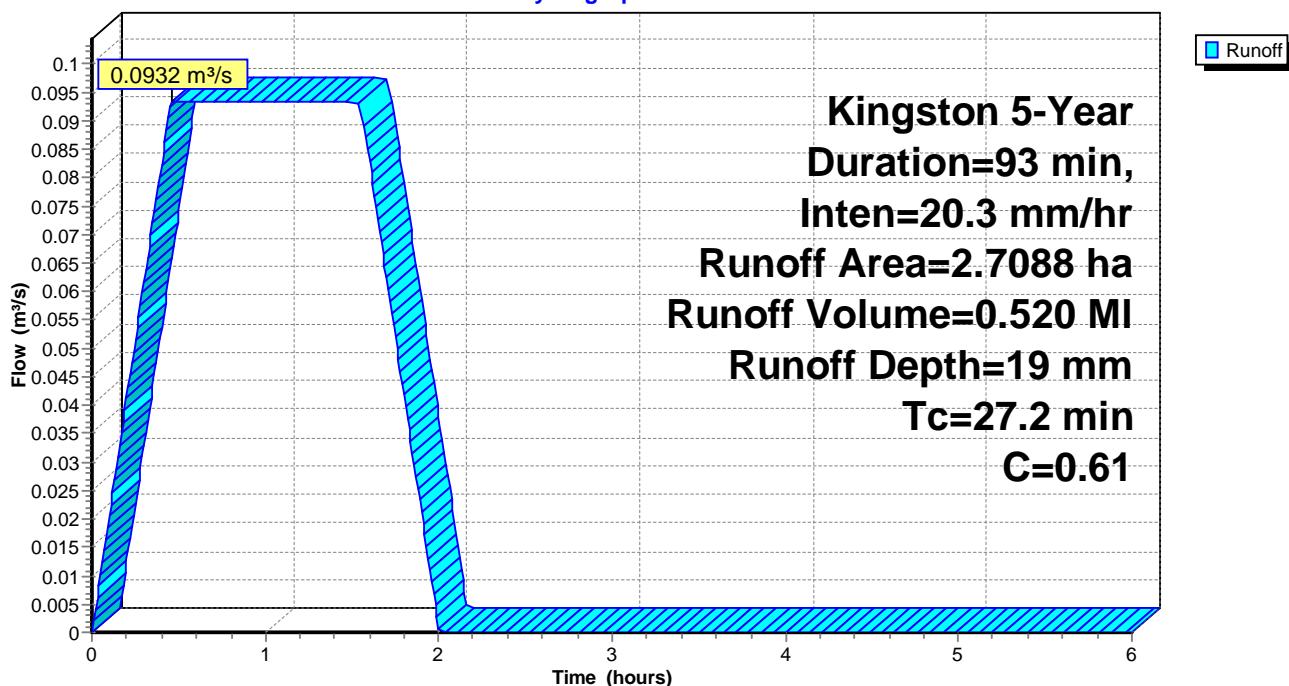
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=93 min, Inten=20.3 mm/hr

Area (ha)	C	Description
0.3775	0.80	Gravel
0.7307	0.90	Asphalt / Concrete
0.0000	0.01	Manure Tank
0.4356	0.90	Roof
1.1650	0.25	Grass
2.7088	0.61	Weighted Average
2.7088		100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 5S: B1-5

**Hydrograph**



### Summary for Subcatchment 6S: B1-6

Runoff = 0.0415 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.231 MI, Depth= 17 mm

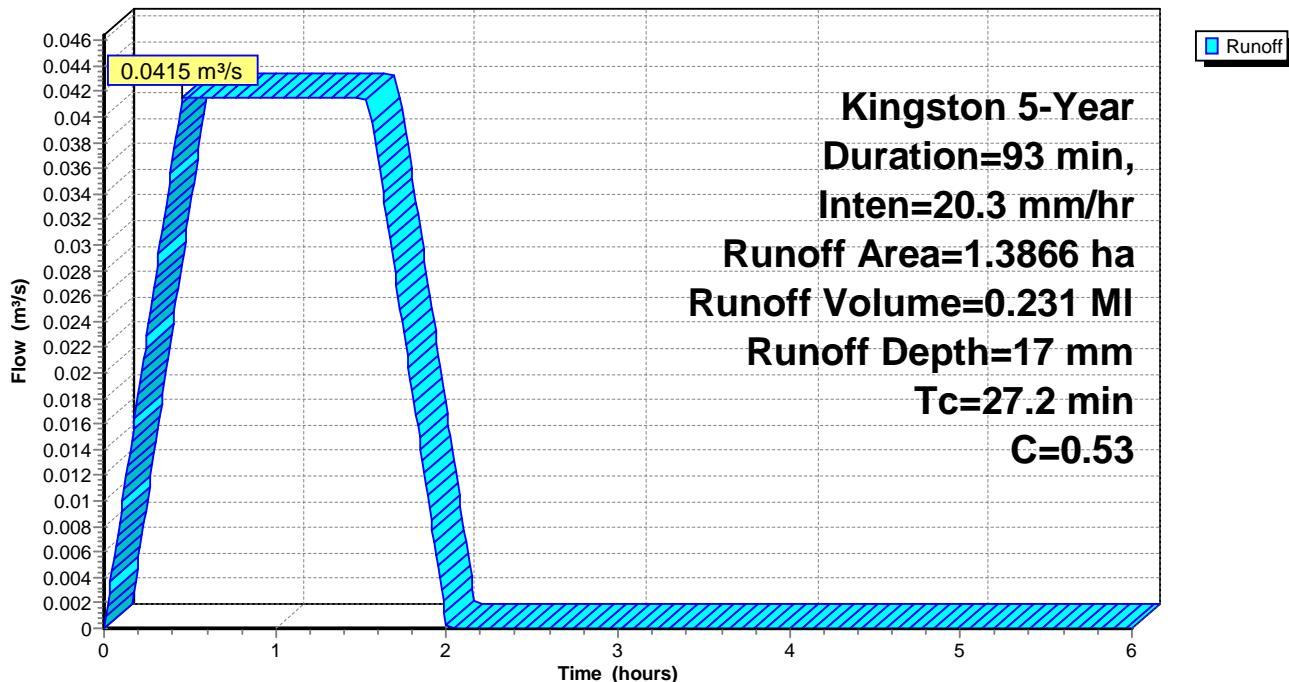
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=93 min, Inten=20.3 mm/hr

Area (ha)	C	Description
0.4917	0.80	Gravel
0.0289	0.90	Asphalt / Concrete
0.0000	0.01	Manure Tank
0.1561	0.90	Roof
0.7099	0.25	Grass
1.3866	0.53	Weighted Average
1.3866		100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 6S: B1-6

**Hydrograph**



### Summary for Subcatchment 7S: B1-7

Runoff = 0.1025 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.572 MI, Depth= 11 mm

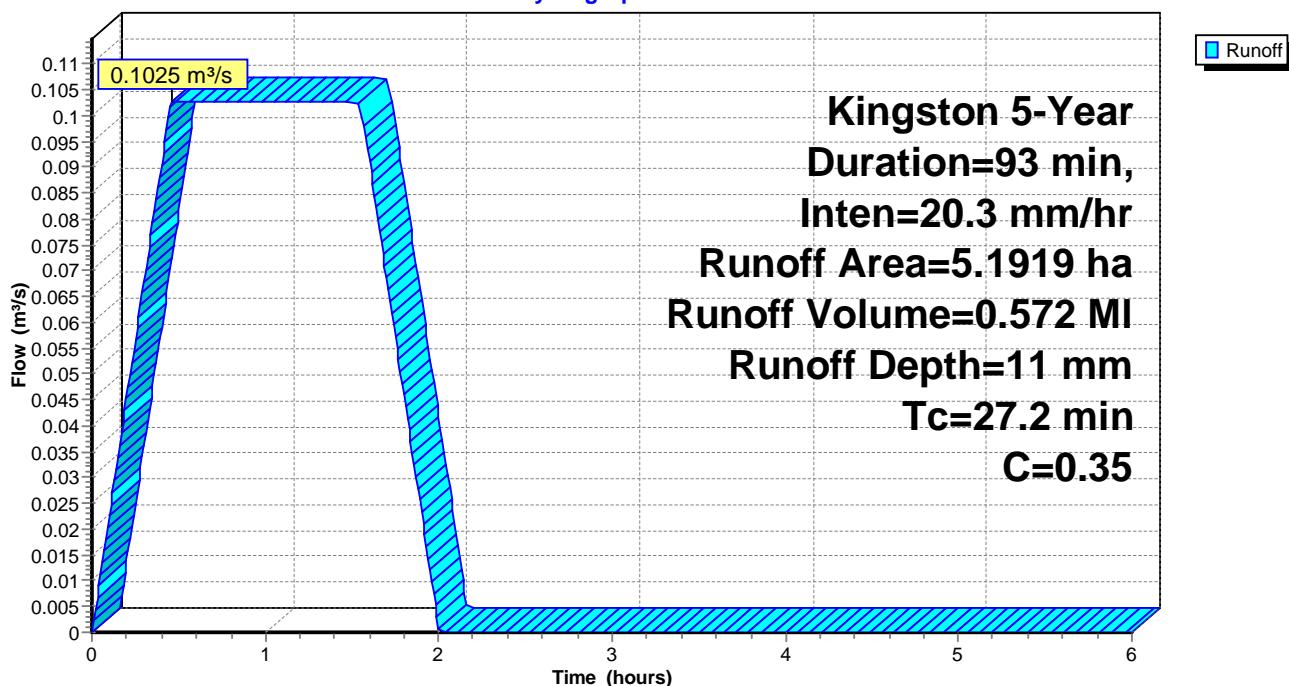
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=93 min, Inten=20.3 mm/hr

Area (ha)	C	Description
0.4987	0.80	Gravel
0.0217	0.90	Asphalt / Concrete
0.0274	0.01	Manure Tank
0.3550	0.90	Roof
4.2891	0.25	Grass
5.1919	0.35	Weighted Average
5.1919		100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 7S: B1-7

**Hydrograph**



### Summary for Subcatchment 8S: B1-8

Runoff = 0.0349 m<sup>3</sup>/s @ 0.25 hrs, Volume= 0.195 MI, Depth= 8 mm

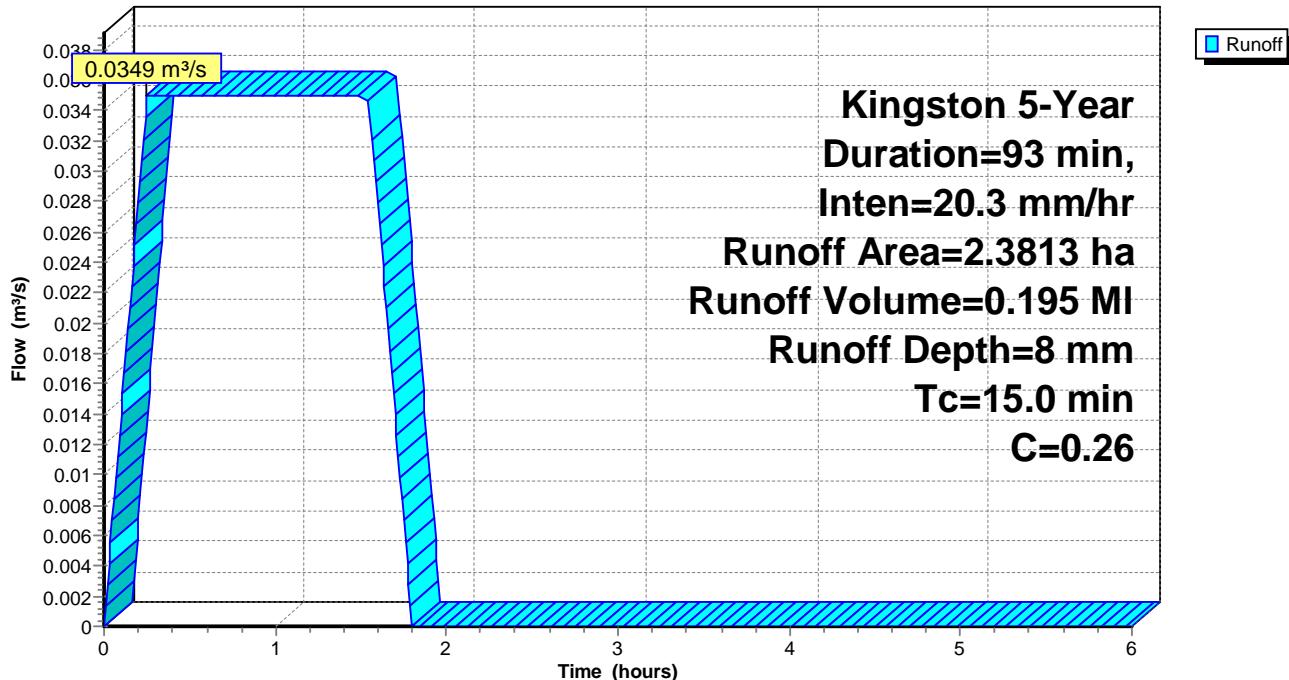
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=93 min, Inten=20.3 mm/hr

Area (ha)	C	Description
0.0513	0.80	Gravel
0.0000	0.90	Asphalt/Concrete
0.0000	0.01	Manure Tank
0.0000	0.90	Roof
2.3300	0.25	Grass
2.3813	0.26	Weighted Average
2.3813		100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
15.0	Direct Entry, Minimum				

### Subcatchment 8S: B1-8

**Hydrograph**



### Summary for Pond P: Retention Area

Inflow Area = 16.9056 ha, 0.00% Impervious, Inflow Depth = 13 mm for 5-Year event  
 Inflow = 0.3999 m<sup>3</sup>/s @ 0.46 hrs, Volume= 2.231 MI  
 Outflow = 0.1675 m<sup>3</sup>/s @ 1.81 hrs, Volume= 1.944 MI, Atten= 58%, Lag= 81.2 min  
 Primary = 0.1675 m<sup>3</sup>/s @ 1.81 hrs, Volume= 1.944 MI  
 Secondary = 0.0000 m<sup>3</sup>/s @ 0.00 hrs, Volume= 0.000 MI

Routing by Stor-Ind method, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Peak Elev= 123.483 m @ 1.81 hrs Surf.Area= 0.0 m<sup>2</sup> Storage= 1,586.2 m<sup>3</sup>

Plug-Flow detention time= 109.6 min calculated for 1.940 MI (87% of inflow)  
 Center-of-Mass det. time= 103.5 min ( 163.6 - 60.1 )

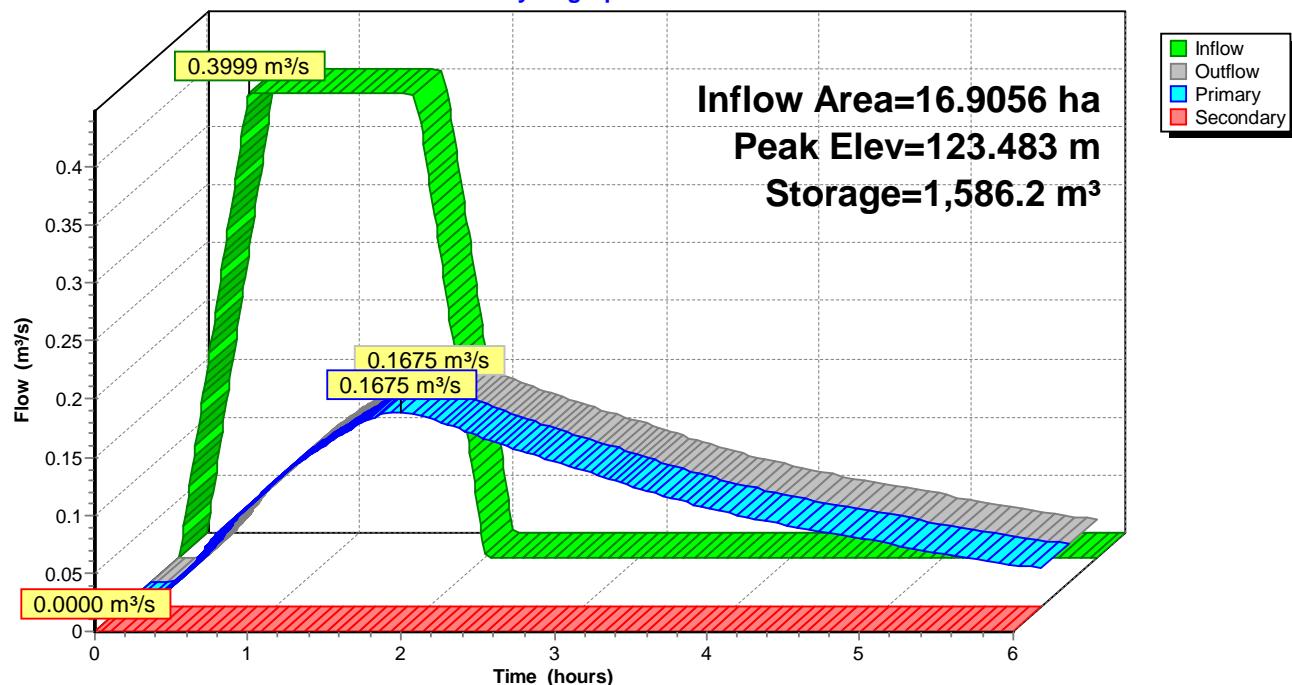
Volume	Invert	Avail.Storage	Storage Description
#1	123.100 m	3,743.6 m <sup>3</sup>	<b>Custom Stage Data</b> Listed below

Elevation (meters)	Cum.Store (cubic-meters)
123.100	0.0
123.200	126.7
123.300	461.6
123.400	1,010.1
123.500	1,708.2
123.600	2,421.7
123.700	3,166.9
123.775	3,743.6

