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WESTERN BROOK BRIDGE REPLACEMENT GROS MORNE NATIONAL PARK NEWFOUNDLAND AND LABRADOR

DRAWING LIST

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DETOUR BRIDGE

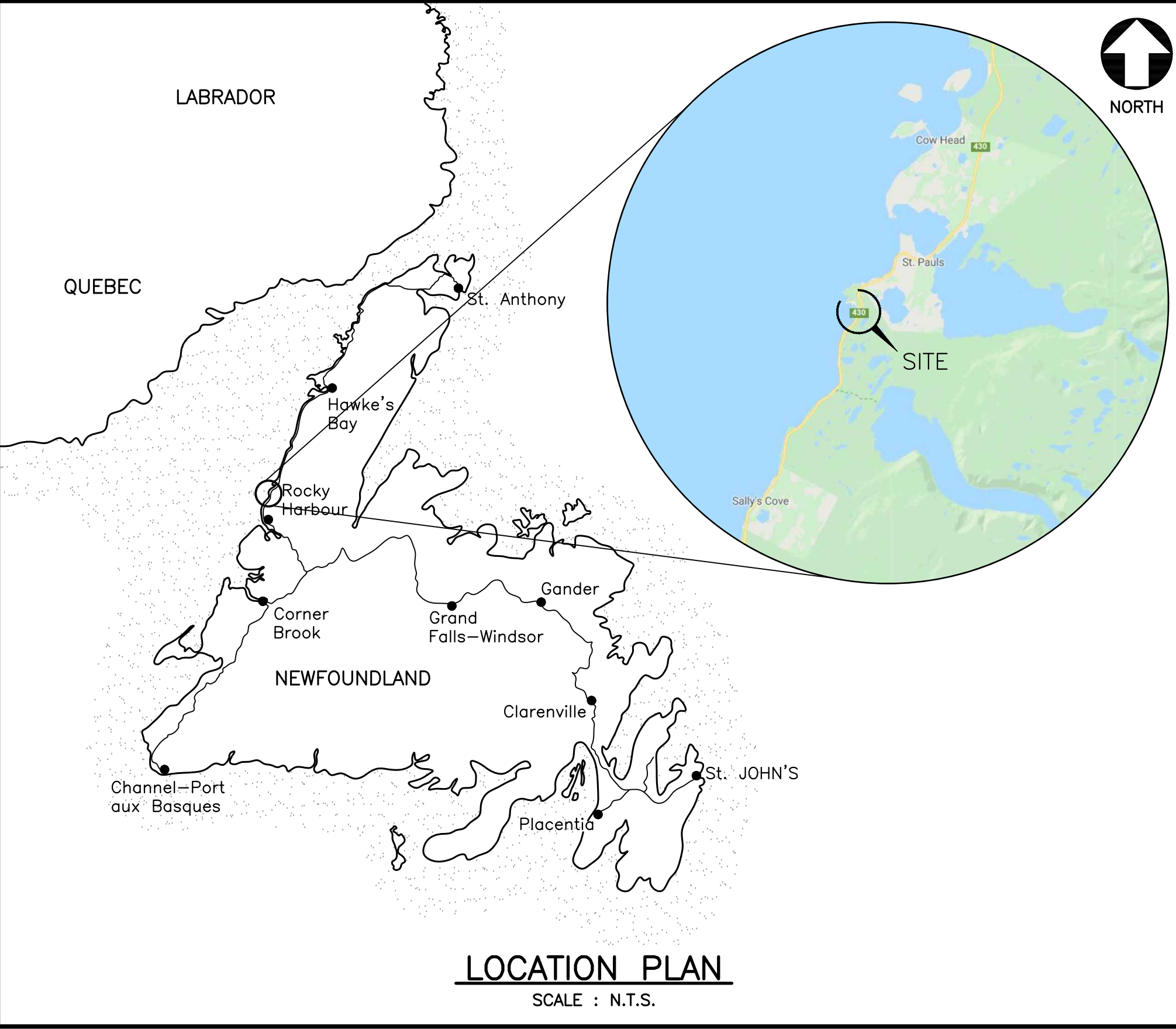
DS1	GENERAL ARRANGEMENT
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DS5	SOUTH ABUTMENT REINFORCING
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LANDSCAPING

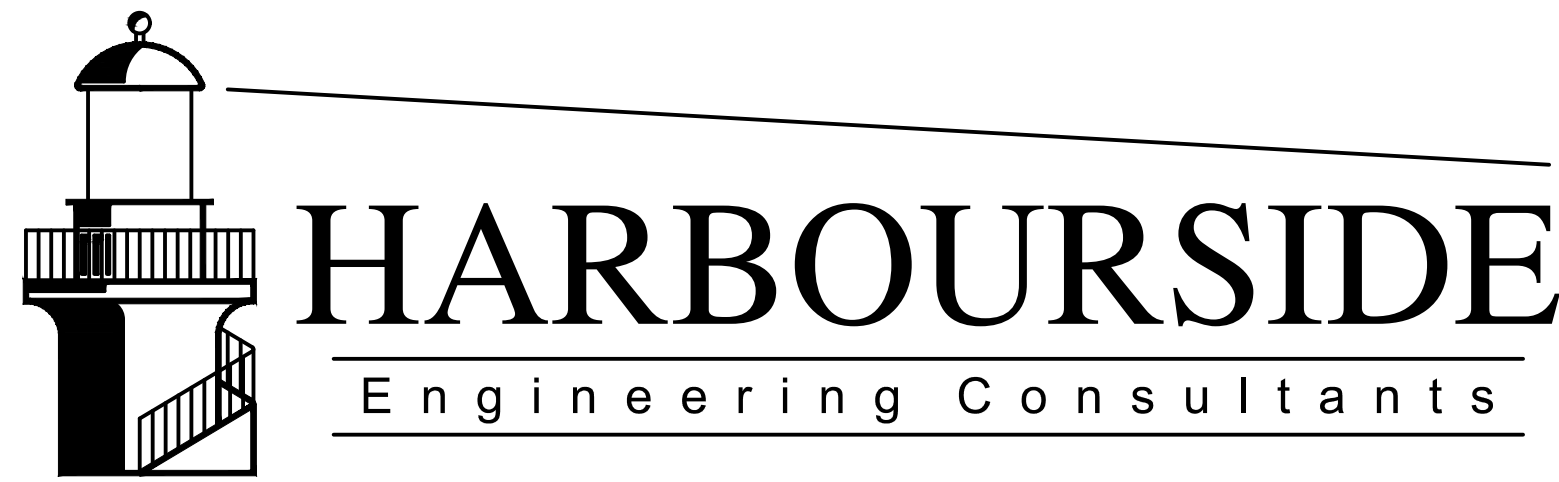
L01	LANDSCAPE FINISH TREATMENT PLAN
L02	LANDSCAPE FINISH TREATMENT PLAN

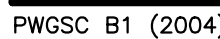
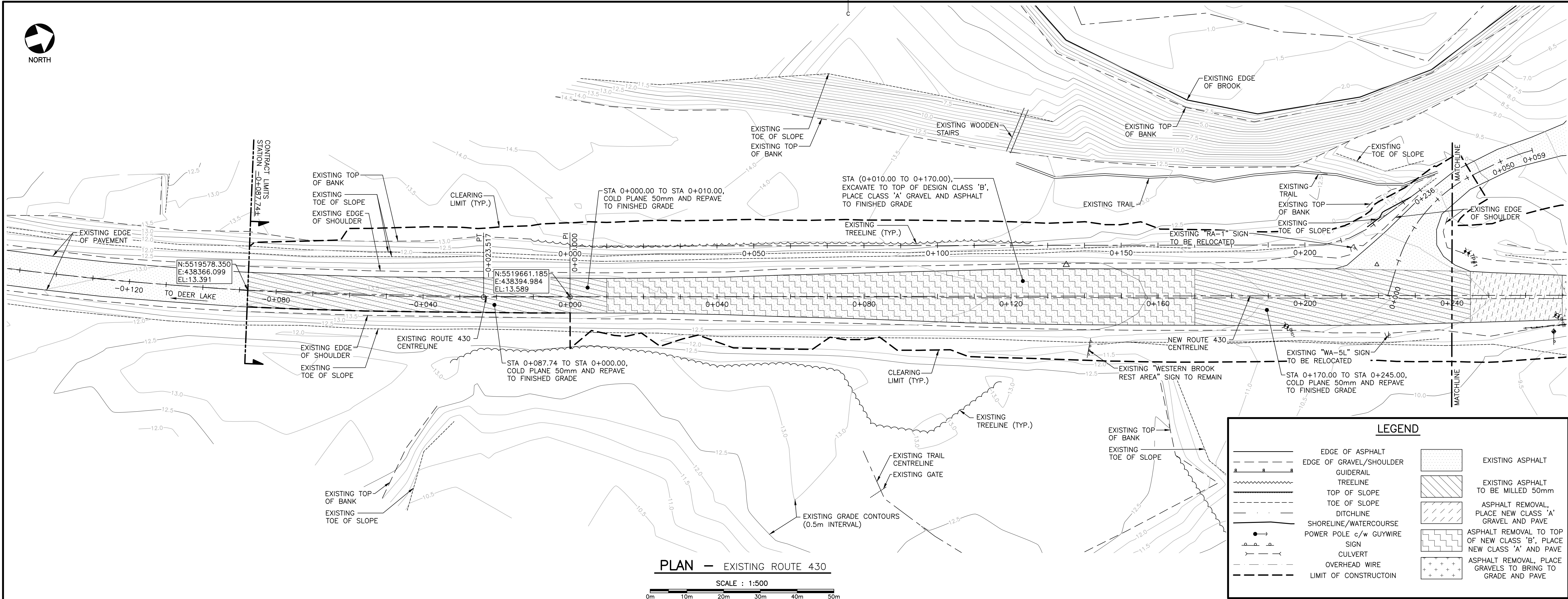
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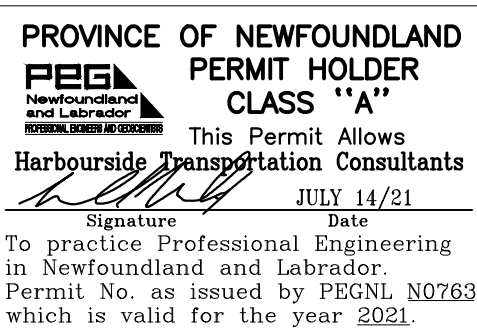
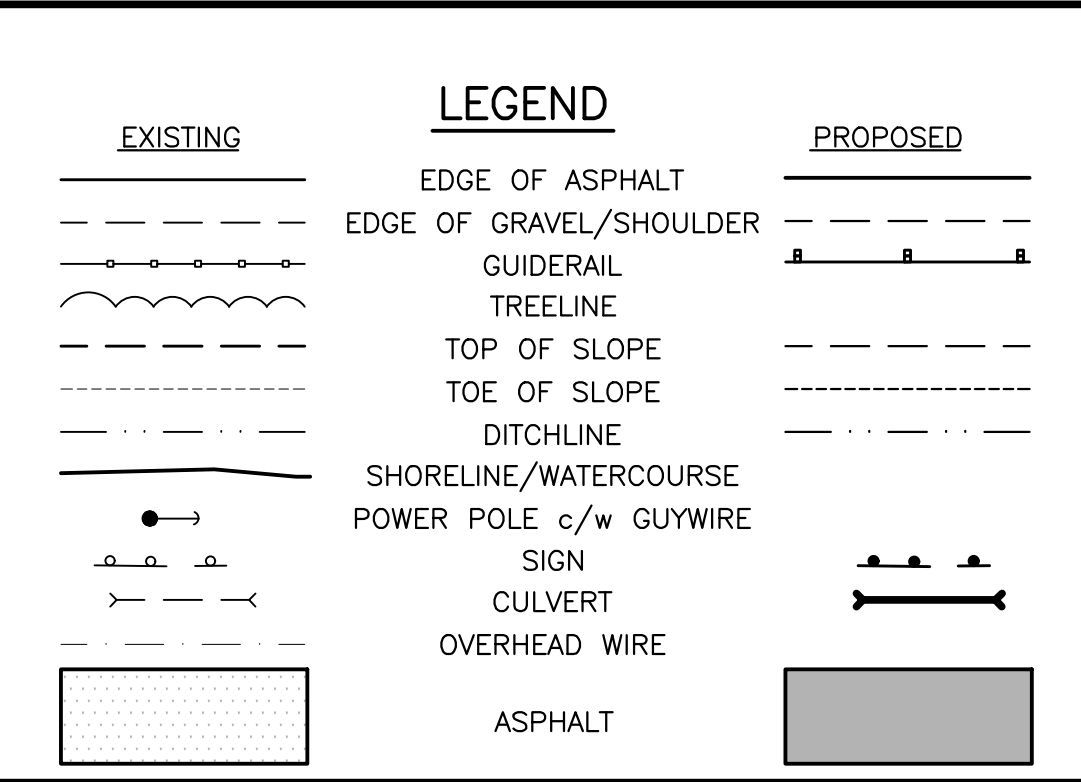
EP1	PHASE 1 AND PHASE 2
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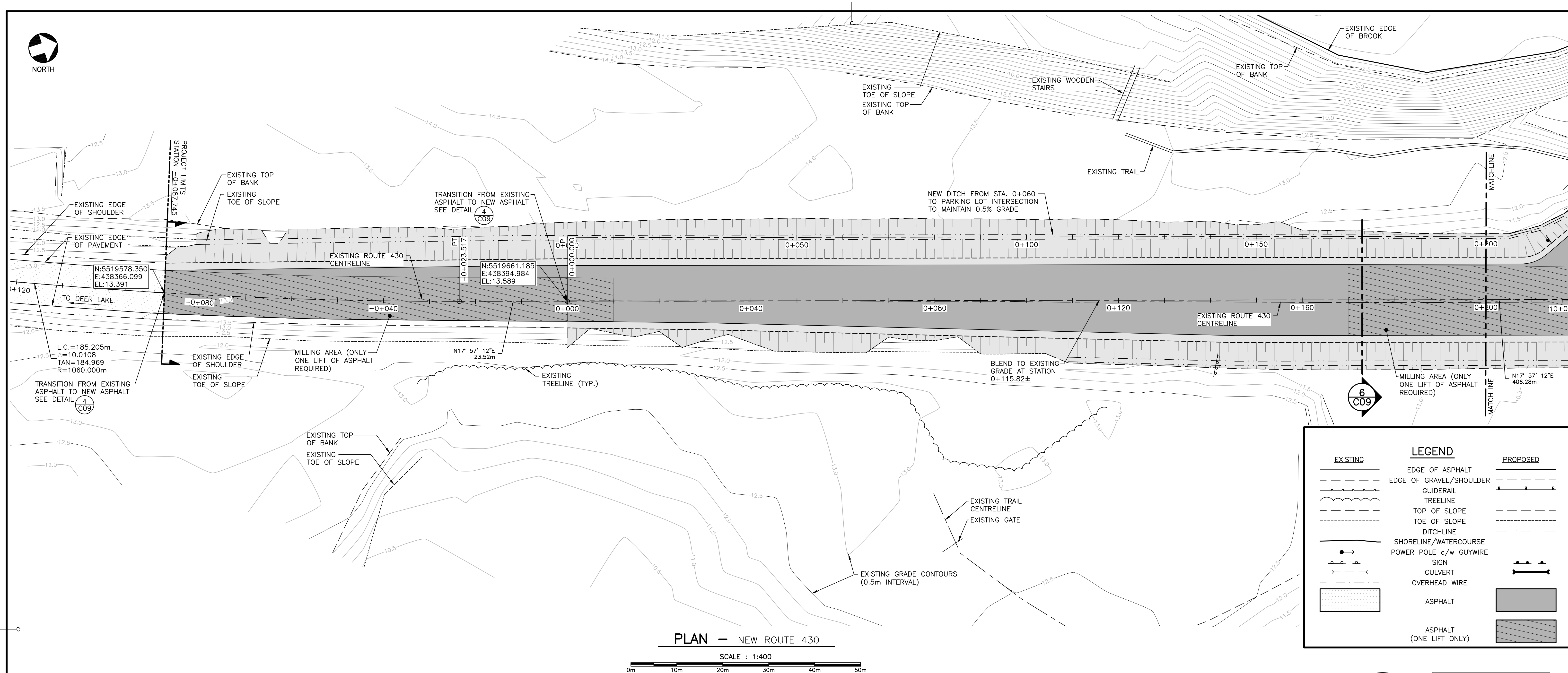


PROJECT NO. 1268



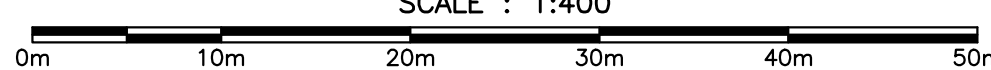




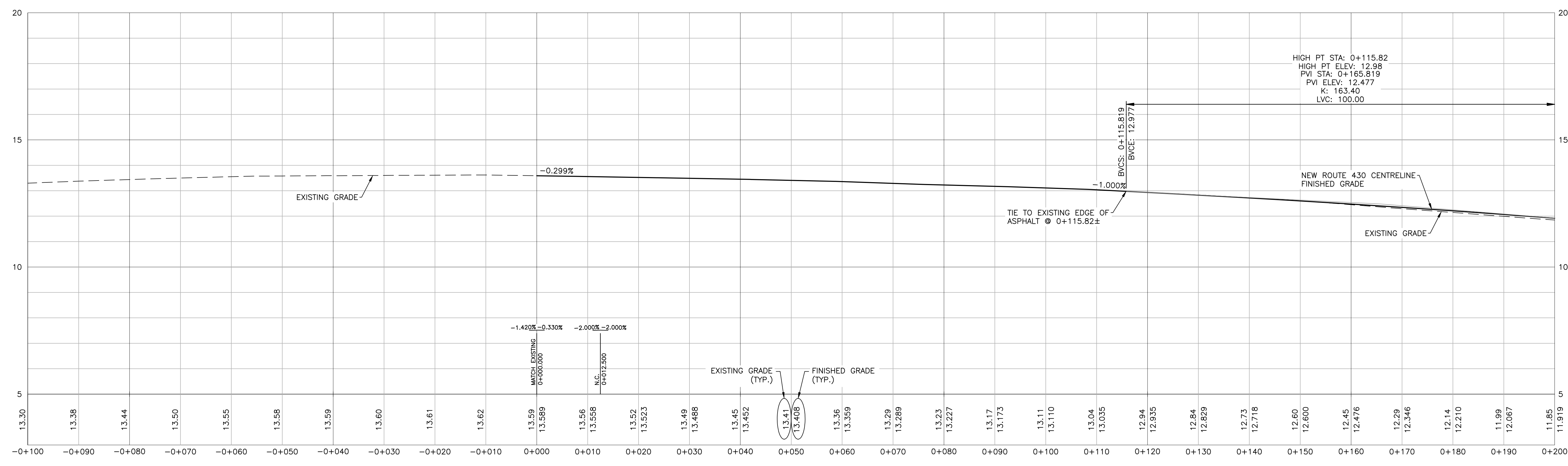


PLAN — NEW ROUTE 430

SCALE : 1:400



LEGEND		
EXISTING		PROPOSED
	EDGE OF ASPHALT	
	EDGE OF GRAVEL/SHOULDER	
	GUIDELINE	
	TREE LINE	
	TOP OF SLOPE	
	TOE OF SLOPE	
	DITCH LINE	
	SHORELINE/WATERCOURSE	
	POWER POLE c/w GUYWIRE	
	SIGN	
	CULVERT	
	OVERHEAD WIRE	
	ASPHALT	
	ASPHALT (ONE LIFT ONLY)	



PROFILE — NEW ROUTE 430 (—0+087.74 TO 0+200.00)

HORIZONTAL SCALE : 1:400

VERTICAL SCALE : 1:80



GENERAL NOTES:

- ALL ELEVATIONS ARE IN METRES UNLESS OTHERWISE NOTED.
 - SURVEY REFERENCED HORIZONTALLY TO UTM ZONE 21 NORTH NAD83 (CSRS) 2010.0 AND VERTICALLY TO CGVD28 (HTV2.0 GEOD MODEL).
 - CONTROL IS DERIVED FROM STATIC GNSS OBSERVATIONS ON A NAIL (POINT #1000) HAVING COORDINATES OF:
N=5463441.253m
E=442956.271m
EL=20.152m
 - RAW GNSS DATA PROCESSED USING NATURAL RESOURCES CANADA PRECISE POINT POSITIONING SOFTWARE.
 - FIELD SURVEY WAS CARRIED OUT BETWEEN MARCH 6—9TH, 2018 AND JULY 25—28TH, 2018.
 - COORDINATES ARE GRID. APPLY COMBINED SCALE FACTOR OF 1.0003631 TO CALCULATE GROUND DISTANCES.
 - CONTOUR INTERVAL IS 0.5 METRE.
- HORIZONTAL ALIGNMENT CONTROL POINTS CONSIST OF:
PT — POINT OF TANGENCY
PC — POINT OF CURVATURE
PRC — POINT OF REVERSE CURVATURE
PCC — POINT OF COMPOUND CURVATURE
- LINE DATA:
AZ — AZIMUTH
L — LENGTH
- CURVE DATA:
D — DELTA
L — LENGTH
R — RADIUS

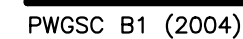
0	ISSUED FOR TENDER	JULY 14 2021
revisions		date

project WESTERN BROOK BRIDGE REPLACEMENT

GROS MORNE NATIONAL PARK


drawing NEW CONDITIONS ROUTE 430 PLAN/PROFILE STA. —0+087.00 TO 0+200.00 (SHEET 1 OF 3)

designed	MICHAEL MACDONALD	conçu
date	MARCH 2019	
drawn	CORY BAKER	dessiné
date	MARCH 2019	
approved	MICHAEL MACDONALD	approuvé
date	MARCH 2019	
Tender		Soumission
PCA Project Manager	Administrateur de projets APC	
project number	182009	no. du projet
drawing no.	C03	no. du dessin



GENERAL NOTES:

1. ALL ELEVATIONS ARE IN METRES UNLESS OTHERWISE NOTED.
2. SURVEY REFERENCED HORIZONTALLY TO UTM ZONE 21 NORTH NAD83 (CSRS) 2010.0 AND VERTICALLY TO CGVD28 (HTV2.0 GEIOD MODEL).
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EL=20.152m
4. RAW GNSS DATA PROCESSED USING NATURAL RESOURCES CANADA PRECISE POINT POSITIONING SOFTWARE.
5. FIELD SURVEY WAS CARRIED OUT BETWEEN MARCH 6-9TH, 2018 AND JULY 25-28TH, 2018.
6. COORDINATES ARE GRID. APPLY COMBINED SCALE FACTOR OF 1.0003631 TO CALCULATE GROUND DISTANCES.
7. CONTOUR INTERVAL IS 0.5 METRE.
 - HORIZONTAL ALIGNMENT CONTROL POINTS CONSIST OF:
 - PT - POINT OF TANGENCY
 - PC - POINT OF CURVATURE
 - PRC - POINT OF REVERSE CURVATURE
 - PCC - POINT OF COMPOUND CURVATURE
 - LINE DATA:
 - AZ - AZIMUTH
 - L - LENGTH
 - CURVE DATA:
 - D - DELTA
 - L - LENGTH
 - R - RADIUS

0	ISSUED FOR TENDER	JULY 14 2021
revisions		date
project	project	
<p align="center">WESTERN BROOK BRIDGE REPLACEMENT</p> <p align="center">GROS MORNE NATIONAL PARK</p>		
drawing	dessin	
<p align="center">NEW CONDITIONS ROUTE 430 PLAN/PROFILE STA. 0+500.00 TO 0+571.94 (SHEET 3 OF 3)</p>		
designed	MICHAEL MACDONALD	conçu
date	MARCH 2019	
drawn	CORY BAKER	dessiné
date	MARCH 2019	
approved	MICHAEL MACDONALD	approuvé
date	MARCH 2019	
Tender		Soumission
PCA Project Manager	Administrateur de projets APC	
project number	182009	no. du projet
drawing no.	005	no. du dessin



- GENERAL NOTES:
- FOR GENERAL NOTES SEE DRAWING C1.
 - SOUTH DETOUR APPROACH TO BE GRUBBED WHERE HEIGHT OF FILL IS LESS THAN 1.5m.
 - SOFT OR LOOSE MATERIAL TO BE EXCAVATED, GEOTEXTILE PLACED AND ROCK FILL.

PROVINCE OF NEWFOUNDLAND
PERMIT HOLDER
CLASS "A"
This Permit Allows
Harbourside Transportation Consultants
To practice Professional Engineering
in Newfoundland and Labrador
Permit No. as issued by PEGNL N0763
which is valid for the year 2021.