Device	Routing	Invert	Outlet Devices
#1	Primary	123.100 m	<b>450 mm Vert. Orifice/Grate</b> C= 0.600
#2	Secondary	123.660 m	<b>Custom Weir/Orifice, Cv= 1.45 (C= 1.81)</b> Head (meters) 0.000 0.500 Width (meters) 5.00 8.00

**Primary OutFlow** Max=0.1675 m<sup>3</sup>/s @ 1.81 hrs HW=123.483 m (Free Discharge)  
 ↗1=Orifice/Grate (Orifice Controls 0.1675 m<sup>3</sup>/s @ 1.16 m/s)

**Secondary OutFlow** Max=0.0000 m<sup>3</sup>/s @ 0.00 hrs HW=123.100 m (Free Discharge)  
 ↗2=Custom Weir/Orifice ( Controls 0.0000 m<sup>3</sup>/s)

**Pond P: Retention Area****Hydrograph**

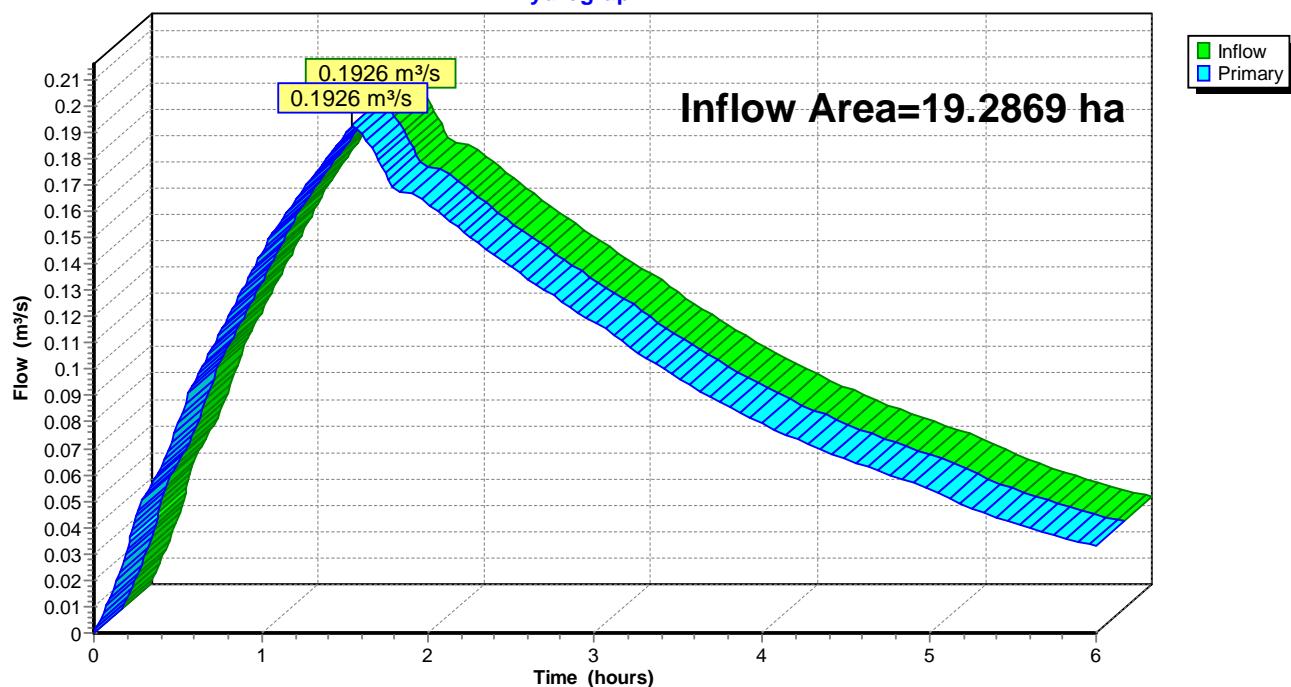
**Summary for Link 2L: Site Outlet**

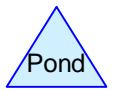
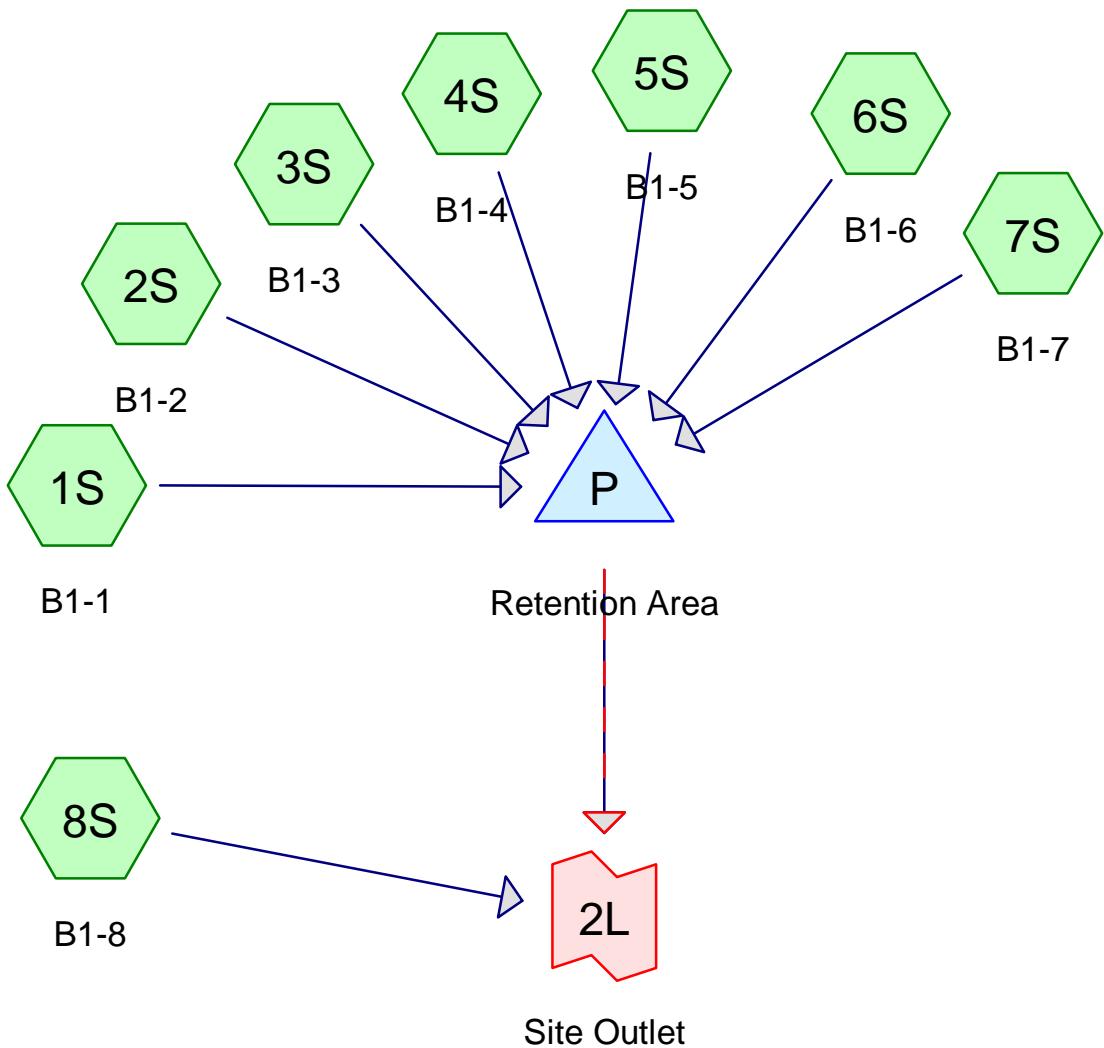
Inflow Area = 19.2869 ha, 0.00% Impervious, Inflow Depth > 11 mm for 5-Year event

Inflow = 0.1926 m<sup>3</sup>/s @ 1.55 hrs, Volume= 2.138 MI

Primary = 0.1926 m<sup>3</sup>/s @ 1.55 hrs, Volume= 2.138 MI, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs

**Link 2L: Site Outlet****Hydrograph**



**Routing Diagram for 191-08574-02 - PostDev\_CTL\_D\_v3**  
 Prepared by WSP Canada Inc., Printed 10/13/2021  
 HydroCAD® 10.00-24 s/n 10698 © 2018 HydroCAD Software Solutions LLC

**Area Listing (all nodes)**

Area (hectares)	C	Description (subcatchment-numbers)
0.8197	0.90	Asphalt / Concrete (2S, 3S, 4S, 5S, 6S, 7S)
14.2656	0.25	Grass (1S, 2S, 3S, 4S, 5S, 6S, 7S, 8S)
2.6264	0.80	Gravel (1S, 2S, 3S, 4S, 5S, 6S, 7S, 8S)
0.1513	0.01	Manure Tank (3S, 7S)
1.4239	0.90	Roof (1S, 2S, 3S, 4S, 5S, 6S, 7S)
<b>19.2869</b>	<b>0.40</b>	<b>TOTAL AREA</b>

### Summary for Subcatchment 1S: B1-1

Runoff = 0.0549 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.376 MI, Depth= 13 mm

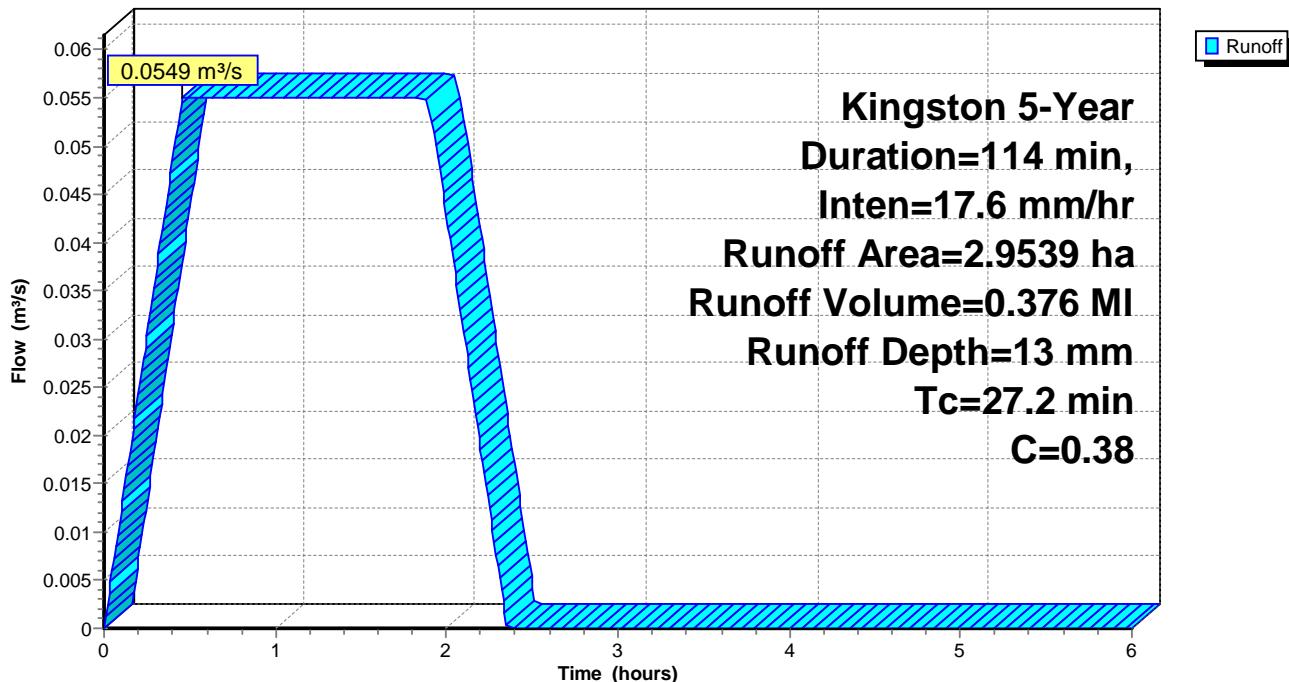
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=114 min, Inten=17.6 mm/hr

Area (ha)	C	Description
0.5181	0.80	Gravel
0.0000	0.90	Asphalt / Concrete
0.0000	0.01	Manure Tank
0.1539	0.90	Roof
2.2819	0.25	Grass
2.9539	0.38	Weighted Average
2.9539		100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2	Direct Entry, Per Ditch Design Sheet				

### Subcatchment 1S: B1-1

**Hydrograph**



### Summary for Subcatchment 2S: B1-2

Runoff = 0.0150 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.103 MI, Depth= 17 mm

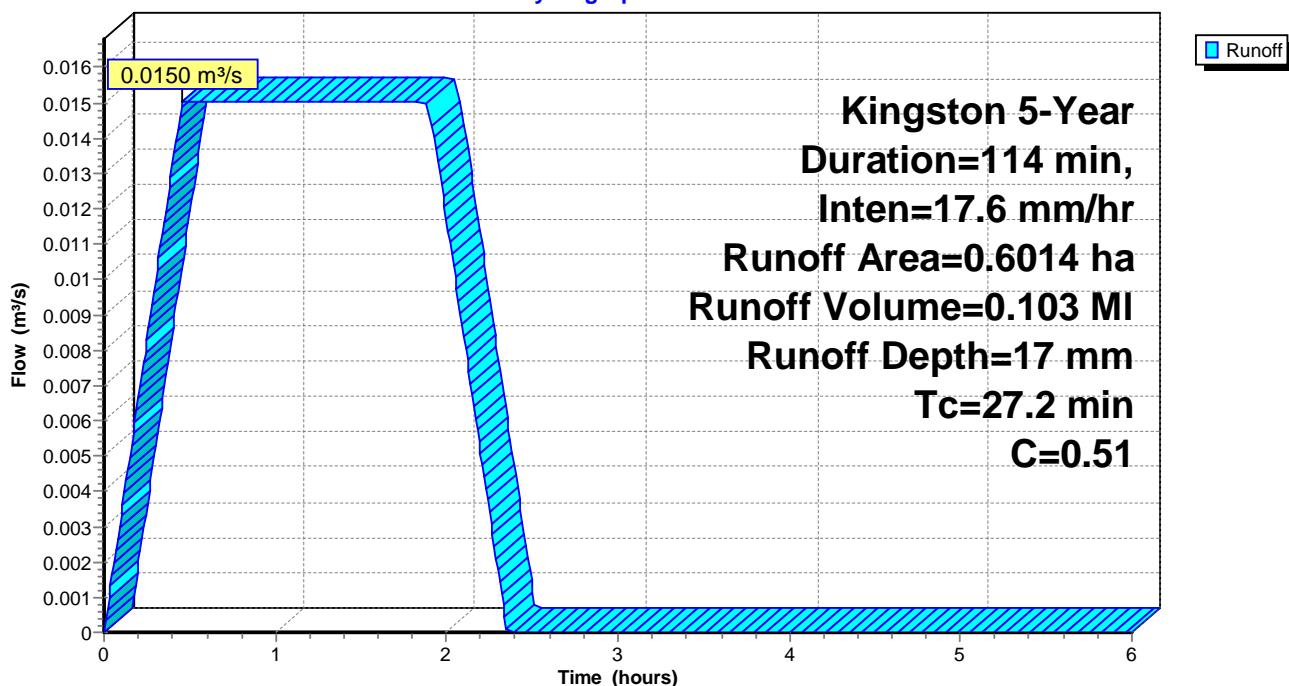
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=114 min, Inten=17.6 mm/hr

Area (ha)	C	Description
0.1347	0.80	Gravel
0.0046	0.90	Asphalt / Concrete
0.0000	0.01	Manure Tank
0.1175	0.90	Roof
0.3446	0.25	Grass
0.6014	0.51	Weighted Average
0.6014		100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 2S: B1-2

**Hydrograph**



### Summary for Subcatchment 3S: B1-3

Runoff = 0.0282 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.193 MI, Depth= 11 mm

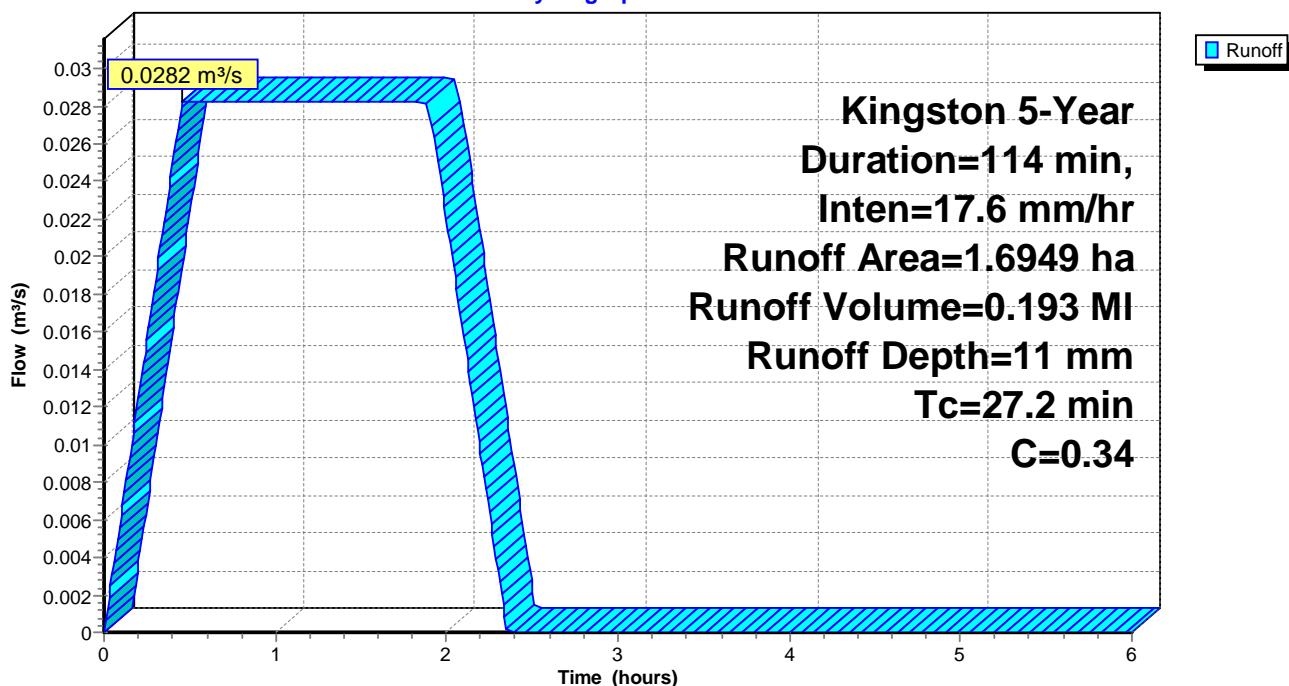
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=114 min, Inten=17.6 mm/hr

Area (ha)	C	Description
0.2254	0.80	Gravel
0.0070	0.90	Asphalt / Concrete
0.1239	0.01	Manure Tank
0.0809	0.90	Roof
1.2577	0.25	Grass
1.6949	0.34	Weighted Average
1.6949		100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 3S: B1-3

**Hydrograph**



### Summary for Subcatchment 4S: B1-4

Runoff = 0.0429 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.293 MI, Depth= 12 mm

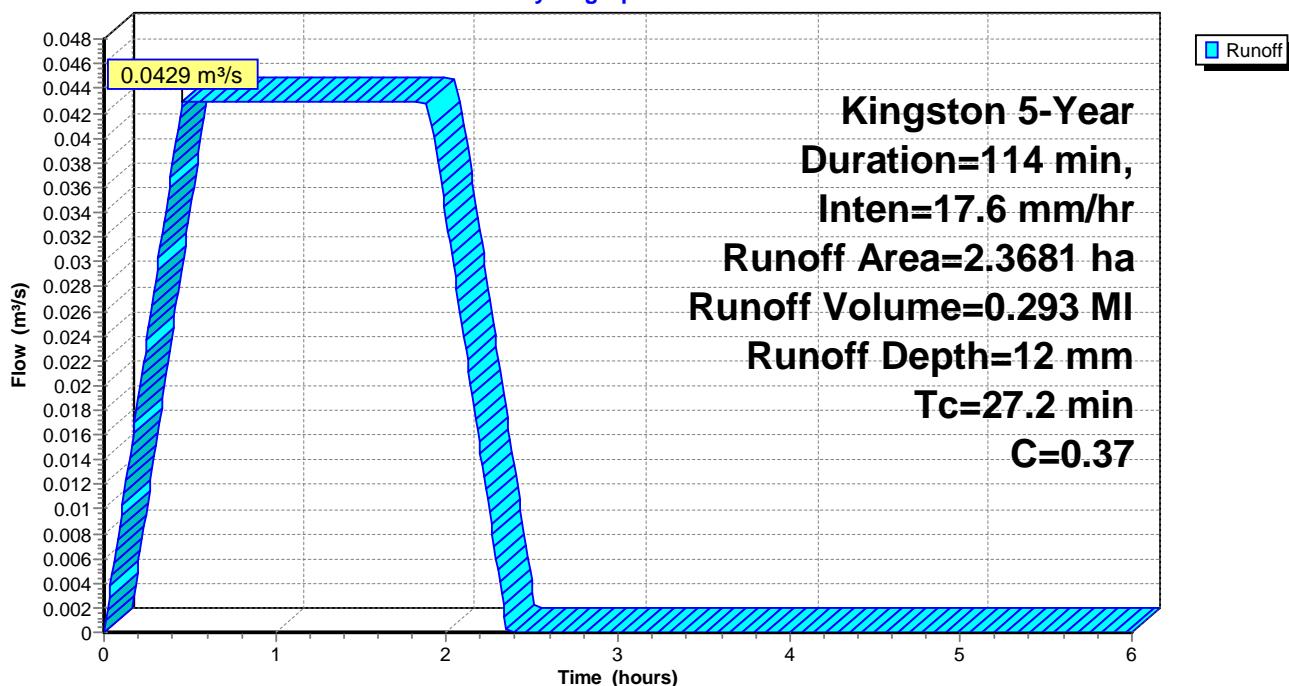
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=114 min, Inten=17.6 mm/hr

Area (ha)	C	Description
0.3290	0.80	Gravel
0.0268	0.90	Asphalt / Concrete
0.0000	0.01	Manure Tank
0.1249	0.90	Roof
1.8874	0.25	Grass
2.3681	0.37	Weighted Average
2.3681		100.00% Pervious Area

Tc (min)	Length (meters)	Slope (m/m)	Velocity (m/sec)	Capacity (m <sup>3</sup> /s)	Description
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 4S: B1-4

**Hydrograph**



### Summary for Subcatchment 5S: B1-5

Runoff = 0.0809 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.553 MI, Depth= 20 mm

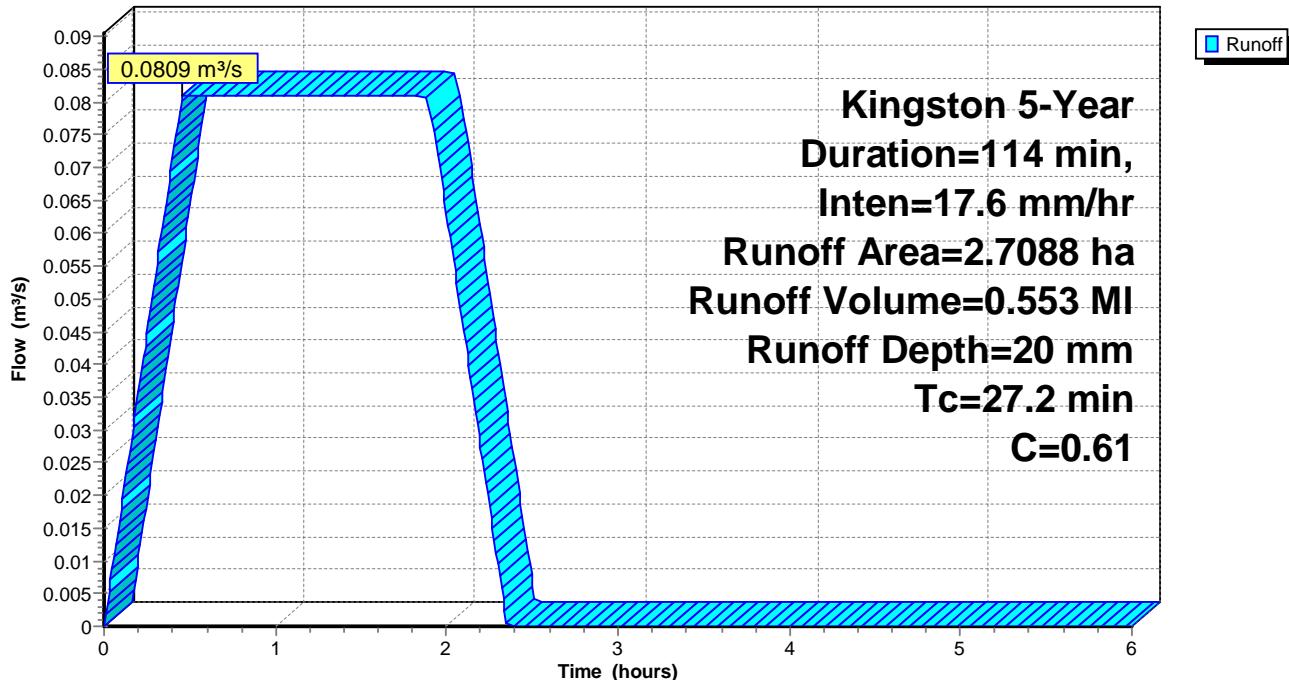
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=114 min, Inten=17.6 mm/hr

Area (ha)	C	Description
0.3775	0.80	Gravel
0.7307	0.90	Asphalt / Concrete
0.0000	0.01	Manure Tank
0.4356	0.90	Roof
1.1650	0.25	Grass
2.7088	0.61	Weighted Average
2.7088		100.00% Pervious Area

Tc (min)	Length (meters)	Slope (m/m)	Velocity (m/sec)	Capacity (m <sup>3</sup> /s)	Description
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 5S: B1-5

**Hydrograph**



### Summary for Subcatchment 6S: B1-6

Runoff = 0.0360 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.246 MI, Depth= 18 mm

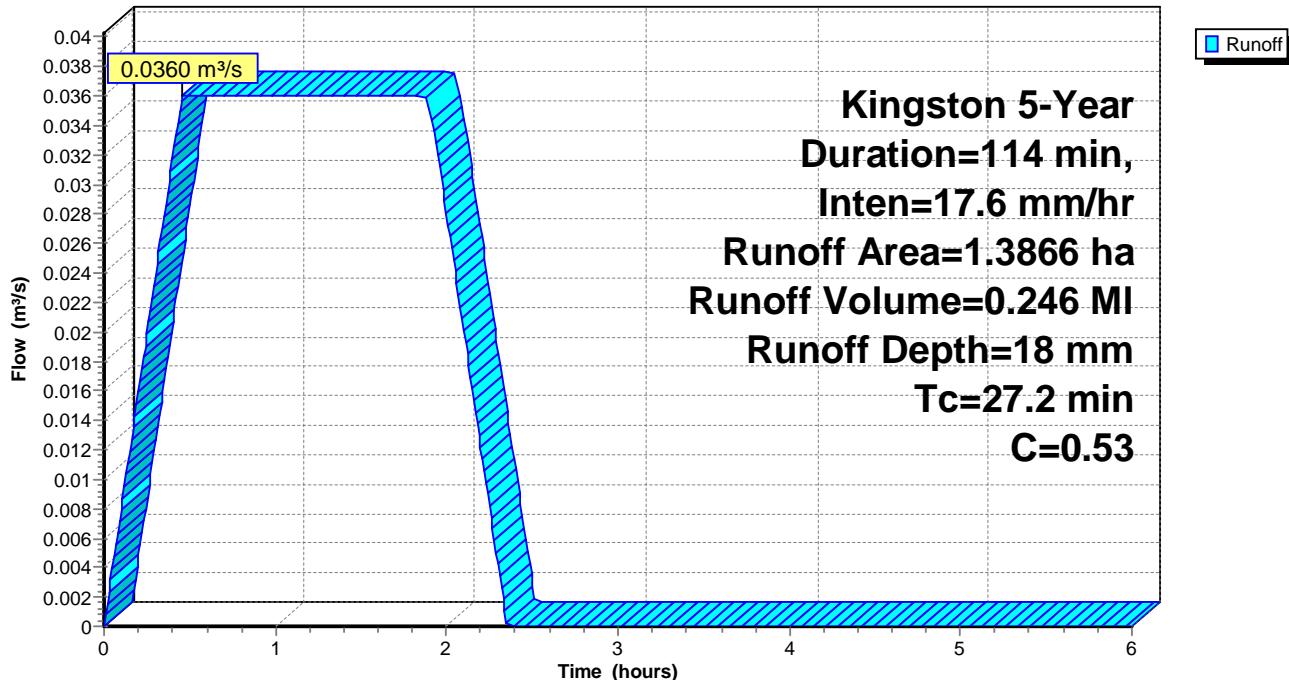
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=114 min, Inten=17.6 mm/hr

Area (ha)	C	Description
0.4917	0.80	Gravel
0.0289	0.90	Asphalt / Concrete
0.0000	0.01	Manure Tank
0.1561	0.90	Roof
0.7099	0.25	Grass
1.3866	0.53	Weighted Average
1.3866		100.00% Pervious Area

Tc (min)	Length (meters)	Slope (m/m)	Velocity (m/sec)	Capacity (m <sup>3</sup> /s)	Description
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 6S: B1-6

**Hydrograph**



### Summary for Subcatchment 7S: B1-7

Runoff = 0.0889 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.608 MI, Depth= 12 mm

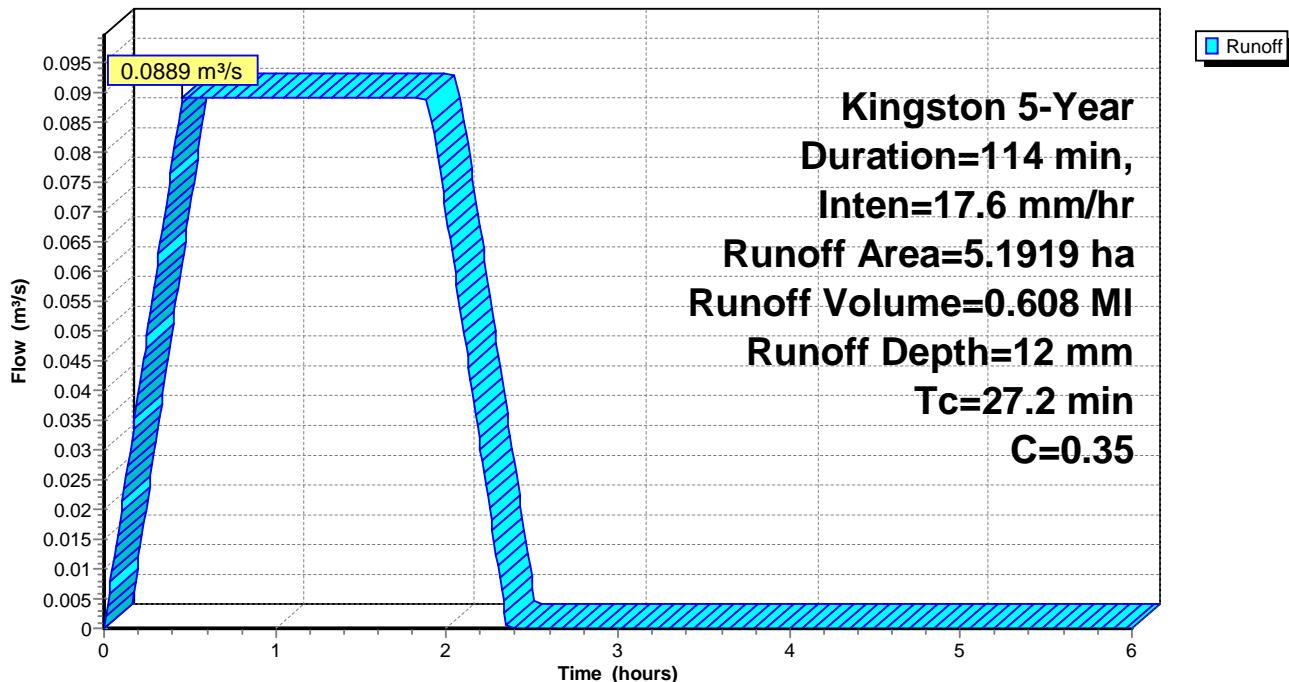
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=114 min, Inten=17.6 mm/hr

Area (ha)	C	Description
0.4987	0.80	Gravel
0.0217	0.90	Asphalt / Concrete
0.0274	0.01	Manure Tank
0.3550	0.90	Roof
4.2891	0.25	Grass
5.1919	0.35	Weighted Average
5.1919		100.00% Pervious Area

Tc (min)	Length (meters)	Slope (m/m)	Velocity (m/sec)	Capacity (m <sup>3</sup> /s)	Description
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 7S: B1-7

**Hydrograph**



### Summary for Subcatchment 8S: B1-8

Runoff = 0.0303 m<sup>3</sup>/s @ 0.25 hrs, Volume= 0.207 MI, Depth= 9 mm

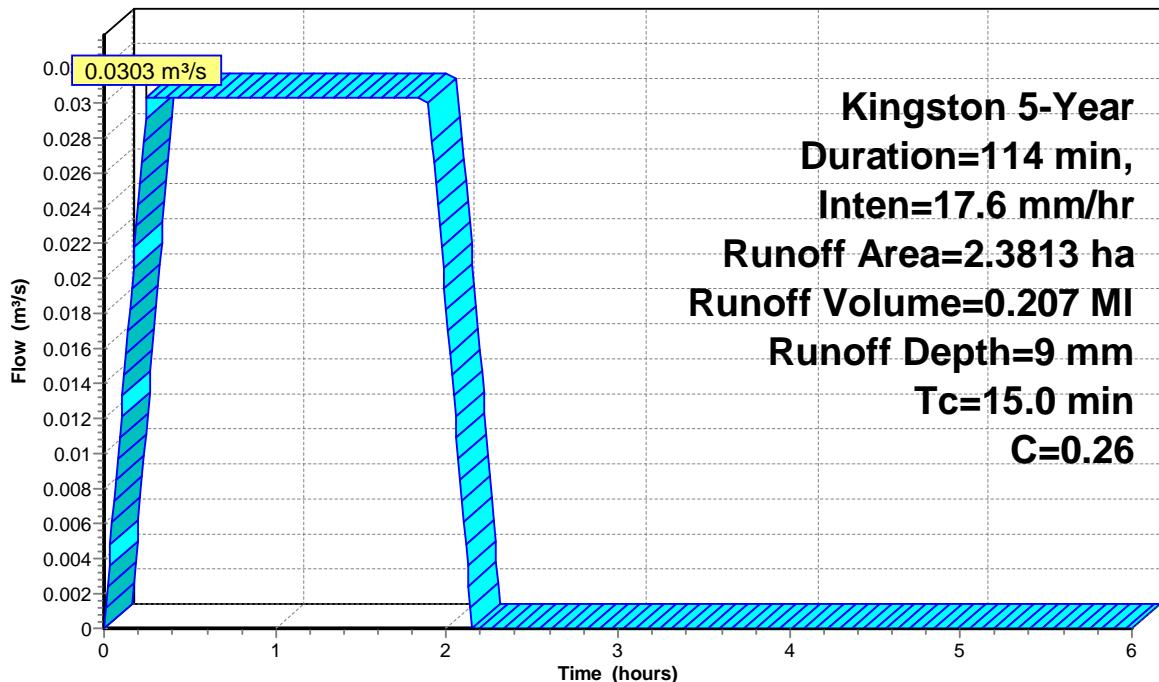
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 5-Year Duration=114 min, Inten=17.6 mm/hr