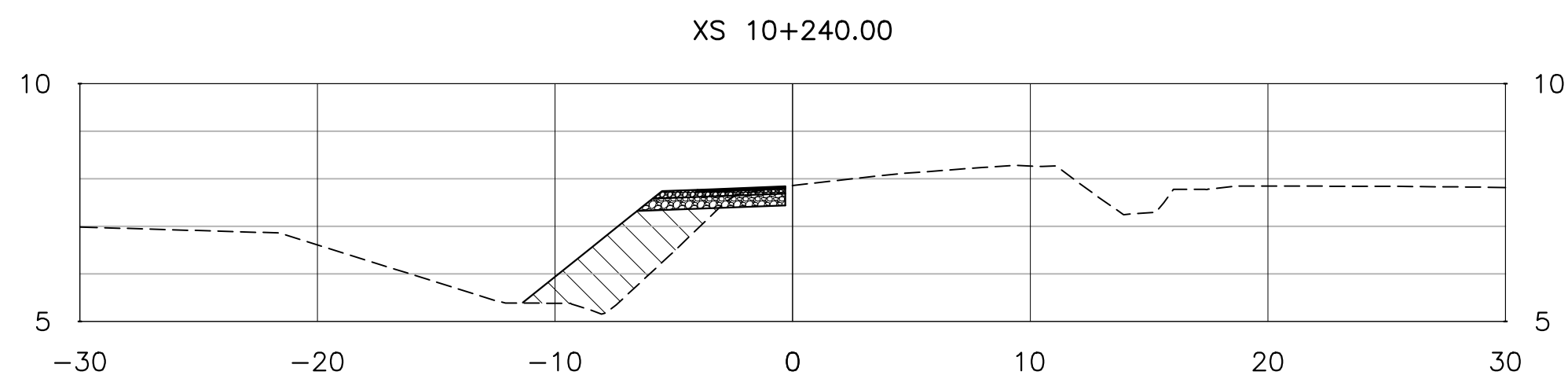
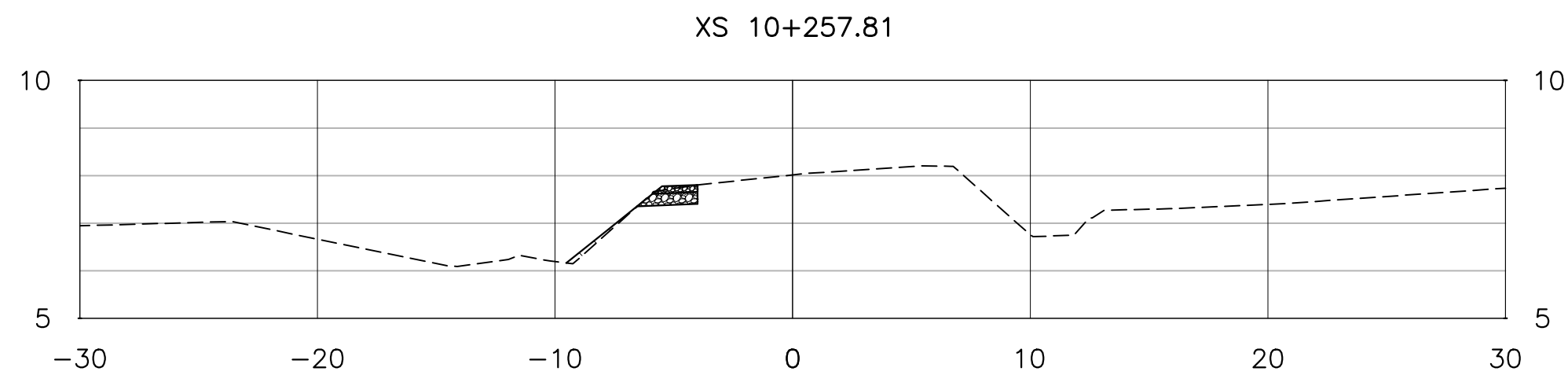
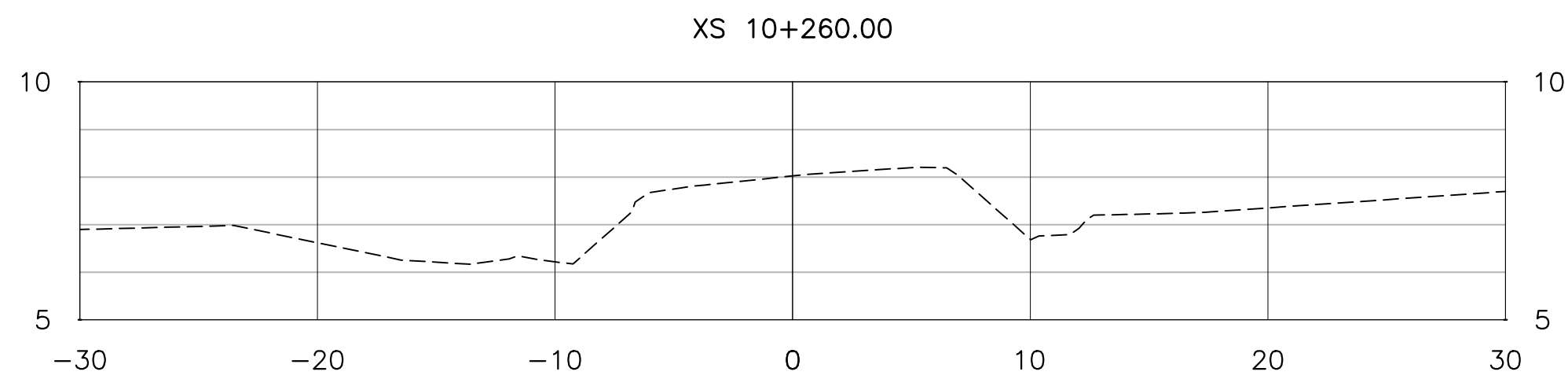
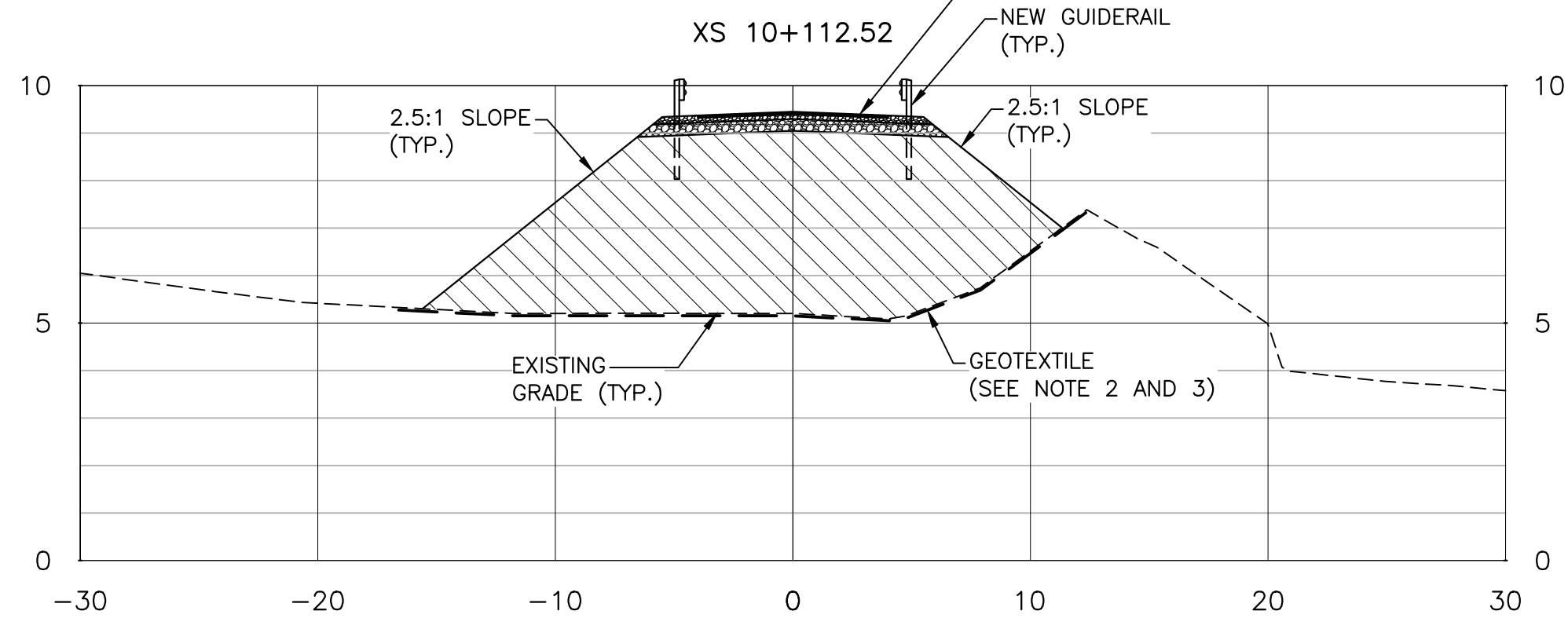
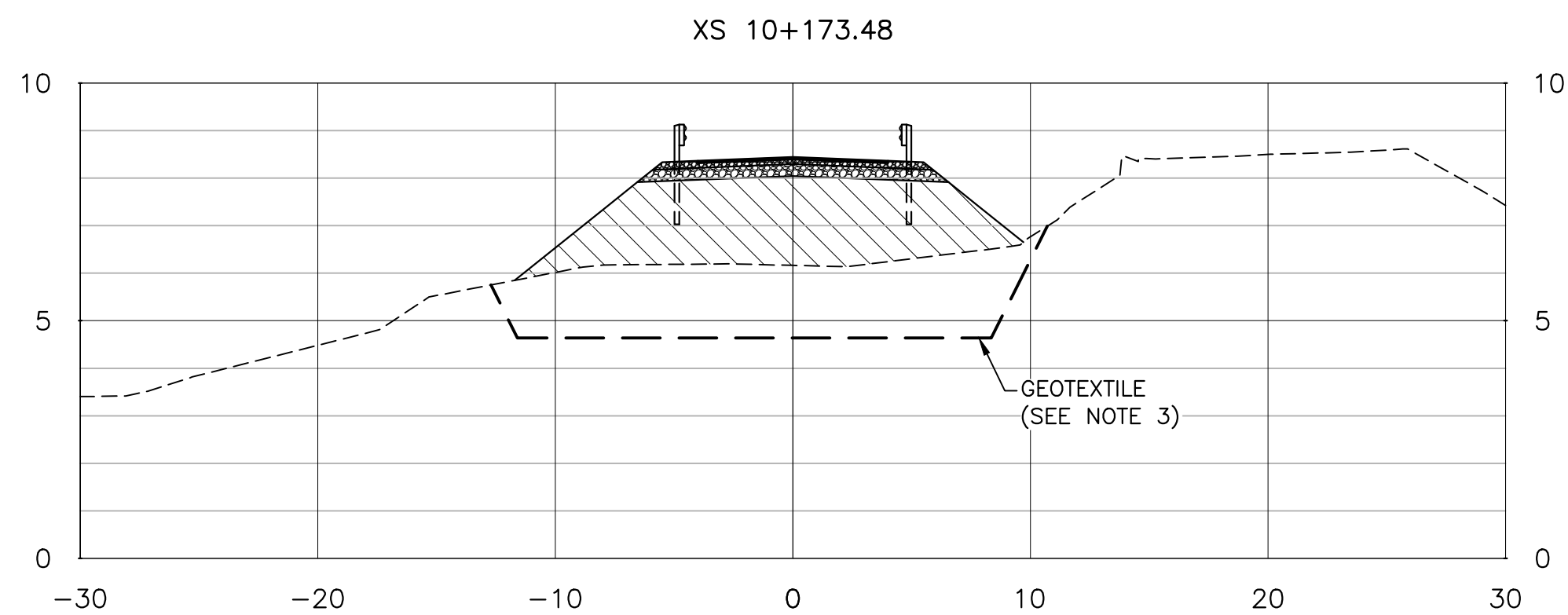
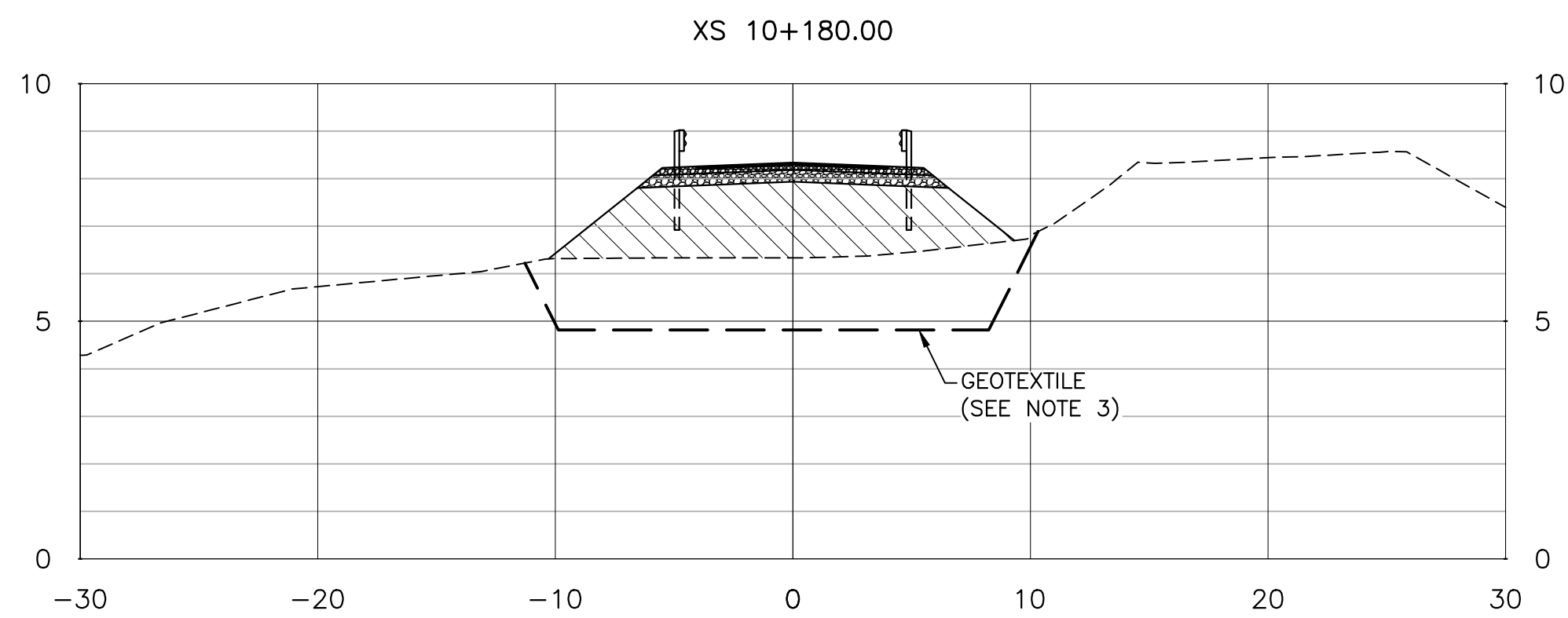
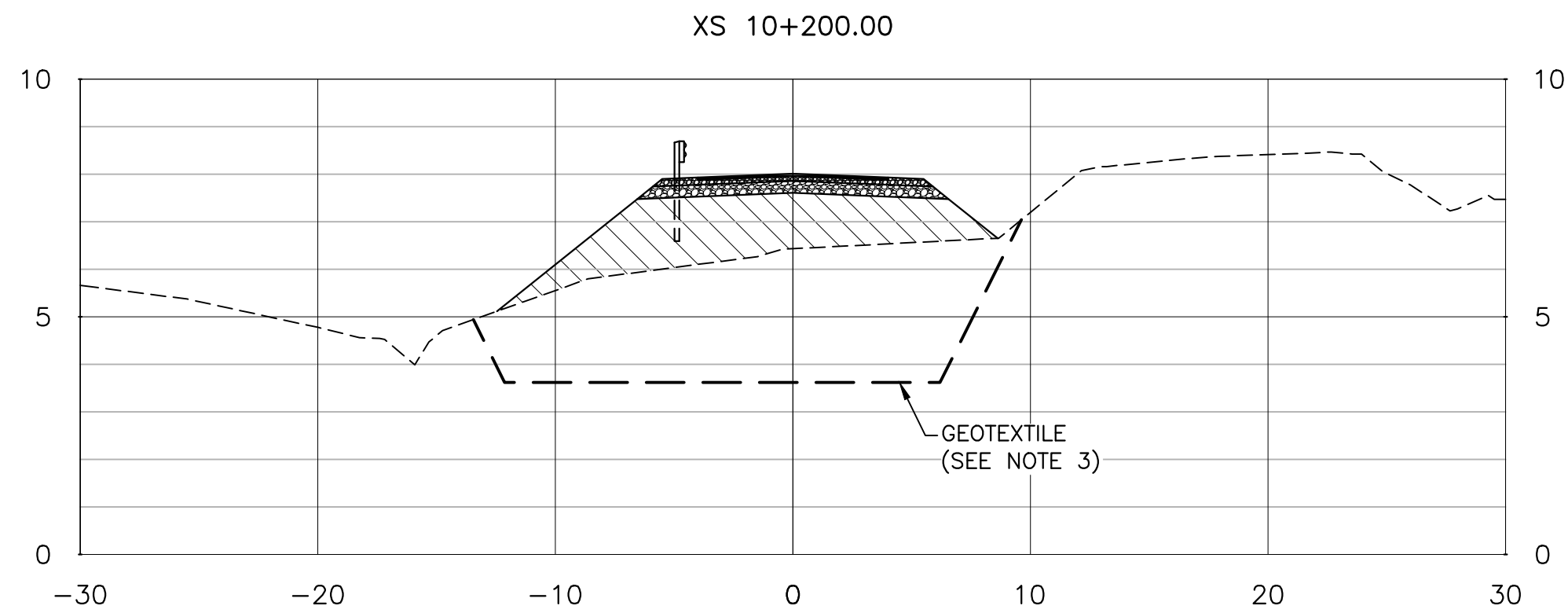
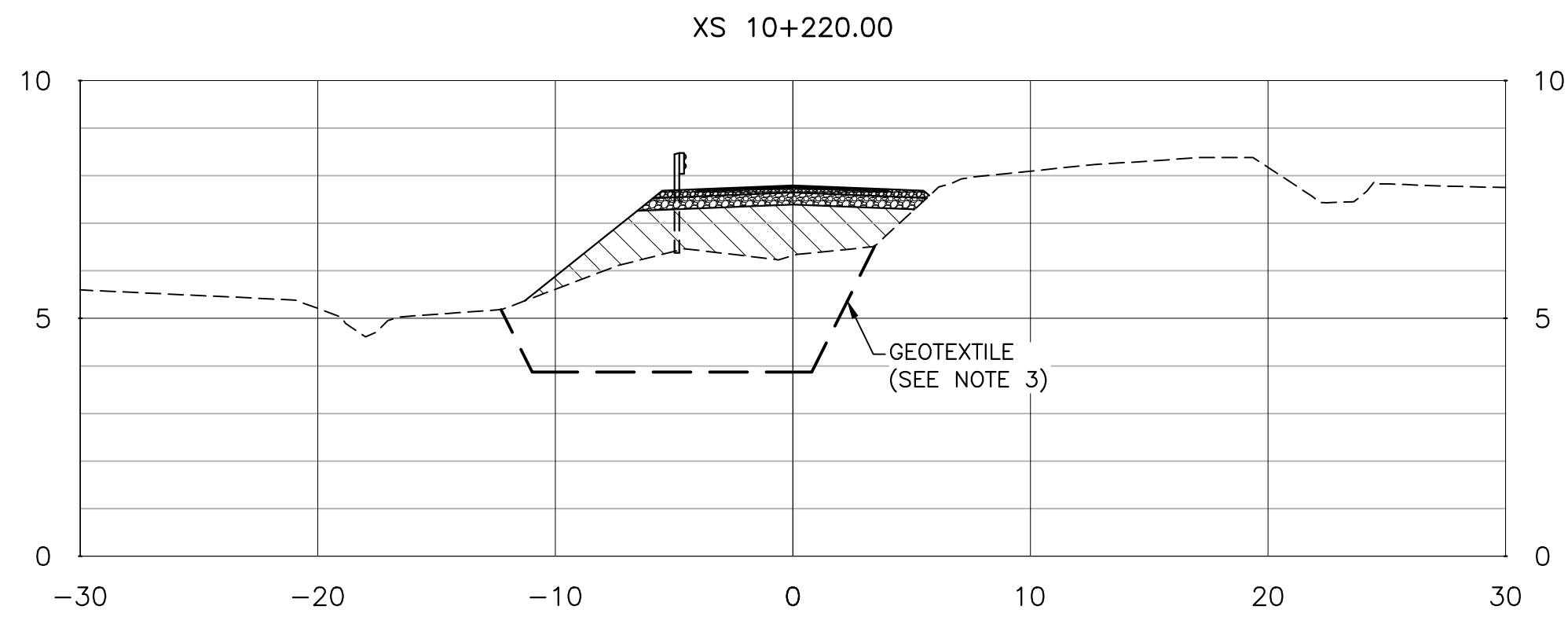
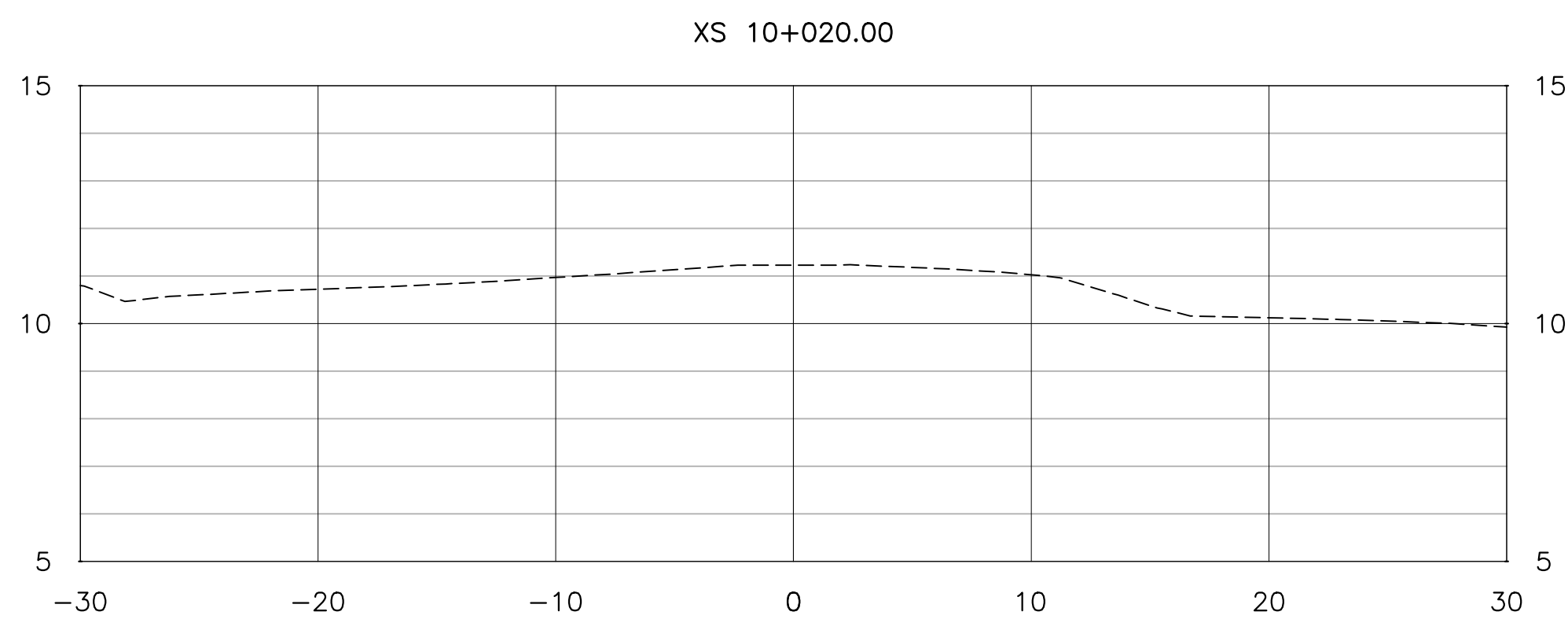
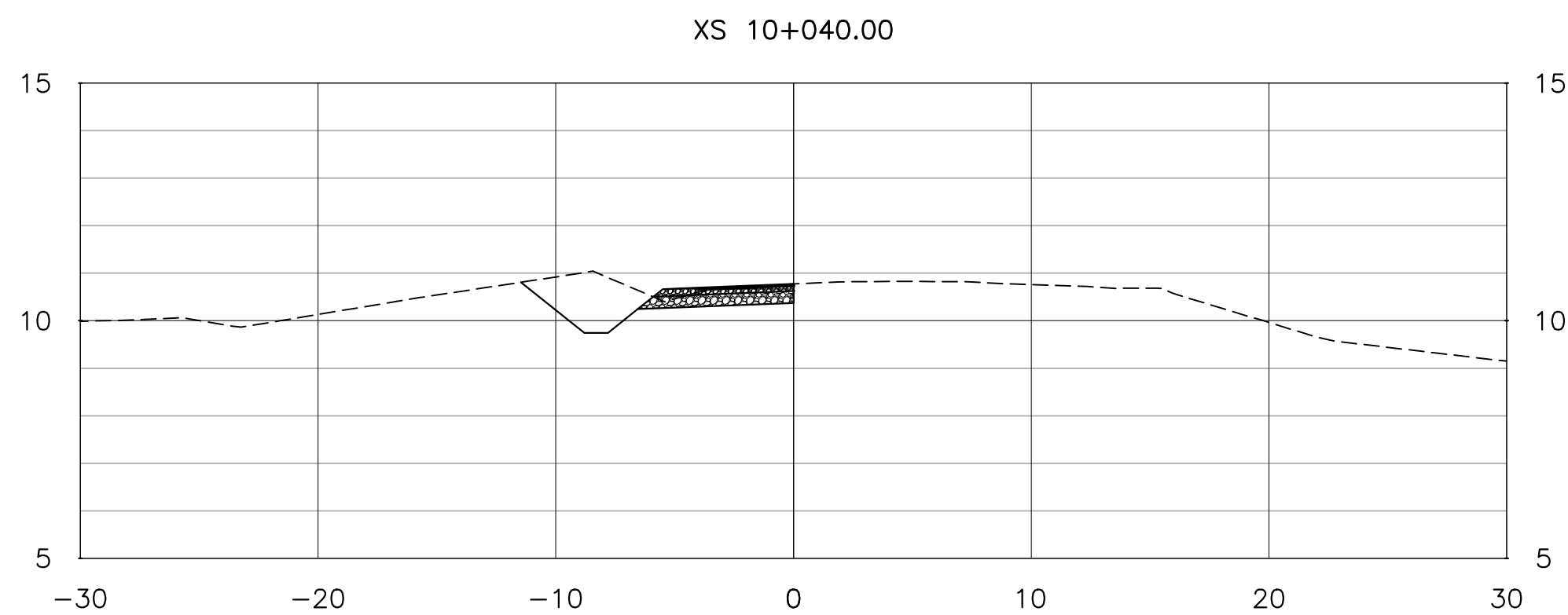
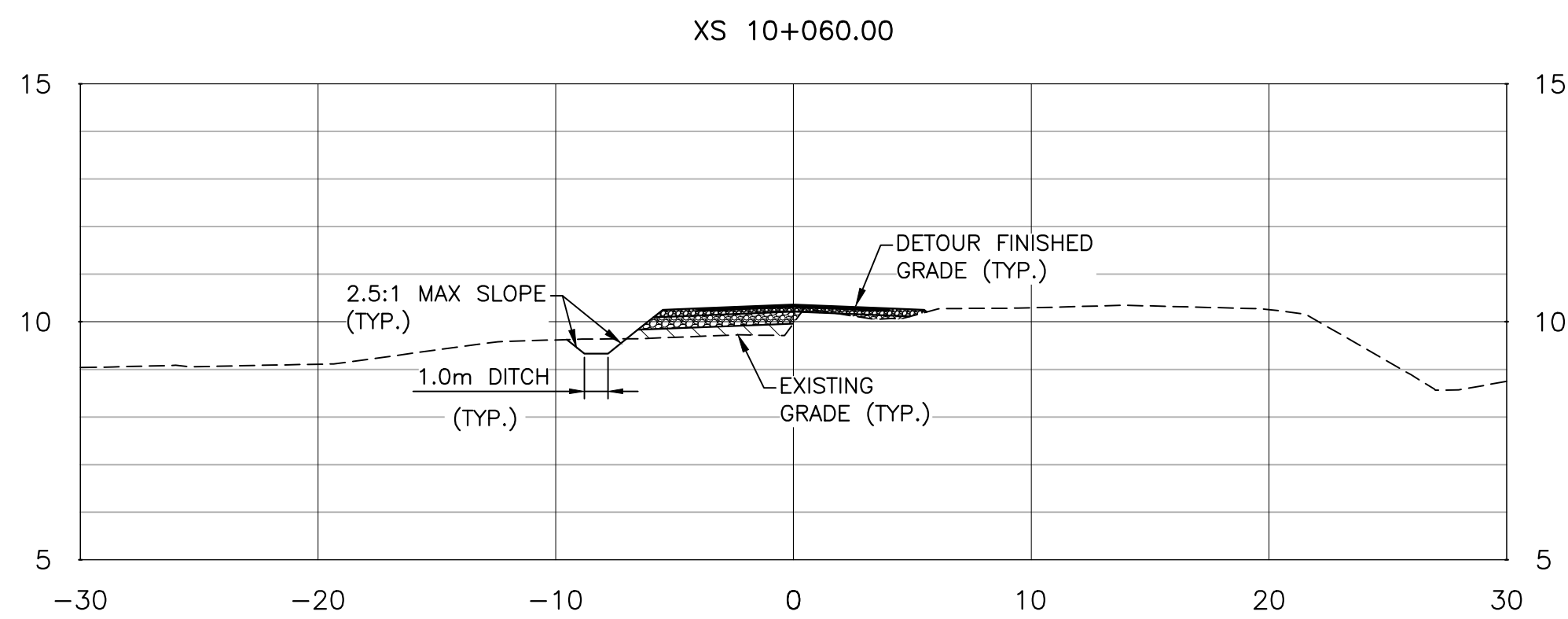
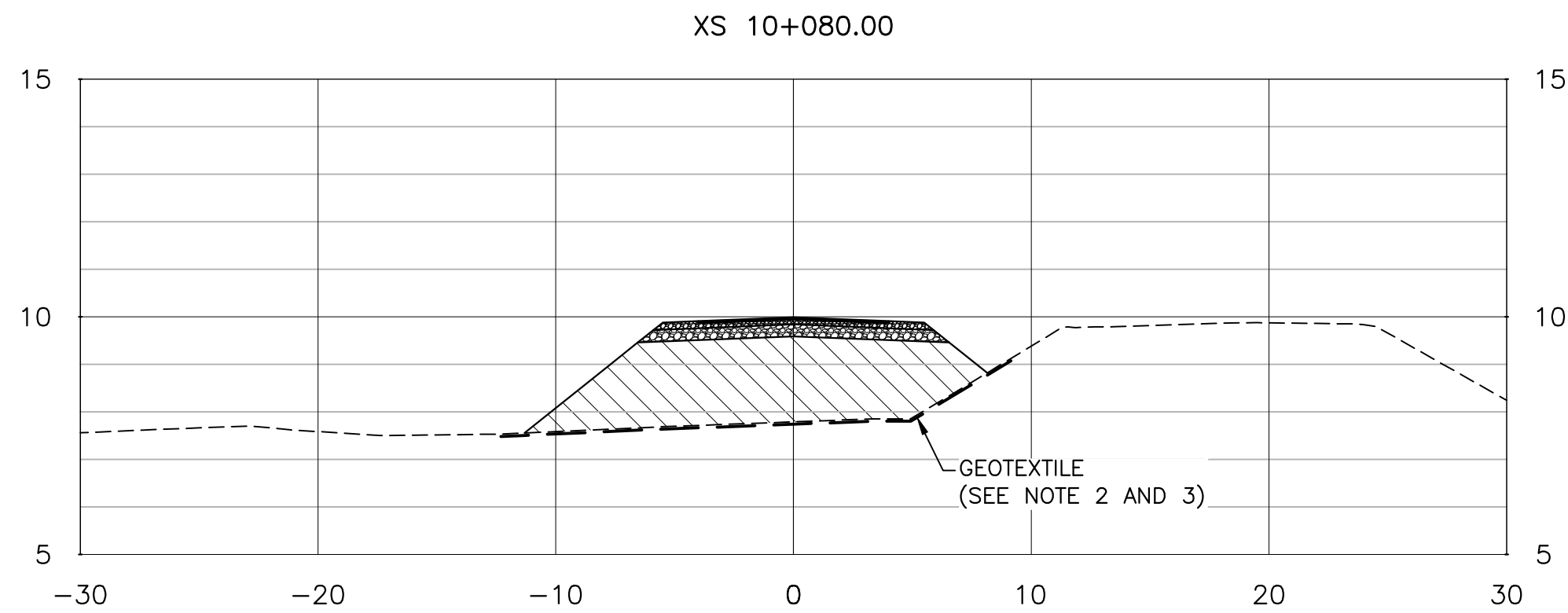
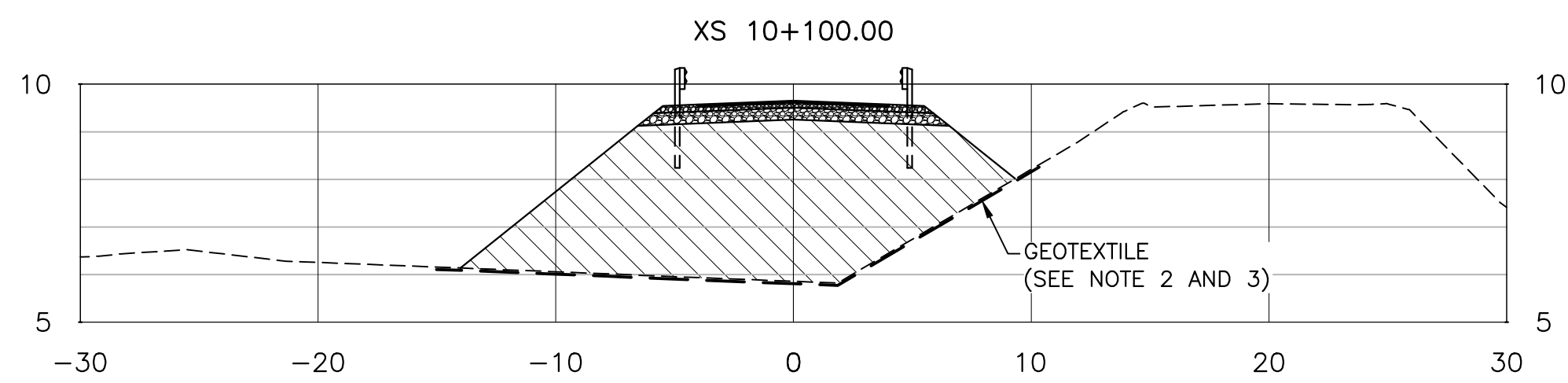
0	ISSUED FOR TENDER	JULY 14 2021
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project	WESTERN BROOK BRIDGE REPLACEMENT	
	GROS MORNE NATIONAL PARK	

drawing
dessin

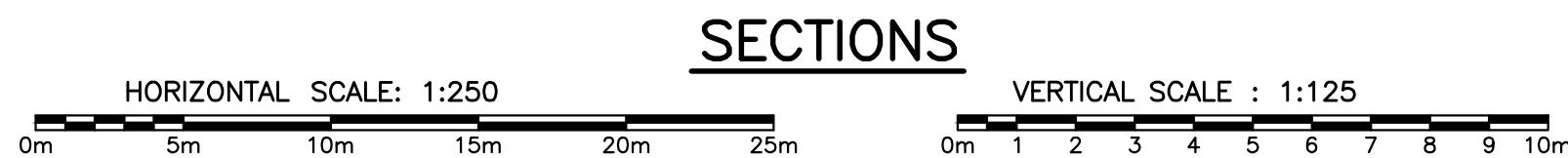
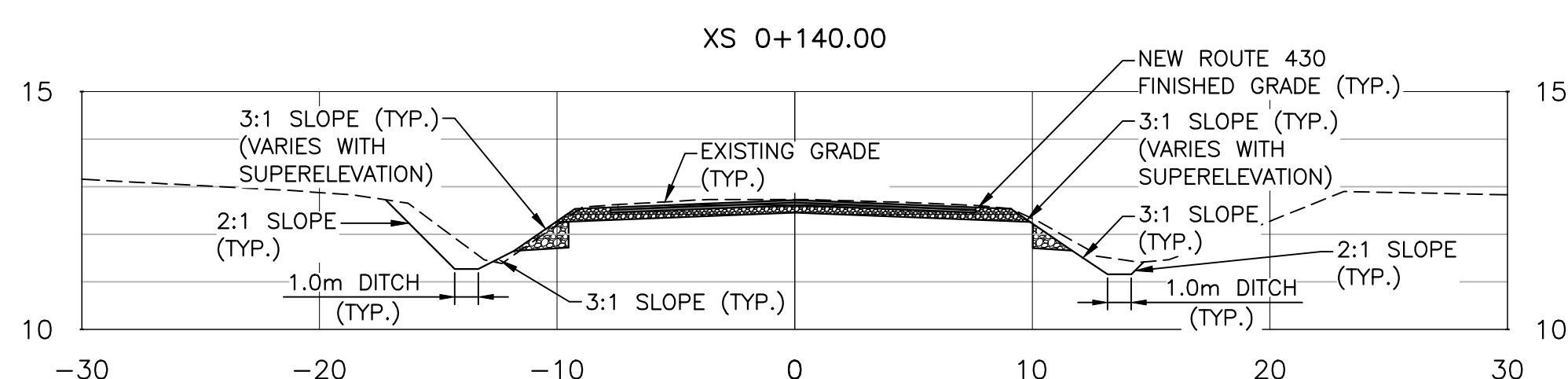
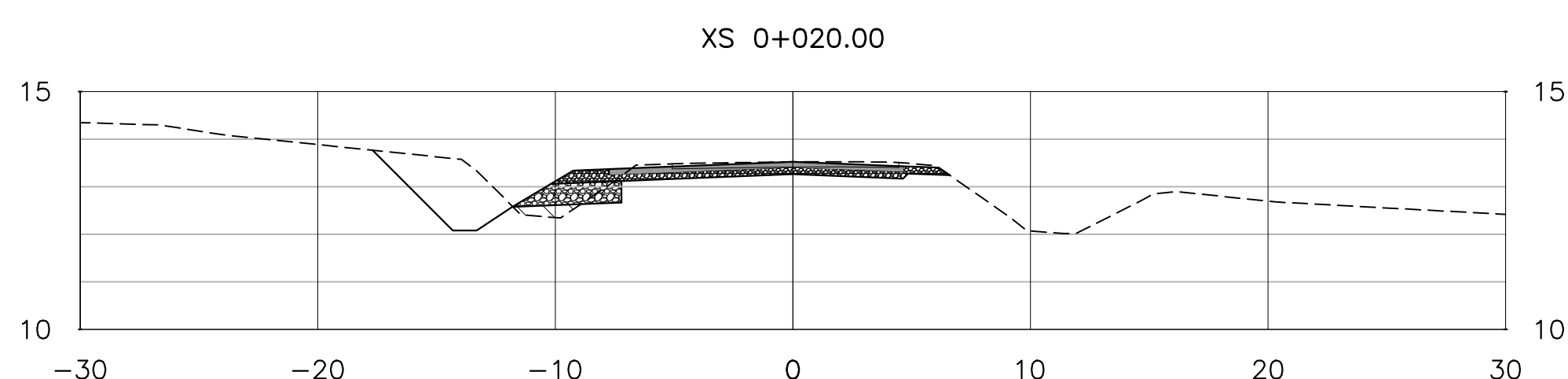
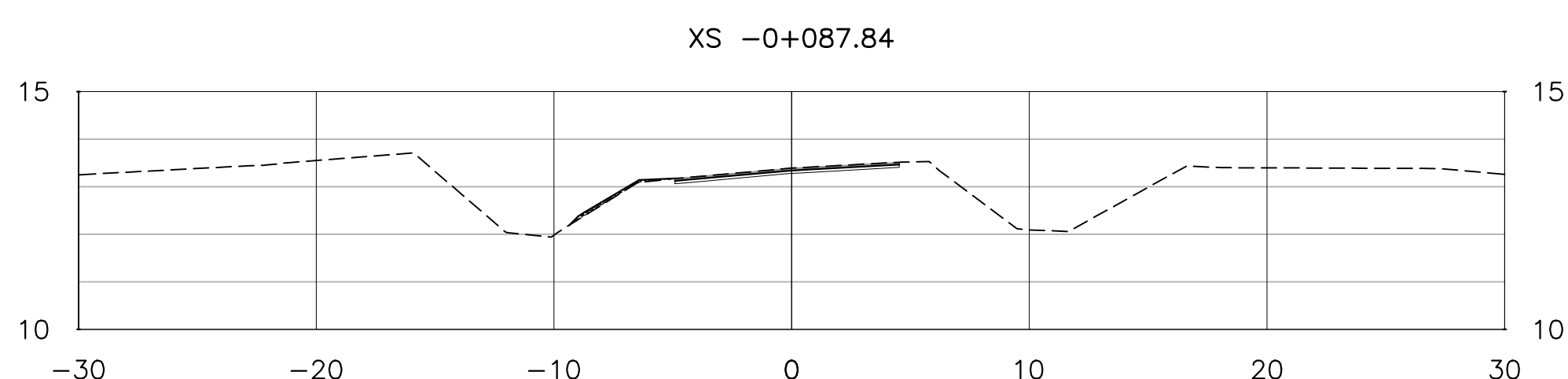
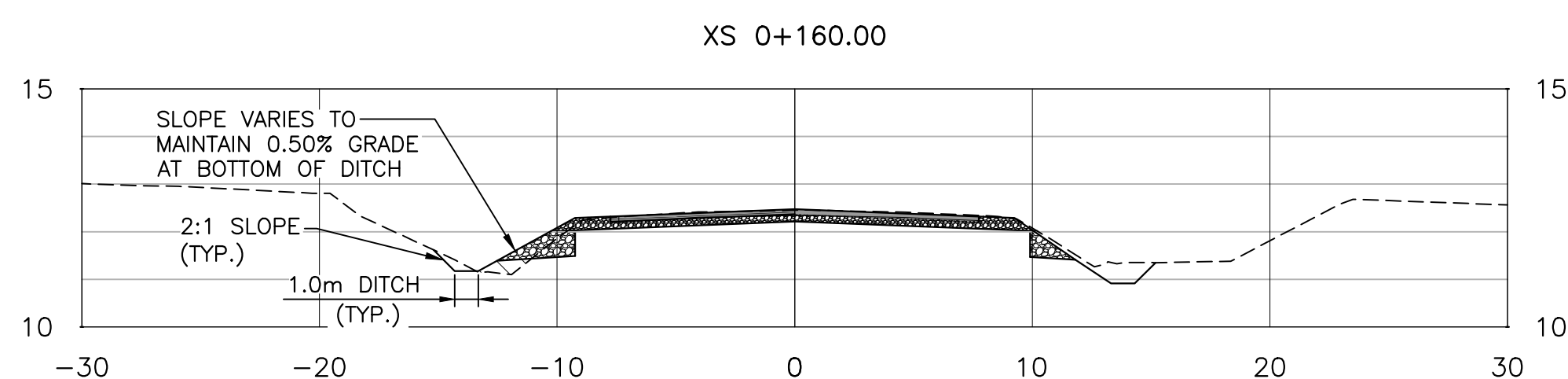
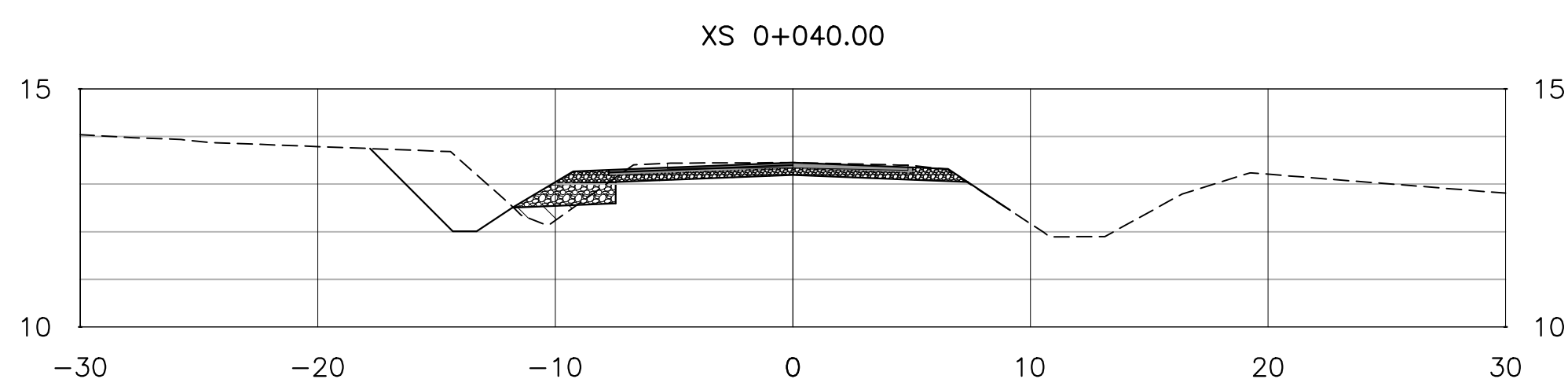
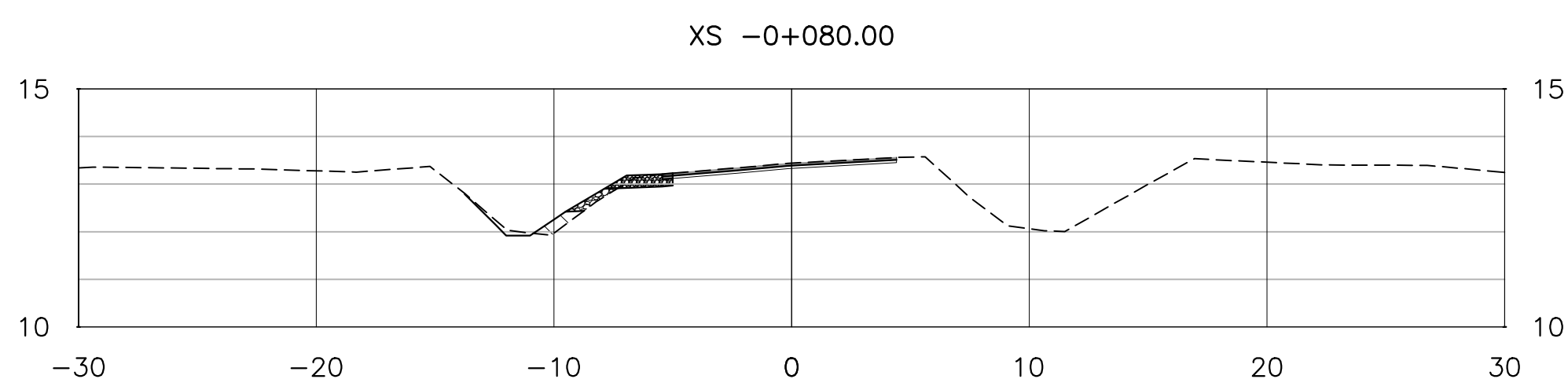
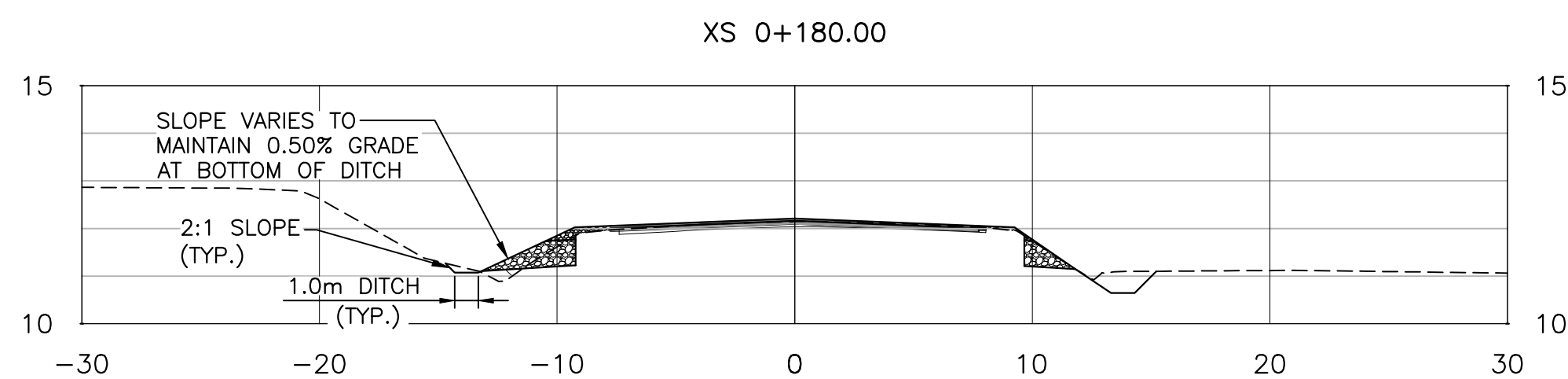
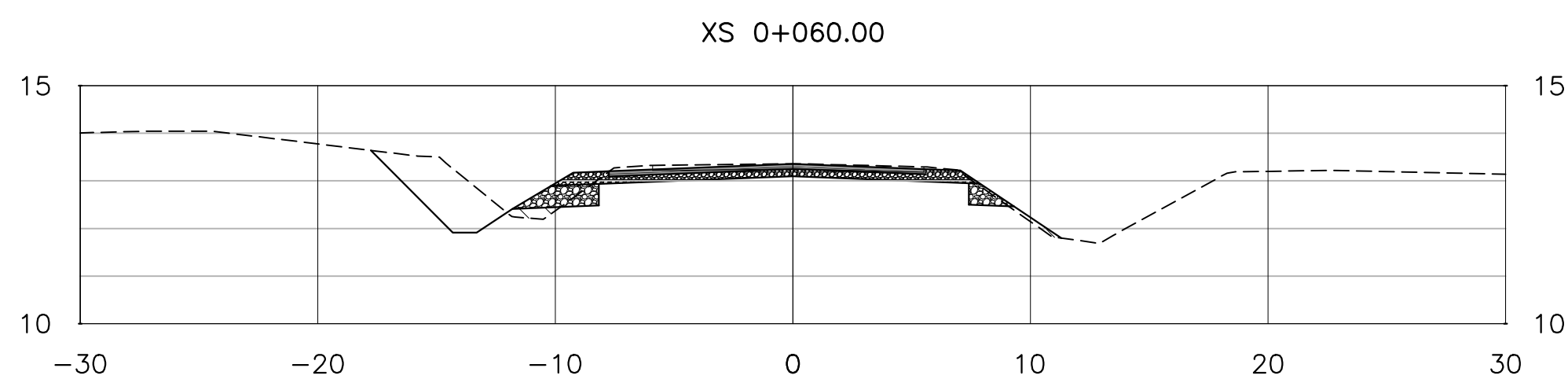
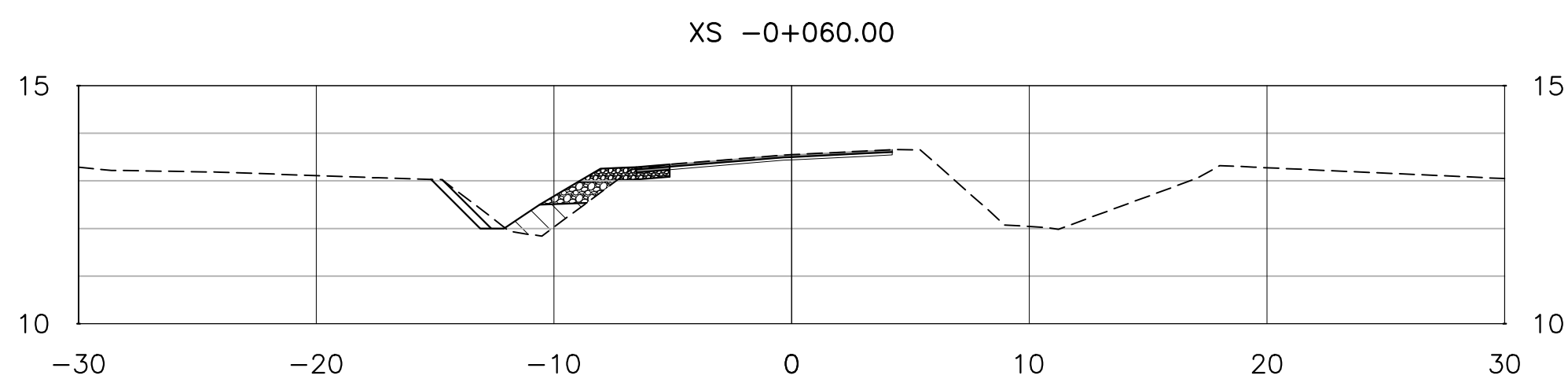
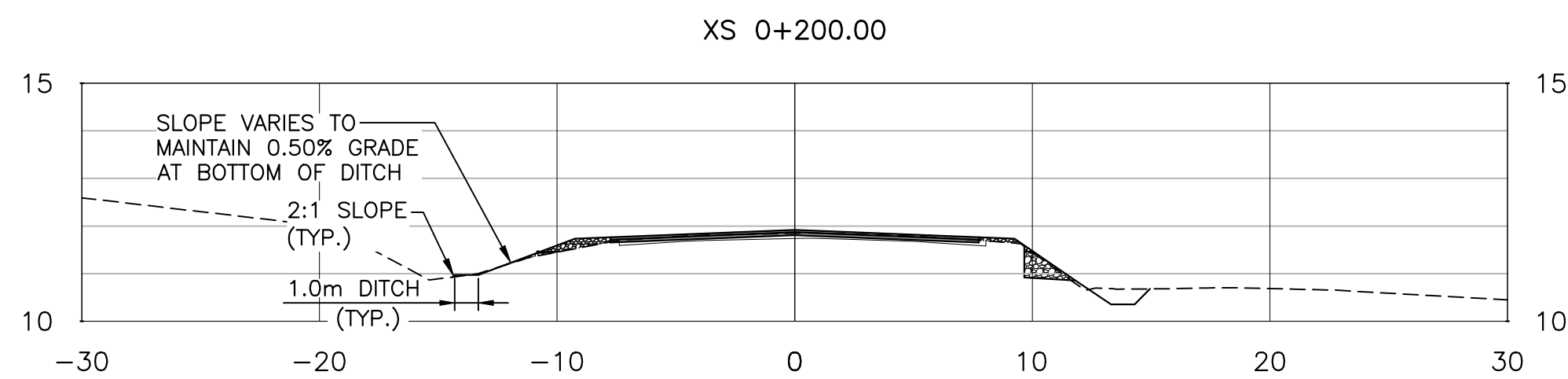
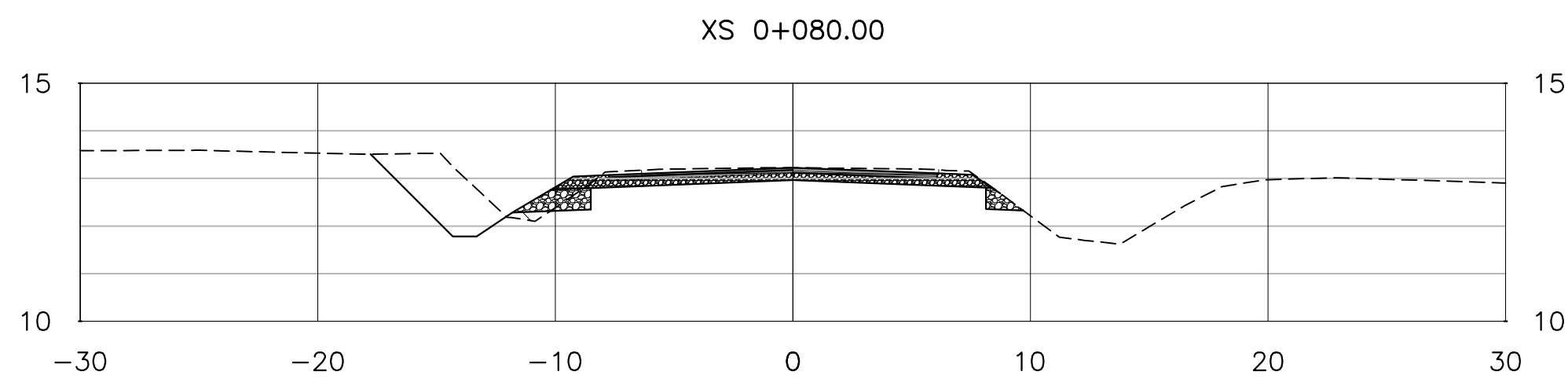
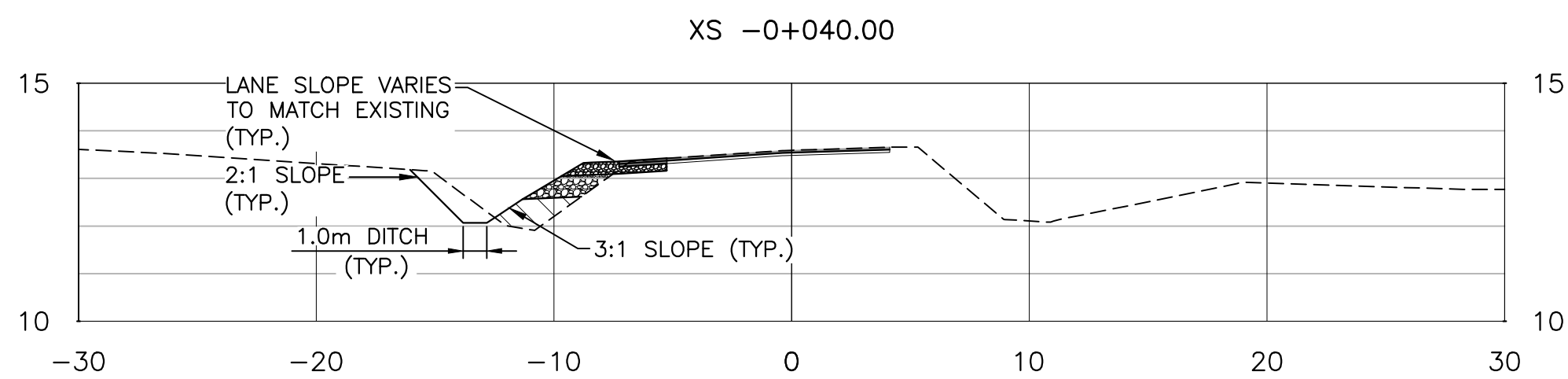
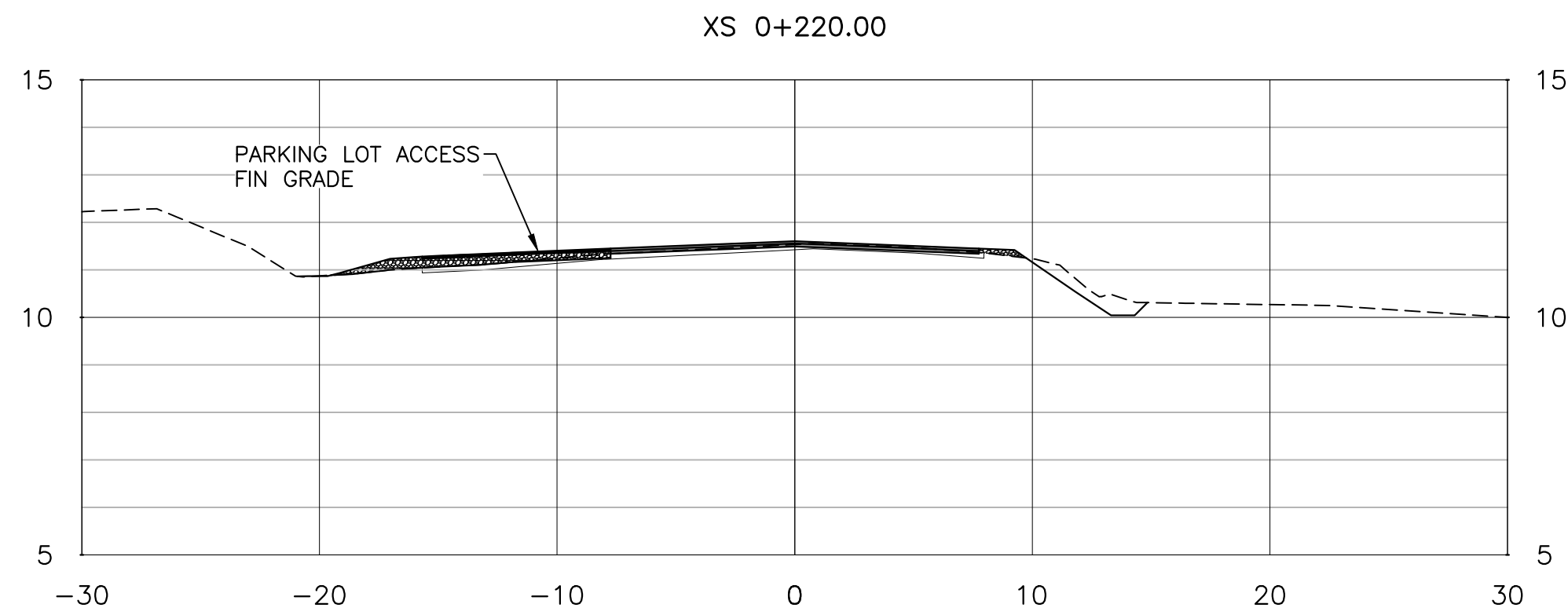
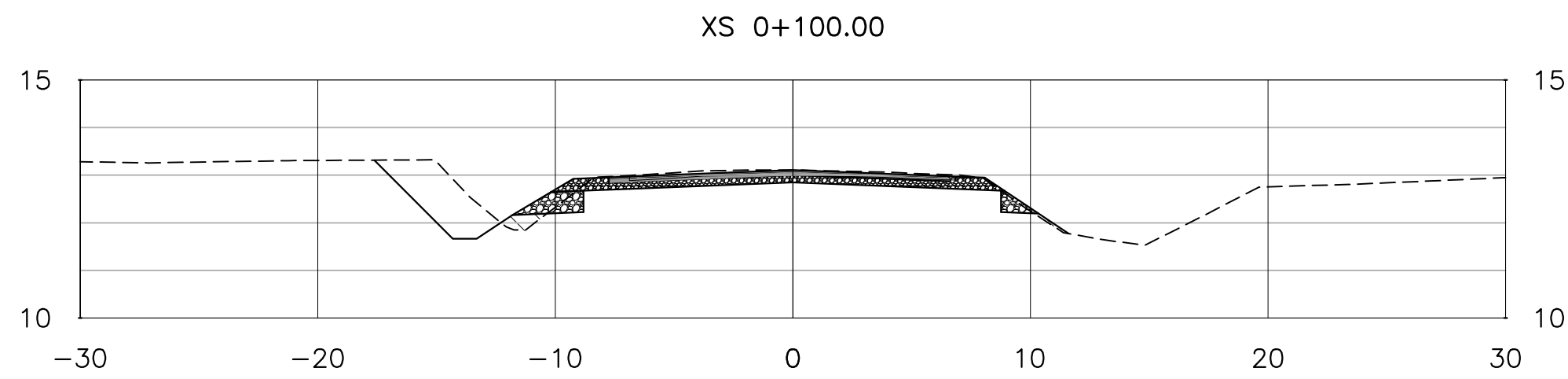
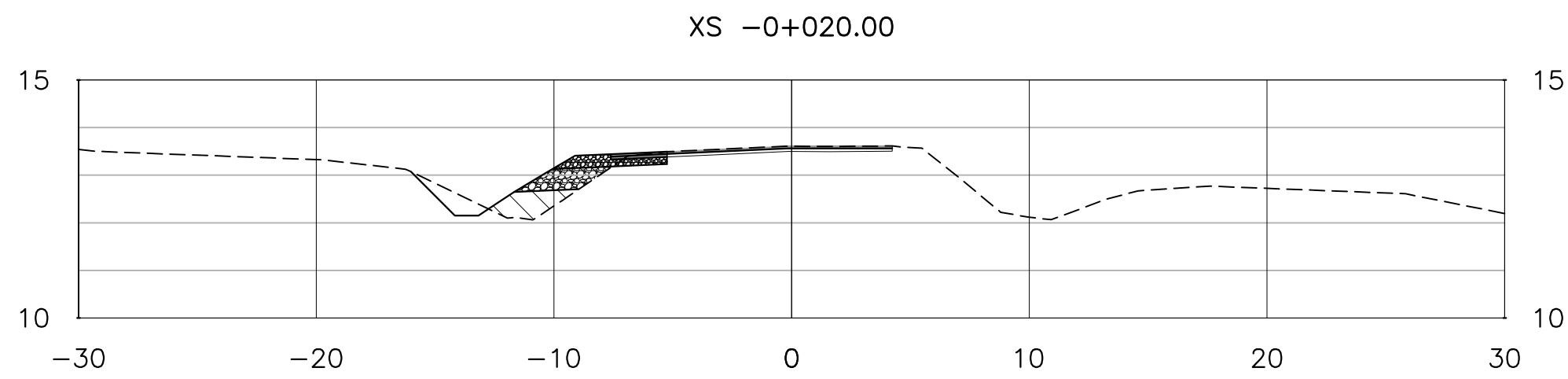
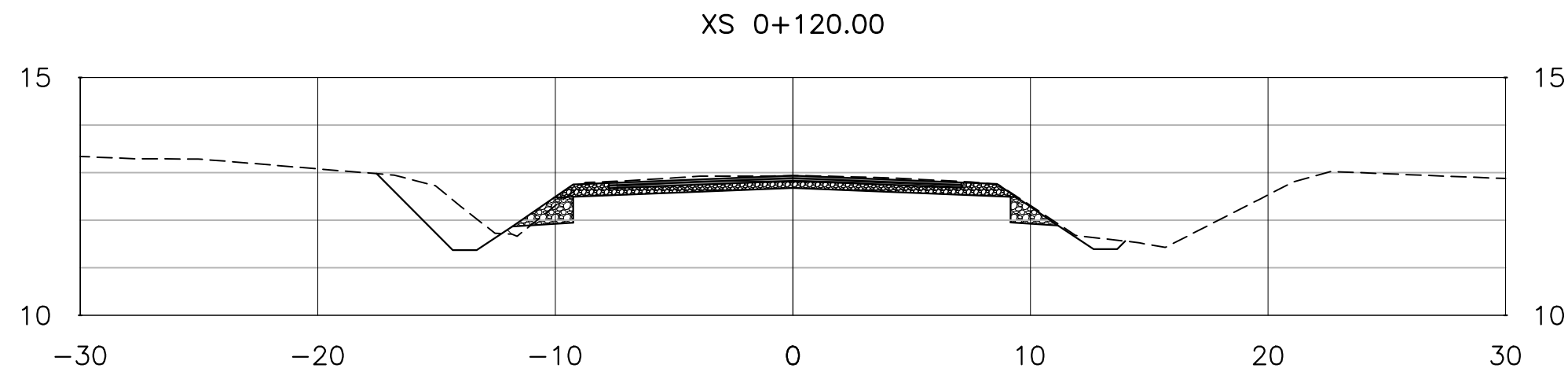
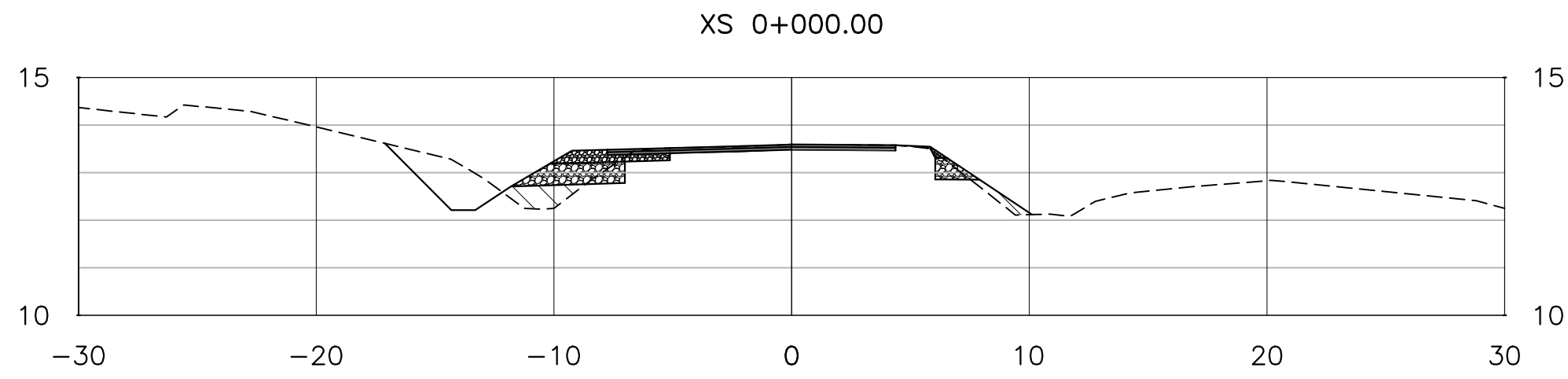
DESIGNATED PROFESSIONAL ENGINEER
MICHAEL S. MACDONALD
SIGNATURE
JULY 14/21
DATE
NEWFOUNDLAND & LABRADOR