Area (ha)	C	Description
0.0513	0.80	Gravel
0.0000	0.90	Asphalt/Concrete
0.0000	0.01	Manure Tank
0.0000	0.90	Roof
2.3300	0.25	Grass
2.3813	0.26	Weighted Average
2.3813		100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
15.0	Direct Entry, Minimum				

### Subcatchment 8S: B1-8

**Hydrograph**



### Summary for Pond P: Retention Area

Inflow Area = 16.9056 ha, 0.00% Impervious, Inflow Depth = 14 mm for 5-Year event  
 Inflow = 0.3468 m<sup>3</sup>/s @ 0.46 hrs, Volume= 2.372 MI  
 Outflow = 0.1683 m<sup>3</sup>/s @ 2.13 hrs, Volume= 2.039 MI, Atten= 51%, Lag= 100.4 min  
 Primary = 0.1683 m<sup>3</sup>/s @ 2.13 hrs, Volume= 2.039 MI  
 Secondary = 0.0000 m<sup>3</sup>/s @ 0.00 hrs, Volume= 0.000 MI

Routing by Stor-Ind method, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Peak Elev= 123.484 m @ 2.13 hrs Surf.Area= 0.0 m<sup>2</sup> Storage= 1,595.9 m<sup>3</sup>

Plug-Flow detention time= 109.3 min calculated for 2.039 MI (86% of inflow)  
 Center-of-Mass det. time= 101.0 min ( 171.6 - 70.6 )

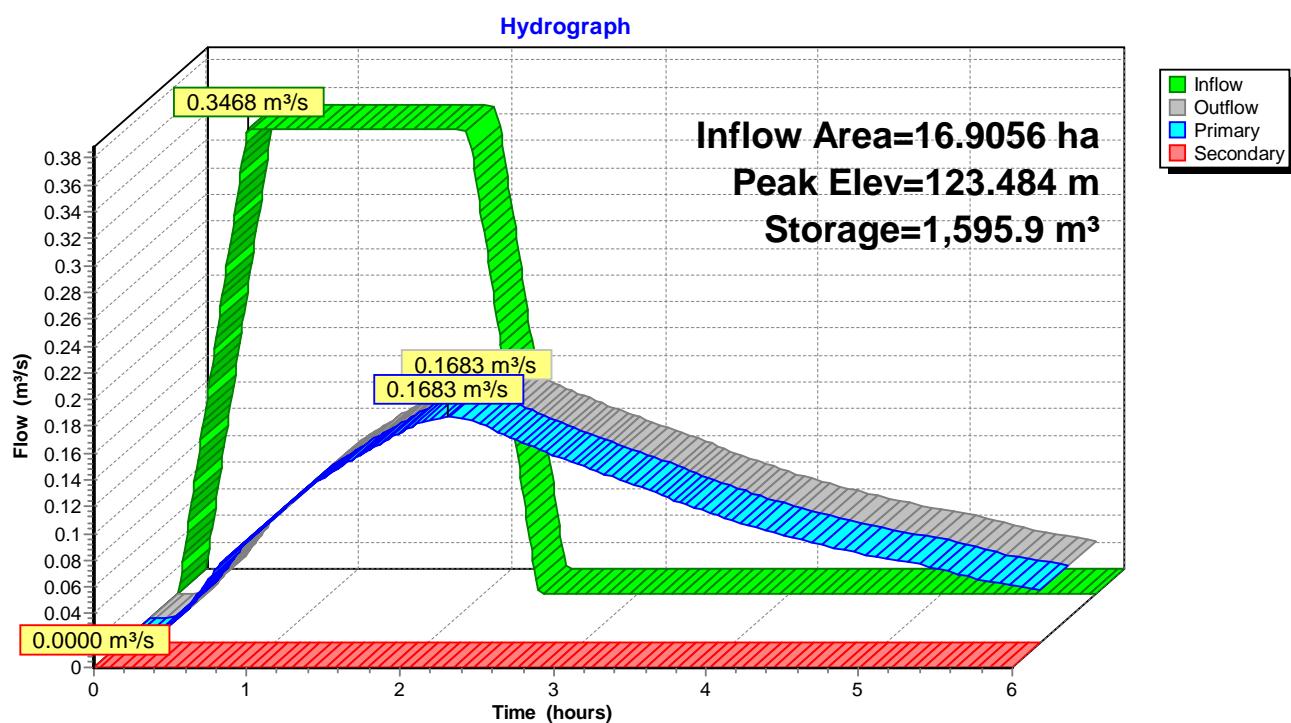
Volume	Invert	Avail.Storage	Storage Description
#1	123.100 m	3,743.6 m <sup>3</sup>	<b>Custom Stage Data</b> Listed below

Elevation (meters)	Cum.Store (cubic-meters)
123.100	0.0
123.200	126.7
123.300	461.6
123.400	1,010.1
123.500	1,708.2
123.600	2,421.7
123.700	3,166.9
123.775	3,743.6

Device	Routing	Invert	Outlet Devices
#1	Primary	123.100 m	<b>450 mm Vert. Orifice/Grate</b> C= 0.600
#2	Secondary	123.660 m	<b>Custom Weir/Orifice, Cv= 1.45 (C= 1.81)</b> Head (meters) 0.000 0.500 Width (meters) 5.00 8.00

**Primary OutFlow** Max=0.1683 m<sup>3</sup>/s @ 2.13 hrs HW=123.484 m (Free Discharge)  
 ↪1=Orifice/Grate (Orifice Controls 0.1683 m<sup>3</sup>/s @ 1.16 m/s)

**Secondary OutFlow** Max=0.0000 m<sup>3</sup>/s @ 0.00 hrs HW=123.100 m (Free Discharge)  
 ↪2=Custom Weir/Orifice (Controls 0.0000 m<sup>3</sup>/s)

**Pond P: Retention Area**

### Summary for Link 2L: Site Outlet

Inflow Area = 19.2869 ha, 0.00% Impervious, Inflow Depth > 12 mm for 5-Year event

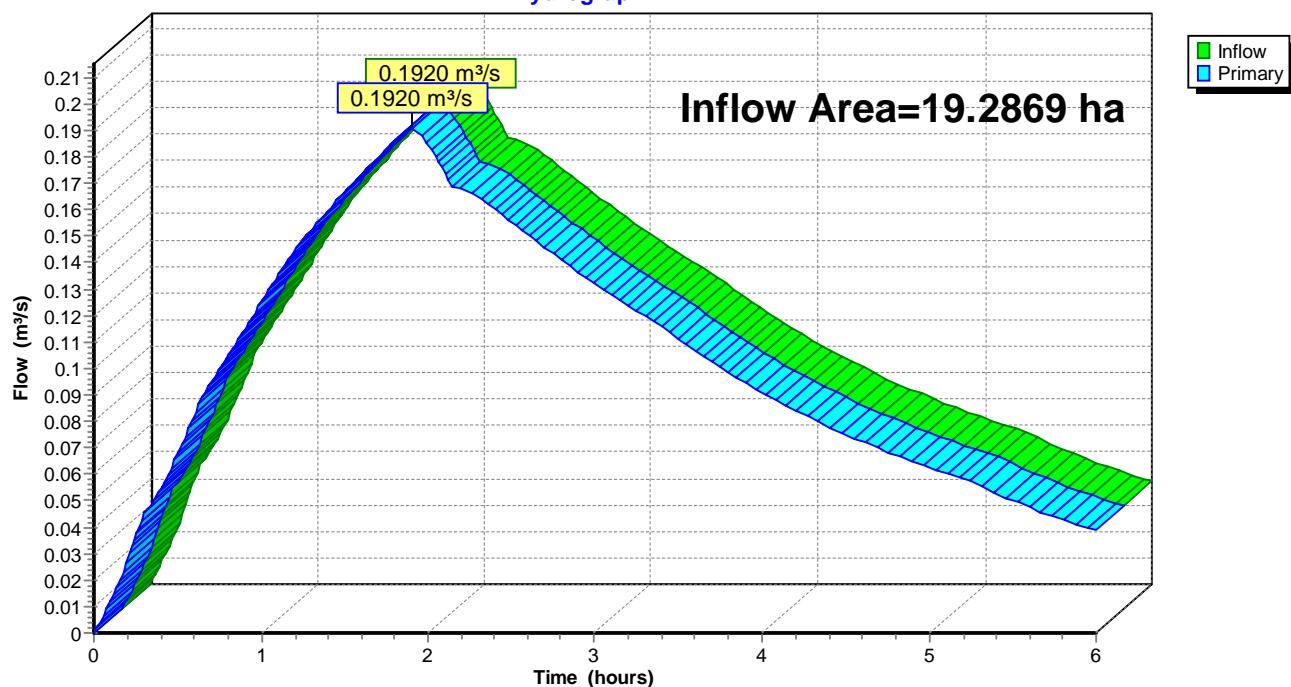
Inflow = 0.1920 m<sup>3</sup>/s @ 1.90 hrs, Volume= 2.246 MI

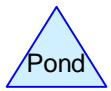
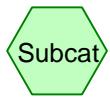
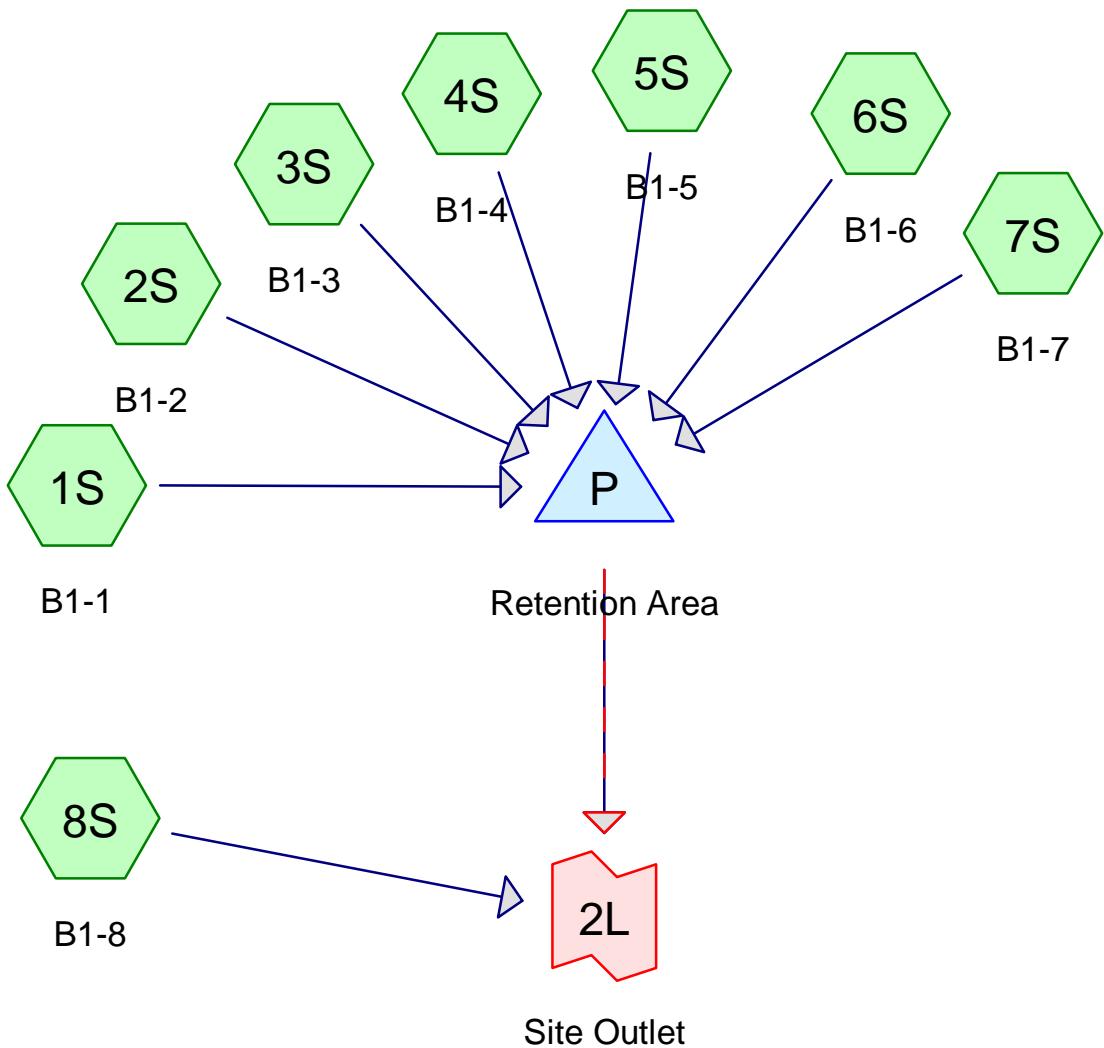
Primary = 0.1920 m<sup>3</sup>/s @ 1.90 hrs, Volume= 2.246 MI, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs

### Link 2L: Site Outlet

Hydrograph





**Routing Diagram for 191-08574-02 - PostDev\_CTL\_D\_v3**  
 Prepared by WSP Canada Inc., Printed 10/13/2021  
 HydroCAD® 10.00-24 s/n 10698 © 2018 HydroCAD Software Solutions LLC

**Area Listing (all nodes)**

Area (hectares)	C	Description (subcatchment-numbers)
0.8197	0.90	Asphalt / Concrete (2S, 3S, 4S, 5S, 6S, 7S)
14.2656	0.25	Grass (1S, 2S, 3S, 4S, 5S, 6S, 7S, 8S)
2.6264	0.80	Gravel (1S, 2S, 3S, 4S, 5S, 6S, 7S, 8S)
0.1513	0.01	Manure Tank (3S, 7S)
1.4239	0.90	Roof (1S, 2S, 3S, 4S, 5S, 6S, 7S)
<b>19.2869</b>	<b>0.40</b>	<b>TOTAL AREA</b>

### Summary for Subcatchment 1S: B1-1

Runoff = 0.1025 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.812 MI, Depth= 27 mm

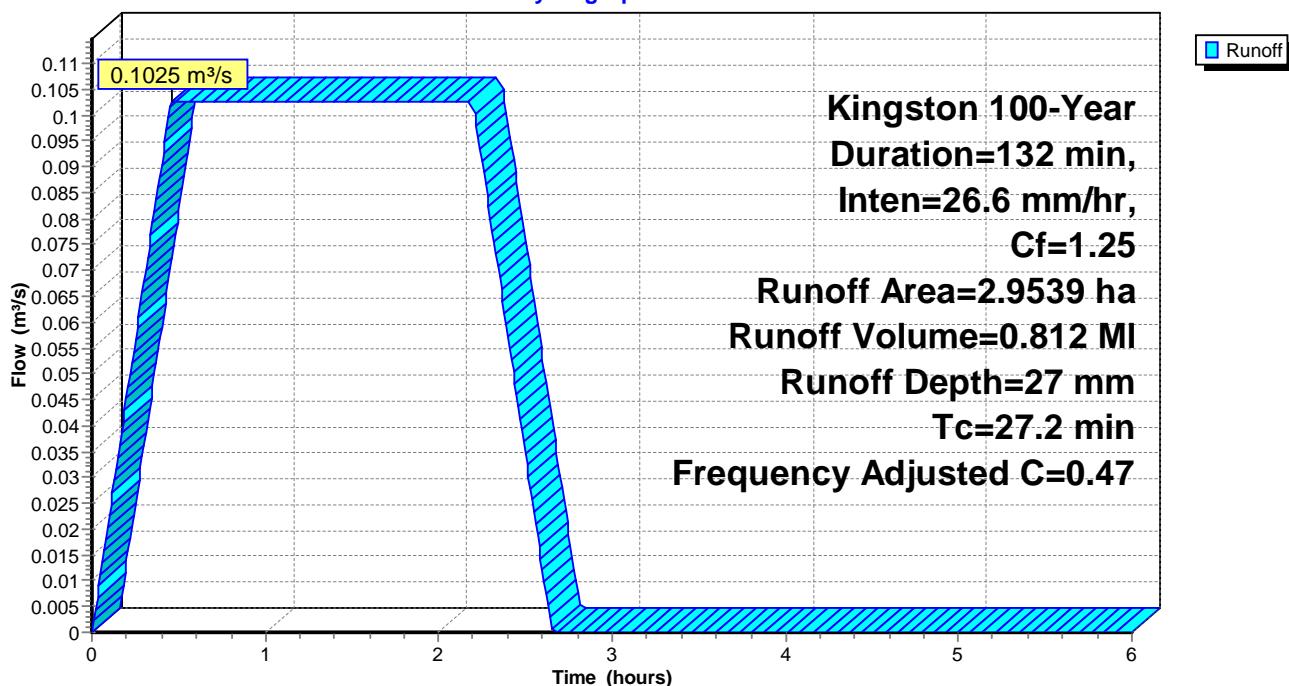
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
Kingston 100-Year Duration=132 min, Inten=26.6 mm/hr, Cf=1.25

Area (ha)	C	Adj	Description
0.5181	0.80		Gravel
0.0000	0.90		Asphalt / Concrete
0.0000	0.01		Manure Tank
0.1539	0.90		Roof
2.2819	0.25		Grass
2.9539	0.38	0.47	Weighted Average, Frequency Adjusted
2.9539			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Per Ditch Design Sheet

### Subcatchment 1S: B1-1

Hydrograph



### Summary for Subcatchment 2S: B1-2

Runoff = 0.0284 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.225 MI, Depth= 37 mm

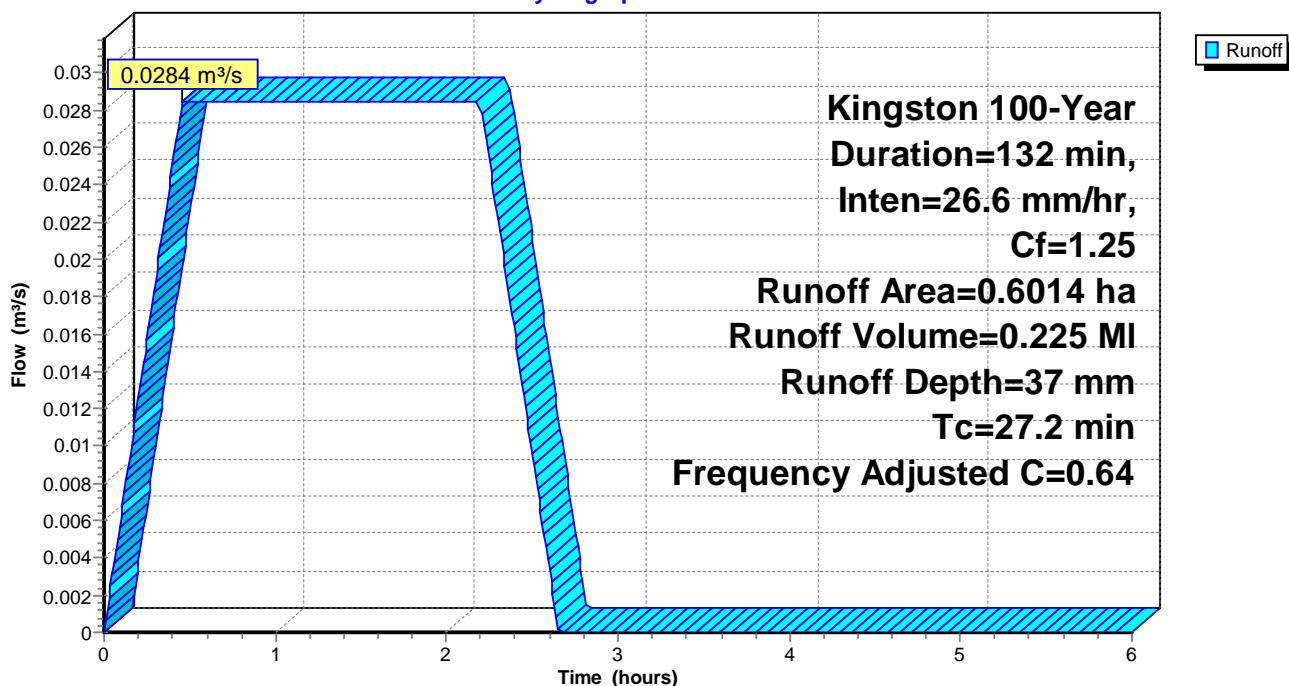
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 100-Year Duration=132 min, Inten=26.6 mm/hr, Cf=1.25

Area (ha)	C	Adj	Description
0.1347	0.80		Gravel
0.0046	0.90		Asphalt / Concrete
0.0000	0.01		Manure Tank
0.1175	0.90		Roof
0.3446	0.25		Grass
0.6014	0.51	0.64	Weighted Average, Frequency Adjusted
0.6014			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 2S: B1-2

**Hydrograph**



### Summary for Subcatchment 3S: B1-3

Runoff = 0.0538 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.426 MI, Depth= 25 mm

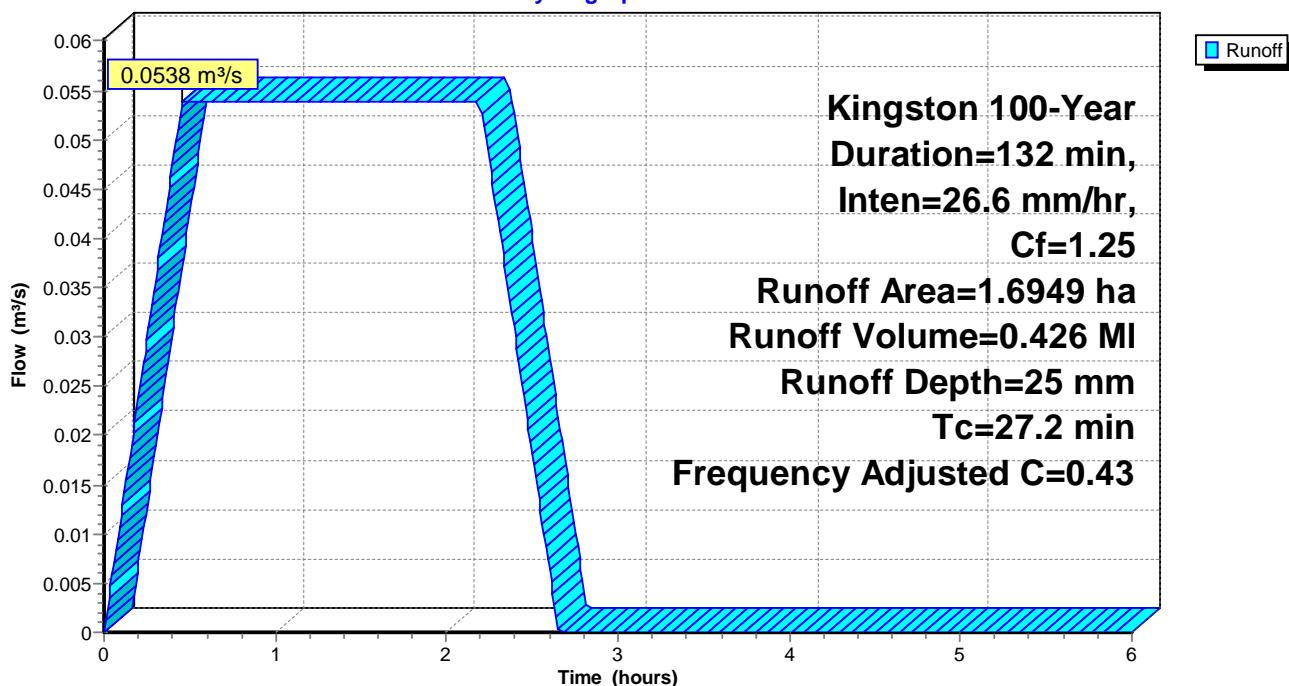
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
Kingston 100-Year Duration=132 min, Inten=26.6 mm/hr, Cf=1.25

Area (ha)	C	Adj	Description
0.2254	0.80		Gravel
0.0070	0.90		Asphalt / Concrete
0.1239	0.01		Manure Tank
0.0809	0.90		Roof
1.2577	0.25		Grass
1.6949	0.34	0.43	Weighted Average, Frequency Adjusted
1.6949			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 3S: B1-3

Hydrograph



### Summary for Subcatchment 4S: B1-4

Runoff = 0.0804 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.637 MI, Depth= 27 mm

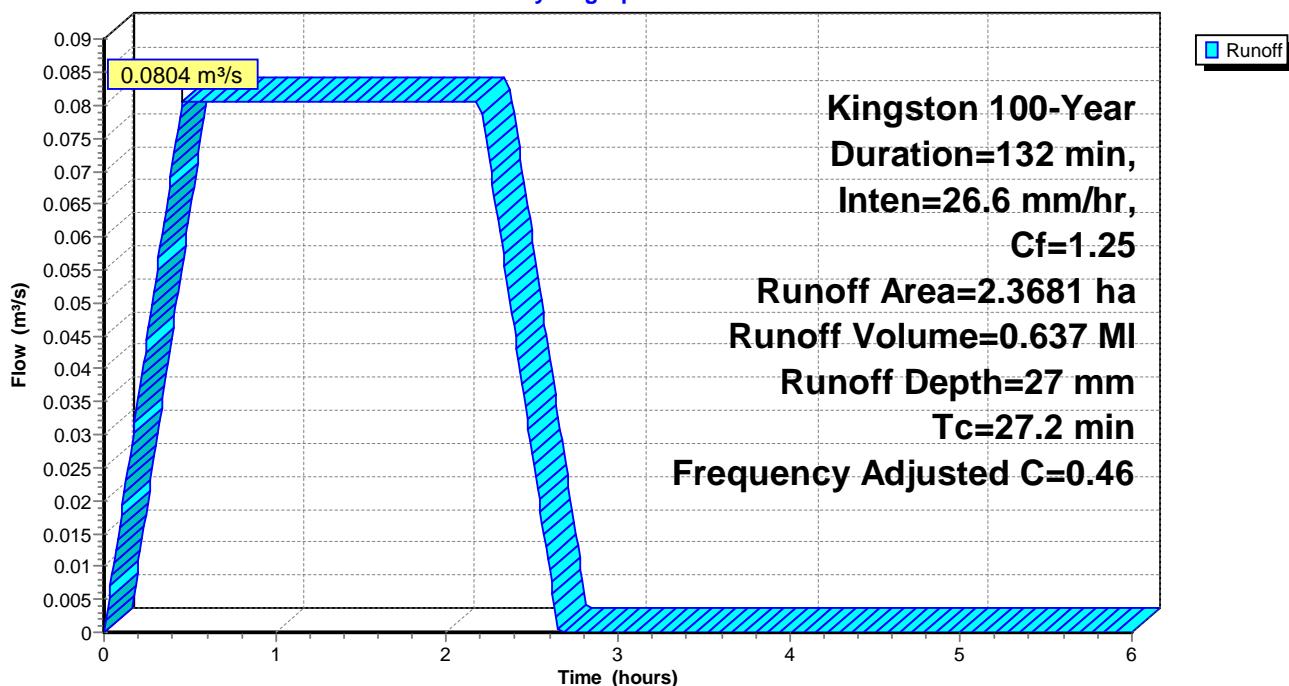
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 100-Year Duration=132 min, Inten=26.6 mm/hr, Cf=1.25

Area (ha)	C	Adj	Description
0.3290	0.80		Gravel
0.0268	0.90		Asphalt / Concrete
0.0000	0.01		Manure Tank
0.1249	0.90		Roof
1.8874	0.25		Grass
2.3681	0.37	0.46	Weighted Average, Frequency Adjusted
2.3681			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 4S: B1-4

Hydrograph



### Summary for Subcatchment 5S: B1-5

Runoff = 0.1520 m<sup>3</sup>/s @ 0.46 hrs, Volume= 1.204 MI, Depth= 44 mm

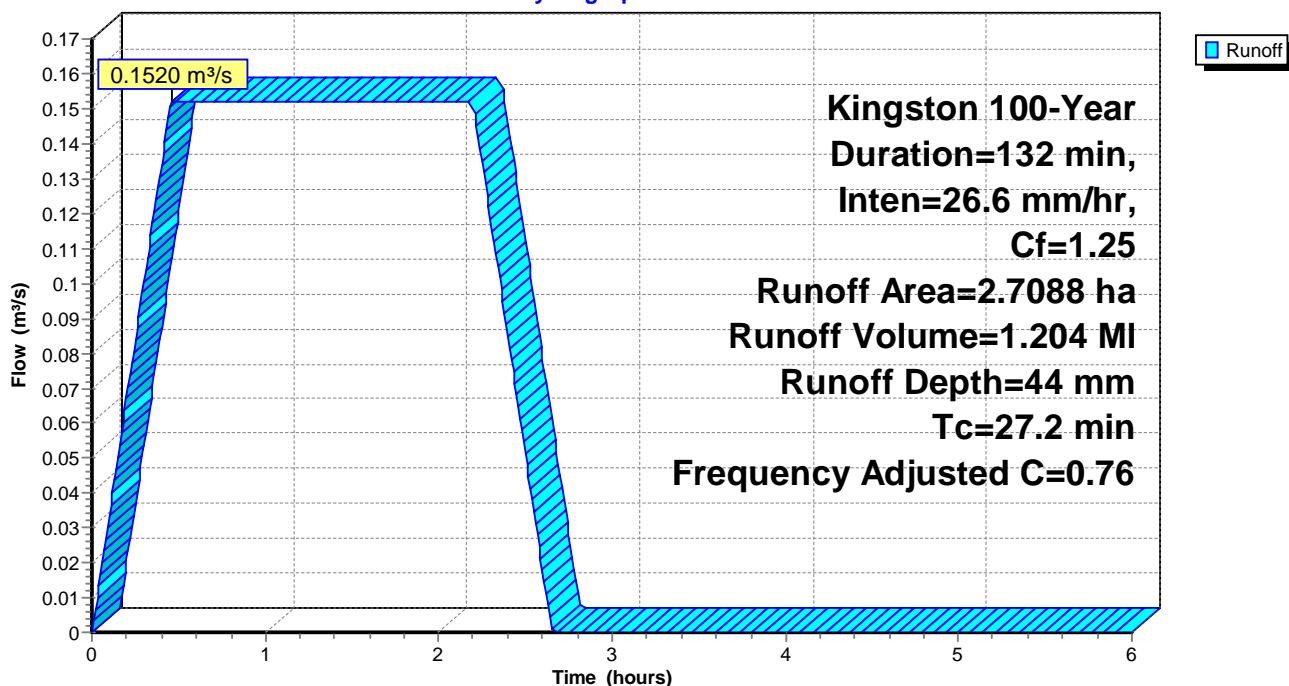
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 100-Year Duration=132 min, Inten=26.6 mm/hr, Cf=1.25

Area (ha)	C	Adj	Description
0.3775	0.80		Gravel
0.7307	0.90		Asphalt / Concrete
0.0000	0.01		Manure Tank
0.4356	0.90		Roof
1.1650	0.25		Grass
2.7088	0.61	0.76	Weighted Average, Frequency Adjusted
2.7088			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 5S: B1-5

**Hydrograph**



### Summary for Subcatchment 6S: B1-6

Runoff = 0.0676 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.535 MI, Depth= 39 mm

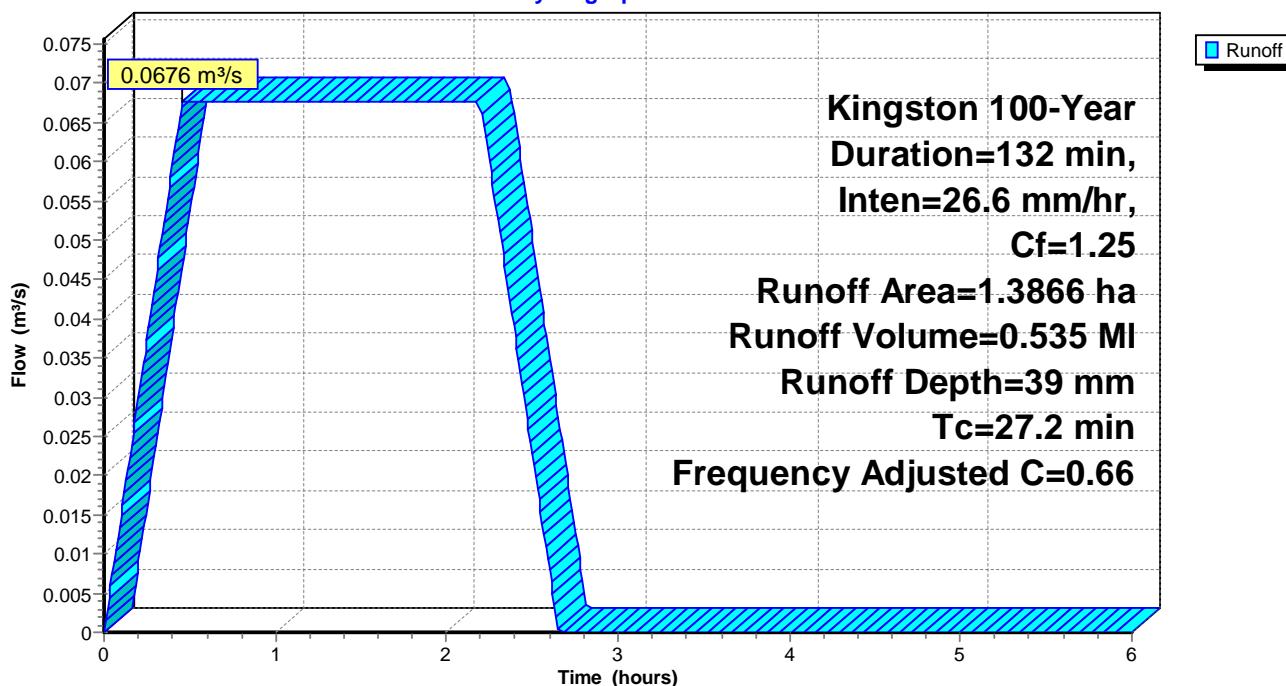
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
Kingston 100-Year Duration=132 min, Inten=26.6 mm/hr, Cf=1.25

Area (ha)	C	Adj	Description
0.4917	0.80		Gravel
0.0289	0.90		Asphalt / Concrete
0.0000	0.01		Manure Tank
0.1561	0.90		Roof
0.7099	0.25		Grass
1.3866	0.53	0.66	Weighted Average, Frequency Adjusted
1.3866			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 6S: B1-6