DETOUR SECTIONS
(10+020.00 TO 10+260.00)
(SHEET 1 OF 1)

designed	MICHAEL MACDONALD	conçu
date	MARCH 2019	
drawn	CORY BAKER	dessiné
date	MARCH 2019	
approved	MICHAEL MACDONALD	approuvé
date	MARCH 2019	
Tender		Soumission
PCA Project Manager	Administrateur de projets APC	
project number	182009	no. du projet
drawing no.	C06	no. du dessin



HORIZONTAL SCALE: 1:250
VERTICAL SCALE: 1:125



GENERAL NOTES:
1. FOR GENERAL NOTES SEE DRAWING C1.

PROVINCE OF NEWFOUNDLAND
PERMIT HOLDER
CLASS "A"
This Permit Allows
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To practice Professional Engineering
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Permit No. as issued by PEGNL N0763
which is valid for the year 2021.



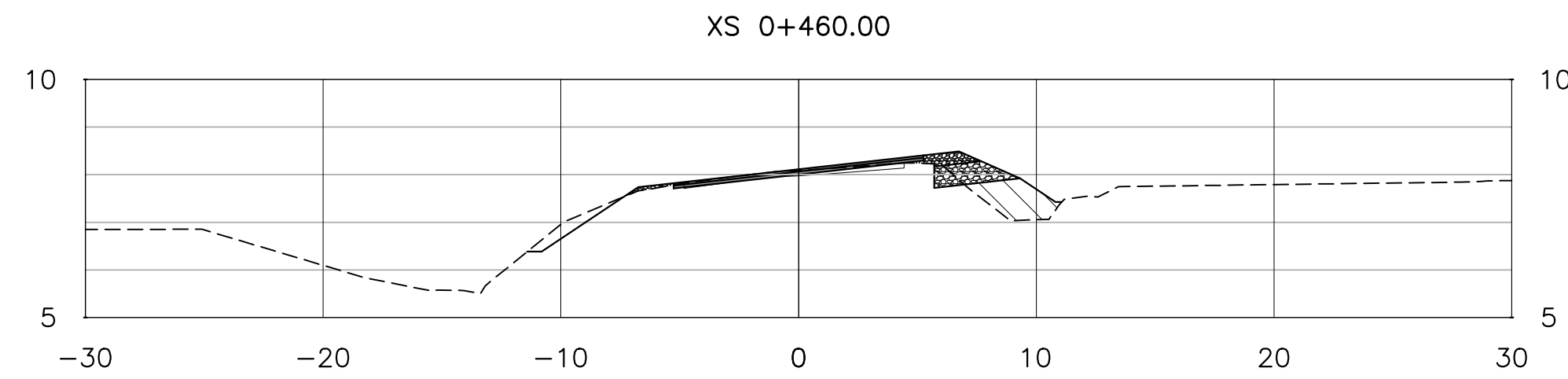
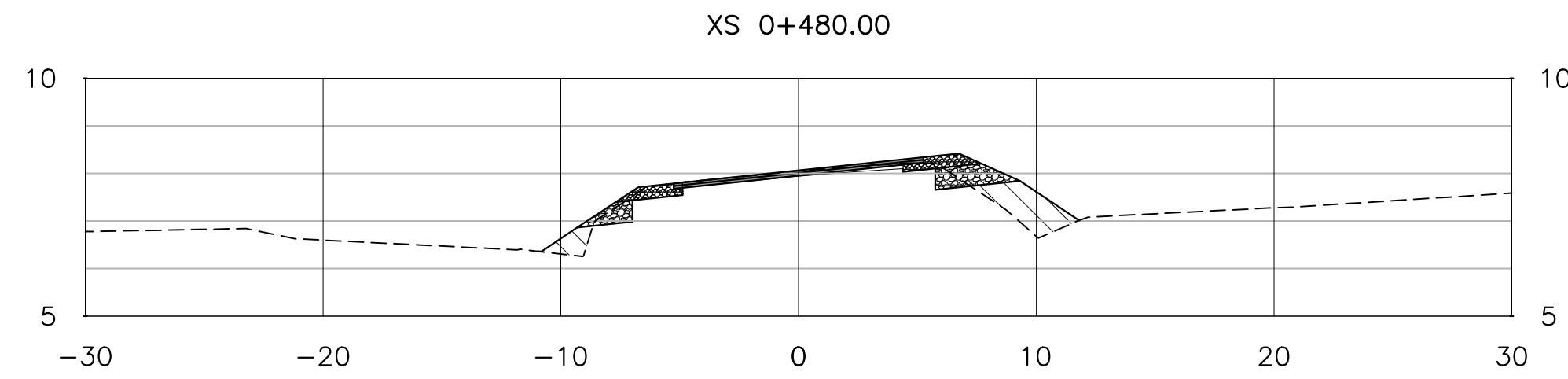
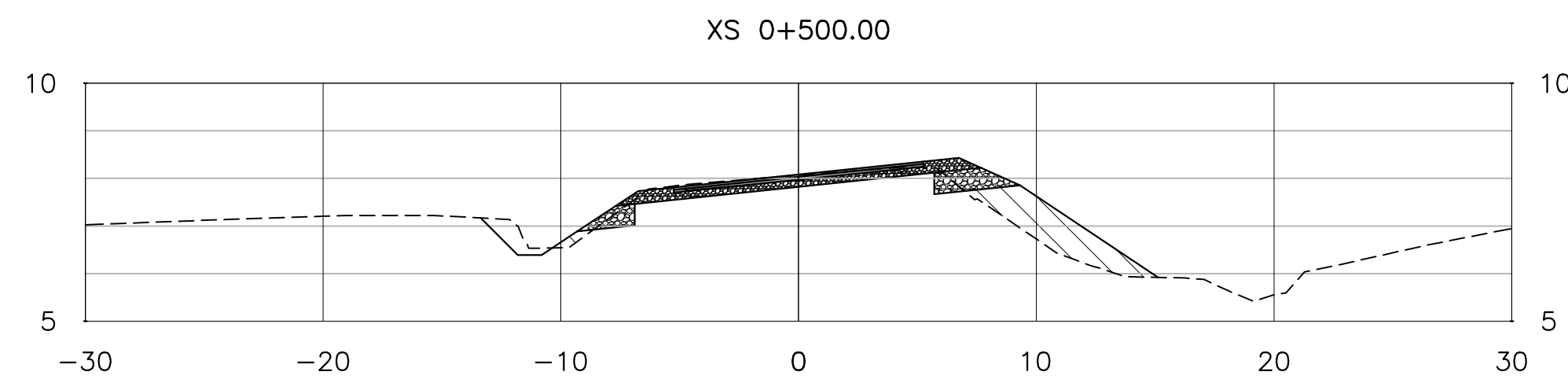
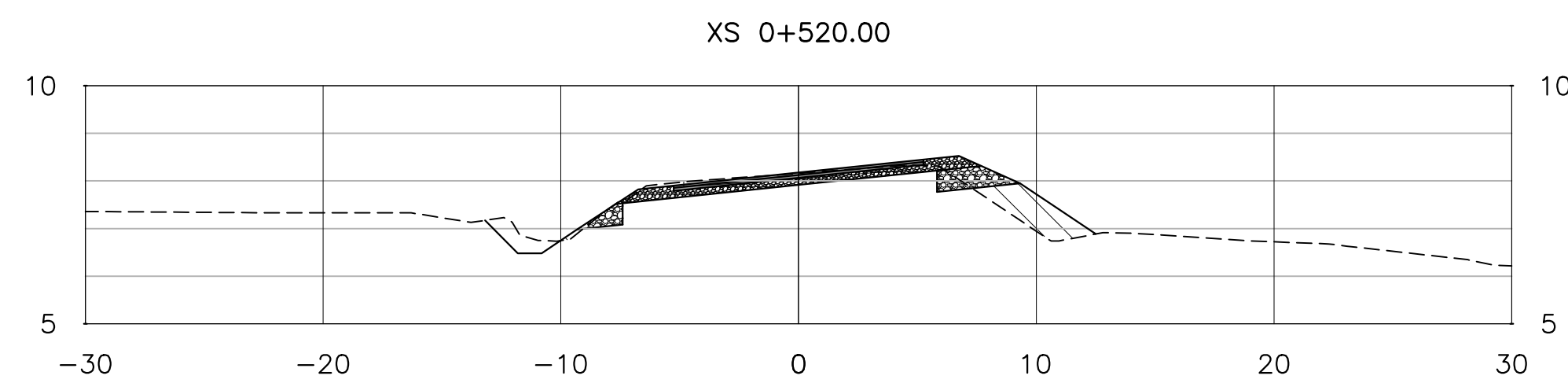
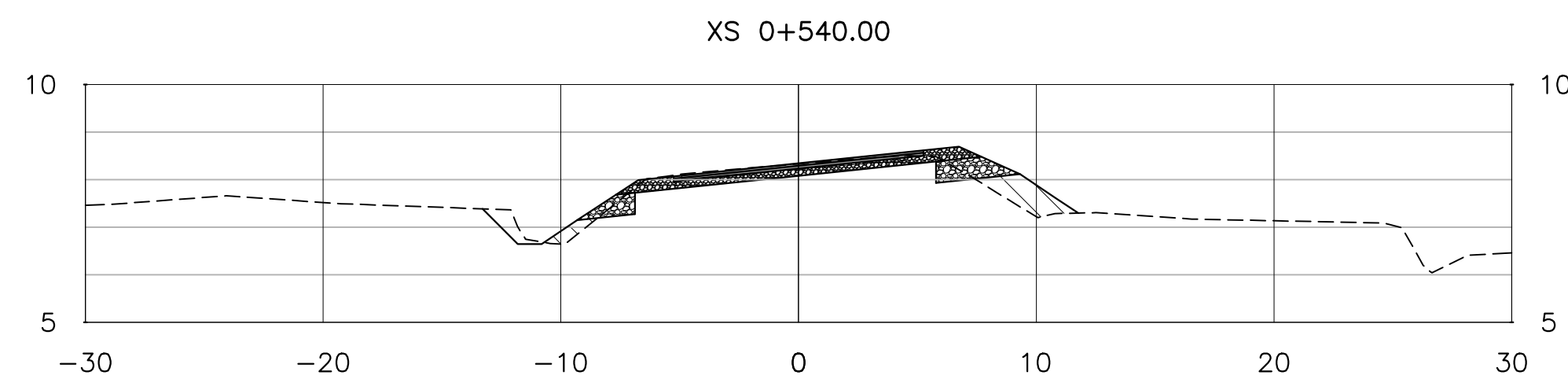
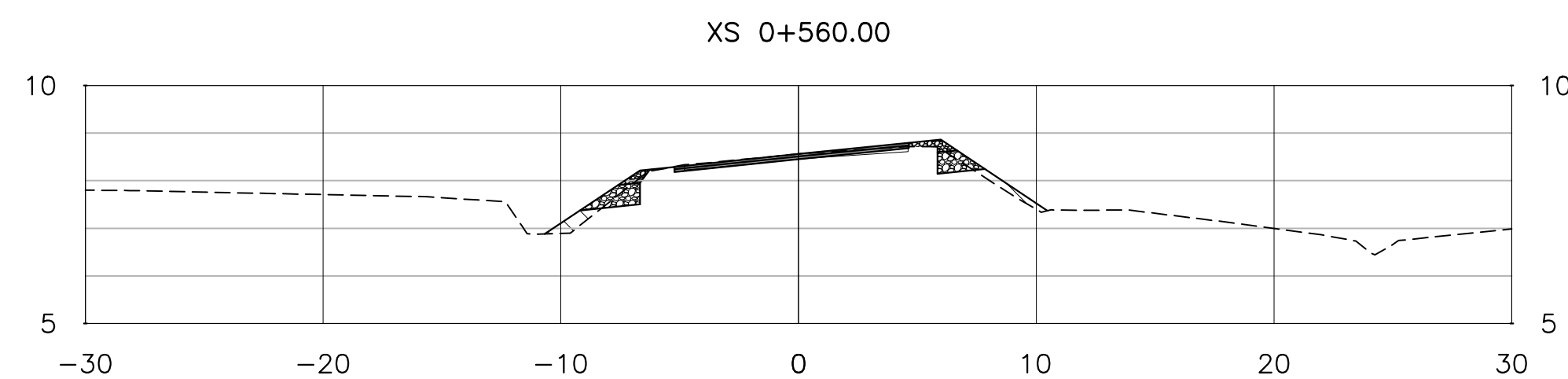
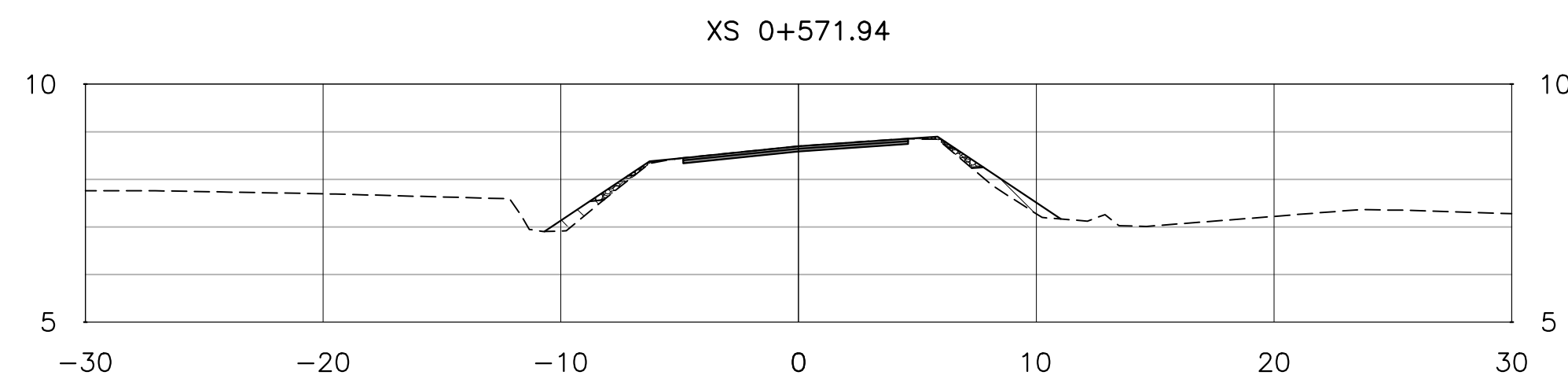
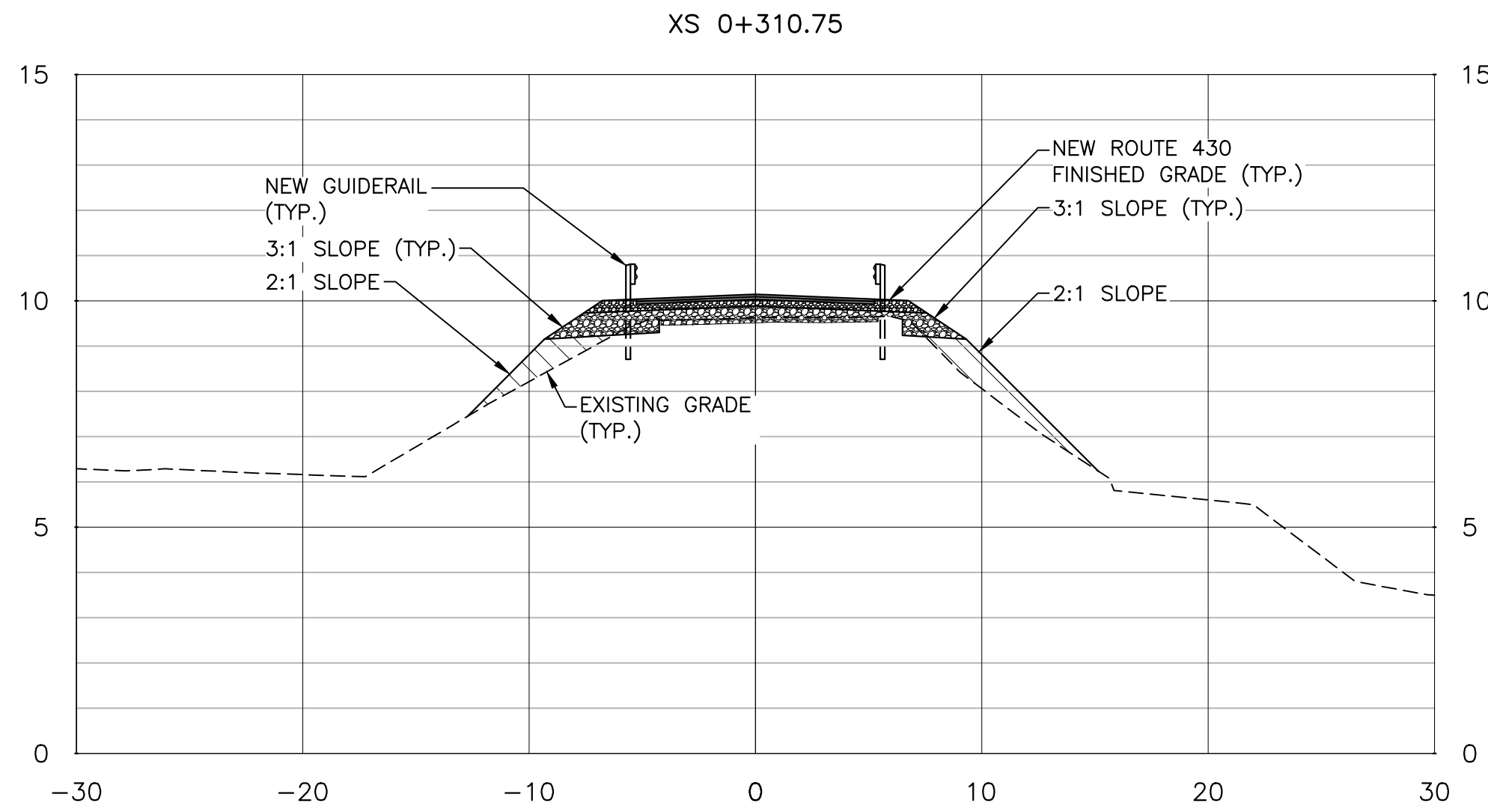
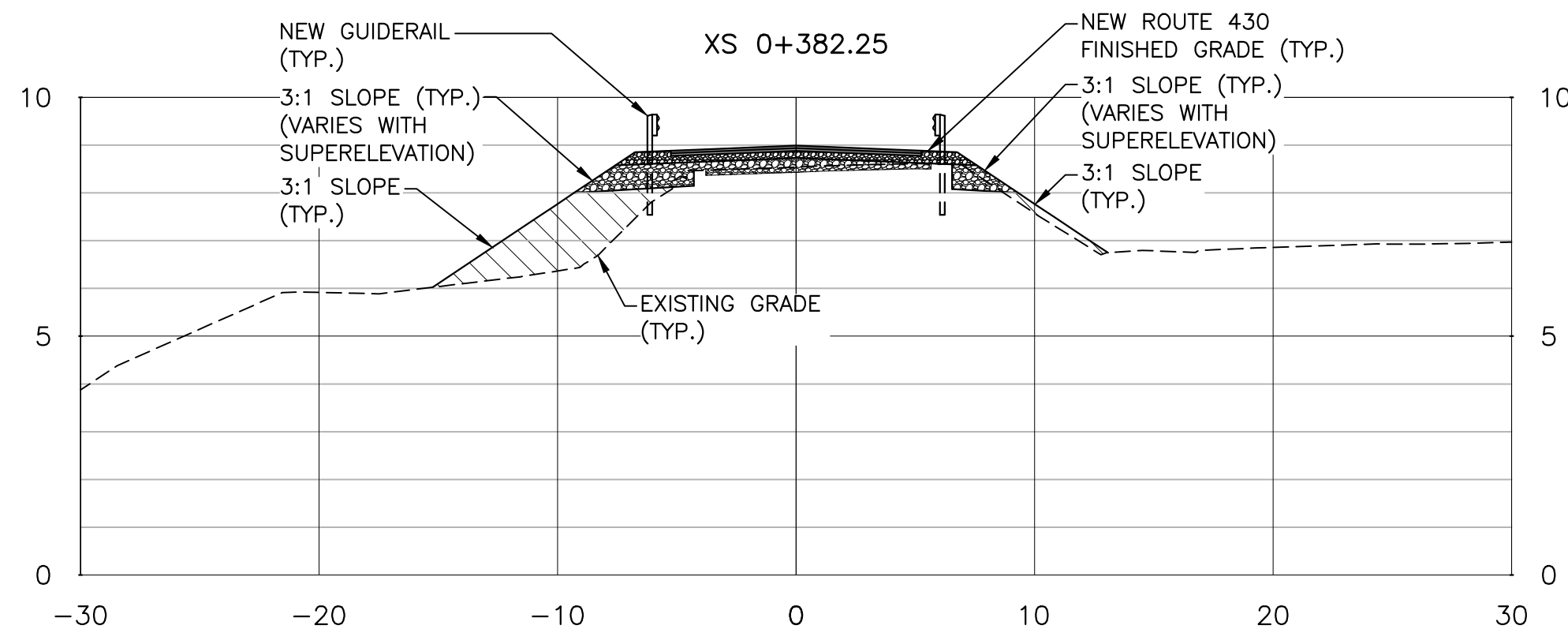
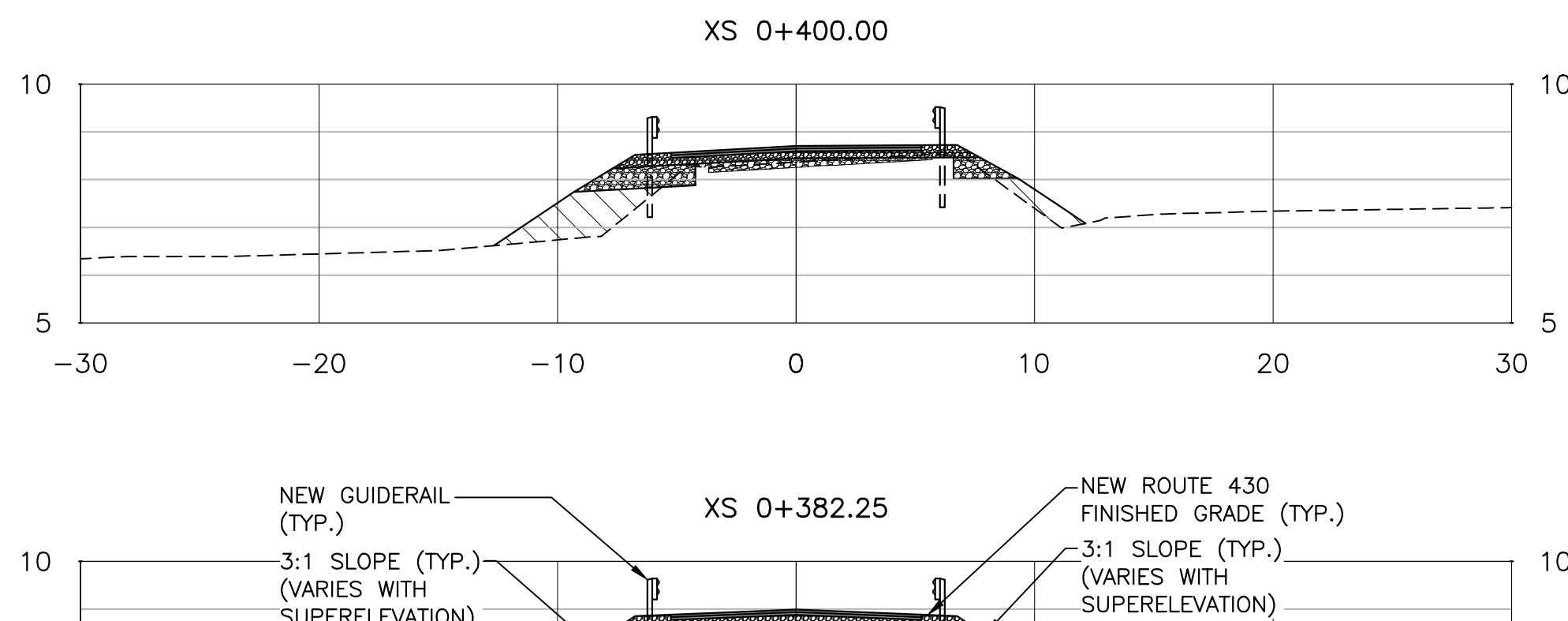
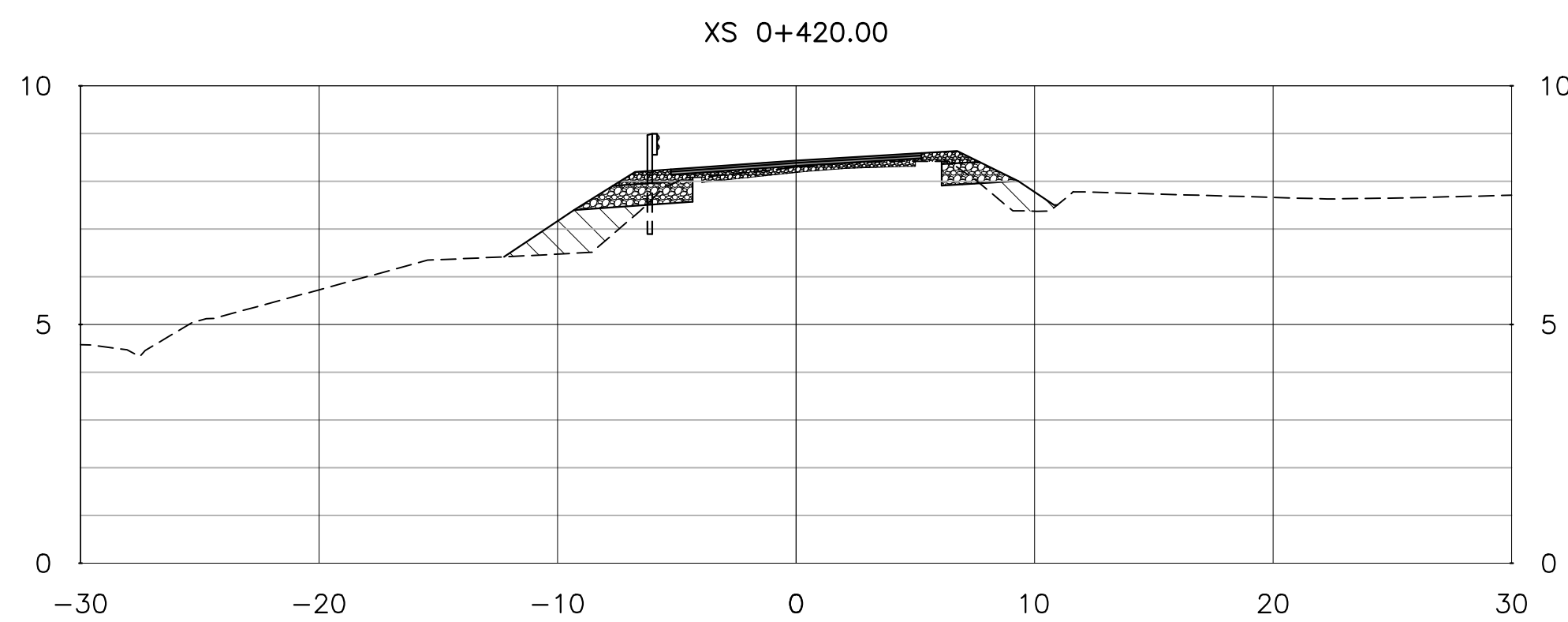
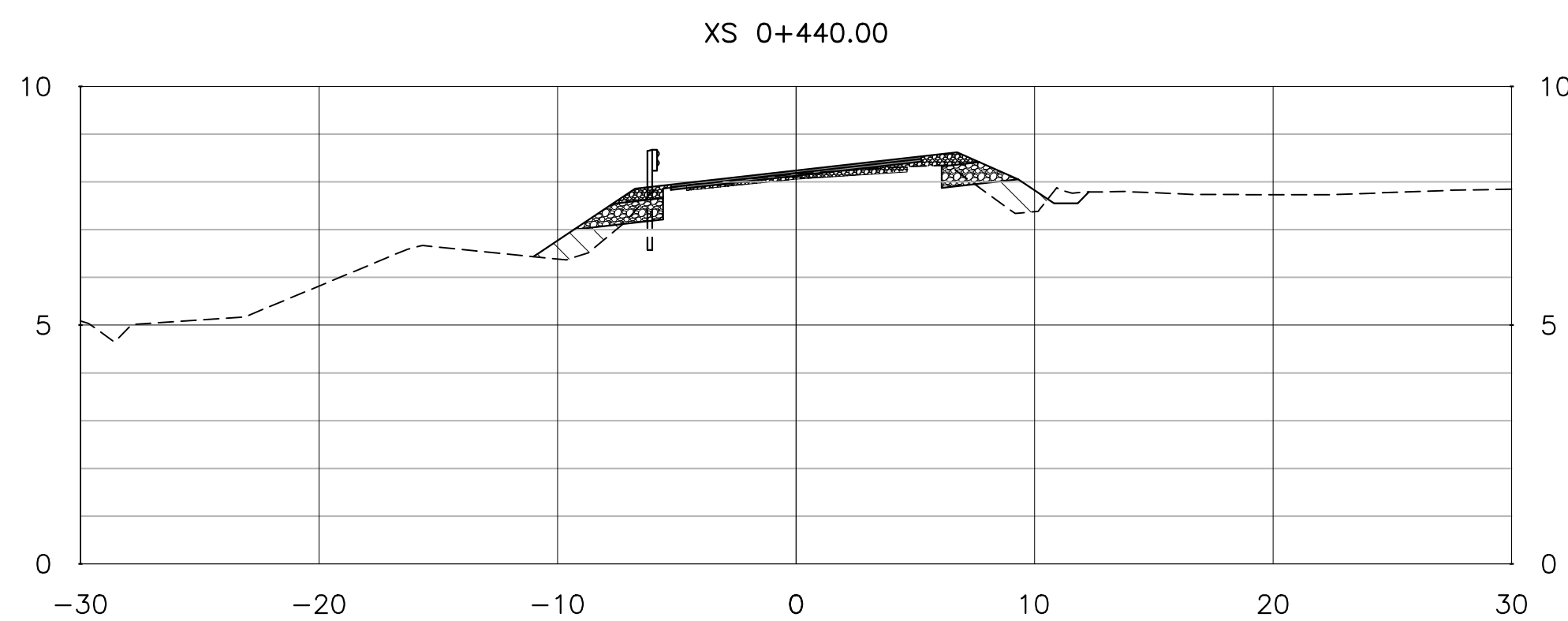
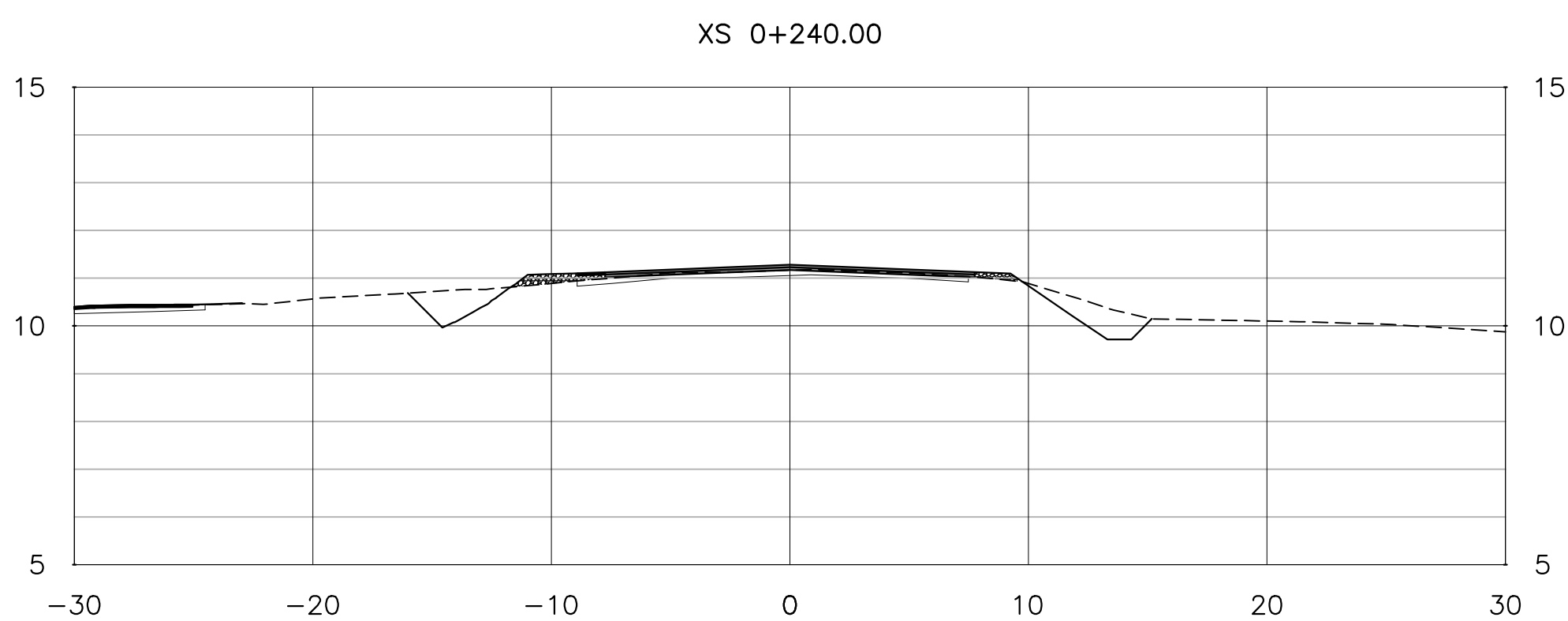
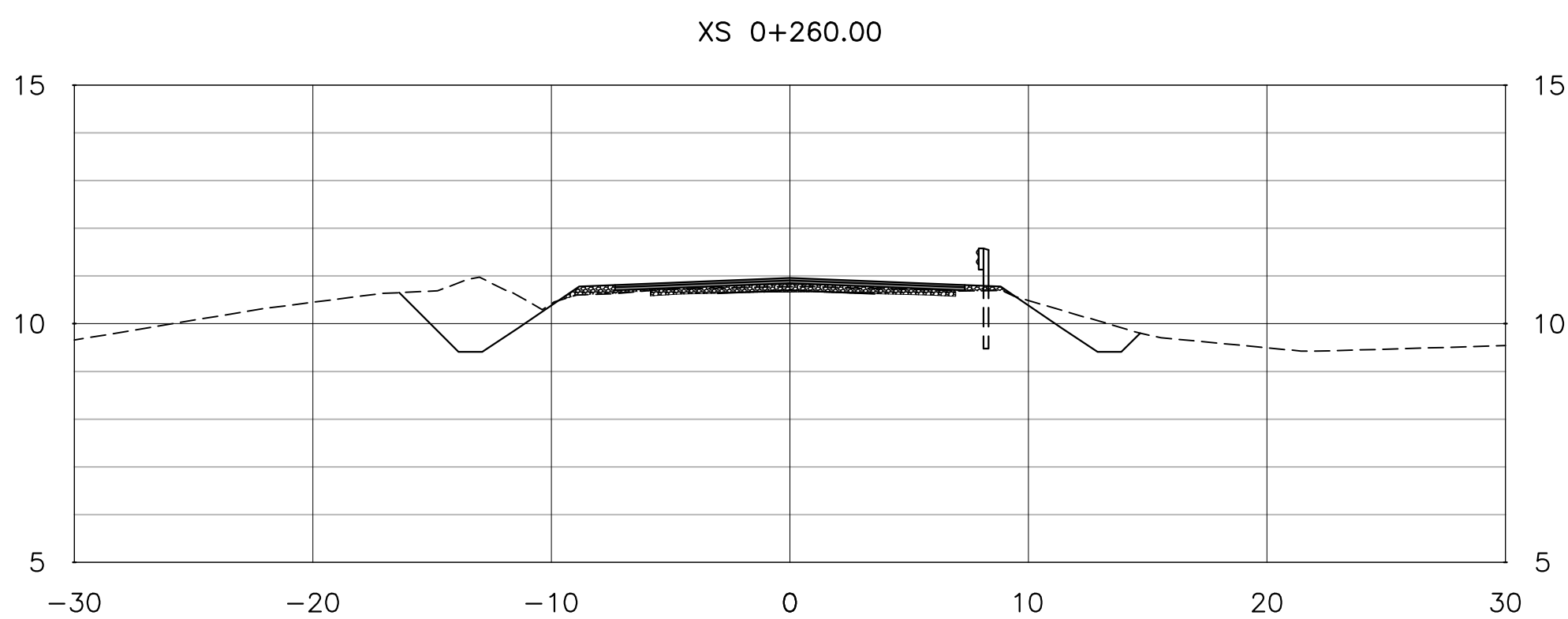
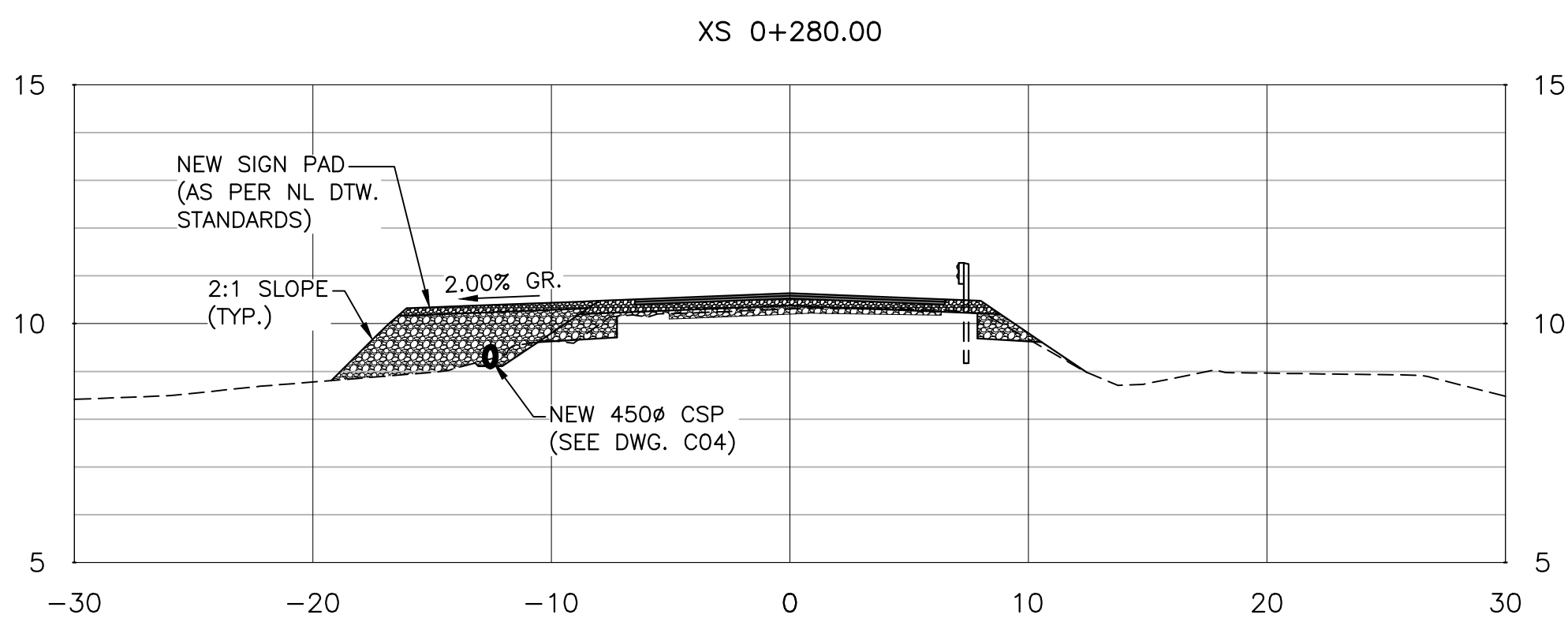
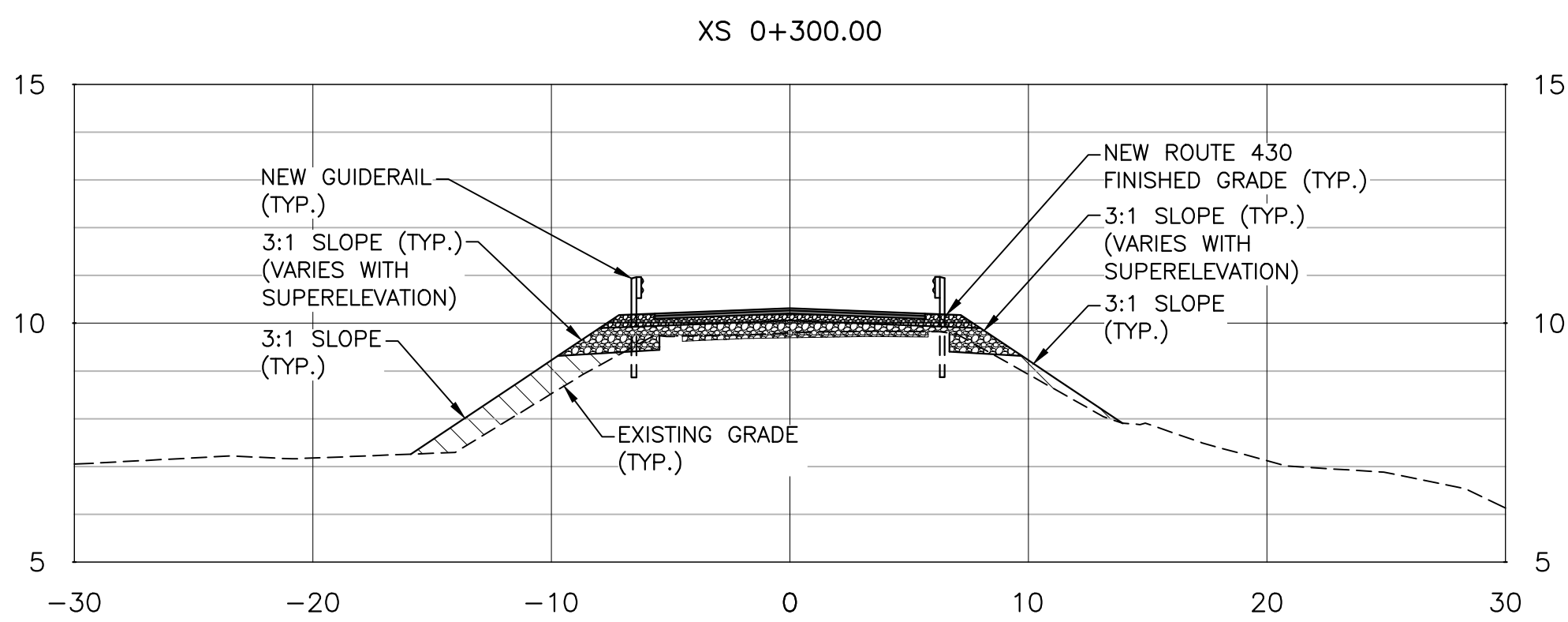
0	ISSUED FOR TENDER	JULY 14 2021
revisions		date