Hydrograph



### Summary for Subcatchment 7S: B1-7

Runoff = 0.1686 m<sup>3</sup>/s @ 0.46 hrs, Volume= 1.336 MI, Depth= 26 mm

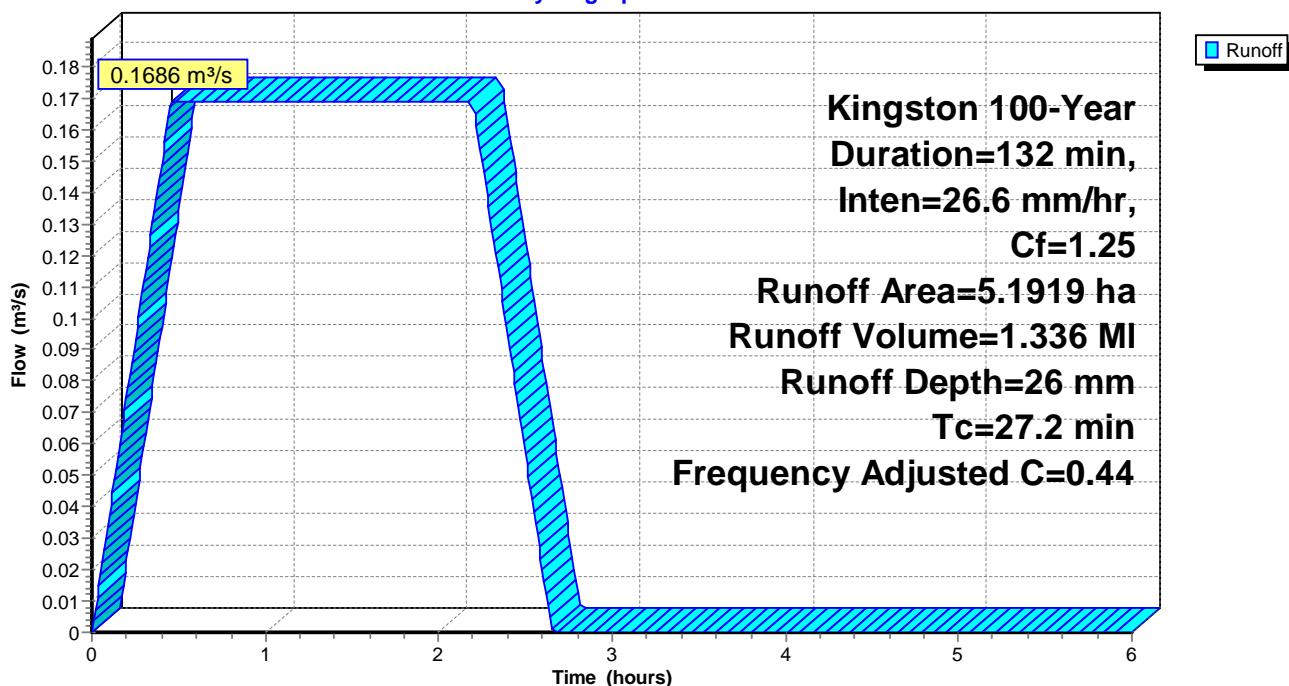
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 100-Year Duration=132 min, Inten=26.6 mm/hr, Cf=1.25

Area (ha)	C	Adj	Description
0.4987	0.80		Gravel
0.0217	0.90		Asphalt / Concrete
0.0274	0.01		Manure Tank
0.3550	0.90		Roof
4.2891	0.25		Grass
5.1919	0.35	0.44	Weighted Average, Frequency Adjusted
5.1919			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 7S: B1-7

**Hydrograph**



### Summary for Subcatchment 8S: B1-8

Runoff = 0.0580 m<sup>3</sup>/s @ 0.25 hrs, Volume= 0.459 MI, Depth= 19 mm

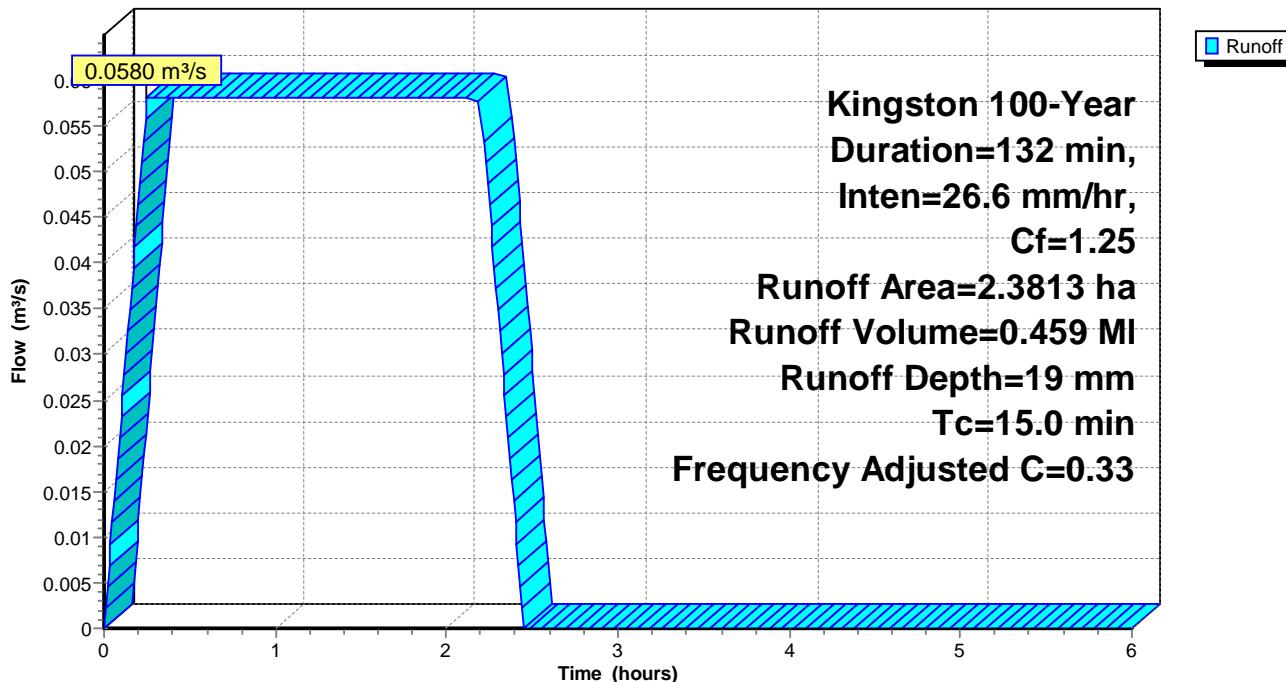
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 100-Year Duration=132 min, Inten=26.6 mm/hr, Cf=1.25

Area (ha)	C	Adj	Description
0.0513	0.80		Gravel
0.0000	0.90		Asphalt/Concrete
0.0000	0.01		Manure Tank
0.0000	0.90		Roof
2.3300	0.25		Grass
2.3813	0.26	0.33	Weighted Average, Frequency Adjusted
2.3813			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
15.0	Direct Entry, Minimum				

### Subcatchment 8S: B1-8

**Hydrograph**



### Summary for Pond P: Retention Area

Inflow Area = 16.9056 ha, 0.00% Impervious, Inflow Depth = 31 mm for 100-Year event  
 Inflow = 0.6532 m<sup>3</sup>/s @ 0.46 hrs, Volume= 5.174 MI  
 Outflow = 0.4381 m<sup>3</sup>/s @ 2.35 hrs, Volume= 4.235 MI, Atten= 33%, Lag= 113.4 min  
 Primary = 0.2686 m<sup>3</sup>/s @ 2.35 hrs, Volume= 3.879 MI  
 Secondary = 0.1695 m<sup>3</sup>/s @ 2.35 hrs, Volume= 0.356 MI

Routing by Stor-Ind method, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Peak Elev= 123.729 m @ 2.35 hrs Surf.Area= 0.0 m<sup>2</sup> Storage= 3,388.7 m<sup>3</sup>

Plug-Flow detention time= 113.2 min calculated for 4.235 MI (82% of inflow)  
 Center-of-Mass det. time= 101.0 min ( 180.6 - 79.6 )

Volume	Invert	Avail.Storage	Storage Description
#1	123.100 m	3,743.6 m <sup>3</sup>	<b>Custom Stage Data</b> Listed below
<hr/>			
Elevation (meters)	Cum.Store (cubic-meters)		
123.100	0.0		
123.200	126.7		
123.300	461.6		
123.400	1,010.1		
123.500	1,708.2		
123.600	2,421.7		
123.700	3,166.9		
123.775	3,743.6		

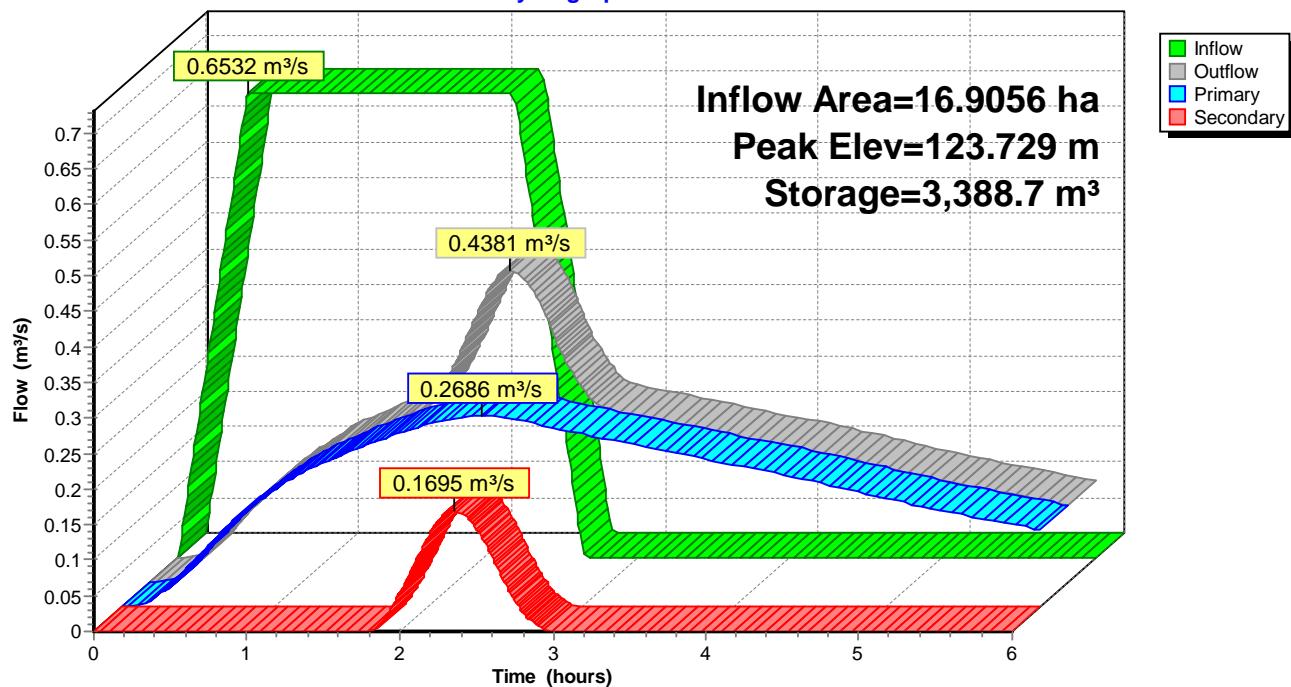
Device	Routing	Invert	Outlet Devices
#1	Primary	123.100 m	<b>450 mm Vert. Orifice/Grate</b> C= 0.600
#2	Secondary	123.660 m	<b>Custom Weir/Orifice, Cv= 1.45 (C= 1.81)</b> Head (meters) 0.000 0.500 Width (meters) 5.00 8.00

**Primary OutFlow** Max=0.2686 m<sup>3</sup>/s @ 2.35 hrs HW=123.729 m (Free Discharge)  
 ↪**1=Orifice/Grate** (Orifice Controls 0.2686 m<sup>3</sup>/s @ 1.69 m/s)

**Secondary OutFlow** Max=0.1691 m<sup>3</sup>/s @ 2.35 hrs HW=123.729 m (Free Discharge)  
 ↪**2=Custom Weir/Orifice** (Weir Controls 0.1691 m<sup>3</sup>/s @ 0.47 m/s)

### Pond P: Retention Area

Hydrograph



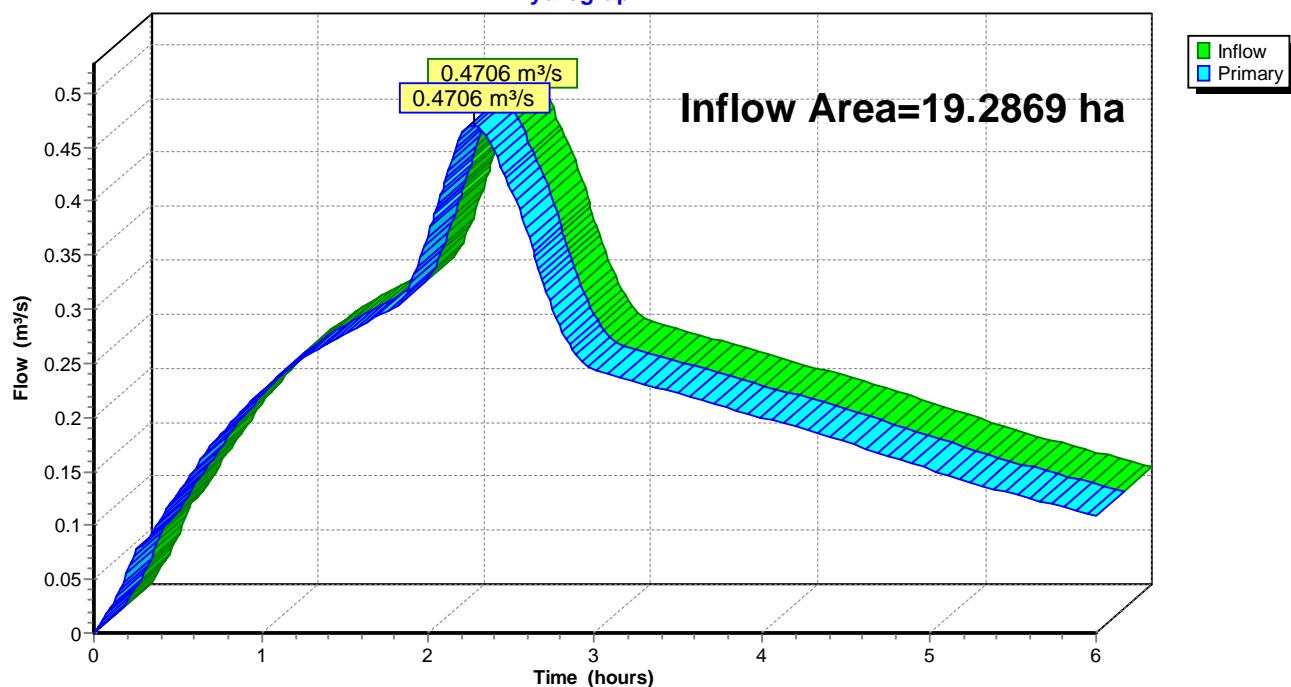
### Summary for Link 2L: Site Outlet

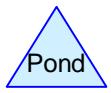
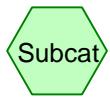
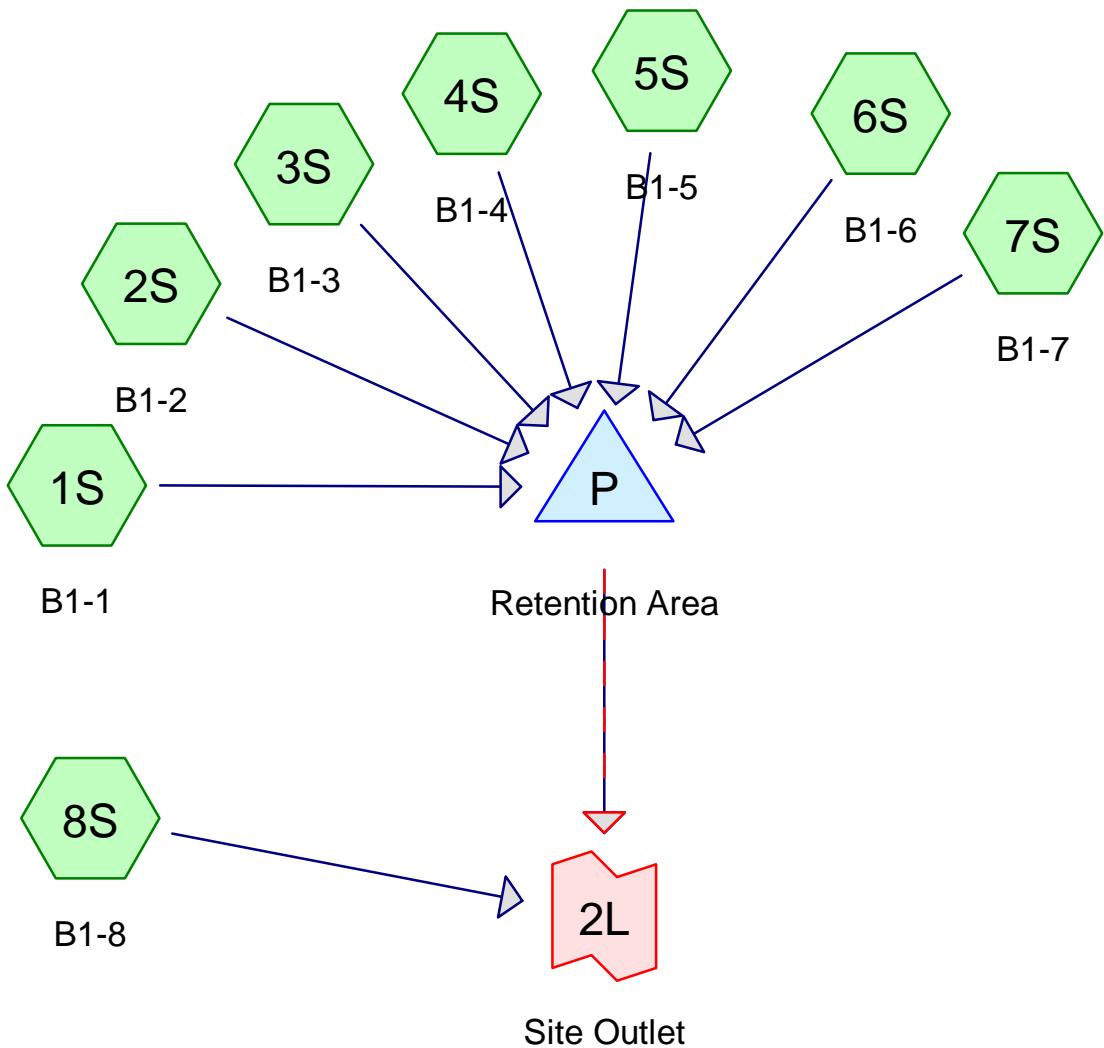
Inflow Area = 19.2869 ha, 0.00% Impervious, Inflow Depth > 24 mm for 100-Year event  
Inflow = 0.4706 m<sup>3</sup>/s @ 2.27 hrs, Volume= 4.694 MI  
Primary = 0.4706 m<sup>3</sup>/s @ 2.27 hrs, Volume= 4.694 MI, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs

### Link 2L: Site Outlet

Hydrograph





**Routing Diagram for 191-08574-02 - PostDev\_CTL\_D\_v3**  
 Prepared by WSP Canada Inc., Printed 10/13/2021  
 HydroCAD® 10.00-24 s/n 10698 © 2018 HydroCAD Software Solutions LLC

**Area Listing (all nodes)**

Area (hectares)	C	Description (subcatchment-numbers)
0.8197	0.90	Asphalt / Concrete (2S, 3S, 4S, 5S, 6S, 7S)
14.2656	0.25	Grass (1S, 2S, 3S, 4S, 5S, 6S, 7S, 8S)
2.6264	0.80	Gravel (1S, 2S, 3S, 4S, 5S, 6S, 7S, 8S)
0.1513	0.01	Manure Tank (3S, 7S)
1.4239	0.90	Roof (1S, 2S, 3S, 4S, 5S, 6S, 7S)
<b>19.2869</b>	<b>0.40</b>	<b>TOTAL AREA</b>

### Summary for Subcatchment 1S: B1-1

Runoff = 0.1077 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.794 MI, Depth= 27 mm

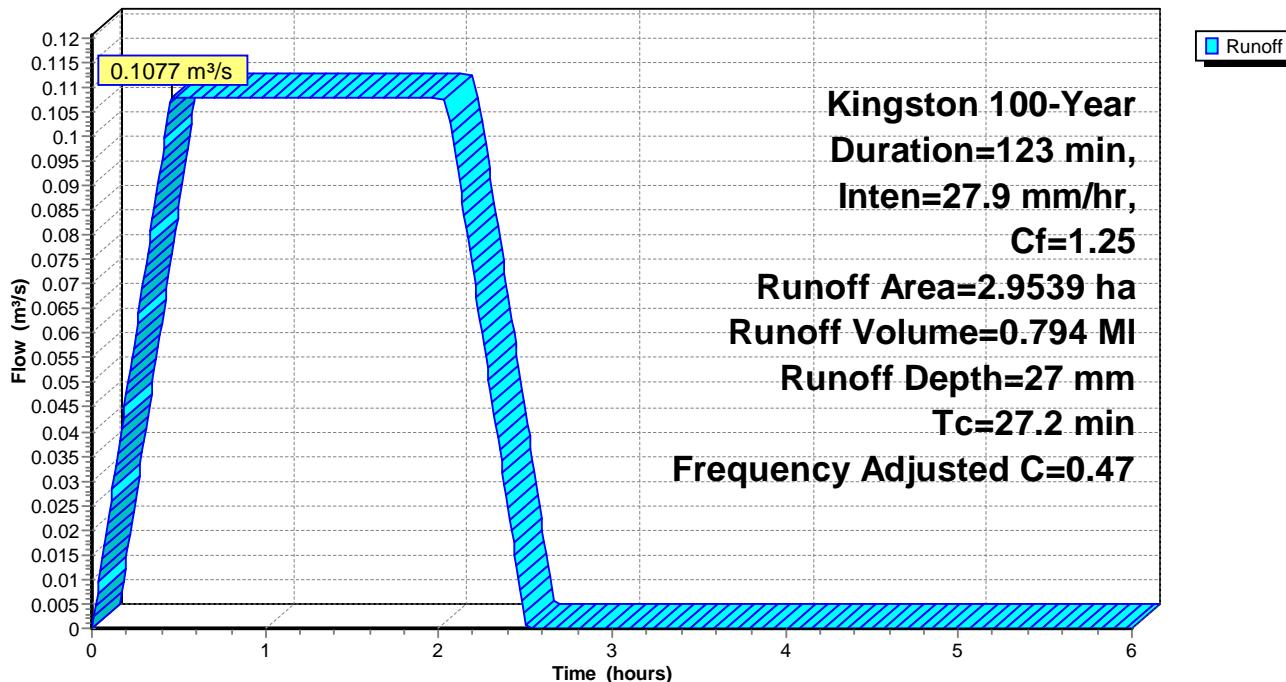
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 100-Year Duration=123 min, Inten=27.9 mm/hr, Cf=1.25

Area (ha)	C	Adj	Description
0.5181	0.80		Gravel
0.0000	0.90		Asphalt / Concrete
0.0000	0.01		Manure Tank
0.1539	0.90		Roof
2.2819	0.25		Grass
2.9539	0.38	0.47	Weighted Average, Frequency Adjusted
2.9539			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2	Direct Entry, Per Ditch Design Sheet				

### Subcatchment 1S: B1-1

Hydrograph



### Summary for Subcatchment 2S: B1-2

Runoff = 0.0298 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.220 MI, Depth= 37 mm

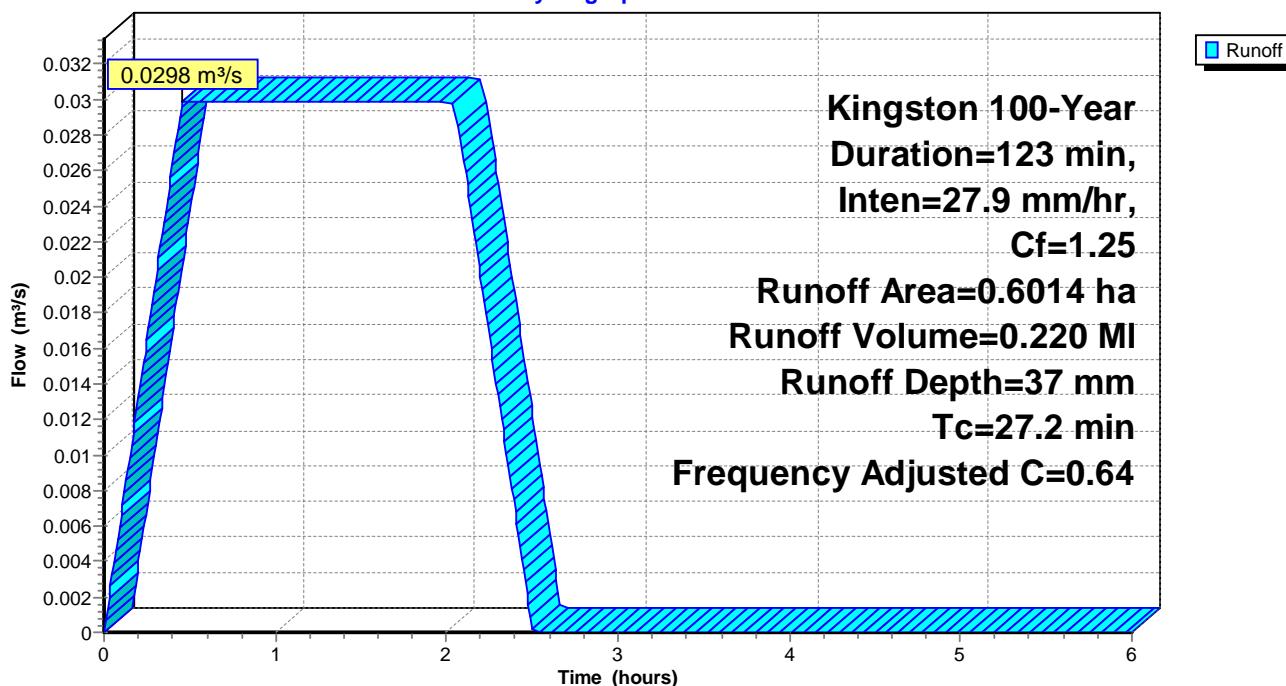
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 100-Year Duration=123 min, Inten=27.9 mm/hr, Cf=1.25

Area (ha)	C	Adj	Description
0.1347	0.80		Gravel
0.0046	0.90		Asphalt / Concrete
0.0000	0.01		Manure Tank
0.1175	0.90		Roof
0.3446	0.25		Grass
0.6014	0.51	0.64	Weighted Average, Frequency Adjusted
0.6014			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 2S: B1-2

**Hydrograph**



### Summary for Subcatchment 3S: B1-3

Runoff = 0.0565 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.417 MI, Depth= 25 mm

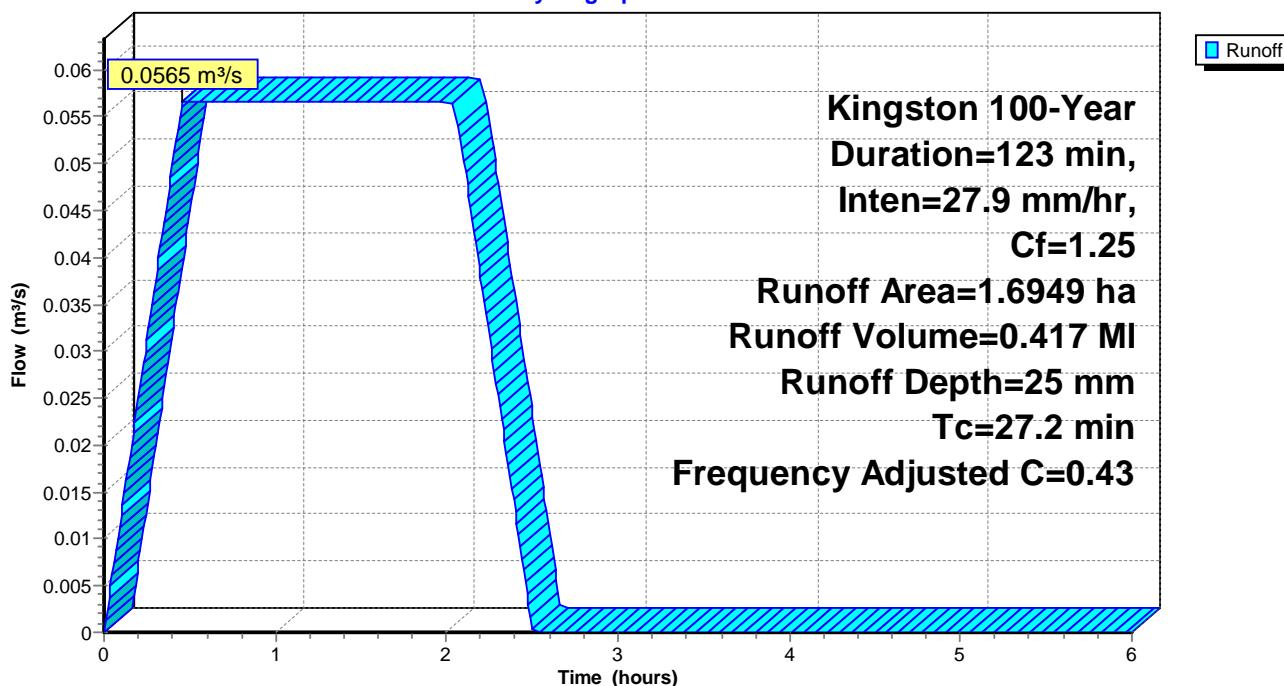
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 100-Year Duration=123 min, Inten=27.9 mm/hr, Cf=1.25

Area (ha)	C	Adj	Description
0.2254	0.80		Gravel
0.0070	0.90		Asphalt / Concrete
0.1239	0.01		Manure Tank
0.0809	0.90		Roof
1.2577	0.25		Grass
1.6949	0.34	0.43	Weighted Average, Frequency Adjusted
1.6949			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 3S: B1-3

**Hydrograph**



### Summary for Subcatchment 4S: B1-4

Runoff = 0.0845 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.623 MI, Depth= 26 mm

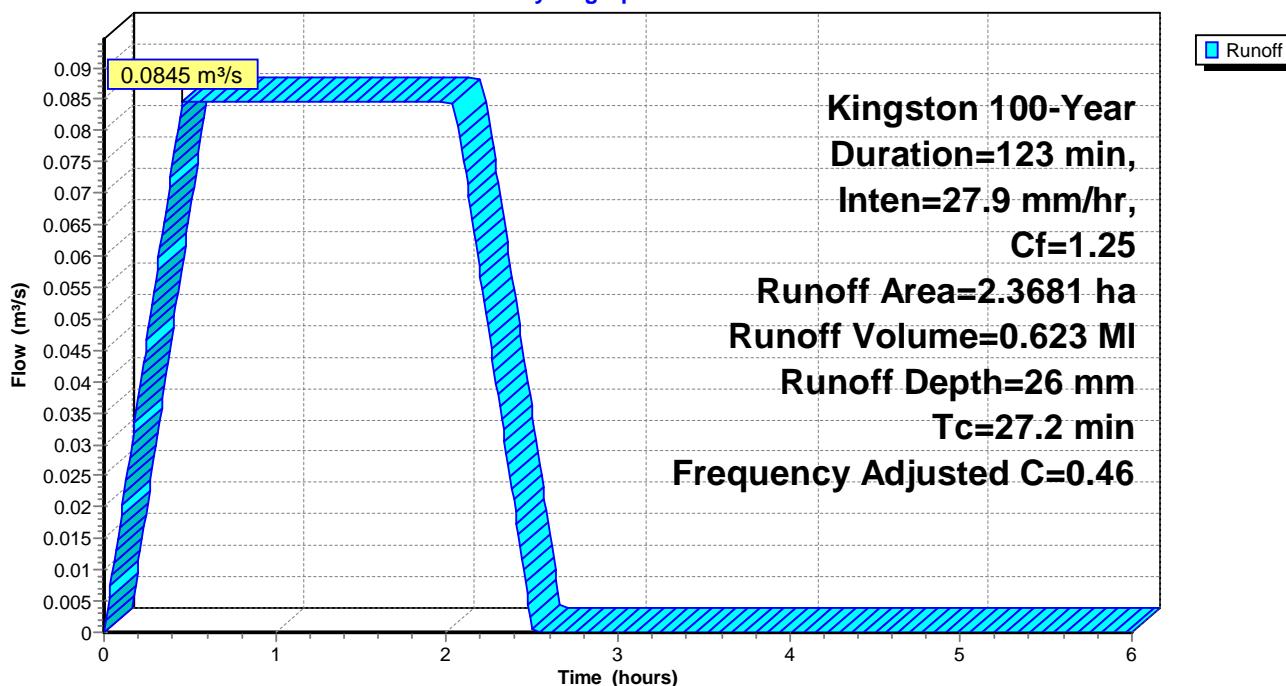
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
Kingston 100-Year Duration=123 min, Inten=27.9 mm/hr, Cf=1.25

Area (ha)	C	Adj	Description
0.3290	0.80		Gravel
0.0268	0.90		Asphalt / Concrete
0.0000	0.01		Manure Tank
0.1249	0.90		Roof
1.8874	0.25		Grass
2.3681	0.37	0.46	Weighted Average, Frequency Adjusted
2.3681			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 4S: B1-4

Hydrograph



### Summary for Subcatchment 5S: B1-5

Runoff = 0.1596 m<sup>3</sup>/s @ 0.46 hrs, Volume= 1.178 MI, Depth= 43 mm

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
Kingston 100-Year Duration=123 min, Inten=27.9 mm/hr, Cf=1.25

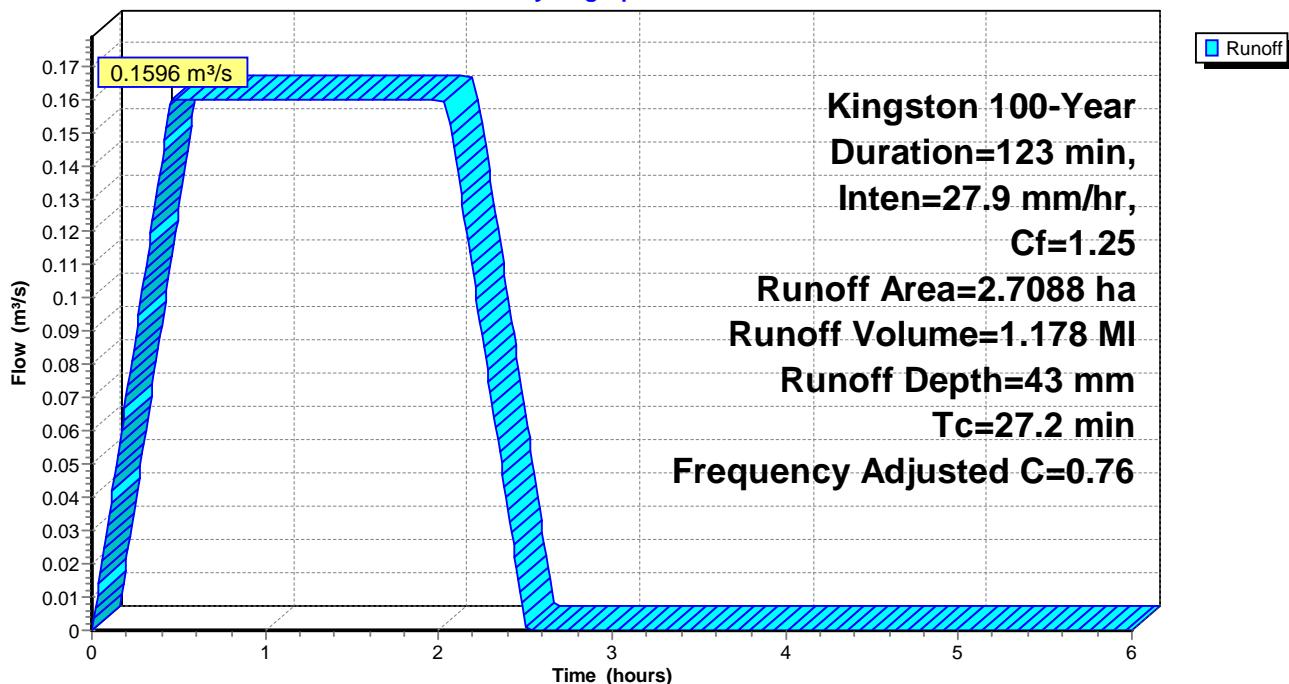
Area (ha)	C	Adj	Description
0.3775	0.80		Gravel
0.7307	0.90		Asphalt / Concrete
0.0000	0.01		Manure Tank
0.4356	0.90		Roof
1.1650	0.25		Grass
2.7088	0.61	0.76	Weighted Average, Frequency Adjusted
2.7088			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 5S: B1-5