project
**WESTERN BROOK
BRIDGE REPLACEMENT**
**GROS MORNE
NATIONAL PARK**
drawing
dessin

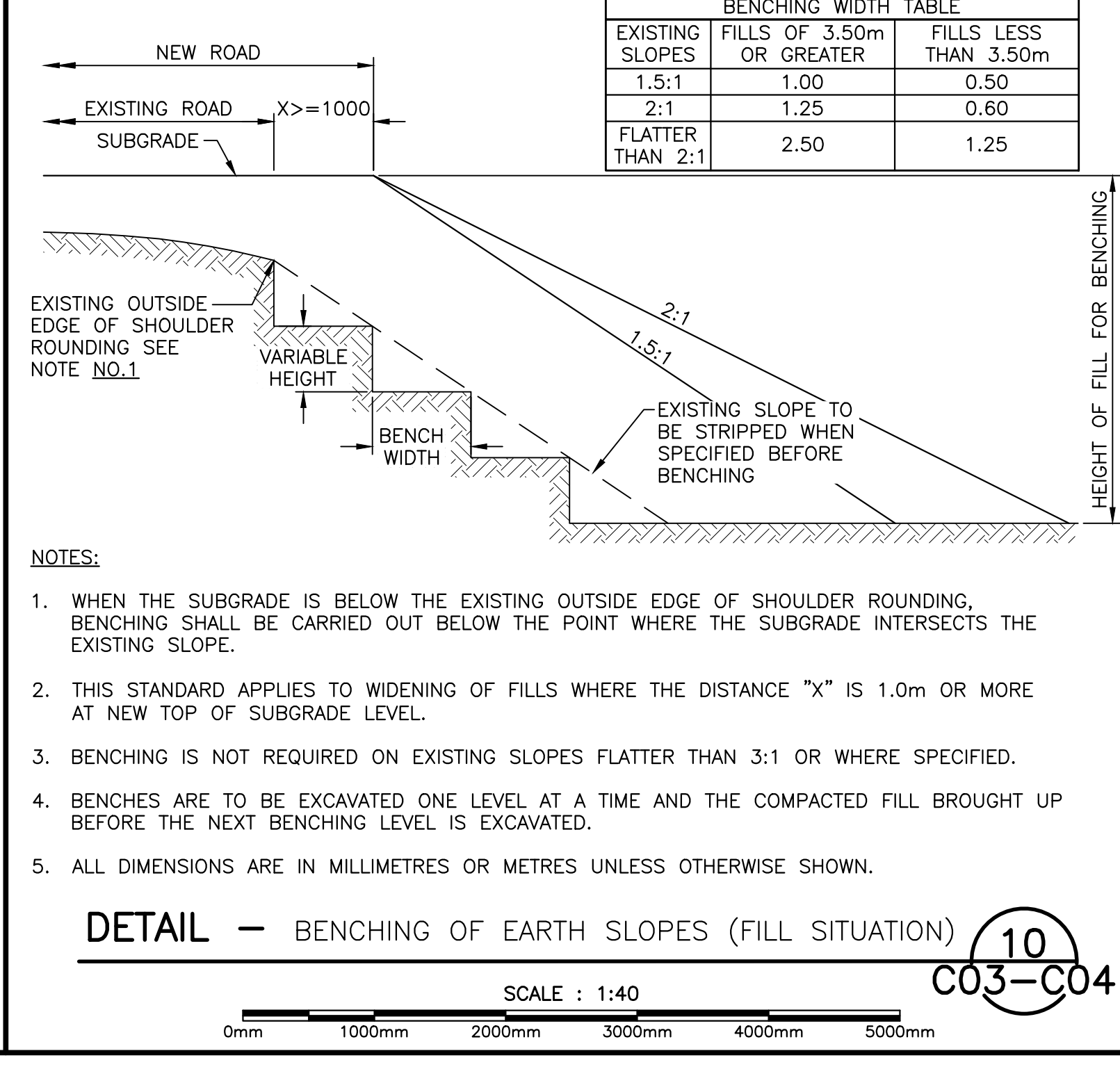
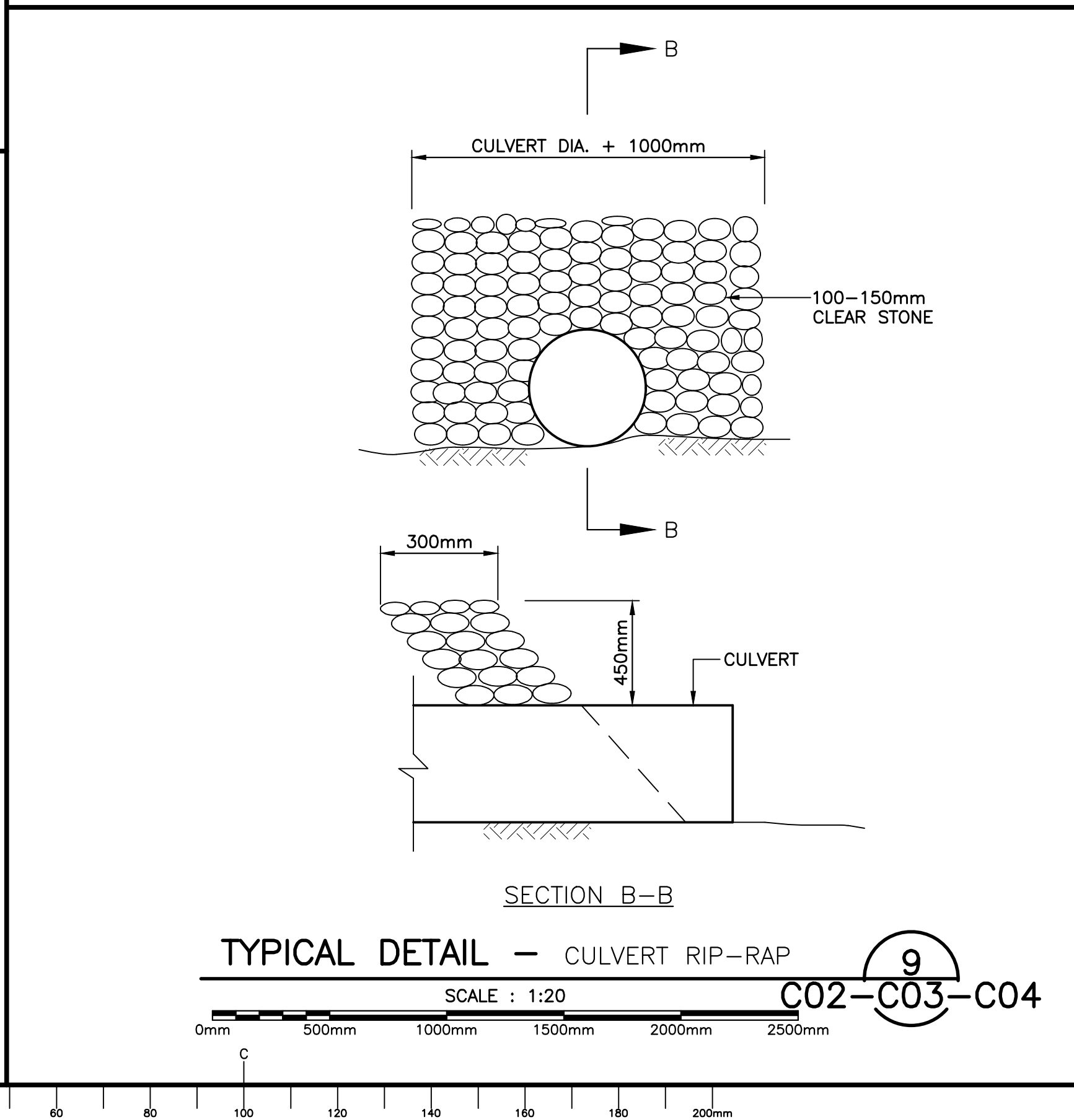
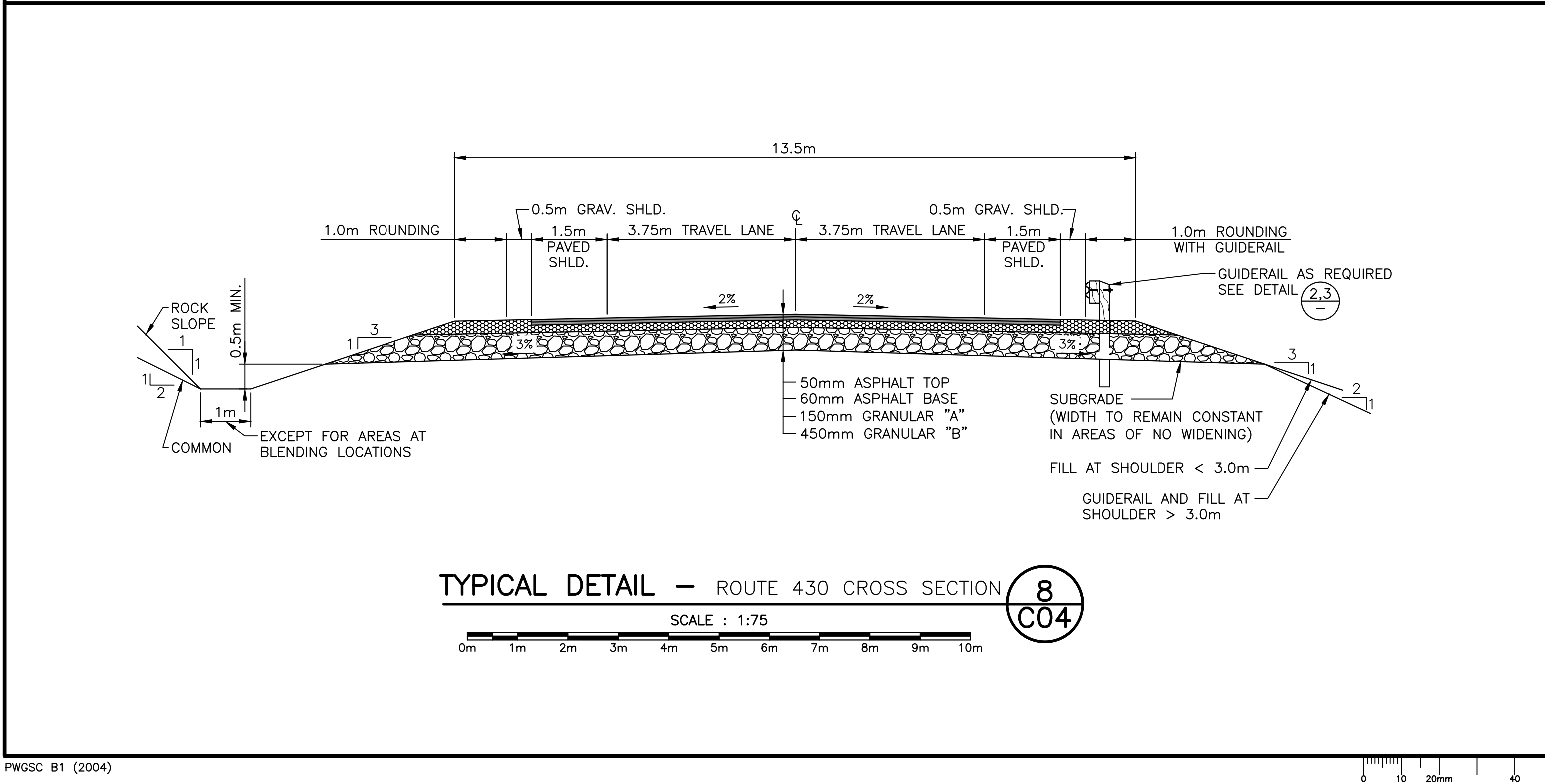
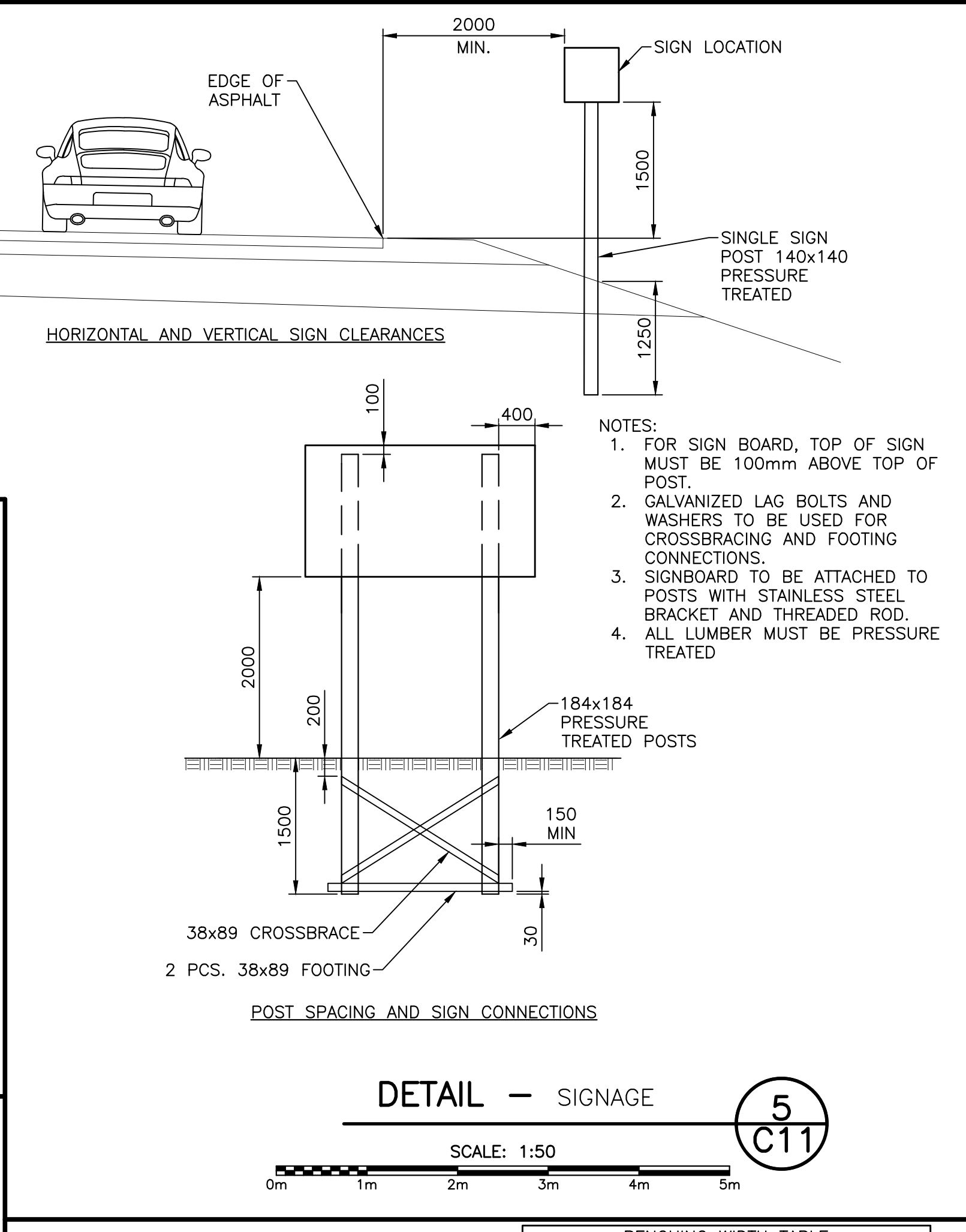
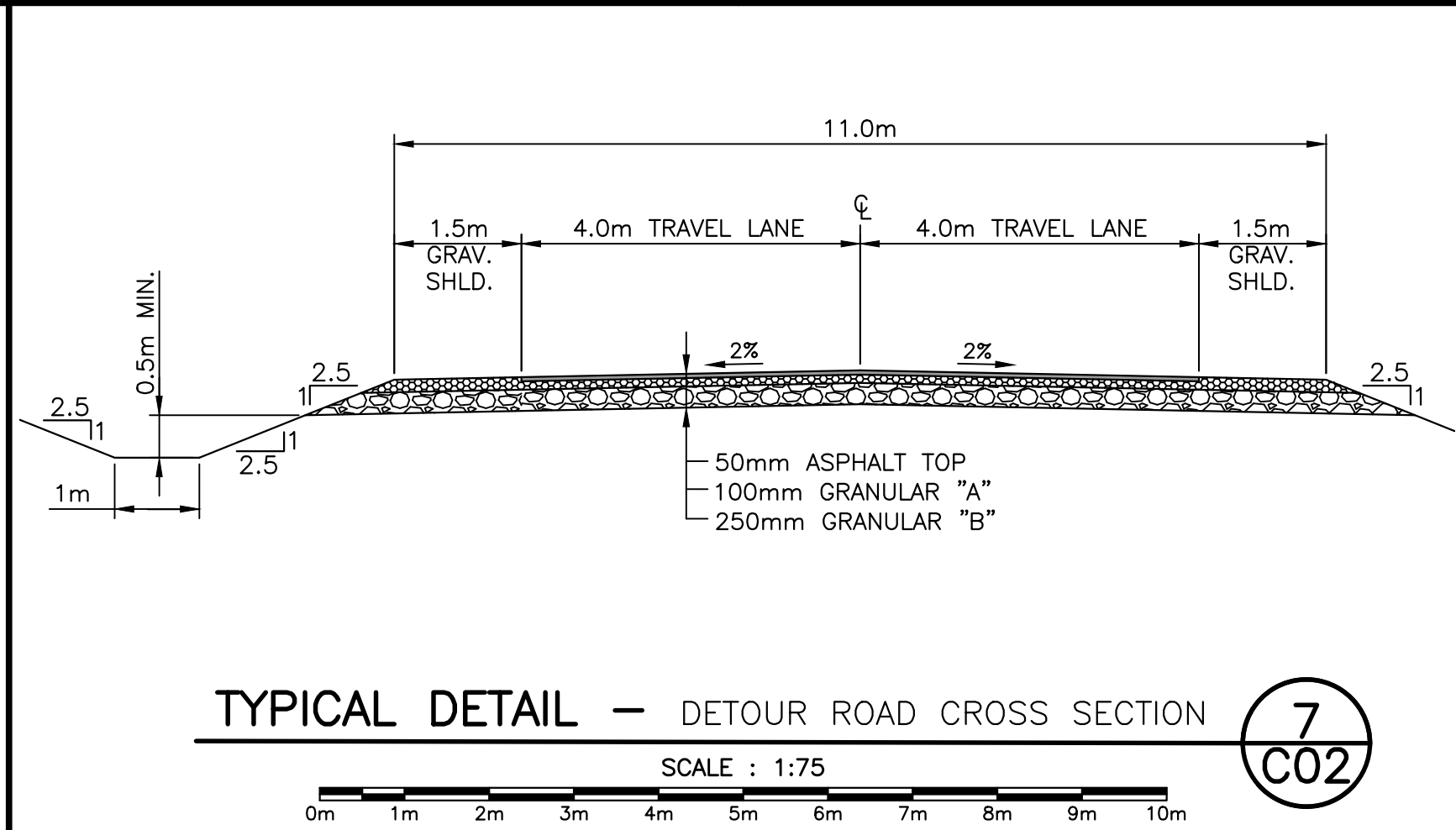
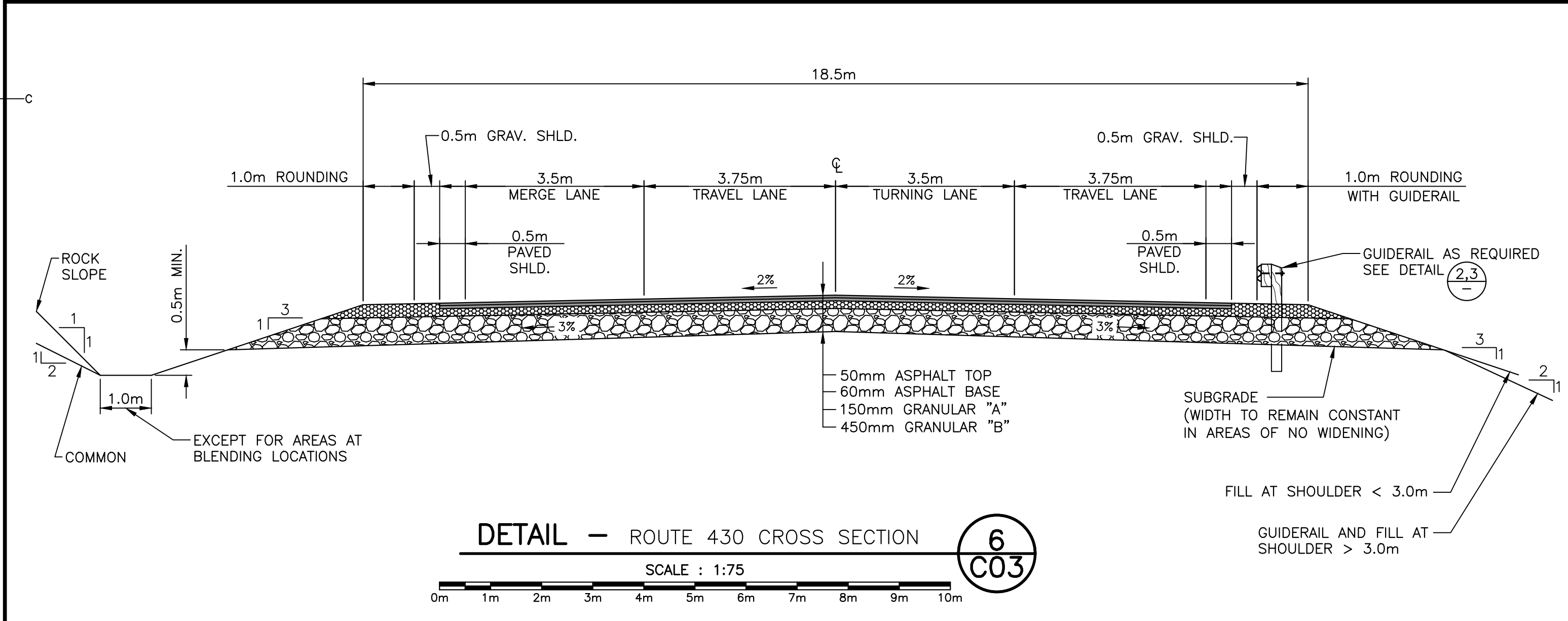
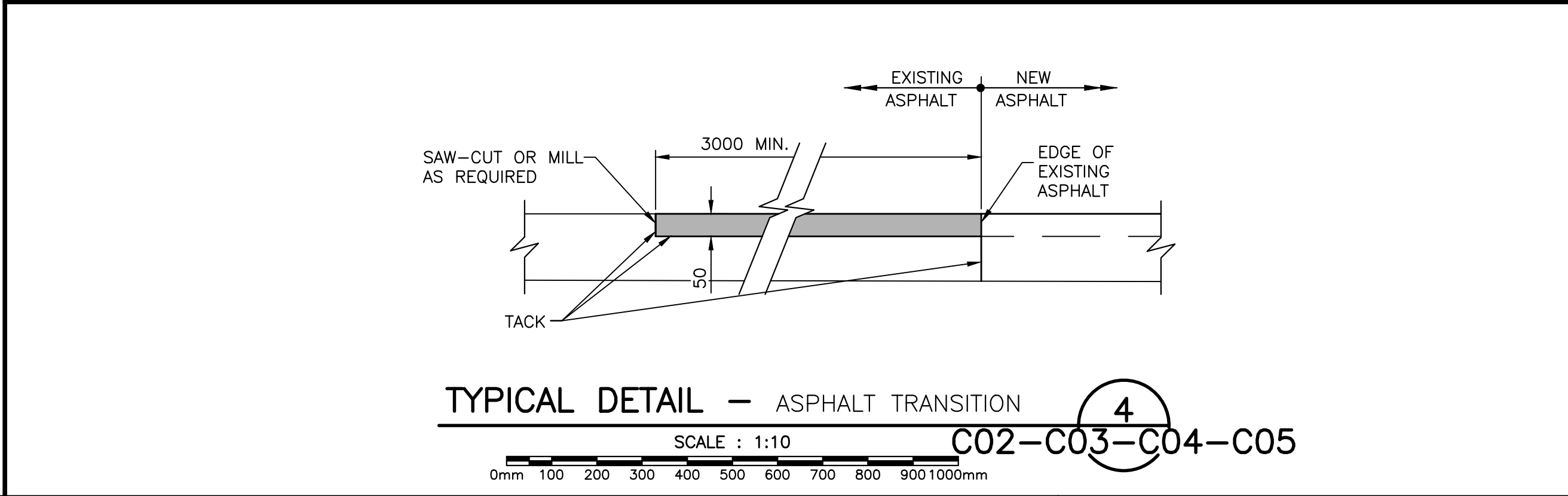
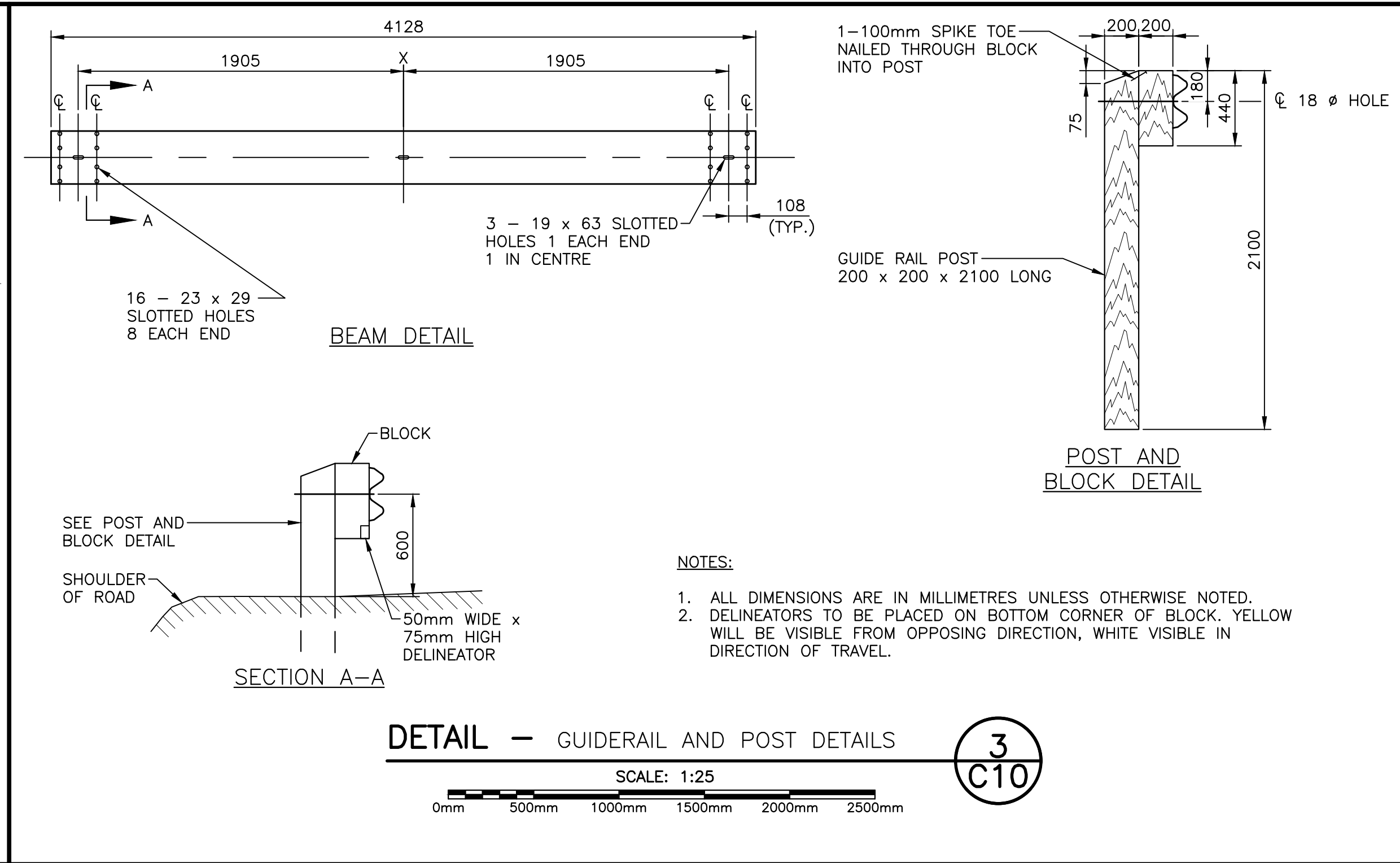
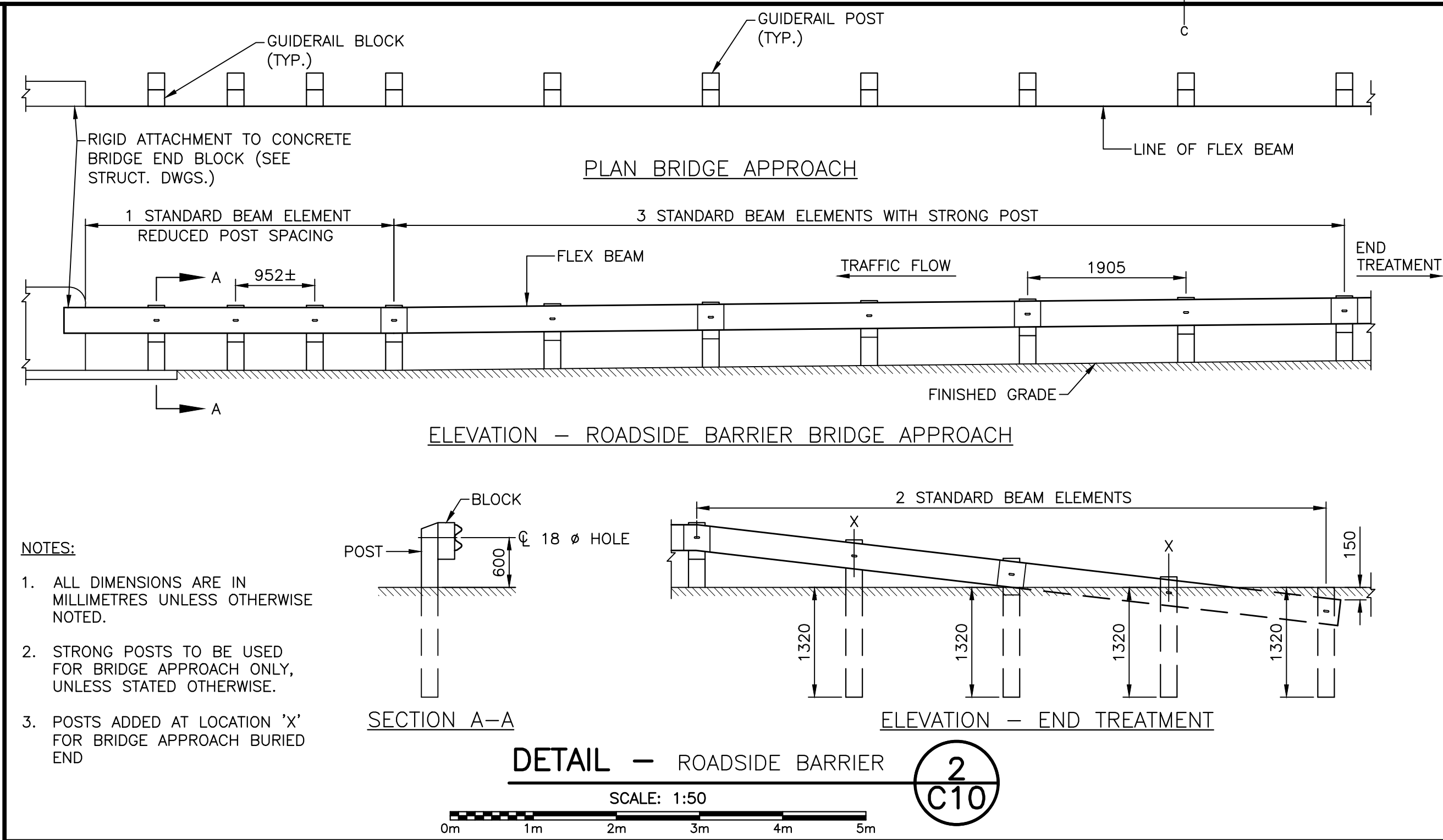
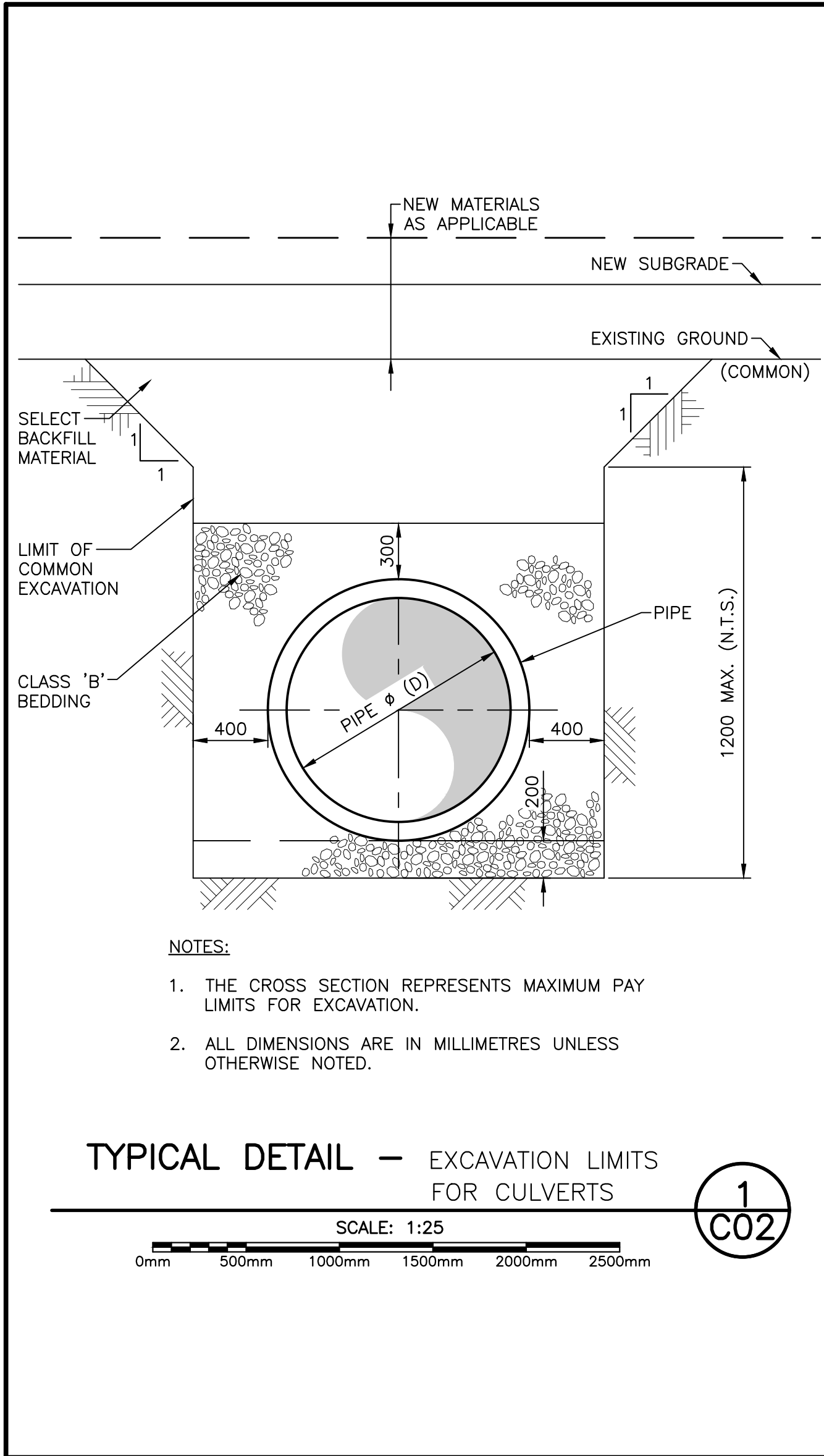
**ROUTE 430 SECTIONS
(0+240.00 TO 0+571.94)
(SHEET 2 OF 2)**