Hydrograph



### Summary for Subcatchment 6S: B1-6

Runoff = 0.0710 m<sup>3</sup>/s @ 0.46 hrs, Volume= 0.524 MI, Depth= 38 mm

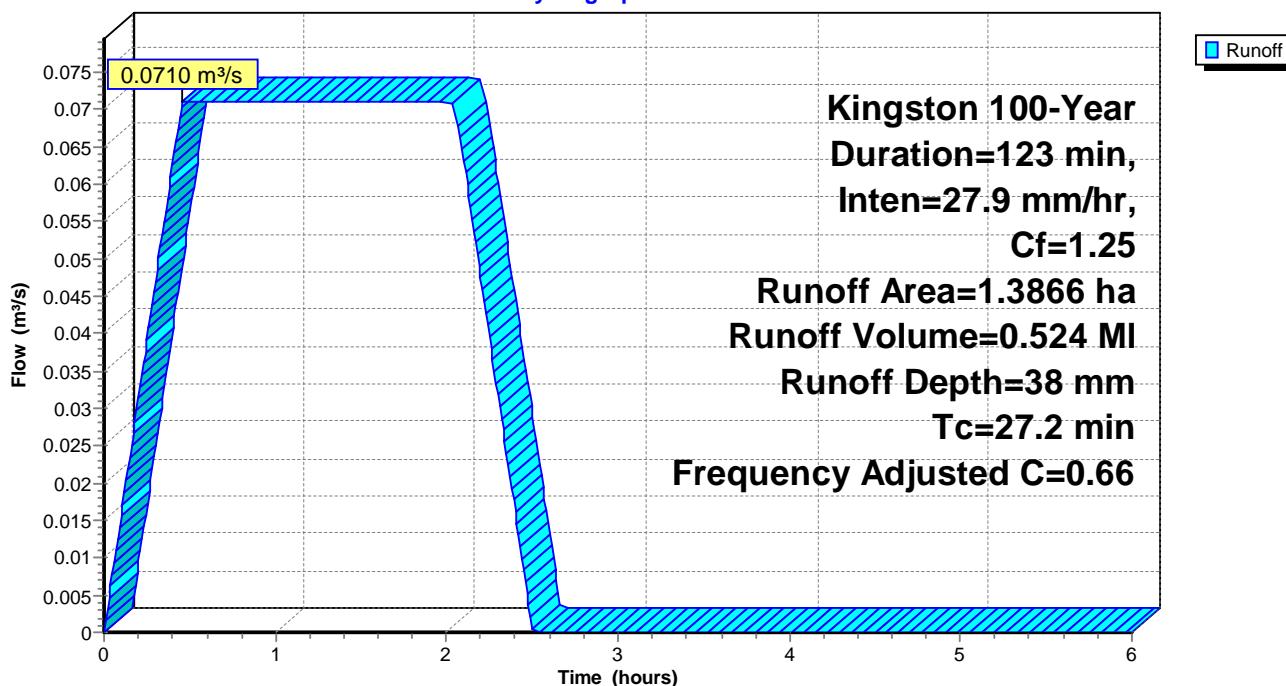
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 100-Year Duration=123 min, Inten=27.9 mm/hr, Cf=1.25

Area (ha)	C	Adj	Description
0.4917	0.80		Gravel
0.0289	0.90		Asphalt / Concrete
0.0000	0.01		Manure Tank
0.1561	0.90		Roof
0.7099	0.25		Grass
1.3866	0.53	0.66	Weighted Average, Frequency Adjusted
1.3866			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 6S: B1-6

**Hydrograph**



### Summary for Subcatchment 7S: B1-7

Runoff = 0.1771 m<sup>3</sup>/s @ 0.46 hrs, Volume= 1.307 MI, Depth= 25 mm

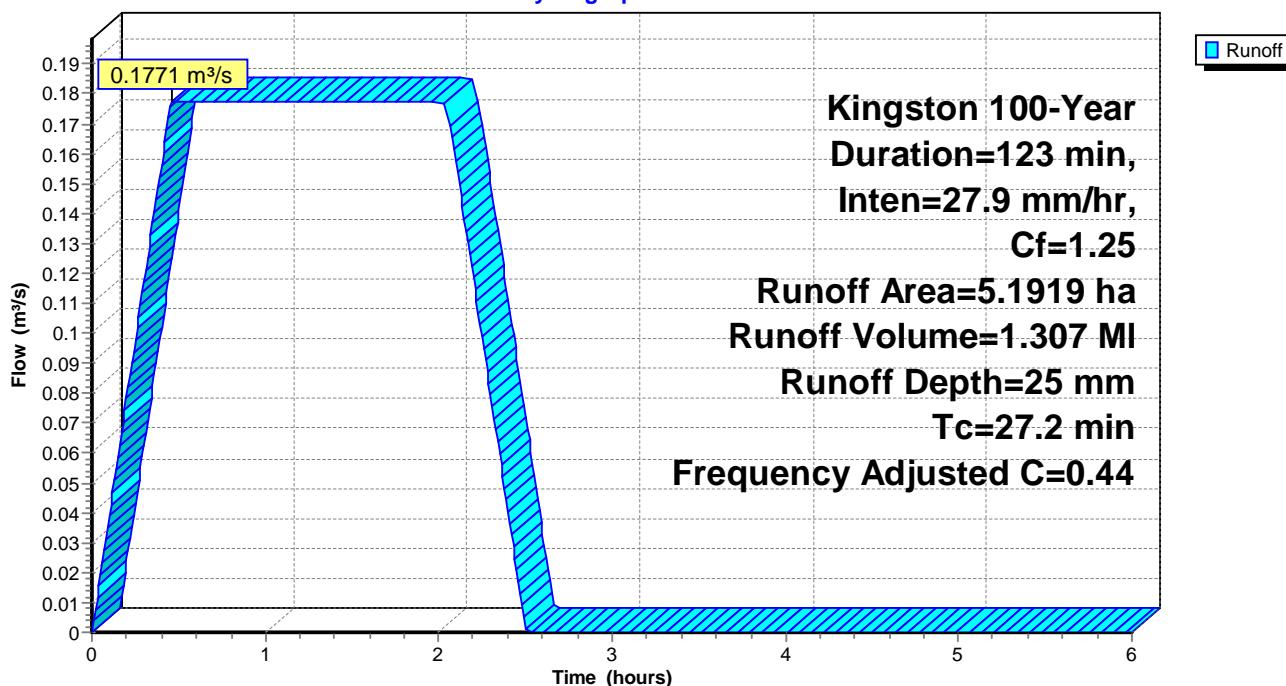
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 100-Year Duration=123 min, Inten=27.9 mm/hr, Cf=1.25

Area (ha)	C	Adj	Description
0.4987	0.80		Gravel
0.0217	0.90		Asphalt / Concrete
0.0274	0.01		Manure Tank
0.3550	0.90		Roof
4.2891	0.25		Grass
5.1919	0.35	0.44	Weighted Average, Frequency Adjusted
5.1919			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
27.2					Direct Entry, Pert Ditch Design Sheet

### Subcatchment 7S: B1-7

Hydrograph



### Summary for Subcatchment 8S: B1-8

Runoff = 0.0609 m<sup>3</sup>/s @ 0.25 hrs, Volume= 0.450 MI, Depth= 19 mm

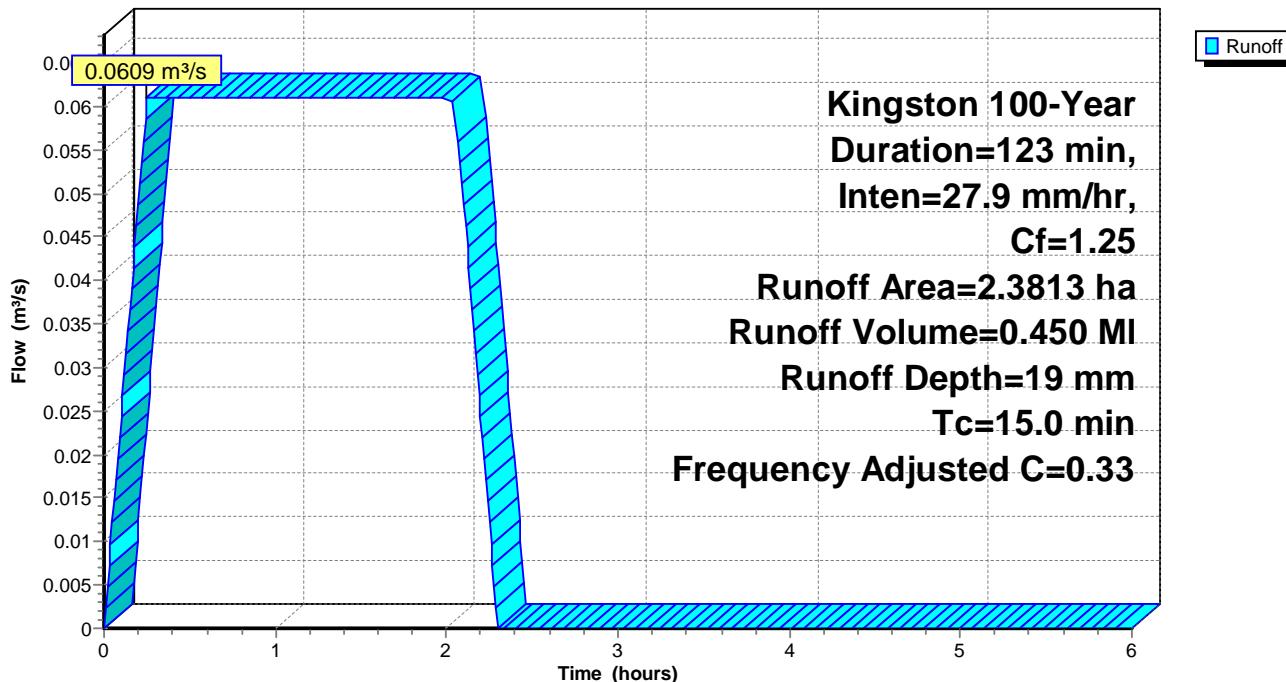
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Kingston 100-Year Duration=123 min, Inten=27.9 mm/hr, Cf=1.25

Area (ha)	C	Adj	Description
0.0513	0.80		Gravel
0.0000	0.90		Asphalt/Concrete
0.0000	0.01		Manure Tank
0.0000	0.90		Roof
2.3300	0.25		Grass
2.3813	0.26	0.33	Weighted Average, Frequency Adjusted
2.3813			100.00% Pervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(meters)	(m/m)	(m/sec)	(m <sup>3</sup> /s)	
15.0	Direct Entry, Minimum				

### Subcatchment 8S: B1-8

**Hydrograph**



### Summary for Pond P: Retention Area

Inflow Area = 16.9056 ha, 0.00% Impervious, Inflow Depth = 30 mm for 100-Year event  
 Inflow = 0.6862 m<sup>3</sup>/s @ 0.46 hrs, Volume= 5.064 MI  
 Outflow = 0.4388 m<sup>3</sup>/s @ 2.21 hrs, Volume= 4.179 MI, Atten= 36%, Lag= 105.2 min  
 Primary = 0.2687 m<sup>3</sup>/s @ 2.21 hrs, Volume= 3.836 MI  
 Secondary = 0.1702 m<sup>3</sup>/s @ 2.21 hrs, Volume= 0.344 MI

Routing by Stor-Ind method, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs  
 Peak Elev= 123.729 m @ 2.21 hrs Surf.Area= 0.0 m<sup>2</sup> Storage= 3,390.0 m<sup>3</sup>

Plug-Flow detention time= 113.6 min calculated for 4.179 MI (83% of inflow)  
 Center-of-Mass det. time= 102.6 min ( 177.7 - 75.1 )

Volume	Invert	Avail.Storage	Storage Description
#1	123.100 m	3,743.6 m <sup>3</sup>	<b>Custom Stage Data</b> Listed below

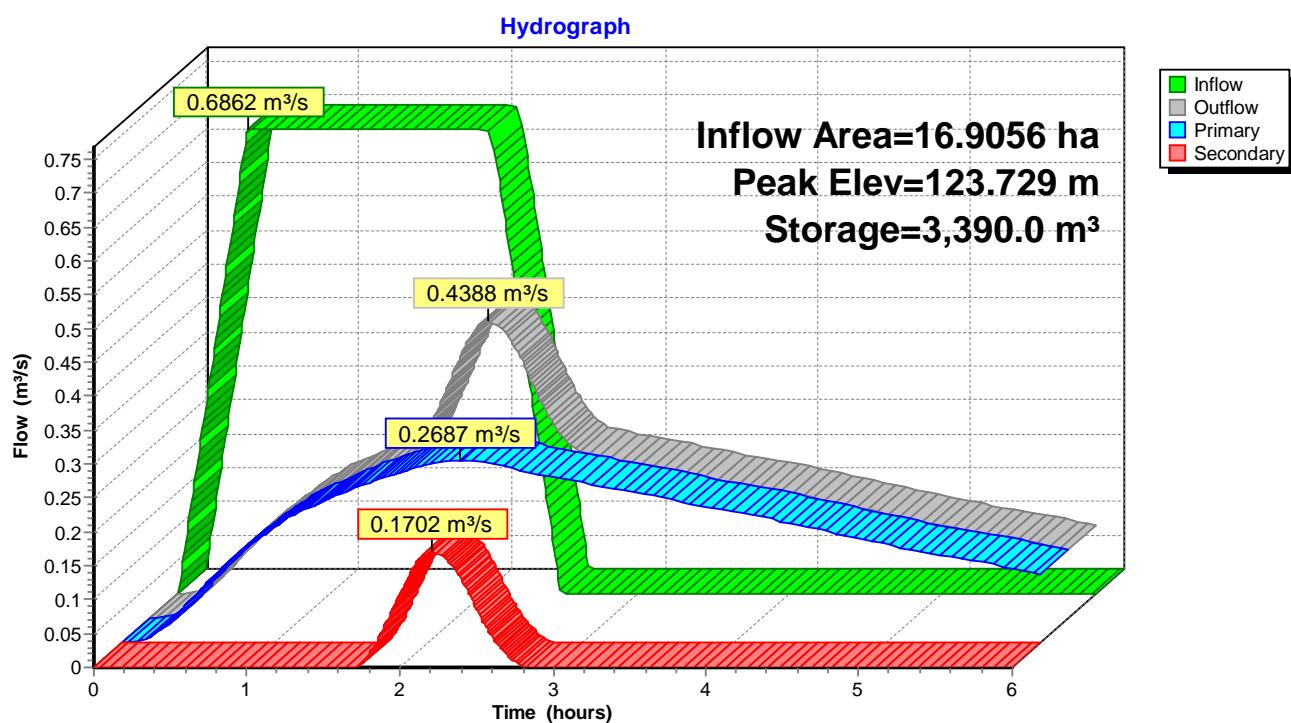
Elevation (meters)	Cum.Store (cubic-meters)
123.100	0.0
123.200	126.7
123.300	461.6
123.400	1,010.1
123.500	1,708.2
123.600	2,421.7
123.700	3,166.9
123.775	3,743.6

Device	Routing	Invert	Outlet Devices
#1	Primary	123.100 m	<b>450 mm Vert. Orifice/Grate</b> C= 0.600
#2	Secondary	123.660 m	<b>Custom Weir/Orifice, Cv= 1.45 (C= 1.81)</b> Head (meters) 0.000 0.500 Width (meters) 5.00 8.00

**Primary OutFlow** Max=0.2687 m<sup>3</sup>/s @ 2.21 hrs HW=123.729 m (Free Discharge)  
 ↗1=Orifice/Grate (Orifice Controls 0.2687 m<sup>3</sup>/s @ 1.69 m/s)

**Secondary OutFlow** Max=0.1697 m<sup>3</sup>/s @ 2.21 hrs HW=123.729 m (Free Discharge)  
 ↗2=Custom Weir/Orifice (Weir Controls 0.1697 m<sup>3</sup>/s @ 0.47 m/s)

### Pond P: Retention Area



### Summary for Link 2L: Site Outlet

Inflow Area = 19.2869 ha, 0.00% Impervious, Inflow Depth > 24 mm for 100-Year event  
Inflow = 0.4695 m<sup>3</sup>/s @ 2.14 hrs, Volume= 4.629 MI  
Primary = 0.4695 m<sup>3</sup>/s @ 2.14 hrs, Volume= 4.629 MI, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-6.00 hrs, dt= 0.01 hrs

### Link 2L: Site Outlet

Hydrograph