designed	MICHAEL MACDONALD	conçu
date	MARCH 2019	
drawn	CORY BAKER	dessiné
date	MARCH 2019	
approved	MICHAEL MACDONALD	approuvé
date	MARCH 2019	
Tender		Soumission

PCA Project Manager Administrateur de projets APC
project number no. du projet
182009
drawing no. no. du dessin
C08



HORIZONTAL SCALE: 1:250
VERTICAL SCALE: 1:125



Parcs Canada

HARBOURSIDE
Engineering Consultants

HARBOURSIDE
Transportation Consultants

GENERAL NOTES:

1. FOR GENERAL NOTES SEE DRAWING C1.

PROVINCE OF NEWFOUNDLAND

PERMIT HOLDER

CLASS "A"

This Permit Allows
Harbourside Transportation Consultants
to practice Professional Engineering
in Newfoundland and Labrador.
Permit No. as issued by PEGNL N0763
which is valid for the year 2021.

PROFESSIONAL ENGINEER

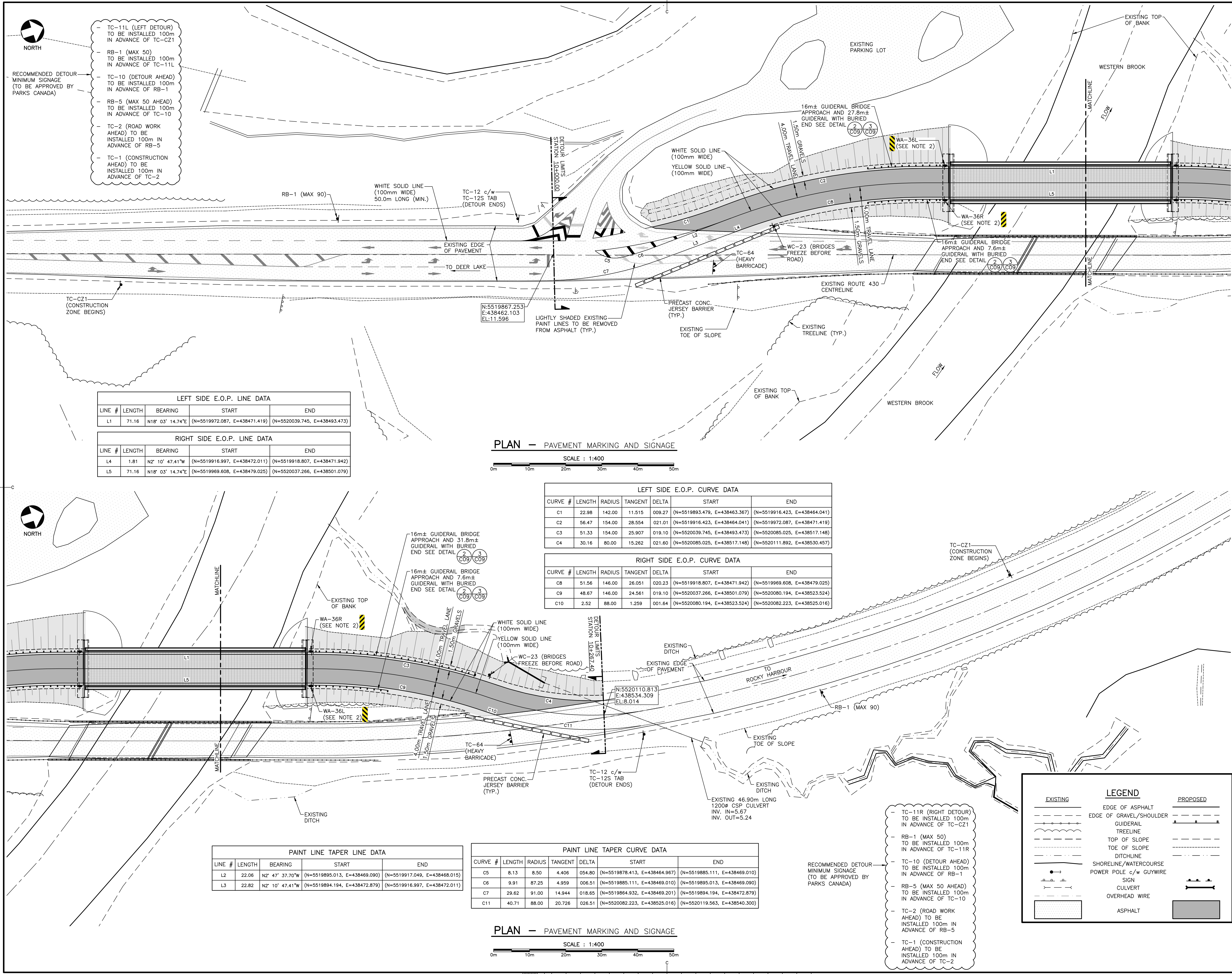
MICHAEL S. MACDONALD

SIGNATURE

July 14/21

DATE

0	ISSUED FOR TENDER	JULY 14 2021
revisions		date
project	WESTERN BROOK BRIDGE REPLACEMENT	project
drawing	GROS MORNE NATIONAL PARK	design
MISCELLANEOUS SECTIONS AND DETAILS		
designed	MICHAEL MACDONALD	conçu
date	MARCH 2019	
drawn	CORY BAKER	dessiné
date	MARCH 2019	
approved	MICHAEL MACDONALD	approuvé
date	MARCH 2019	
Tender		Soumission
PCA Project Manager	Administrateur de projets APC	
project number	182009	no. du projet
drawing no.	C09	no. du dessin



Parcs Canada Parks Canada

HARBORSIDE Engineering Consultants

HARBORSIDE Transportation Consultants

GENERAL NOTES:

- CONTRACTOR MUST PREPARE TRAFFIC CONTROL PLANS, PARKS CANADA APPROVAL OF THESE PLANS IS REQUIRED BEFORE WORK CAN PROCEED.
- NEW SIGNS (WA-36L AND WA-36R) FASTENED TO NEAREST APPROACH GUIDERAIL POST USING 150mm X 150mm POST.

PROVINCE OF NEWFOUNDLAND PERMIT HOLDER CLASS "A"

This Permit Allows Harbourside Transportation Consultants to practice Professional Engineering in Newfoundland and Labrador. Permit No. as issued by PEGNL 00763 which is valid for the year 2021.

WESTERN BROOK BRIDGE REPLACEMENT

GROS MORNE NATIONAL PARK

PAVEMENT MARKING AND SIGNAGE PLAN (TEMPORARY DETOUR)

designed MICHAEL MACDONALD conçu
date MARCH 2019
drawn CORY BAKER dessiné
date MARCH 2019
approved MICHAEL MACDONALD approuvé
date MARCH 2019
Tender Soumission
PCA Project Manager Administrateur de projets APC
project number no. du projet
drawing no. no. du dessin

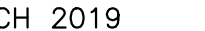
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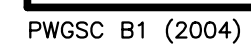
C10

- | | | |
|---|-------------------|-----------------|
| 0 | ISSUED FOR TENDER | JULY 14
2021 |
|---|-------------------|-----------------|

GROS MORNE
NATIONAL PARK

PAVEMENT MARKING
AND SIGNAGE PLAN
(ROUTE 430)

designed	MICHAEL MACDONALD	conçu
drawn	MARCH 2019	
date	CORY BAKER	dessiné
date	MARCH 2019	
approved	MICHAEL MACDONALD	approuvé
date	MARCH 2019	
sender		Sourmission
PCA Project Manager	Administrateur de projets APC	
project number	182009	no. du projet
drawing no.	C11	no. du dessin





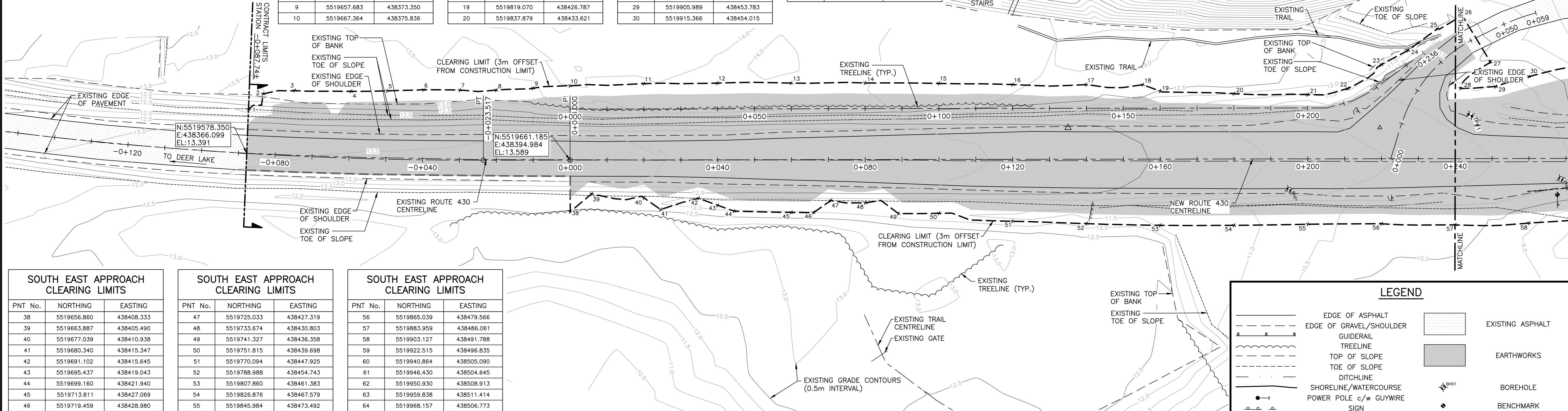
NORTH

SOUTH WEST APPROACH CLEARING LIMITS		
PNT No.	NORTHING	EASTING
1	5519582.540	438354.871
2	5519587.528	438352.130
3	5519596.011	438353.920
4	5519610.480	438358.865
5	5519619.876	438361.775
6	5519629.310	438364.551
7	5519638.616	438367.689
8	5519648.018	438370.738
9	5519657.683	438373.350
10	5519667.364	438375.836

SOUTH WEST APPROACH CLEARING LIMITS		
PNT No.	NORTHING	EASTING
11	5519686.589	438381.464
12	5519705.591	438387.511
13	5519724.666	438393.718
14	5519743.693	438399.881
15	5519762.671	438406.194
16	5519781.590	438412.690
17	5519800.602	438418.898
18	5519815.187	438423.669
19	5519819.070	438426.787
20	5519837.879	438433.621

SOUTH WEST APPROACH CLEARING LIMITS		
PNT No.	NORTHING	EASTING
21	5519856.812	438440.075
22	5519867.618	438441.865
23	5519878.331	438438.546
24	5519886.403	438437.090
25	5519895.869	438433.959
26	5519903.314	438432.620
27	5519906.704	438447.049
28	5519896.757	438451.290
29	5519905.989	438453.783
30	5519915.366	438454.015

SOUTH WEST APPROACH CLEARING LIMITS		
PNT No.	NORTHING	EASTING
31	5519935.834	438455.728
32	5519956.873	438455.684
33	5519967.778	438456.917
34	5519977.004	438458.437
35	5519996.311	438463.736
36	5520002.216	438470.105
37	5520000.603	438476.850



PLAN - CLEARING LIMITS

SCALE : 1:500

SOUTH EAST APPROACH CLEARING LIMITS		
PNT No.	NORTHING	EASTING
38	5519656.860	438408.333
39	5519663.887	438405.490
40	5519677.039	438410.938
41	5519680.340	438415.347
42	5519691.102	438415.645
43	5519695.437	438419.043
44	5519699.160	438421.940
45	5519713.811	438427.069
46	5519719.459	438428.980

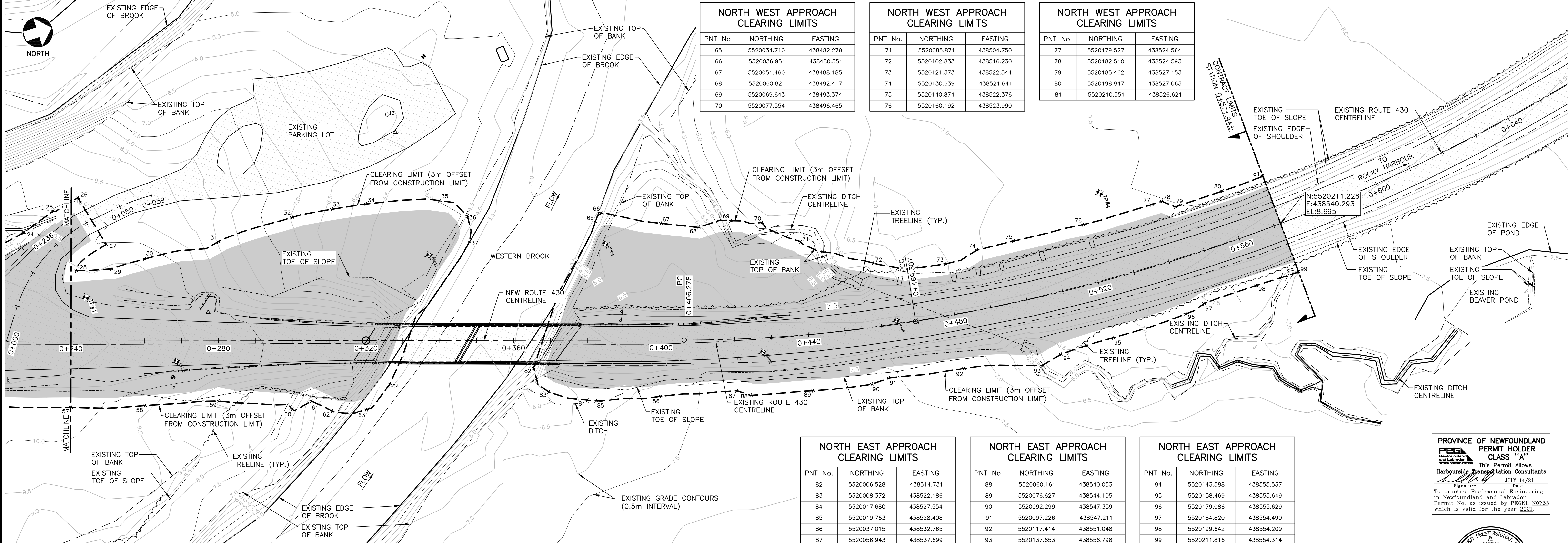
SOUTH EAST APPROACH CLEARING LIMITS		
PNT No.	NORTHING	EASTING
47	5519725.033	438427.319
48	5519733.674	438430.803
49	5519741.327	438436.358
50	5519751.815	438439.698
51	5519770.094	438447.925
52	5519788.988	438454.743
53	5519807.860	438461.383
54	5519826.876	438467.579
55	5519845.984	438473.492

SOUTH EAST APPROACH CLEARING LIMITS		
PNT No.	NORTHING	EASTING
56	5519865.039	438479.566
57	5519883.959	438486.061
58	5519903.127	438491.788
59	5519922.515	438496.835
60	5519940.864	438505.090
61	5519946.430	438504.645
62	5519950.930	438508.913
63	5519959.838	438511.414
64	5519968.157	438506.773

NORTH WEST APPROACH CLEARING LIMITS		
PNT No.	NORTHING	EASTING
65	5520034.710	438482.279
66	5520036.951	438480.551
67	5520051.460	438488.185
68	5520060.821	438492.417
69	5520069.643	438493.374
70	5520077.554	438496.465

NORTH WEST APPROACH CLEARING LIMITS		
PNT No.	NORTHING	EASTING
71	5520085.871	438504.750
72	5520102.833	438516.230
73	5520121.373	438522.544
74	5520130.639	438521.641
75	5520140.874	438522.376
76	5520160.192	438523.990

NORTH WEST APPROACH CLEARING LIMITS		
PNT No.	NORTHING	EASTING
77	5520179.527	438524.564
78	5520182.510	438524.593
79	5520185.462	438527.153
80	5520198.947	438527.063
81	5520210.551	438526.621



PLAN - CLEARING LIMITS

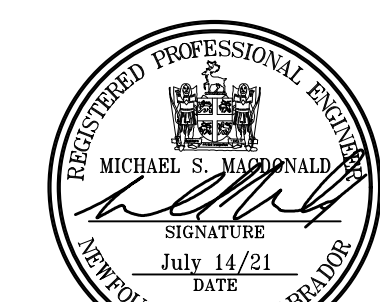
SCALE : 1:500

NORTH EAST APPROACH CLEARING LIMITS		
PNT No.	NORTHING	EASTING
82	5520006.528	438514.731
83	5520008.372	438522.186
84	5520017.680	438527.554
85	5520019.763	438528.408
86	5520037.015	438532.765
87	5520056.943	438537.699

NORTH EAST APPROACH CLEARING LIMITS		
PNT No.	NORTHING	EASTING
88	5520060.161	438540.053
89	5520076.627	438544.105
90	5520092.299	438547.359
91	5520097.226	438547.211
92	5520117.414	438551.048
93	5520137.653	438556.798

NORTH EAST APPROACH CLEARING LIMITS		
PNT No.	NORTHING	EASTING
94	5520143.588	438555.537
95	5520158.469	438555.649
96	5520179.086	438555.629
97	5520184.820	438554.490
98	5520199.642	438554.209
99	5520211.816	438554.314

PROVINCE OF NEWFOUNDLAND
PERMIT HOLDER
CLASS "A"
This Permit Allows
Harbourside Engineering Consultants
To practice Professional Engineering
in Newfoundland and Labrador.
Permit No. as issued by PEGNL 02763
which is valid for the year 2021.



GENERAL NOTES:

- ALL ELEVATIONS ARE IN METRES UNLESS OTHERWISE NOTED.
 - SURVEY REFERENCED HORIZONTALLY TO UTM ZONE 21 NORTH NAD83 (CSRS) 2010.0 AND VERTICALLY TO CGVD28 (HTV2.0 GEOD MODEL).
 - CONTROL IS DERIVED FROM STATIC GNSS OBSERVATIONS ON A NAIL (POINT #1000) HAVING COORDINATES OF:
N=5483441.253m
E=442956.271m
EL=20.152m
 - RAW GNSS DATA PROCESSED USING NATURAL RESOURCES CANADA PRECISE POINT POSITIONING SOFTWARE.
 - FIELD SURVEY WAS CARRIED OUT BETWEEN MARCH 6-9TH, 2018 AND JULY 25-28TH, 2018.
 - COORDINATES ARE GRID. APPLY COMBINED SCALE FACTOR OF 1.0003631 TO CALCULATE GROUND DISTANCES.
 - CONTOUR INTERVAL IS 0.5 METRE.
- HORIZONTAL ALIGNMENT CONTROL POINTS CONSIST OF:
PT - POINT OF TANGENCY
PC - POINT OF CURVATURE
PRC - POINT OF REVERSE CURVATURE
PCC - POINT OF COMPOUND CURVATURE
- LINE DATA:
AZ - AZIMUTH
L - LENGTH
- CURVE DATA:
D - DELTA
L - LENGTH
R - RADIUS

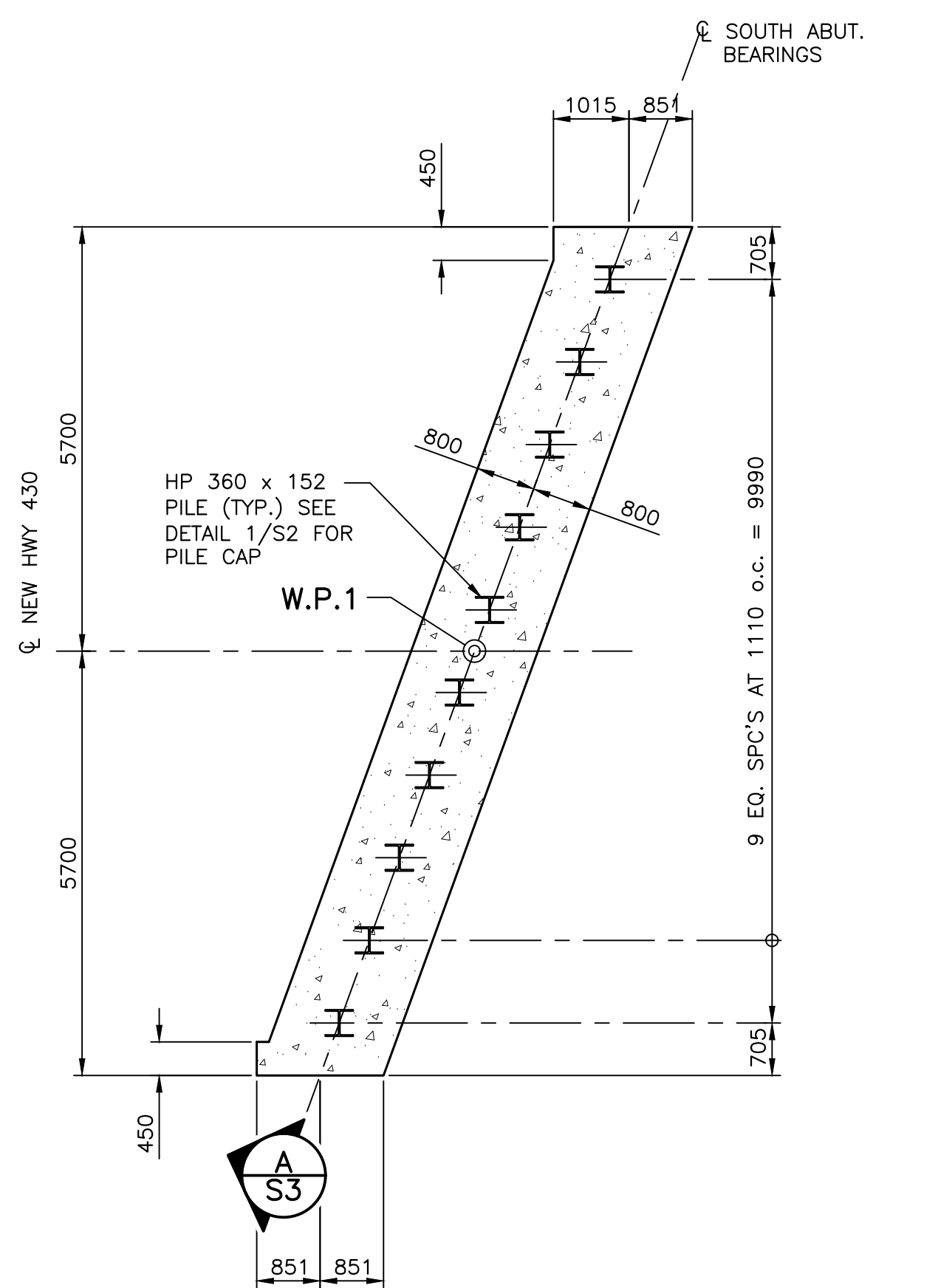
0	ISSUED FOR TENDER	JULY 14 2021
revisions		date

WESTERN BROOK
BRIDGE REPLACEMENT

GROS MORNE
NATIONAL PARK

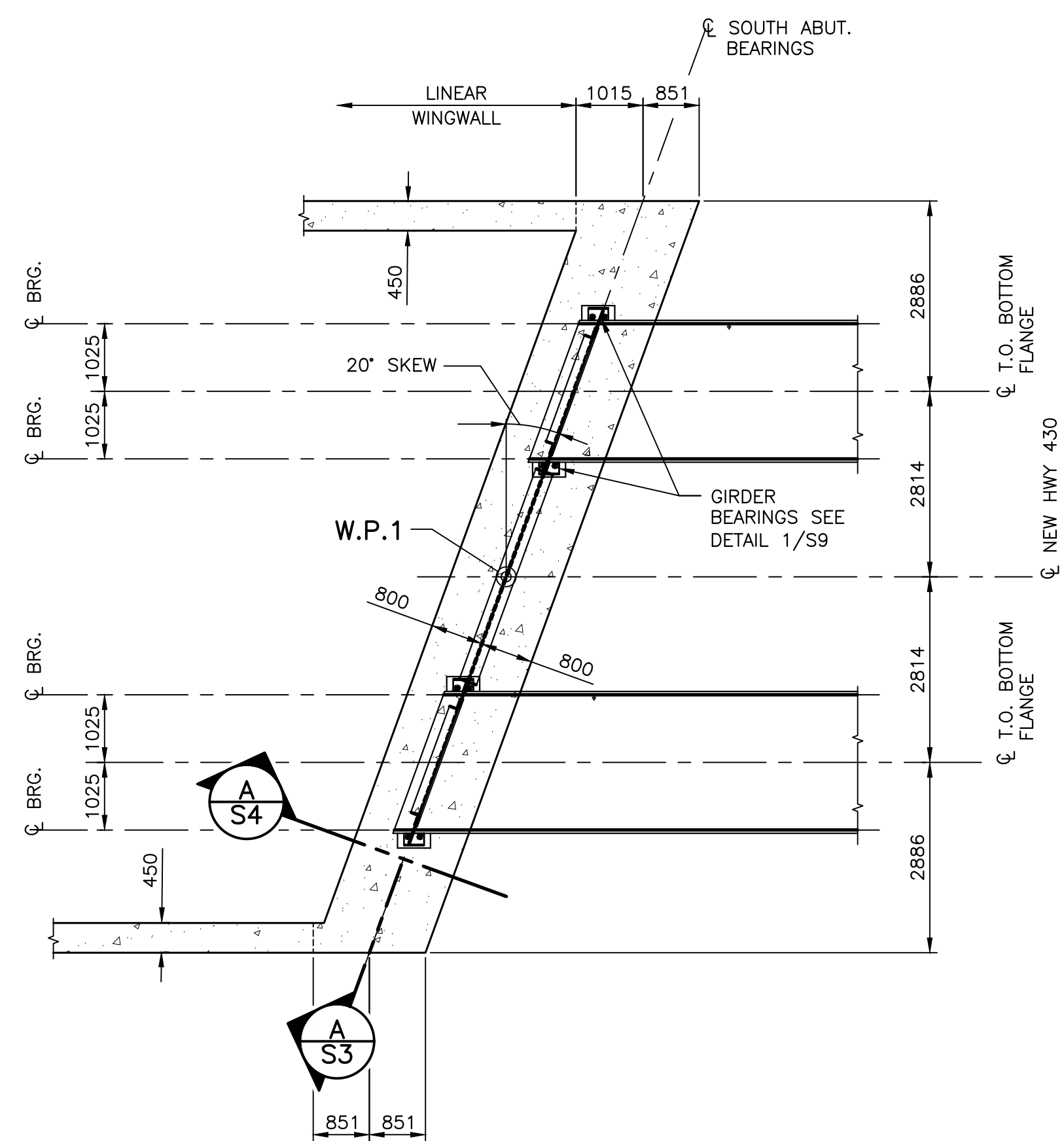
PROPOSED
CLEARING PLAN

designed	MICHAEL MACDONALD	conçu
date	MARCH 2019	
drawn	CORY BAKER	dessiné
date	MARCH 2019	
approved	MICHAEL MACDONALD	approuvé
date	MARCH 2019	
Tender		Soumission
PCA Project Manager	Administrateur de projets APC	
project number	182009	no. du projet
drawing no.	C12	no. du dessin



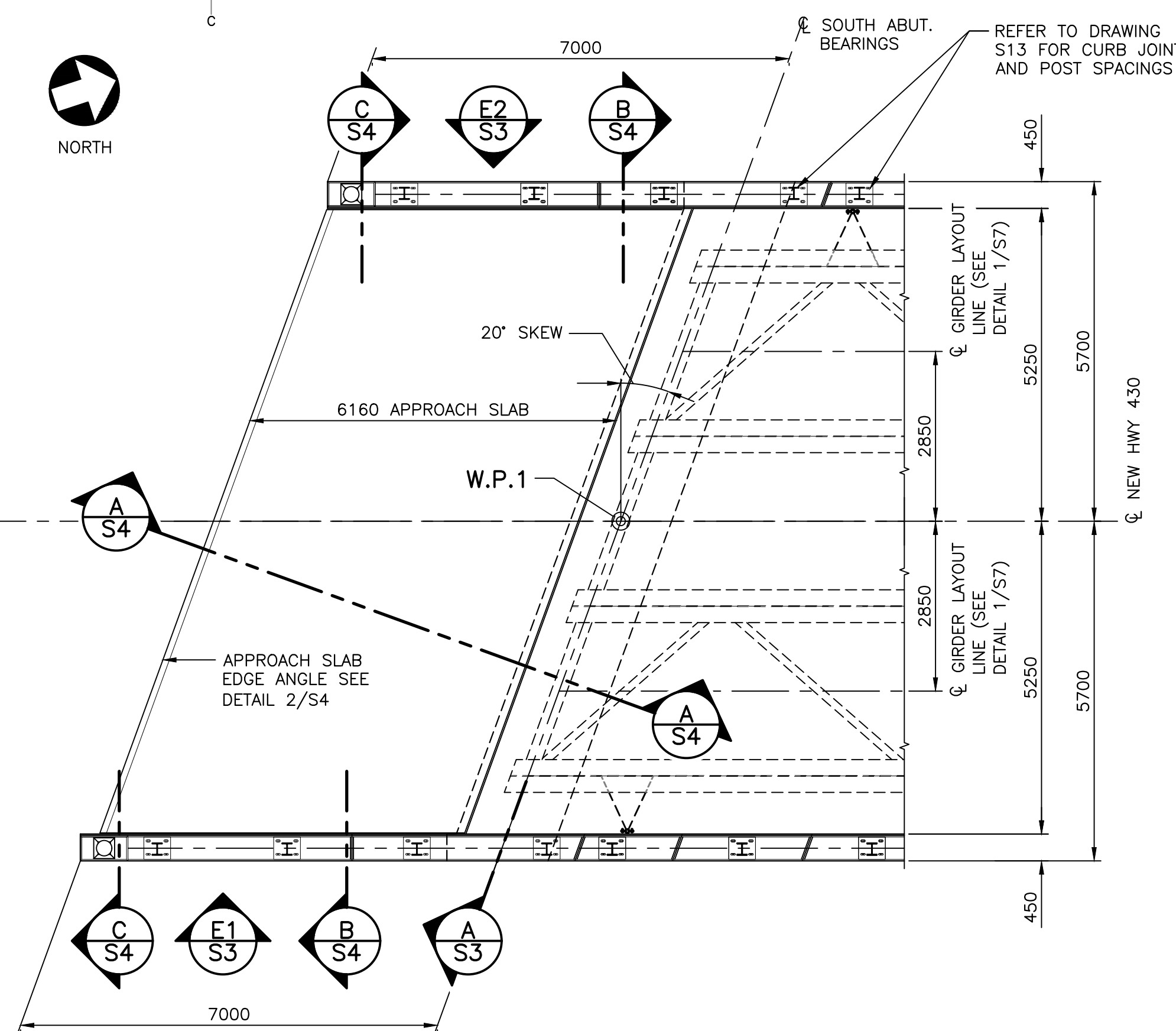
SOUTH ABUTMENT – PILE LAYOUT PLAN

SCALE : 1:75



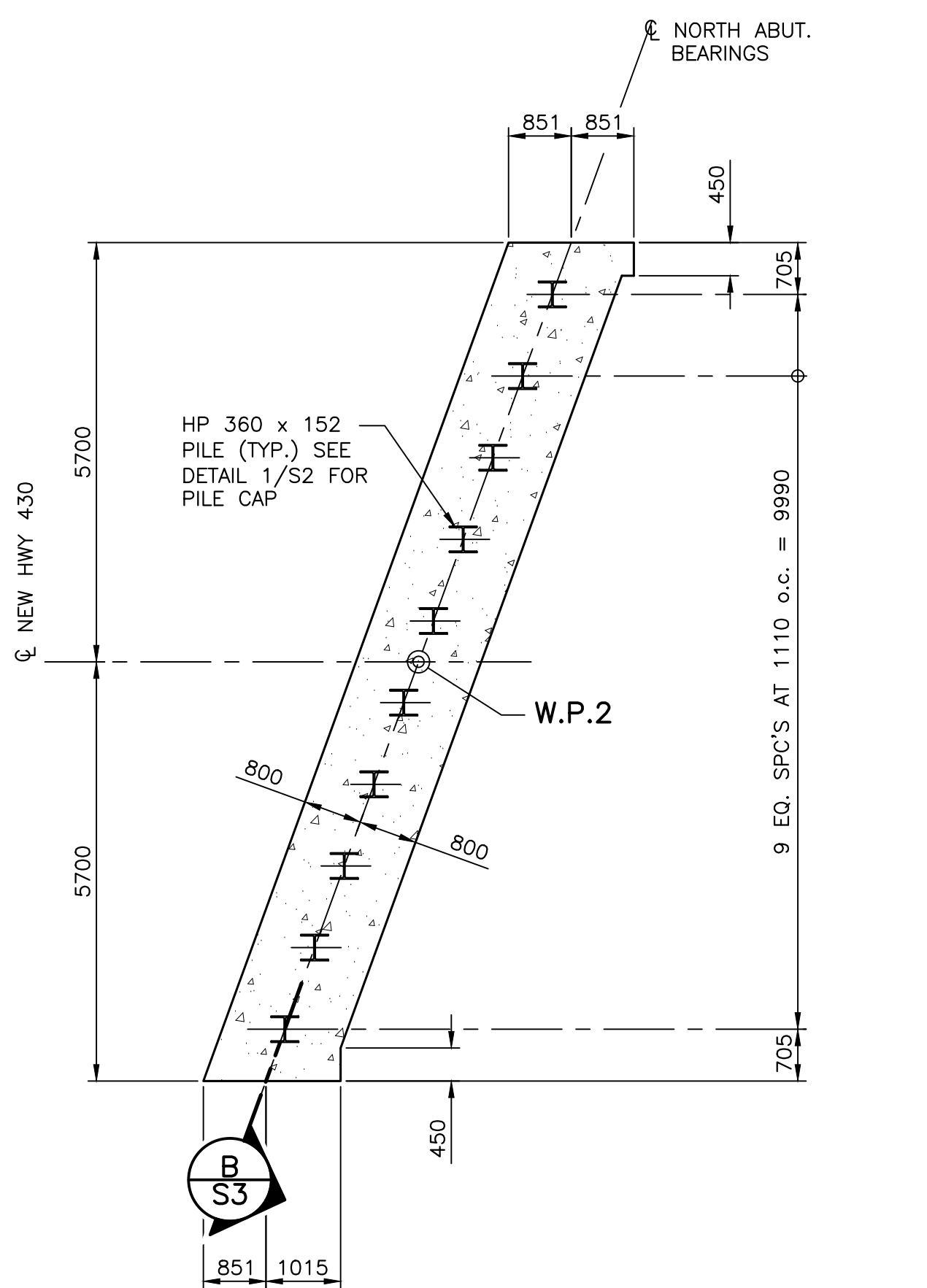
SOUTH ABUTMENT – BEAM SEAT PLAN

SCALE : 1:75



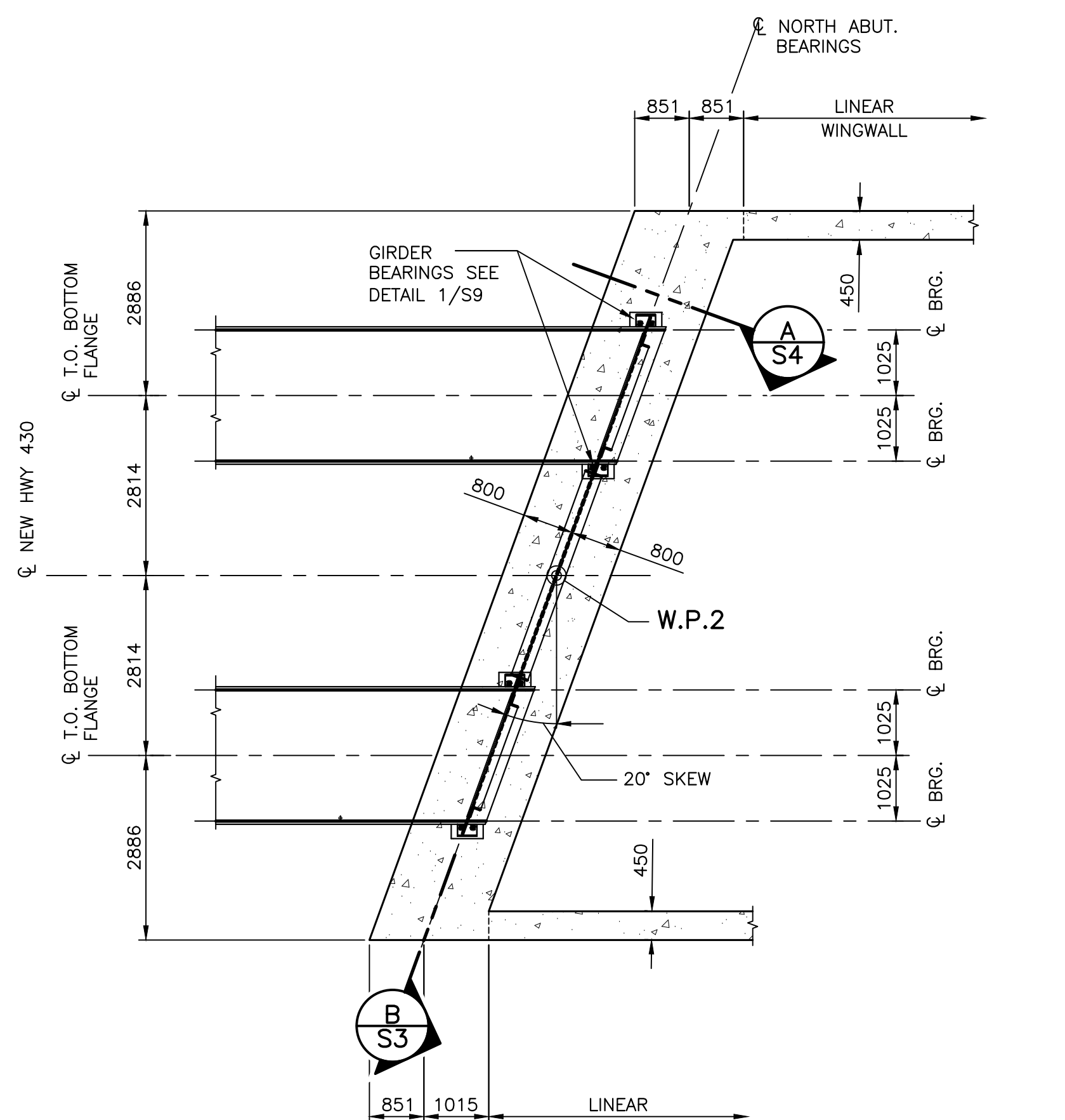
SOUTH ABUTMENT – TOP PLAN

SCALE : 1:75



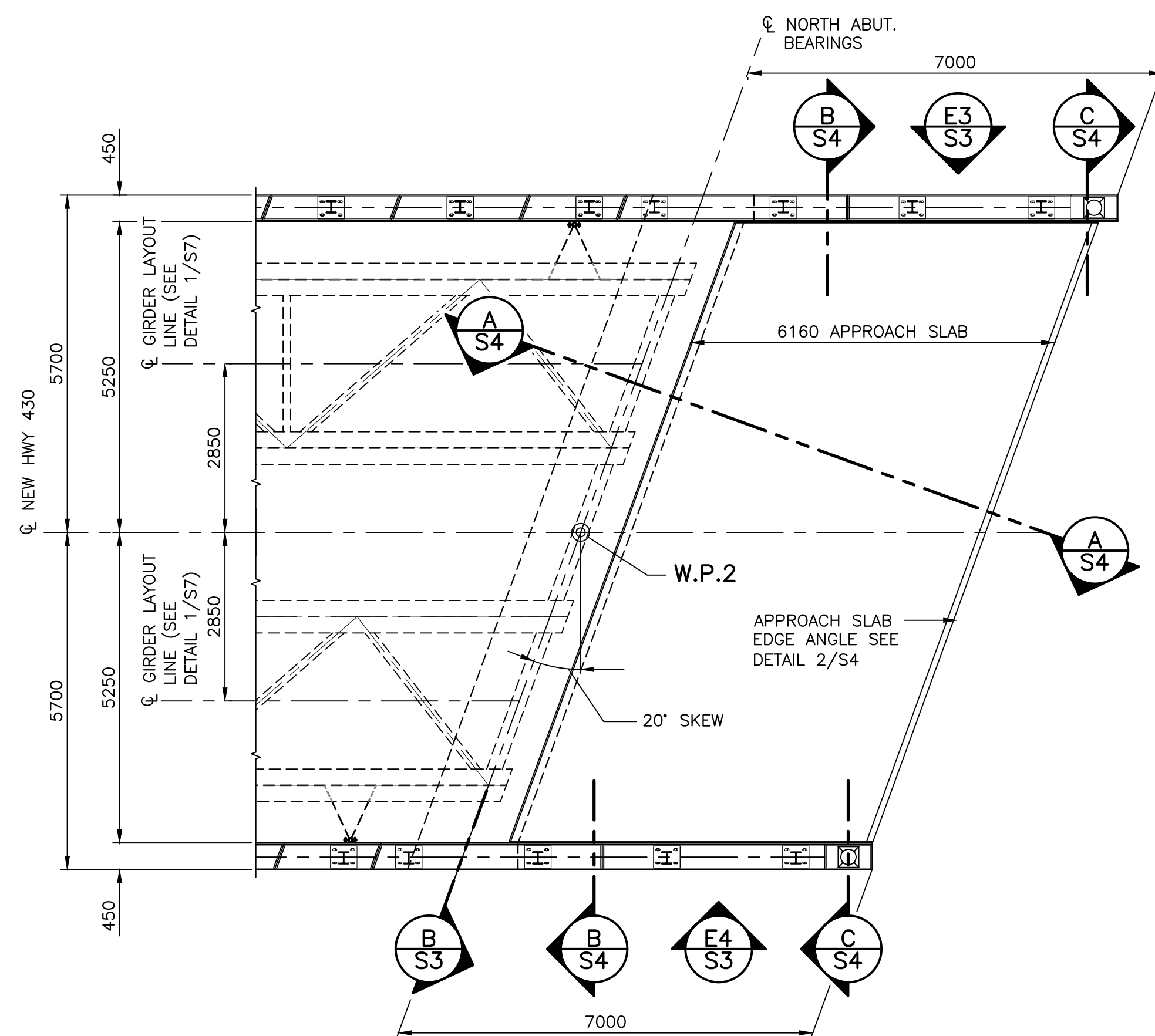
NORTH ABUTMENT – PILE LAYOUT PLAN

SCALE : 1:75



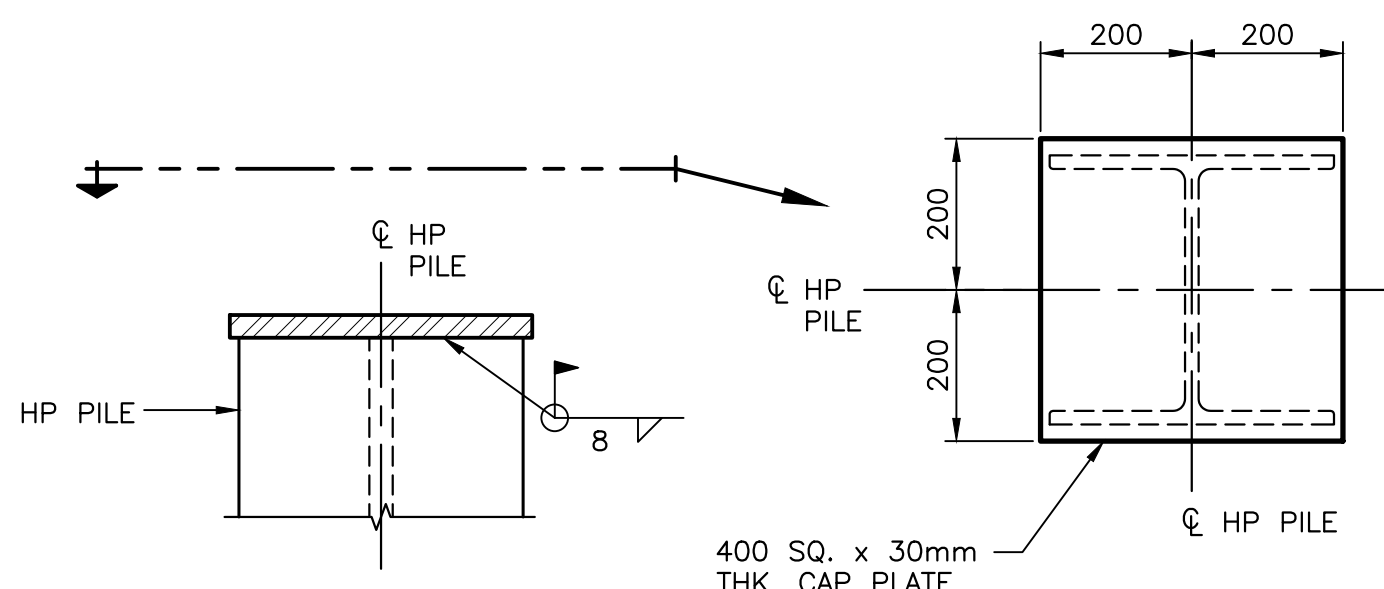
NORTH ABUTMENT – BEAM SEAT PLAN

SCALE : 1:75



NORTH ABUTMENT – TOP PLAN

SCALE : 1:75



DETAIL – PILE CAP

SCALE : 1:10

1
S2

C.I.P. CONCRETE NOTES

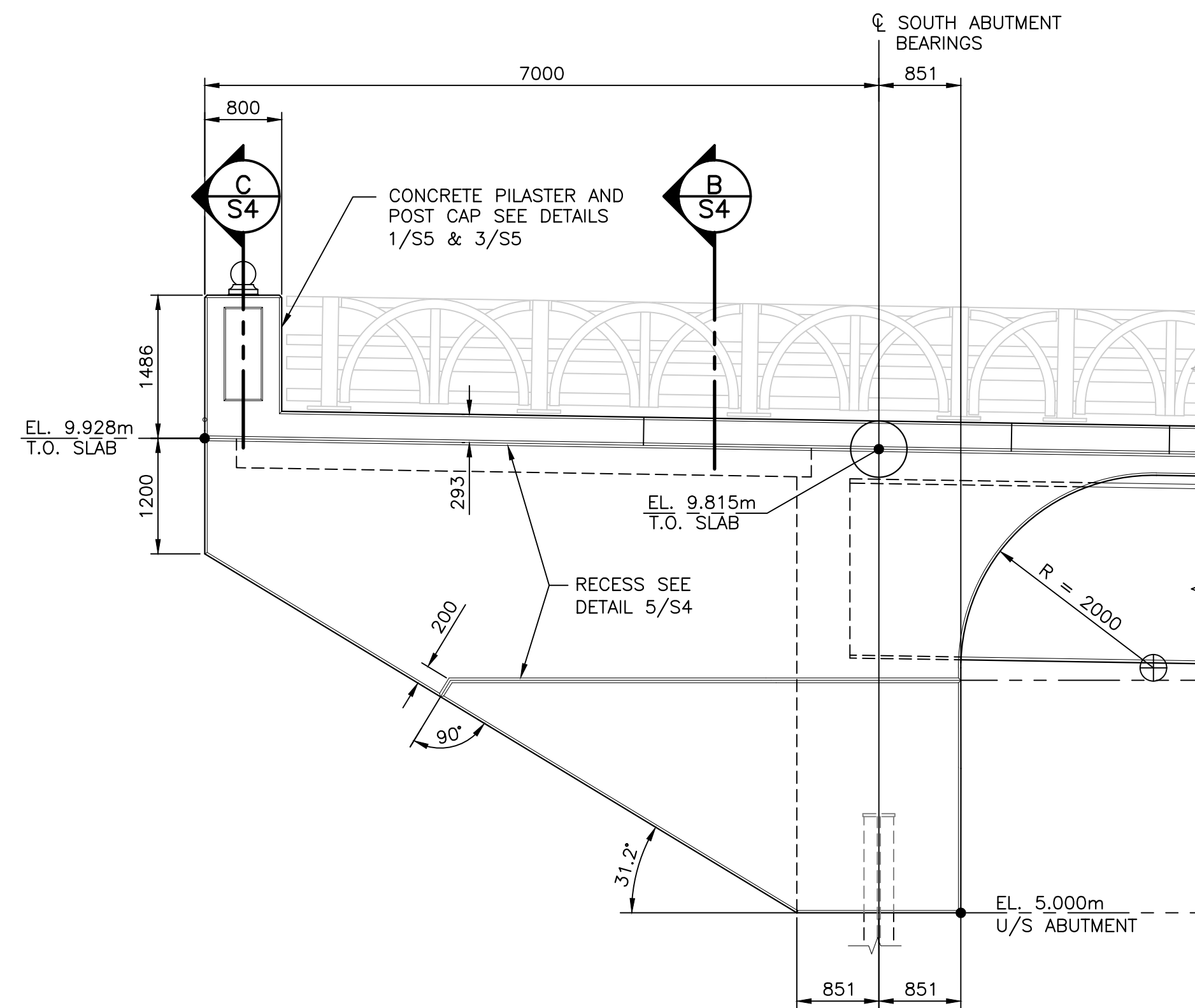
- ALL EXPOSED CORNERS OF CONCRETE TO HAVE 25mm CHAMFERS.
- LOCATION OF CONSTRUCTION JOINTS AND SEQUENCE OF CONCRETE PLACEMENT TO BE APPROVED BY THE DEPARTMENTAL REPRESENTATIVE.
- CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS:
 - ABUTMENTS, WINGWALLS, APPROACH SLABS, CONCRETE DECK AND CURBS 45 MPa WITH 20mm MAX. AGGREGATE SIZE AND 6% ± 1% AIR ENTRAINMENT (AIR VOID SPACING REQUIREMENTS AS PER PROJECT SPECIFICATIONS), MAX. WATER-CEMENT RATIO 0.35
 - BEARING PLINTHS 35 MPa WITH 20mm MAX. AGGREGATE SIZE AND 6% ± 1% AIR ENTRAINMENT (AIR VOID SPACING REQUIREMENTS AS PER PROJECT SPECIFICATIONS), MAX. WATER-CEMENT RATIO 0.35.
- CONCRETE COVER TO REINFORCING STEEL AS NOTED ON DRAWINGS.
- REINFORCING STEEL TO BE GRADE 400W DEFORMED BARS AS PER PROJECT SPECIFICATIONS WITH YIELD STRENGTH OF 400 MPa (WELDABLE). ALL REINFORCING TO BE GALVANIZED IN ACCORDANCE WITH PROJECT SPECIFICATIONS. BEND DIAMETERS PRIOR TO GALVANIZING AS PER PROJECT SPECIFICATIONS, FIELD BENDING OF GALVANIZED BARS IS NOT PERMITTED.
- ALL REINFORCEMENT TO BE INSPECTED BY THE DEPARTMENTAL REPRESENTATIVE PRIOR TO CLOSING FORMWORK OR PLACING CONCRETE.
- COMPACTING IMMEDIATELY ADJACENT TO BACK WALL SHALL BE ACCOMPLISHED WITH LIGHT COMPACTING EQUIPMENT. MODERATE COMPACTING WITH A TRENCH ROLLER IN 300mm LIFTS ELSEWHERE (ALL COMPACTION SHALL BE TO 100% STD. PROCTOR MAXIMUM DRY DENSITY). BACKFILLING SHALL NOT BE UNDERTAKEN UNTIL GIRDERS ARE ERECTED AND SLAB AND ABUTMENT CAPS ARE COMPLETED ($f'c \geq 35$ MPa) AND SHALL BE ACCOMPLISHED IN EQUAL/BALANCED LIFTS BEHIND EACH ABUTMENT. WHEEL LOADS SHALL BE KEPT 5.0m MINIMUM CLEAR OF ABUTMENTS UNTIL CONCRETE REACHES DESIGN STRENGTH AND BACKFILLING IS COMPLETED BEHIND BOTH ABUTMENTS. SURCHARGING FROM CONSTRUCTION EQUIPMENT TO BE AVOIDED UNLESS OTHERWISE APPROVED BY DEPARTMENTAL REPRESENTATIVE IN WRITING.
- FOR BENT REINFORCING BAR TYPES REFER TO R.S.I.C. REINFORCING MANUAL OF STANDARD PRACTICE TYPICAL BAR BENDS EXCEPT BAR BEND DIAMETERS AS PER PROJECT SPECIFICATIONS (U.N.O.).
- EACH PHASE OF WORK TO BE INSPECTED BY THE DEPARTMENT REPRESENTATIVE PRIOR TO PROCEEDING TO THE NEXT PHASE OF WORK.
- BACKFILL IMMEDIATELY BEHIND ABUTMENTS TO BE "FILL AGAINST STRUCTURE" MATERIAL AS PER PROJECT SPECIFICATIONS. LIMITS AS INDICATED ON DRAWING S15.

PRECAST CONCRETE NOTES

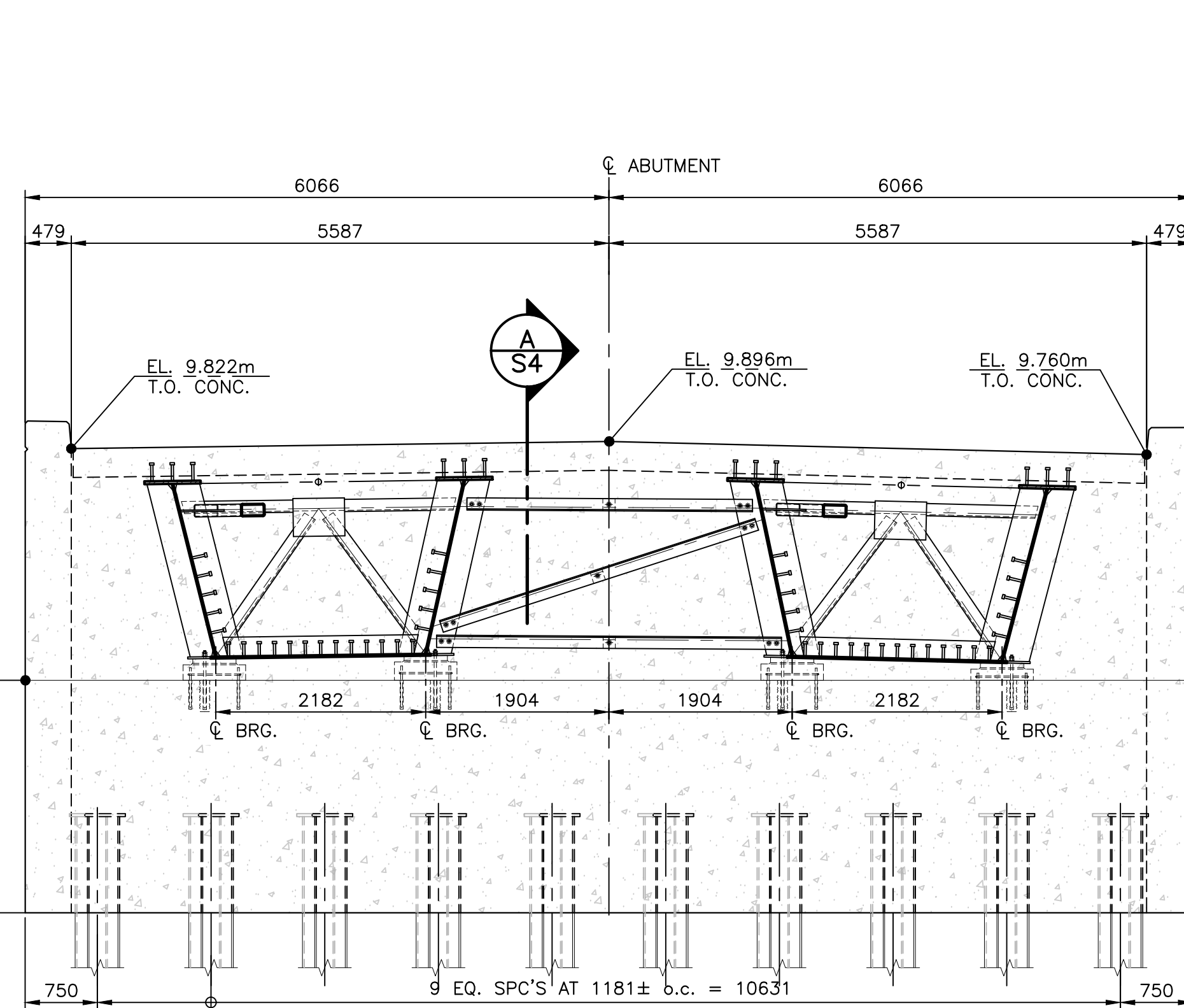
- ALL EXPOSED CORNERS OF CONCRETE TO HAVE 25mm CHAMFERS.
- CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS:
 - ROAD DRAINS: 35MPa, NON-REINFORCED, 6% ± 1% AIR ENTRAINMENT, AS PER PROJECT SPECIFICATIONS.

PILE NOTES

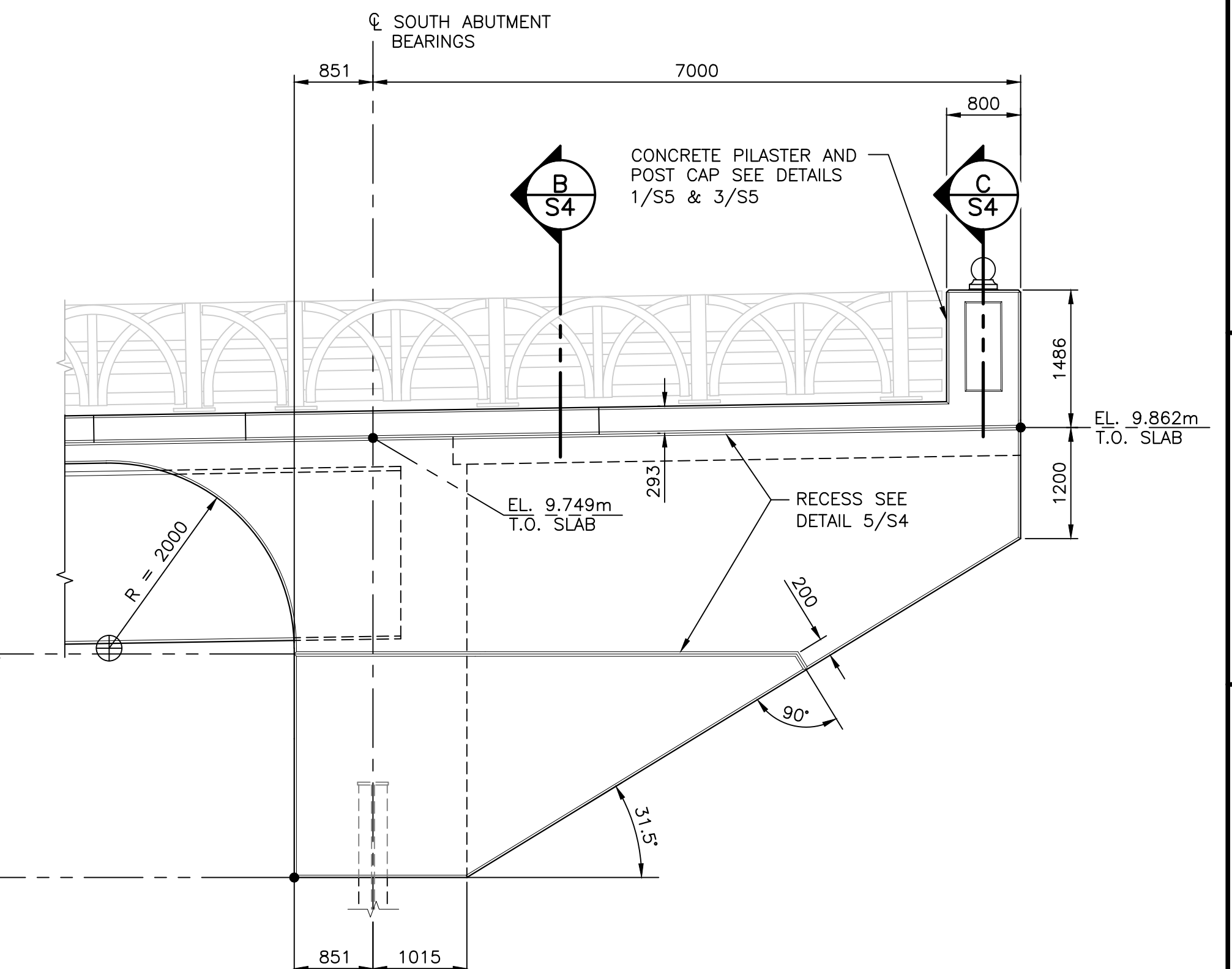
- PILE MATERIAL
 - STEEL H-PILES IN ABUTMENT, HP360 x 152, $F_y = 350$ MPa (MIN.)
 - ALL PILE SPLICES, IF REQUIRED AND AT THE APPROVAL OF THE DEPARTMENTAL REPRESENTATIVE, SHALL BE FULL STRENGTH WELDED CONNECTIONS (LIMIT 1 SPLICE PER PILE)
 - CAP PLATE, $F_y = 350$ MPa MINIMUM
 - WELDING MATERIAL TO CSA G40.1 – LATEST EDITION
 - WELDING TO BE IN ACCORDANCE TO CSA W59 – LATEST EDITION
- PILE INSTALLATION CRITERIA AS PER HGC REPORT No. 193132 R1 DATED MAR. 23, 2020.
- SEE PROJECT SPECIFICATION FOR PILE SET CRITERIA.
- ALL PILES SHALL BE DRIVEN WITH A PROTECTIVE H-PILE DRIVING SHOE. ALL POINTS SHALL MATCH PILE SIZE AND SHALL BE WELDED TO PILE TIPS AS PER MANUFACTURER'S RECOMMENDATIONS. PILE TIP DETAILS SHALL BE FORWARDED TO THE DEPARTMENTAL REPRESENTATIVE FOR REVIEW AND ACCEPTANCE PRIOR TO DRIVING.
- FULL TIME INSPECTION SHALL BE UNDERTAKEN DURING PILE DRIVING AND COMPLETE DRIVING RECORDS SHALL BE KEPT BY CONTRACTOR.
- PILE CAPACITIES OF AT LEAST TWO PILES PER ABUTMENT SHALL BE CONFIRMED BY PDA TESTING WITH ADDITIONAL PDA TESTING TO BE COMPLETED AT THE DISCRETION OF THE DEPARTMENTAL REPRESENTATIVE.
- DESIGN PILE CAPACITY AT ULS:
 - HP360 x 152 PILES ----- 1360 kN (COMPRESSION)
- MINIMUM RATED HAMMER ENERGY OF 450 J/cm² OF STEEL CROSS SECTION-SECTIONAL AREA SHALL BE USED. A PILE DRIVING HAMMER CAPABLE OF DELIVERING VARIABLE AMOUNTS OF ENERGY SHALL BE USED ON THIS PROJECT.
- PRACTICAL REFUSAL TAKEN AS PILE PENETRATION OF LESS THAN 25mm FOR 15 BLOWS AT THE RATED ENERGY FOR 4 CONSECUTIVE 25mm INCREMENTS.
- RE-STRIKING OF 2 PILES PER ABUTMENT SHALL BE UNDERTAKEN NO SOONER THAN 24 HOURS AFTER ACHIEVING THE REFUSAL CRITERIA AND SUFFICIENTLY DRIVEN TO RE-ESTABLISH THE REFUSAL CRITERIA AS PER GEOTECHNICAL ENGINEER RECOMMENDATIONS (REFERENCE HARBOURSIDE GEOTECHNICAL CONSULTANTS PROJECT GEOTECHNICAL REPORT).



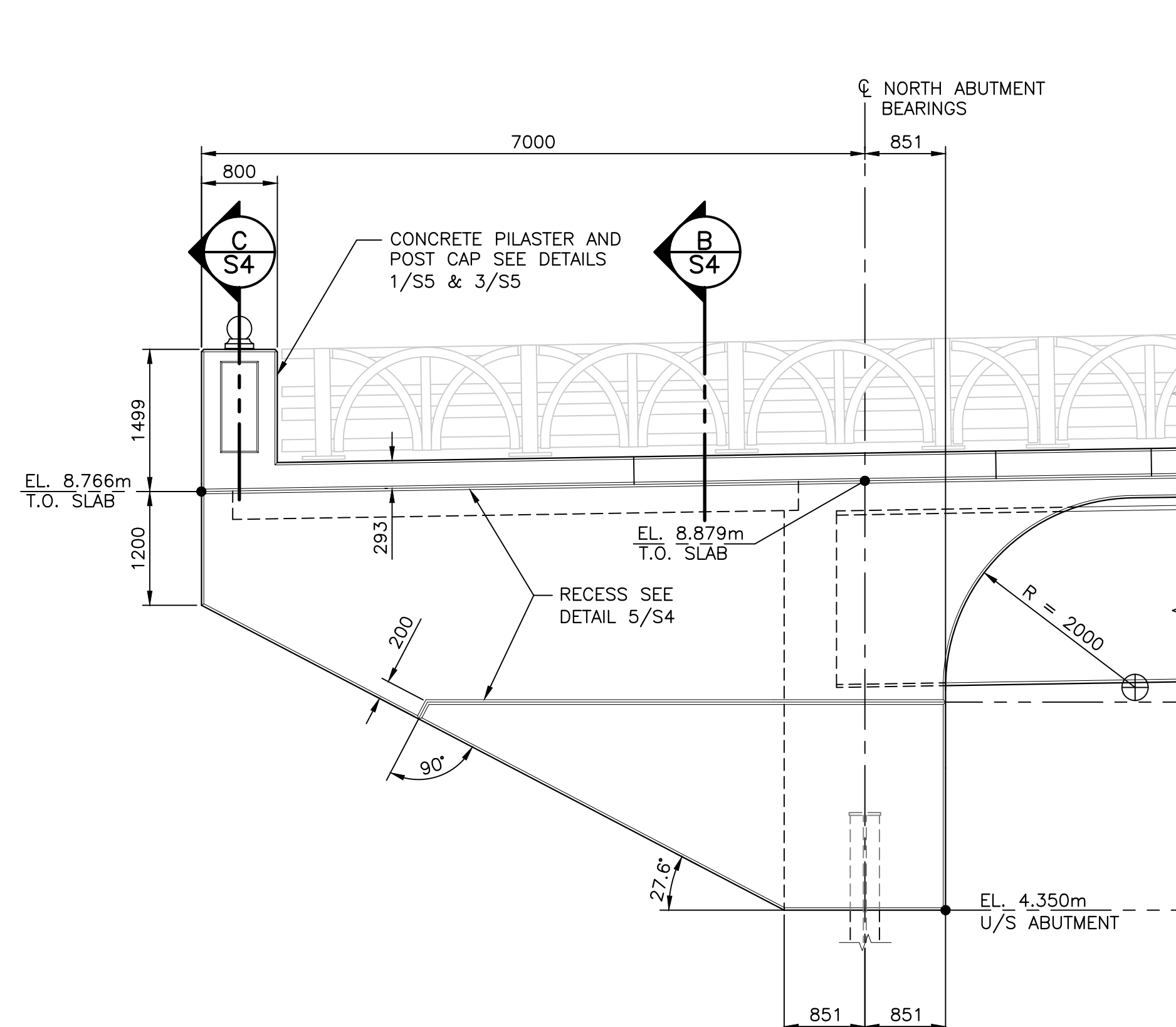
ELEVATION - SOUTH EAST WINGWALL (E1 S2)
SCALE : 1:50



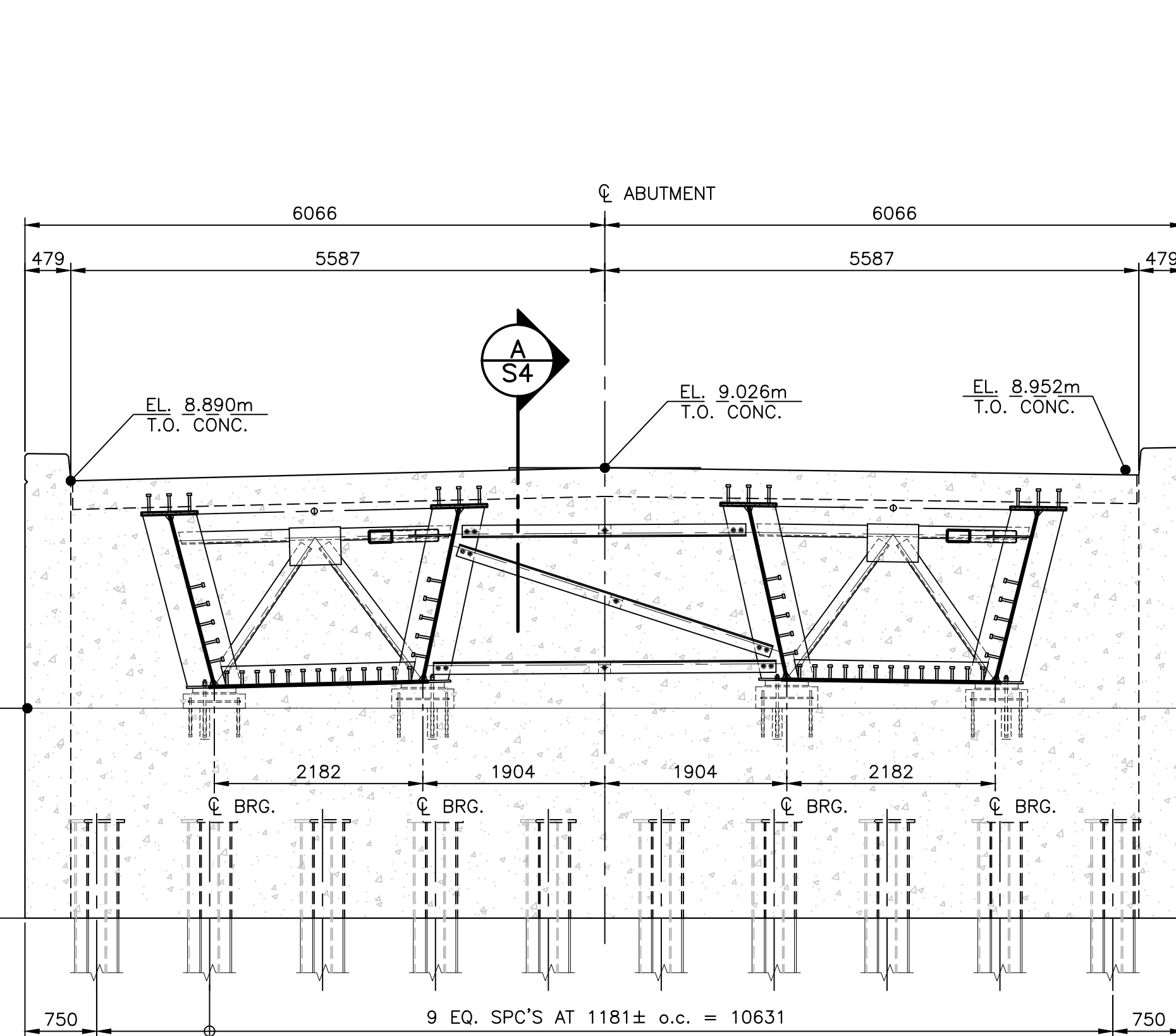
SECTION - SOUTH ABUTMENT (A S2)
SCALE : 1:50



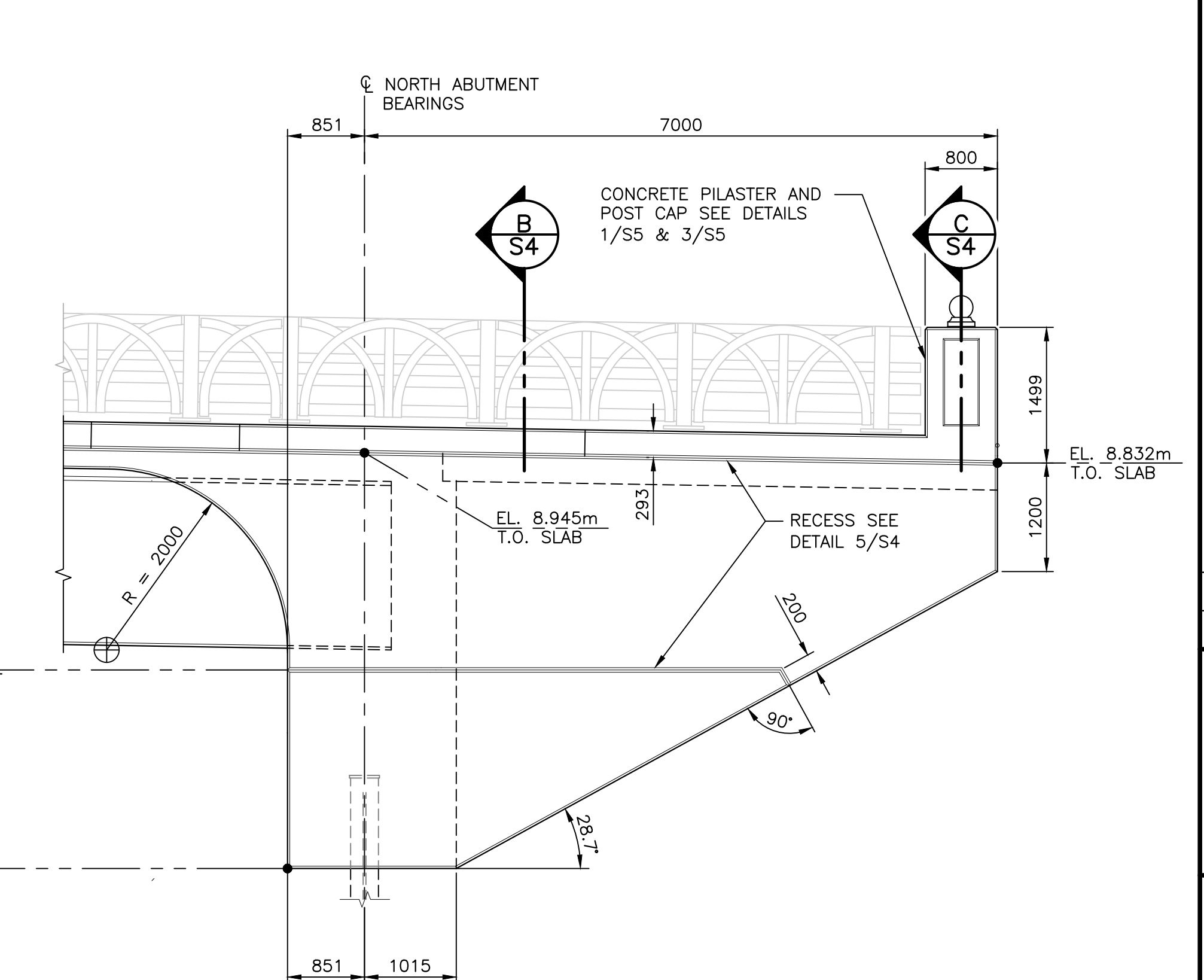
ELEVATION - SOUTH WEST WINGWALL (E2 S2)
SCALE : 1:50



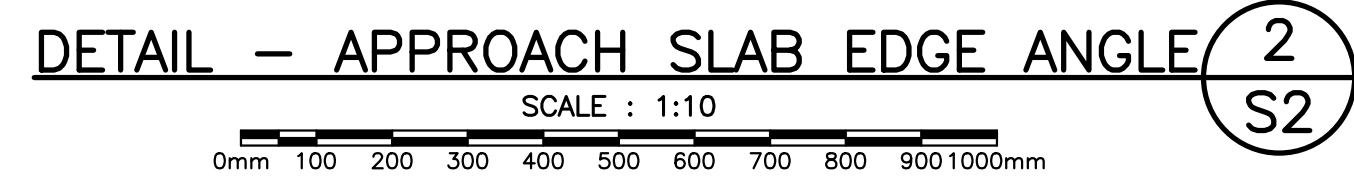
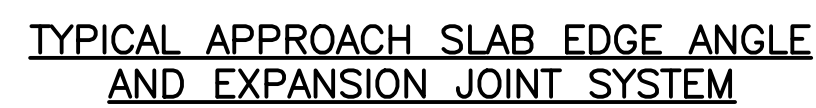
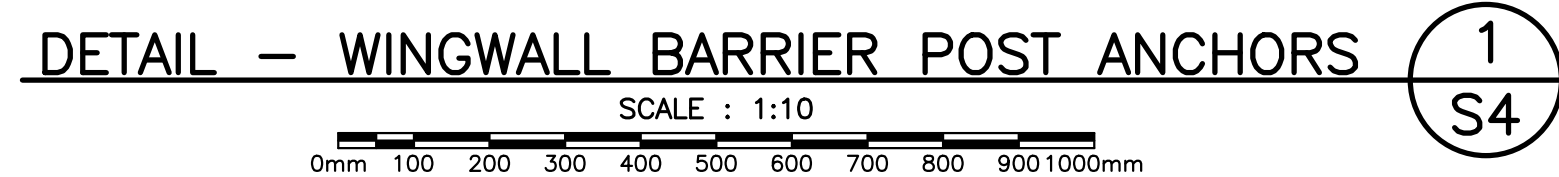
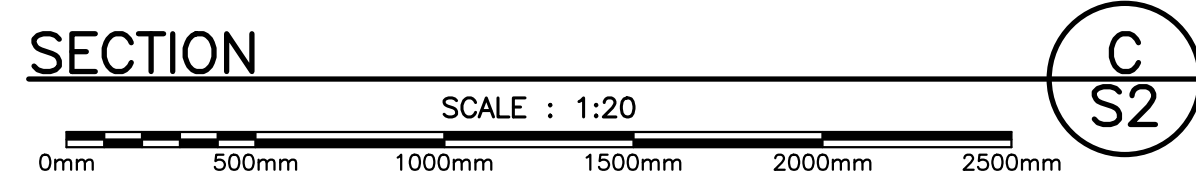
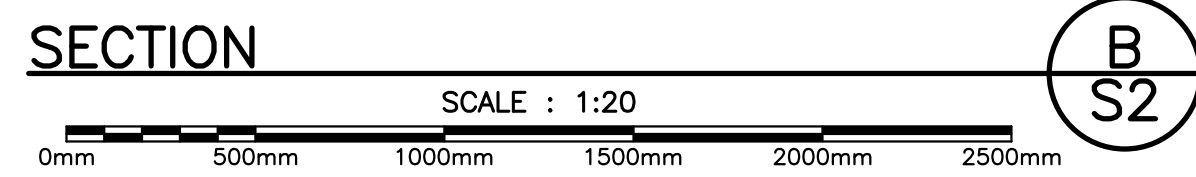
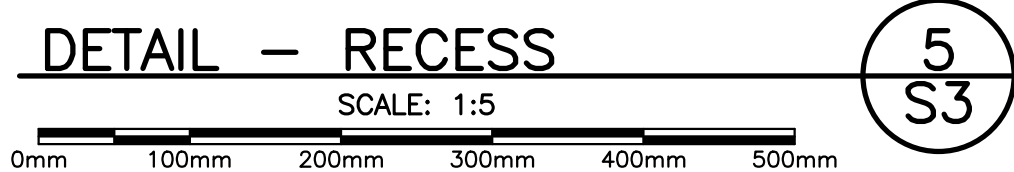
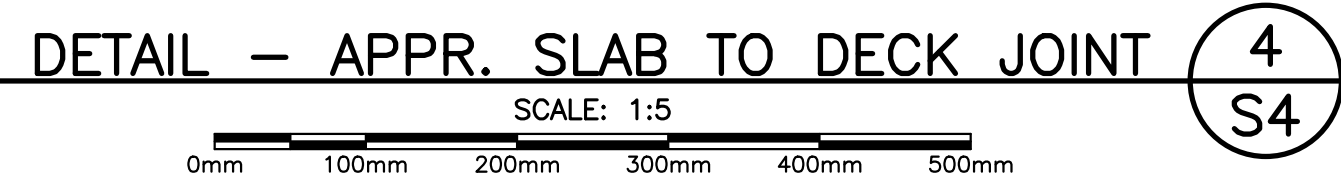
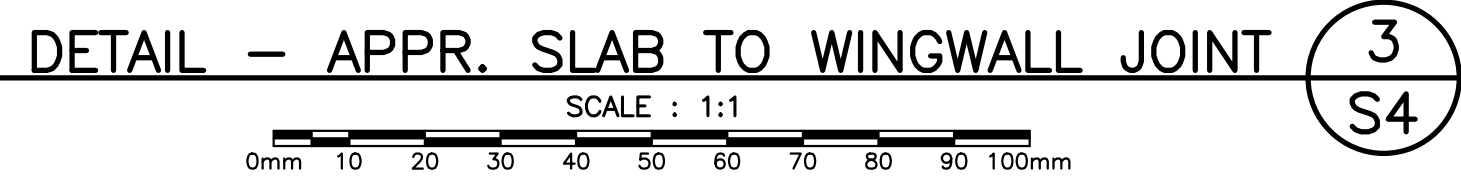
ELEVATION - NORTH WEST WINGWALL (E3 S2)
SCALE : 1:50


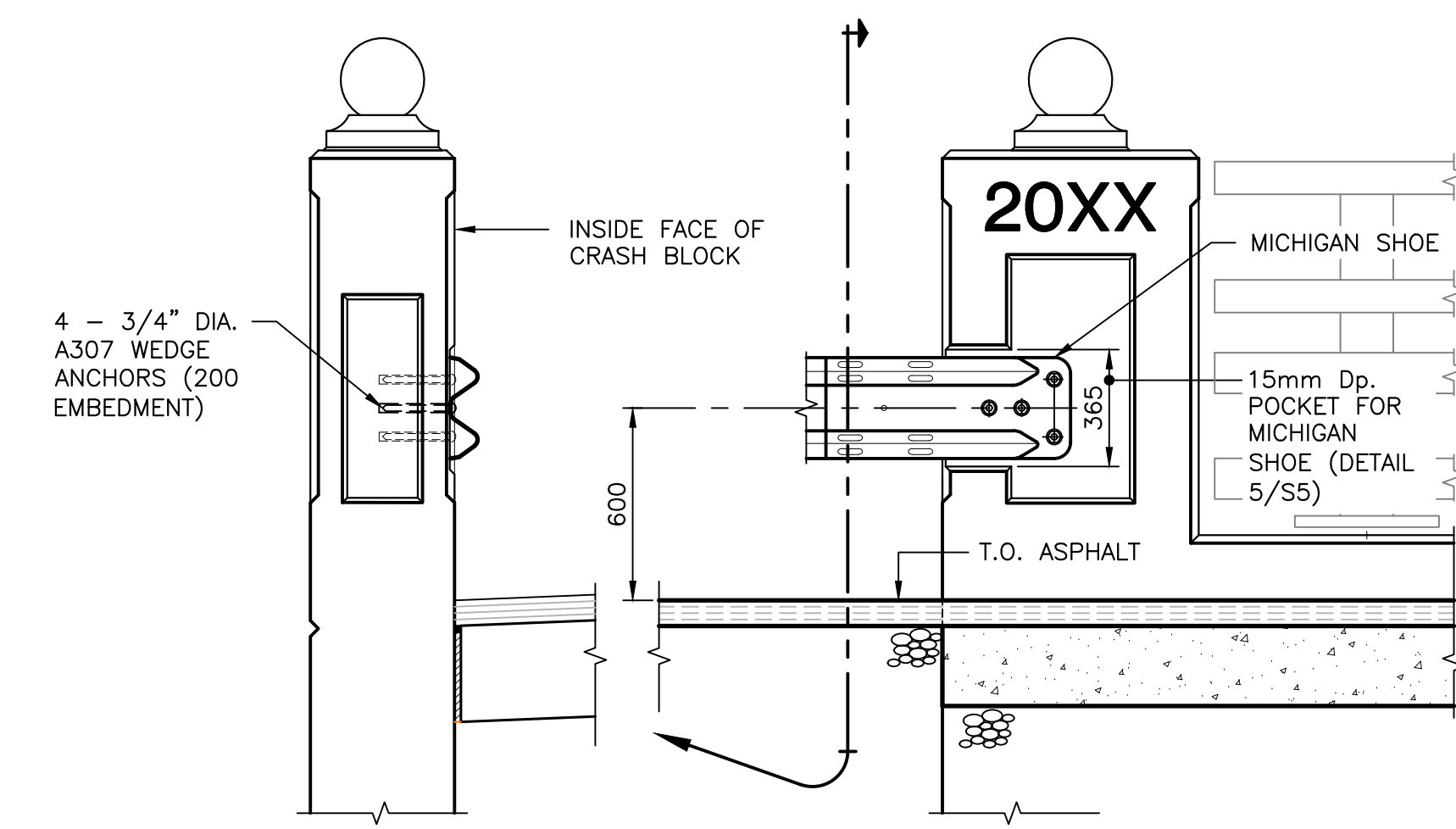


SECTION - NORTH ABUTMENT (B S2)
SCALE : 1:50



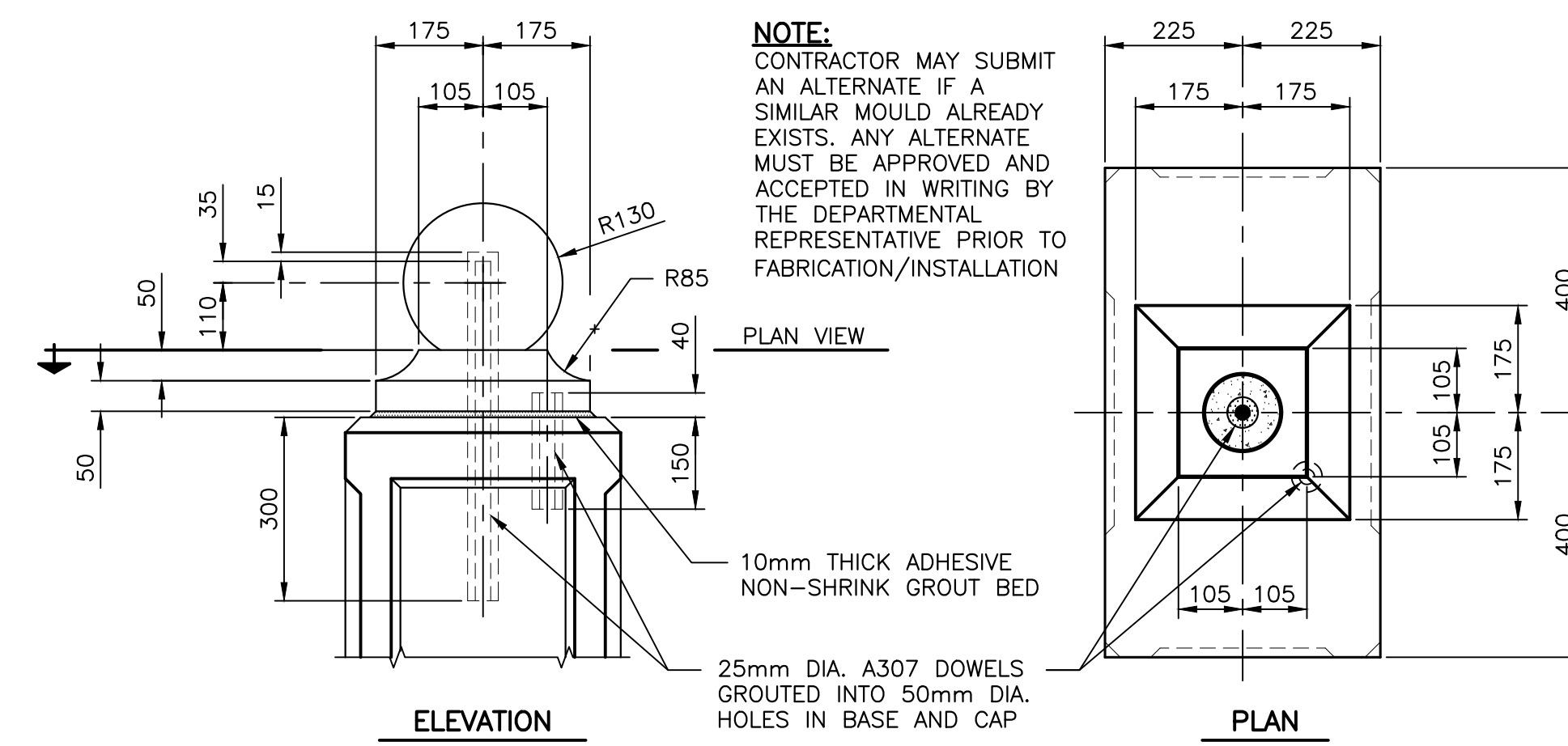

ELEVATION - NORTH EAST WINGWALL (E4 S2)
SCALE : 1:50





SCALE : 1:20


0mm 500mm 1000mm 1500mm 2000mm 2500mm



The logo of the American Society of Safety Engineers (ASSE) is a circular emblem. It is divided horizontally by a line. The upper half of the circle contains the letter 'A', and the lower half contains the letters 'S5'.

SCALE : 1:10

0mm 100 200 300 400 500 600 700 800 900 1000

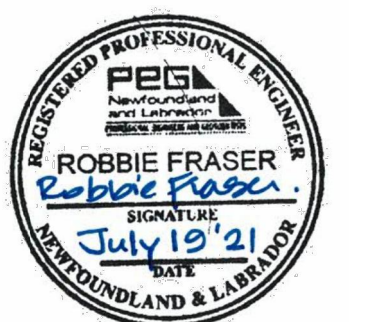


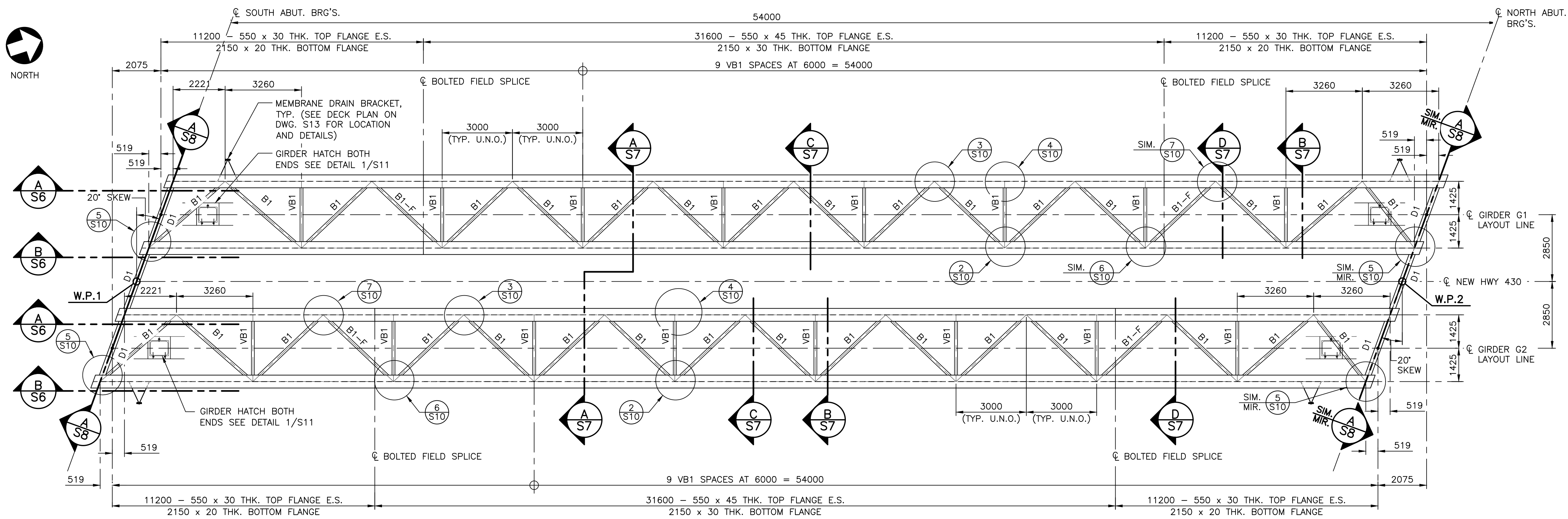
4
S1

SCALE : 1:10

0mm 100 200 300 400 500 600 700 800

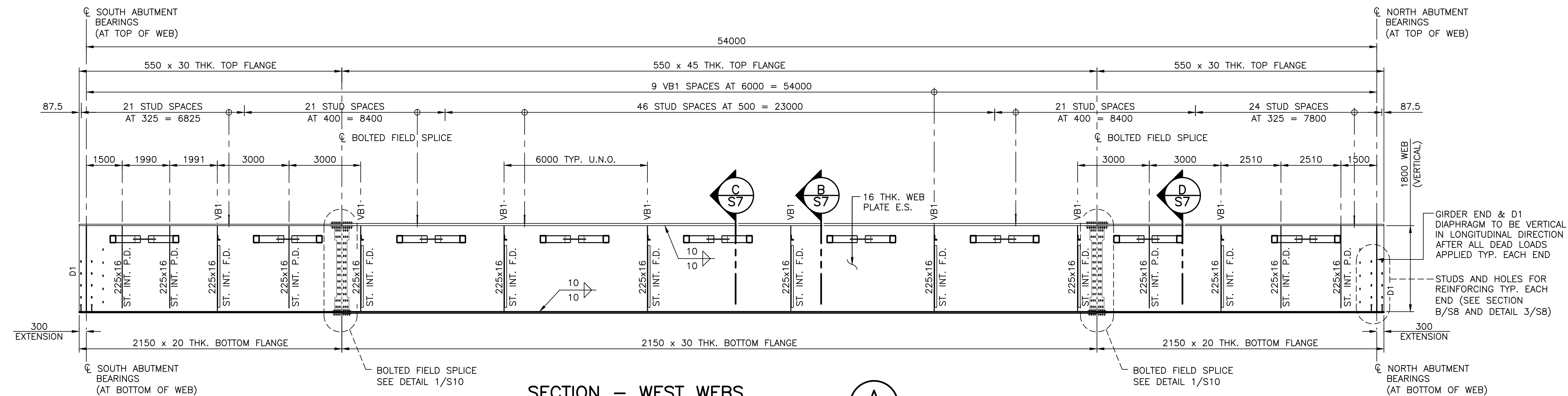
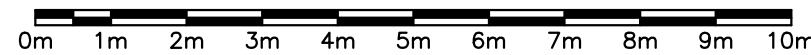
5
S5

65



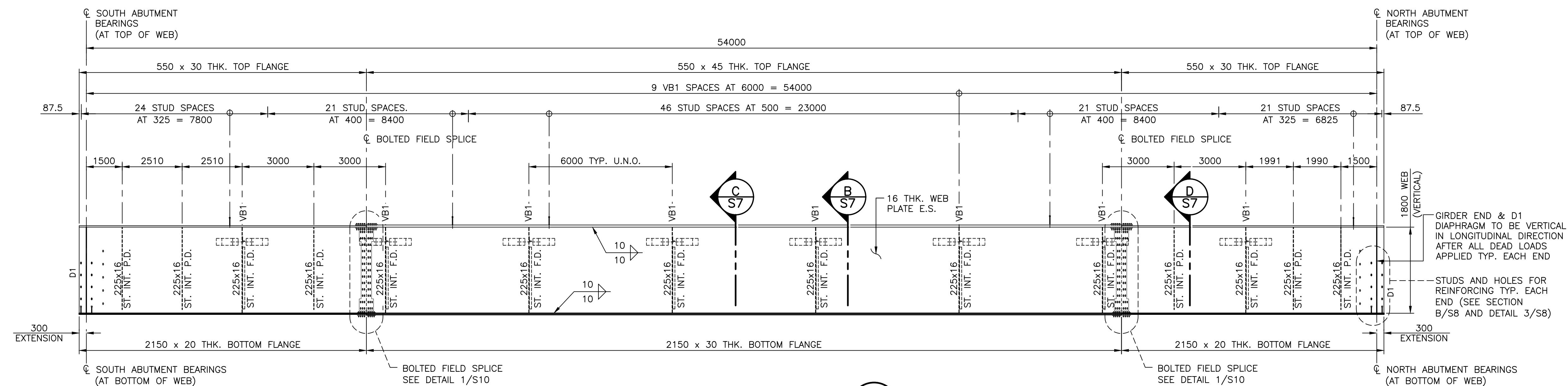
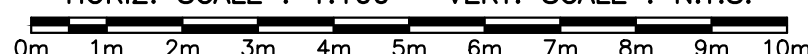
GIRDER LAYOUT PLAN

SCALE : 1:100



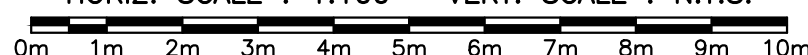
SECTION - WEST WEBS

HORIZ. SCALE : 1:100 VERT. SCALE : N.T.S.



SECTION - EAST WEBS

HORIZ. SCALE : 1:100 VERT. SCALE : N.T.S.

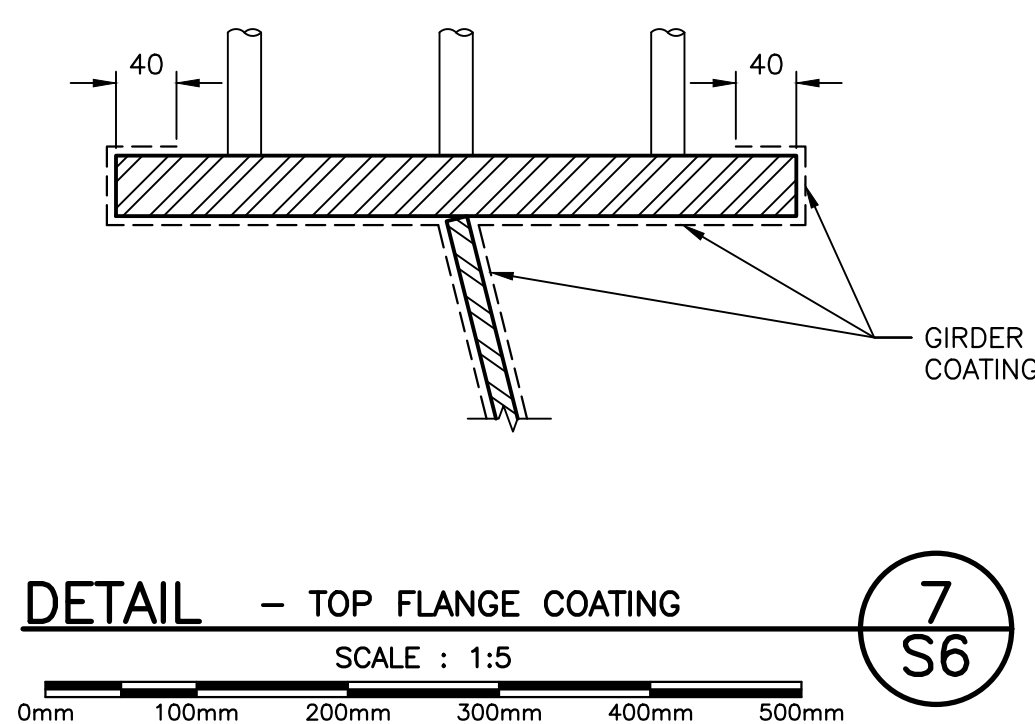
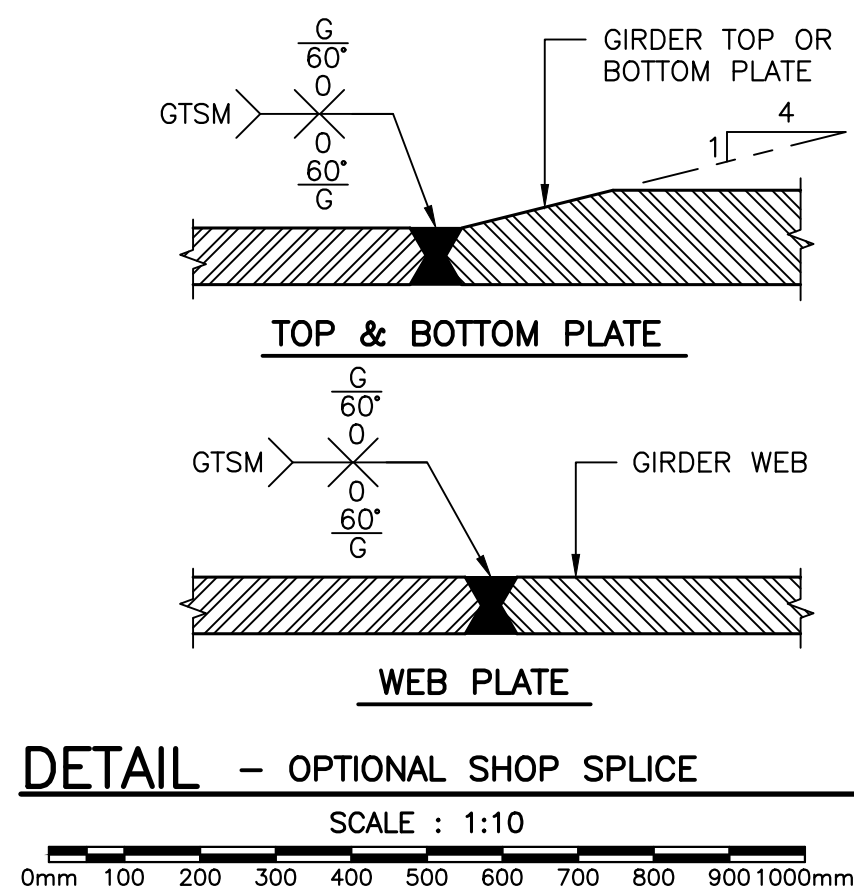
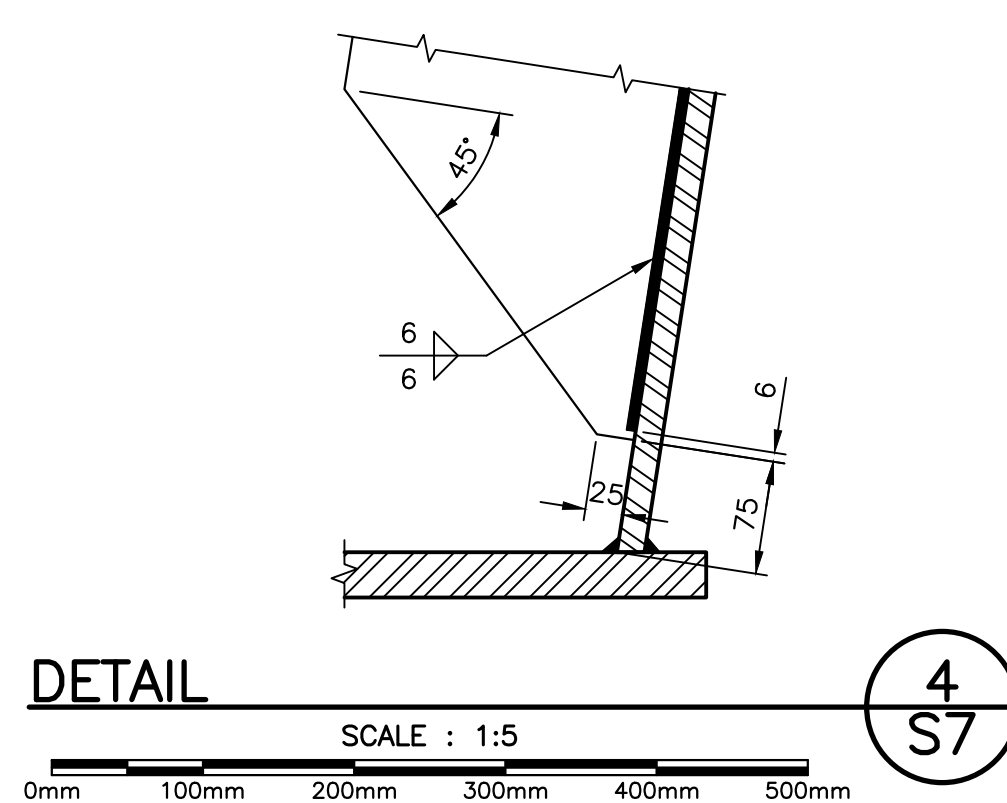
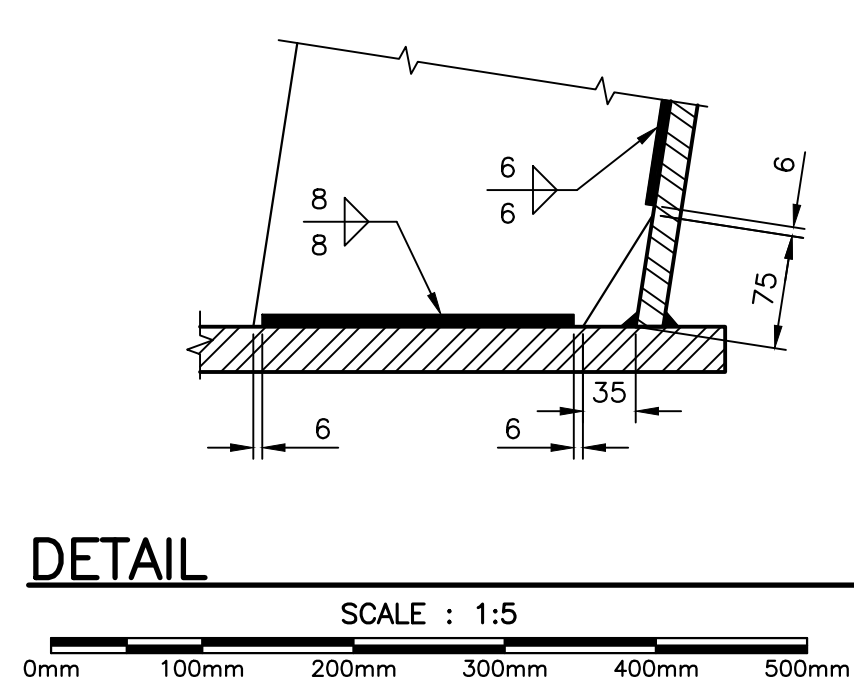
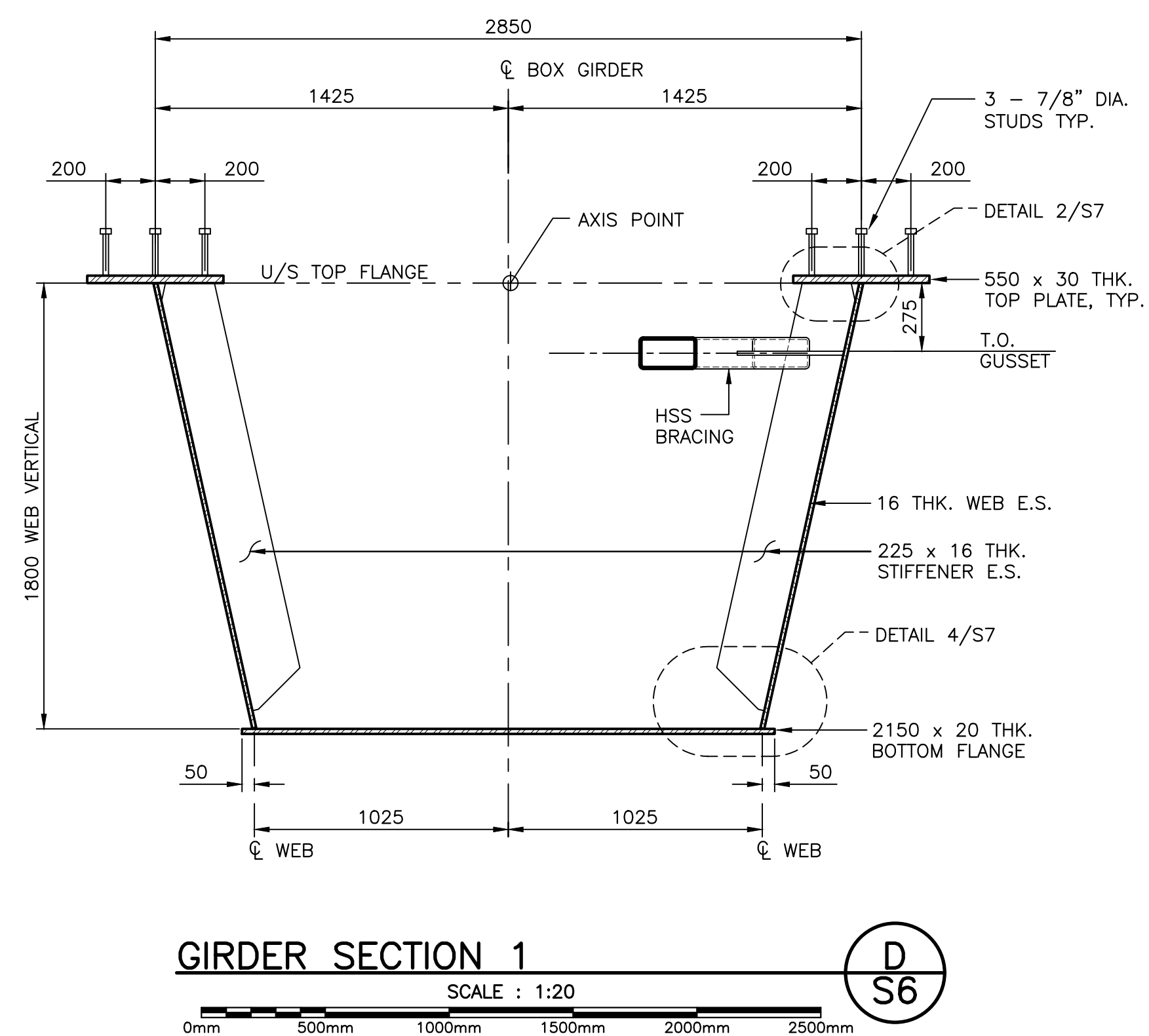
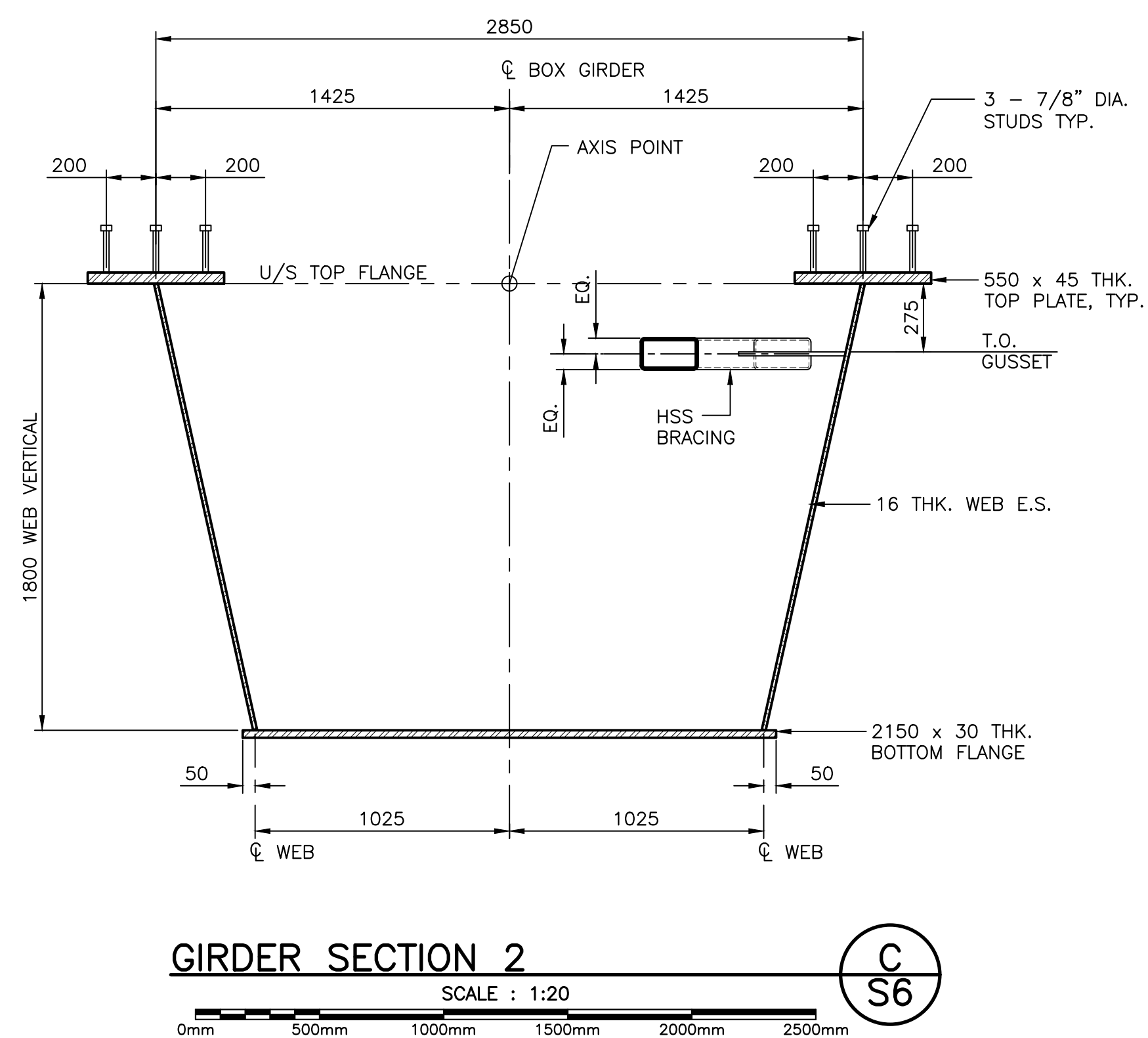
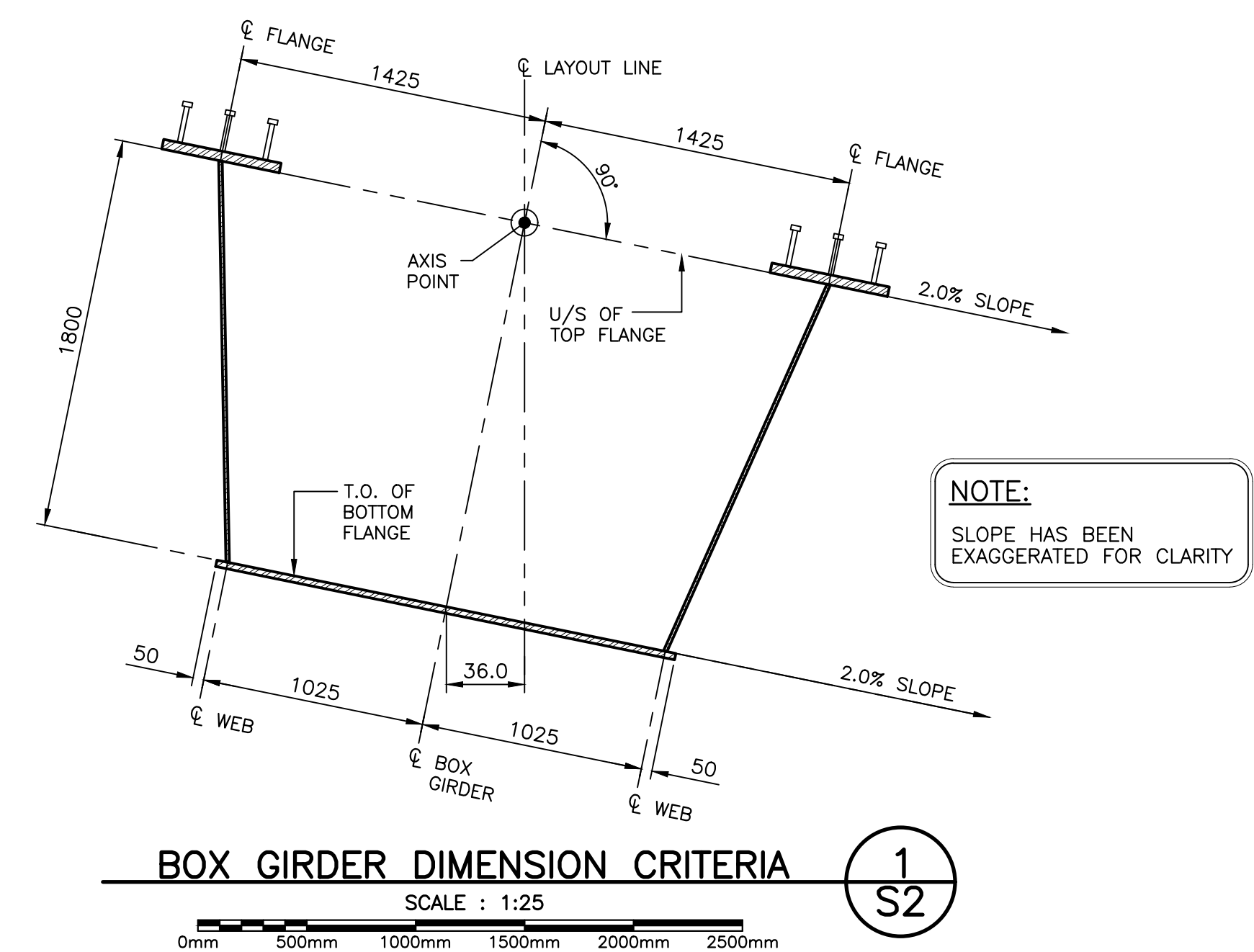


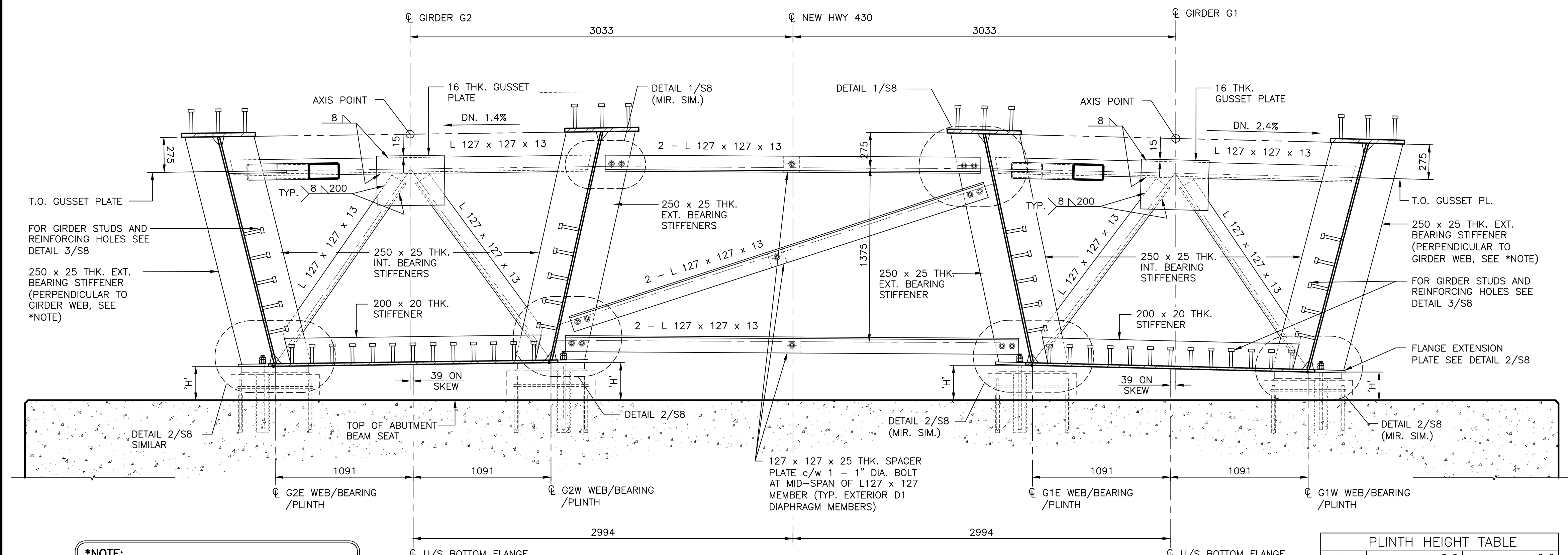
STEEL NOTES:

- STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING C.S.A. STANDARDS AND A.S.T.M. STANDARDS:
 - BOX GIRDERS: WEBS, FLANGE PLATES, STIFFENERS, SPLICE PLATES, GUSSET PLATES TO G40.21M - 350WT CAT 2.
 - ROLLED SECTIONS, INCLUDING, ANGLES, WIDE FLANGE BARRIER POSTS, AND MISC. PLATES TO G40.21M - 350W OR EQUIVALENT.
 - HSS RAILS & BRACES: ASTM A500 GRADE C.
 - HIGH STRENGTH BOLTS ASTM F3125 GRADE A325 TYPE 1, THREADS TO BE EXCLUDED FROM ALL SHEAR PLANES (U.N.O.). ALL BOLT HOLES TO BE DRILLED.
 - BEARING PLATE THREADED ANCHOR RODS ASTM A615 (75 ksi).
- ALL WELDING SHALL BE IN ACCORDANCE WITH C.S.A. STANDARD W59, LATEST EDITION.
- SHEAR STUD CONNECTORS AND THE QUALIFICATIONS OF THE SHEAR CONNECTION BASE SHALL COMPLY WITH CSA W59, ANNEX H, AND MEET THE REQUIREMENTS OF ASTM A108 GRADES G10100 TO G10200. ONLY TYPE B STUDS ARE ACCEPTABLE. STUD WELDS SHALL BE FULL STRENGTH WELDS.
- FABRICATE, DELIVER TO SITE AND ERECT STEELWORK IN ACCORDANCE WITH CAN/CSA-S6-19.
- COAT ALL STEEL IN ACCORDANCE WITH PROJECT SPECIFICATIONS. PRIMER COAT ONLY ON ALL INSIDE SURFACES OF BOX. NO PAINT ON TOP SURFACES OF TOP FLANGES EXCEPT AS SHOWN IN DETAIL 7/S7. DRILLED HOLES IN GIRDER WEBS AND BOTTOM FLANGE PLATES FOR INTEGRAL ABUTMENT REINFORCING SHALL BE FULLY COATED TO ENSURE ISOLATION BETWEEN GIRDER AND GALVANIZED ABUTMENT REINFORCING STEEL TO AVOID POTENTIAL OF GALVANIC REACTION. BOX GIRDER DRAIN HOLES AND MEMBRANE DRAIN DOWNSPOUTS SHALL ALSO BE FULLY COATED. CONFIRM TOP COAT COLOR WITH DEPARTMENTAL REPRESENTATIVE PRIOR TO FABRICATION.
- GRIND ALL BEARING STIFFENERS AT ABUTMENTS TO BEAR AT BOTTOM, THEN WELD.
- ALL FAYING SURFACES AT BOLTED CONNECTIONS SHALL BE CLASS 2 OR BETTER. ALL BOLTS BROUGHT TO SLIP CRITICAL CONDITION BY TURN OF NUT METHOD.
- CONTRACTOR RESPONSIBLE FOR LIFTING & STABILITY OF GIRDERS DURING ALL PHASES OF CONSTRUCTION.
- D1 DIAPHRAGMS TO BE VERTICAL IN THE LONGITUDINAL DIRECTION AFTER ALL DEAD LOAD DEFLECTIONS OCCUR. ALL OTHER STIFFENERS (INCLUDING VB1 LOCATIONS) TO BE PERPENDICULAR TO FLANGES.
- STUD HEIGHTS VARY ALONG SPAN AND ARE DEPENDENT ON AS-BUILT GIRDER ELEVATIONS AND CAMBERS. THEORETICAL STUD HEIGHTS CAN BE DETERMINED USING DETAIL 5/S7 (MAX. AND MIN. PROJECTION INTO DECK), ALONG WITH THEORETICAL HAUNCH DEPTHS ALONG SPAN. THEORETICAL HAUNCH DEPTHS CAN BE DETERMINED AT EACH SCROD STATION USING FINAL GIRDER ELEVATIONS AT EACH ABUTMENT, THE THEORETICAL CAMBER PROFILE, THE SELF WEIGHT AND TOTAL DEAD LOAD GIRDER DEFLECTIONS AND THE TOP OF DECK FINAL ELEVATIONS. INDICATED ON S13. HAUNCH THICKNESS NEAR ABUTMENTS MAY EXCEED READILY AVAILABLE STUD LENGTHS. IT IS PERMISSIBLE TO USE 2 SHEAR CONNECTORS (WELDED ABOVE ONE ANOTHER) TO OBTAIN ADEQUATE STUD LENGTHS. AS-BUILT STUD HEIGHTS ARE BASED ON AS-BUILT CAMBERS, AND BEARING ELEVATIONS, AND THEREFORE THE REQUIRED STUD HEIGHTS REMAINS THE RESPONSIBILITY OF THE CONTRACTOR.
- BOTTOM BEARING POINT OF SLAB OVERHANG BRACKET SHALL BE ORIENTATED NO HIGHER THAN 25mm ABOVE EXTERIOR GIRDER BOTTOM FLANGE/WEB INTERFACE DURING DECK CASTING. CONTRACTOR SHALL ENSURE STABILITY OF GIRDERS DURING ALL PHASES OF CONSTRUCTION.
- BOX GIRDERS ARE FRACTURE CRITICAL MEMBERS AS PER SECTION 12 OF CSA W59-13 AND SECTION 10 OF CSA S6-19. SPECIFICALLY, THE BOTTOM FLANGE AND THE LOWER 1500mm PORTION OF GIRDER WEB ALONG THE ENTIRE SPAN, ALONG WITH THE BOTTOM FLANGE AND WEB SPLICE PLATES, SHALL ALL BE CONSIDERED FRACTURE CRITICAL COMPONENTS OF THE STRUCTURE.
- ABUTMENT D1 BRACING BETWEEN BOXES TO BE MATCH DRILLED WITH BOXES IN SELF WEIGHT ONLY CONDITION.
- IT IS ACCEPTABLE TO REMOVE ONE OR BOTH BOLTED FIELD SPLICES IN FAVOR OF APPROVED WELDED SHOP SPLICES. IT IS NOT ACCEPTABLE TO CHANGE THE LOCATIONS OF THE BOLTED FIELD SPLICES. REFER TO DETAIL 6/S7 FOR ACCEPTABLE WELDED SHOP SPLICES.

LEGEND:

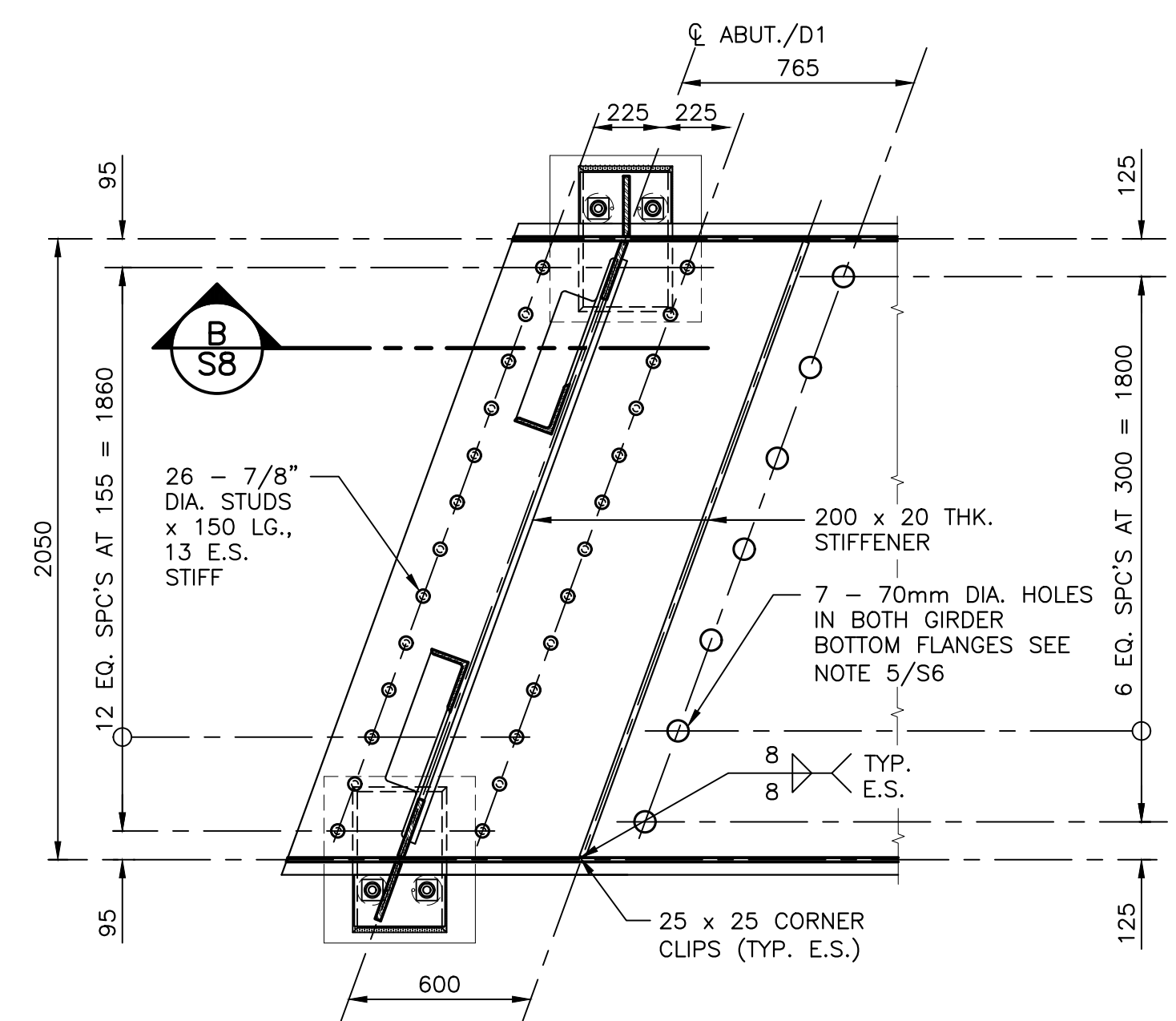
- D1 --- TYPICAL ABUTMENT DIAPHRAGM SECTION A/S8
- VB1 --- TYPICAL INTERIOR DIAPHRAGM SECTION B/S7
- B1 --- HSS 127 x 127 x 8.0 BRACING
- B1-F --- HSS 127 x 127 x 8.0 BRACING c/w BOLTED FIELD CONNECTIONS EACH END
- ST. INT. F.D. --- STIFFENER INTERIOR FULL DEPTH. SEE DETAILS 2/S7 AND 3/S7
- ST. INT. P.D. --- STIFFENER INTERIOR PARTIAL DEPTH. SEE DETAILS 2/S7 AND 4/S7





SECTION - DIAPHRAGM D1
SCALE : 1:20

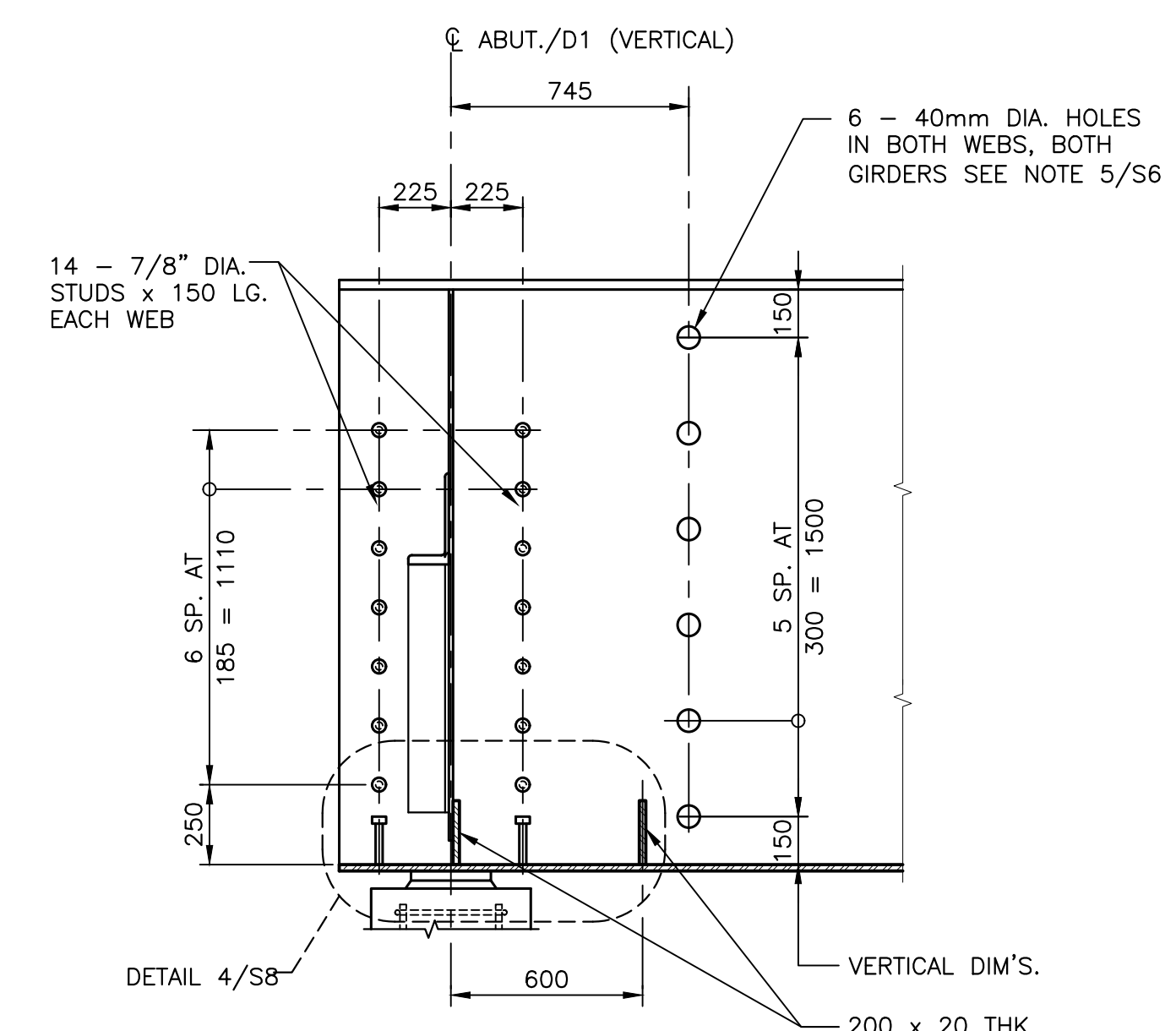
A
S6



PLAN - GIRDER BOTTOM FLANGE (G1 SHOWN AT SOUTH ABUT.)

DETAIL - REINFORCING HOLES / STUDS
SCALE : 1:20

3
S6

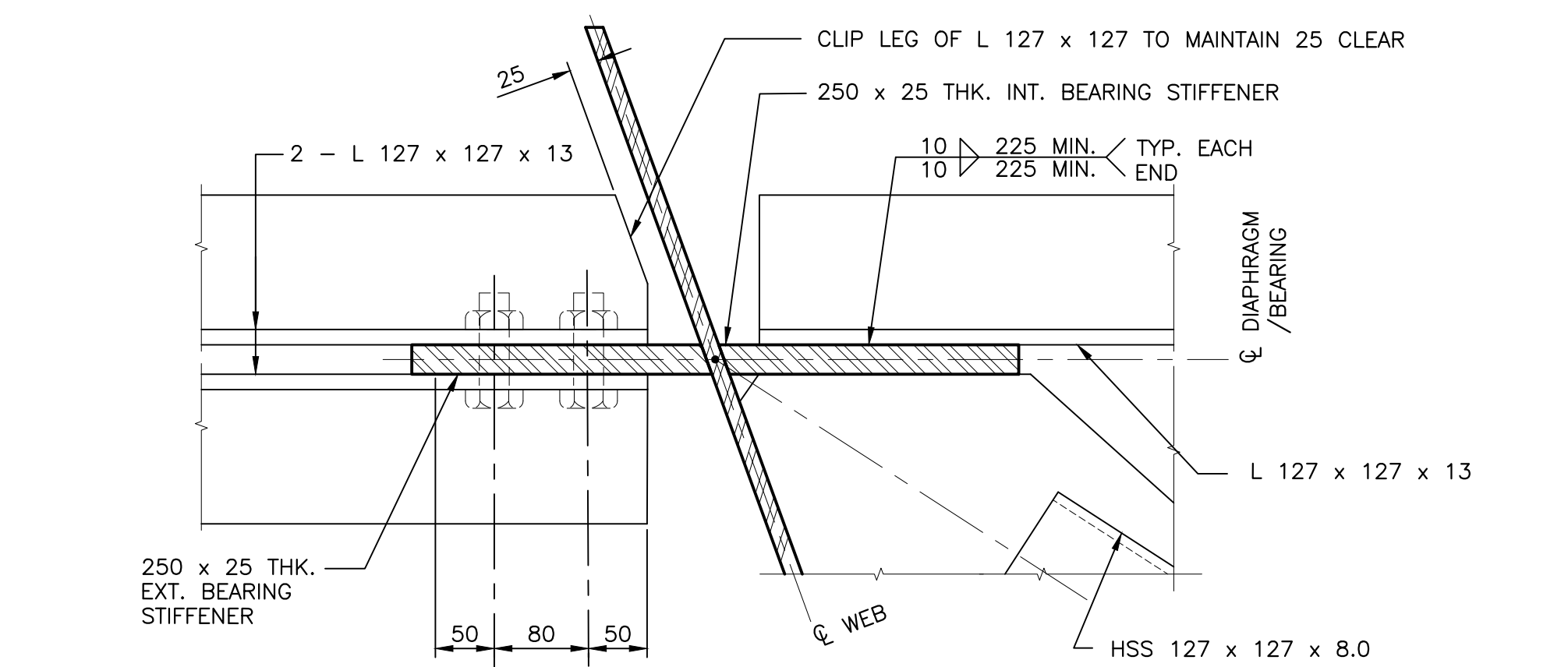


ELEVATION - WEB PLATE

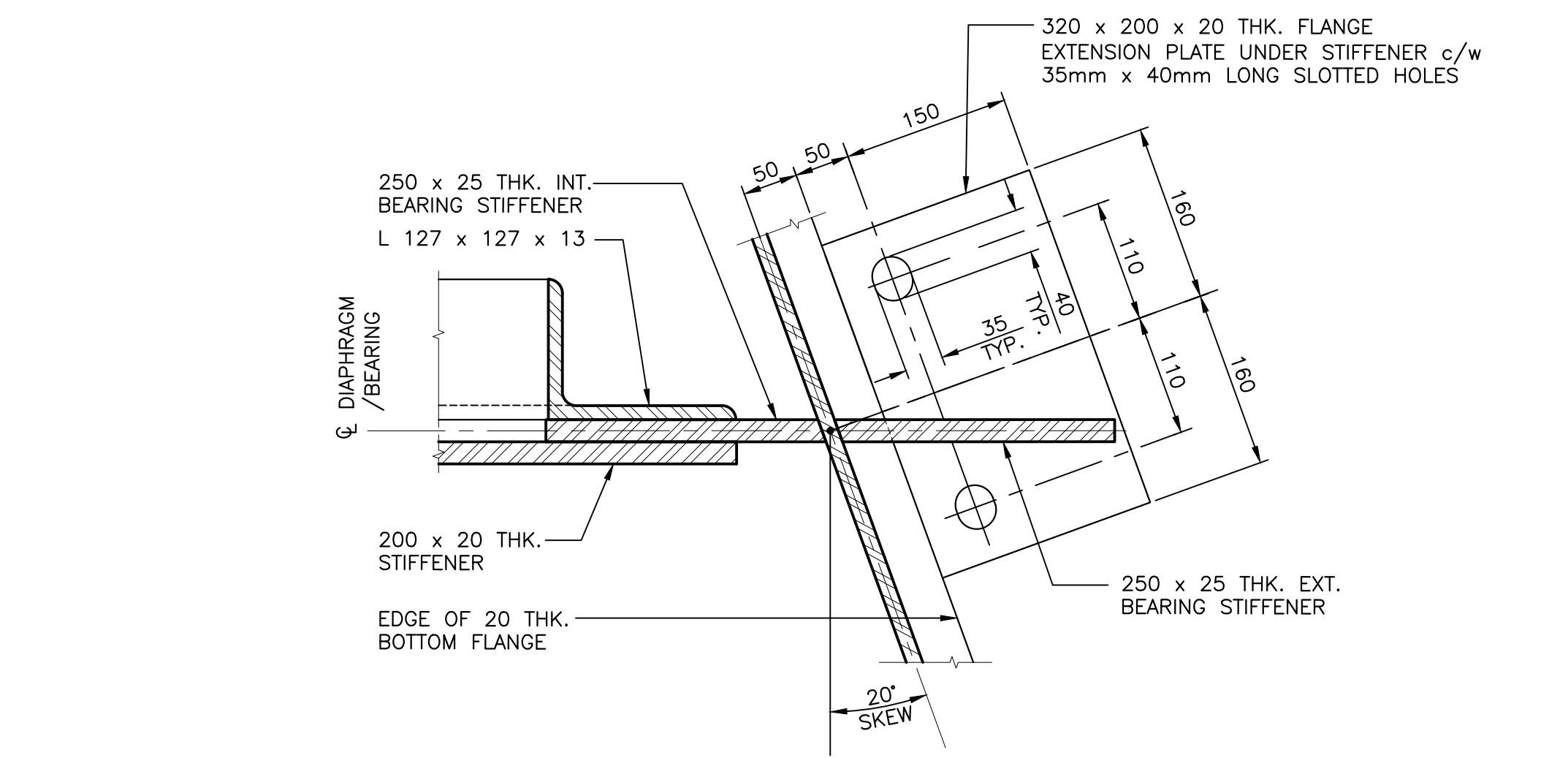
SECTION - REINFORCING HOLES / STUDS
SCALE : 1:20

B
S6

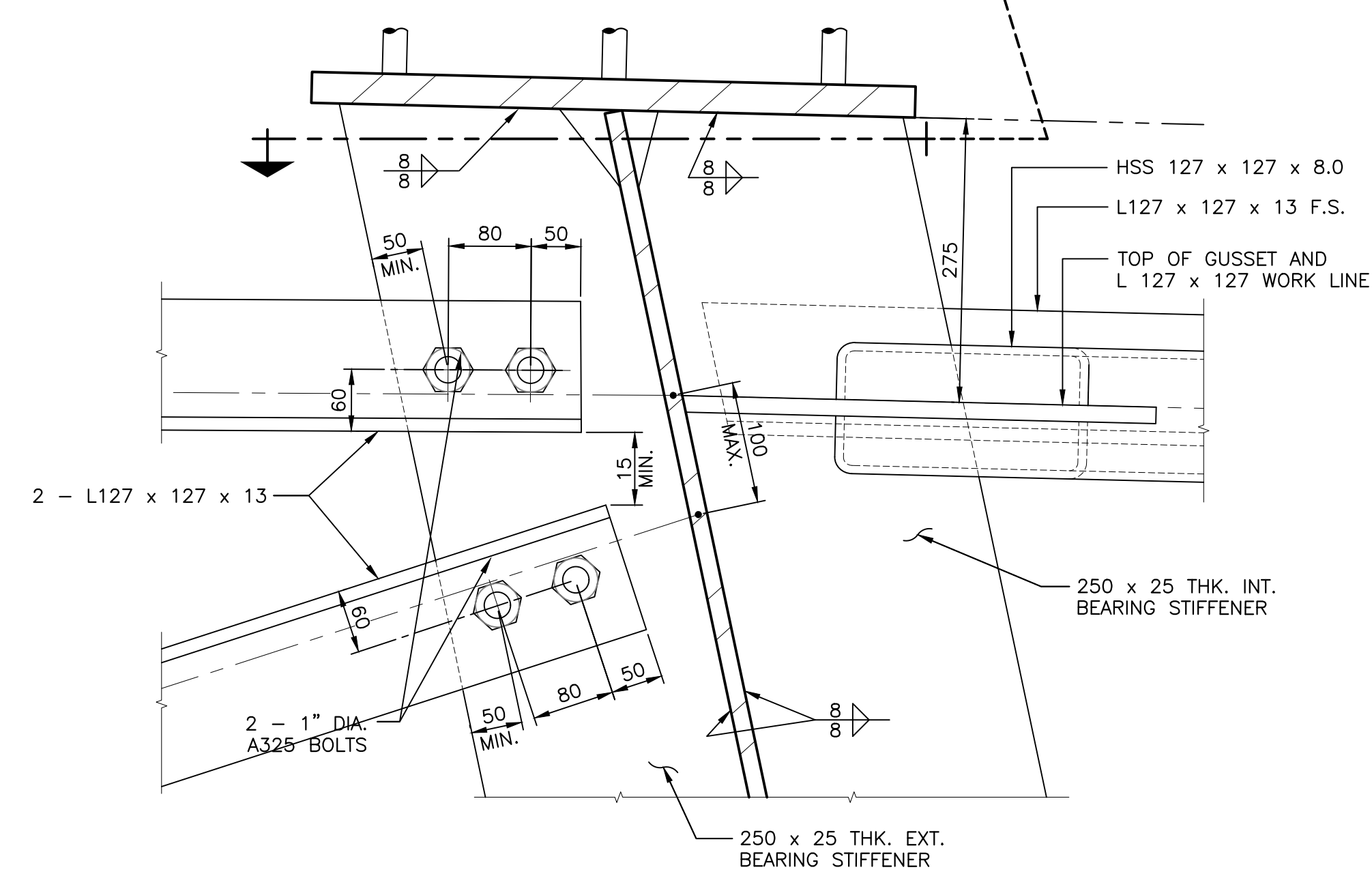
PRELIMINARY
Not For Construction



PLAN SECTION

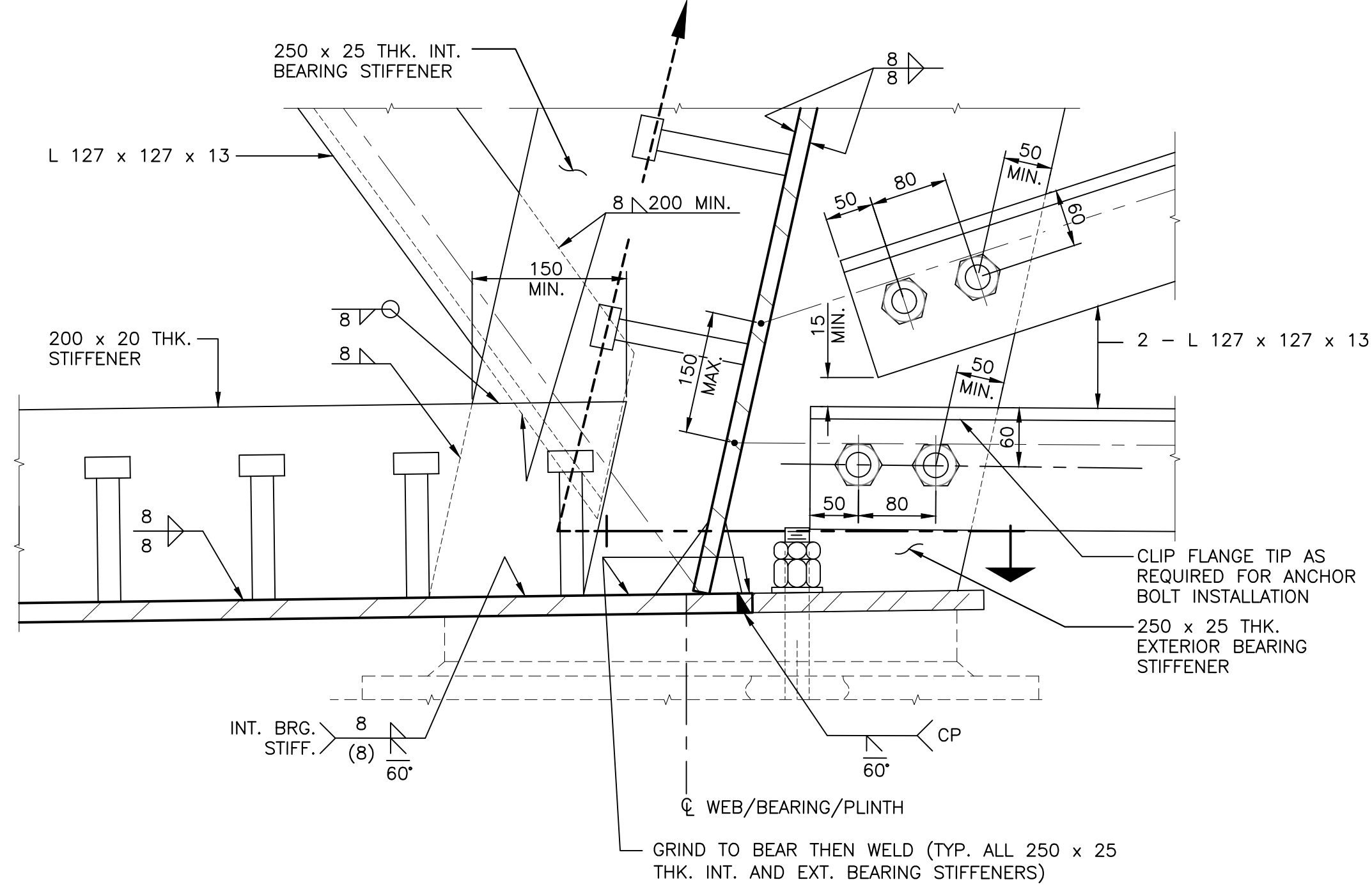


PLAN SECTION



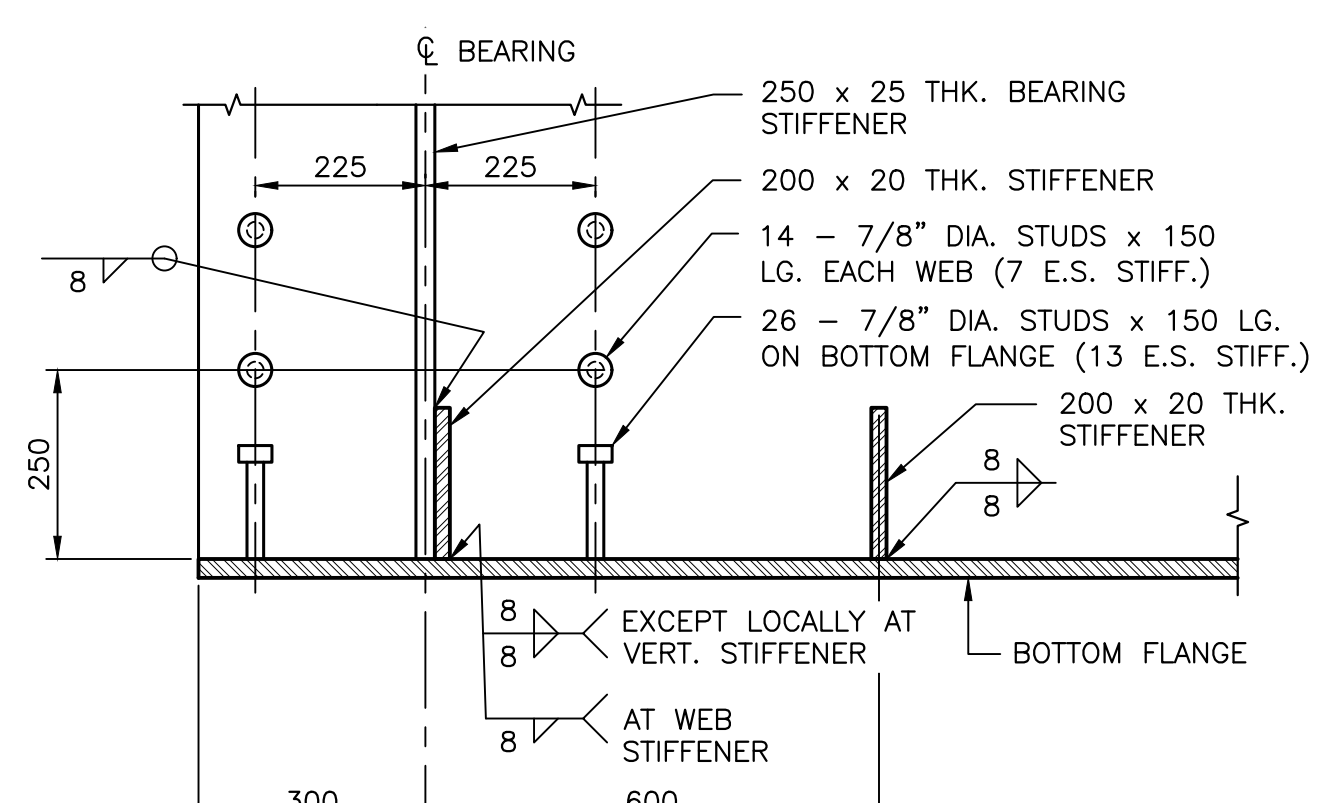
DETAIL
SCALE : 1:5

1
S8



DETAIL
SCALE : 1:5

2
S8



DETAIL
SCALE : 1:10

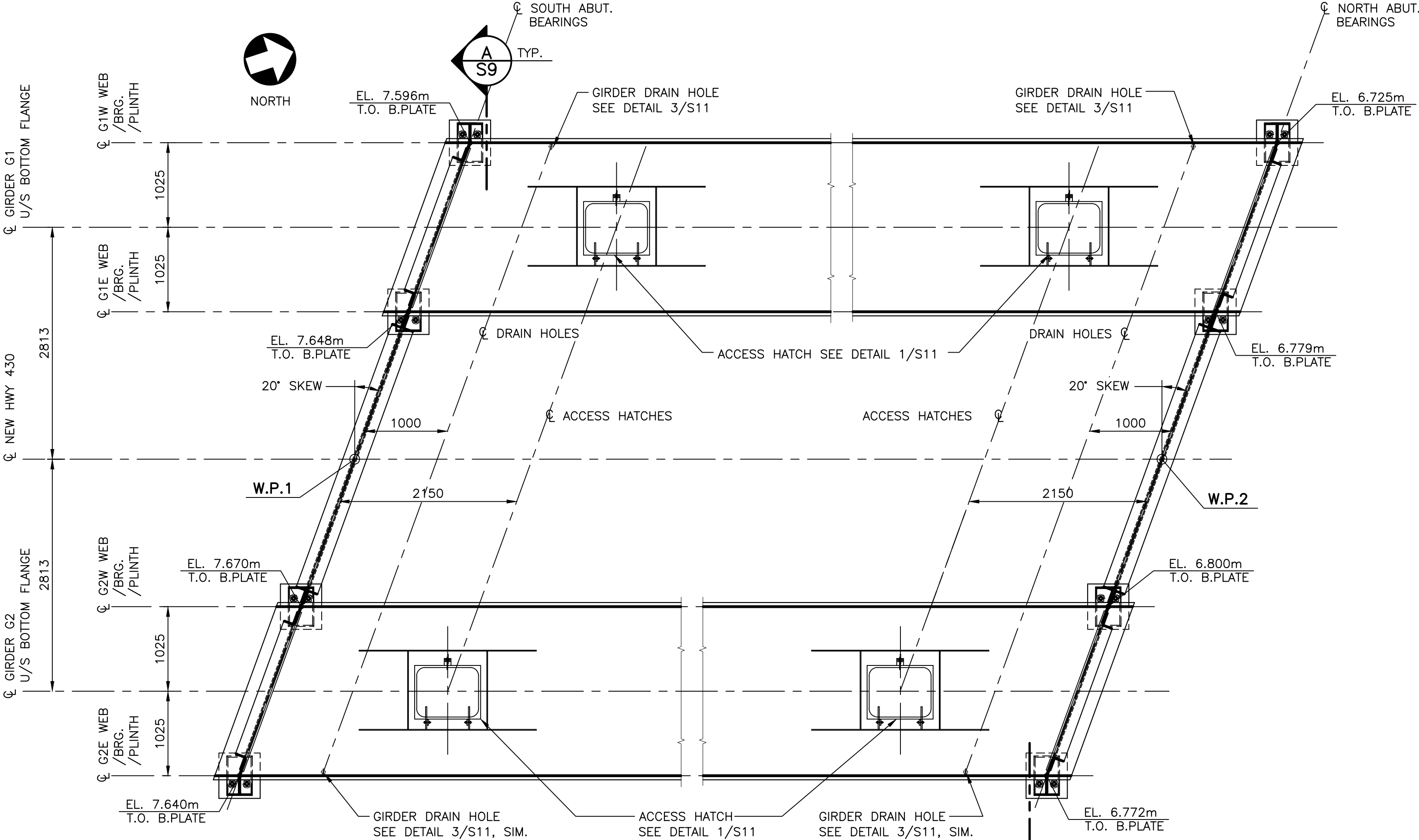
4
S8

0 ISSUED FOR TENDER JULY 19 2021
revisions
project WESTERN BROOK BRIDGE REPLACEMENT
GROS MORNE NATIONAL PARK

drawing DIAPHRAGM D1 SECTIONS AND DETAILS
desain

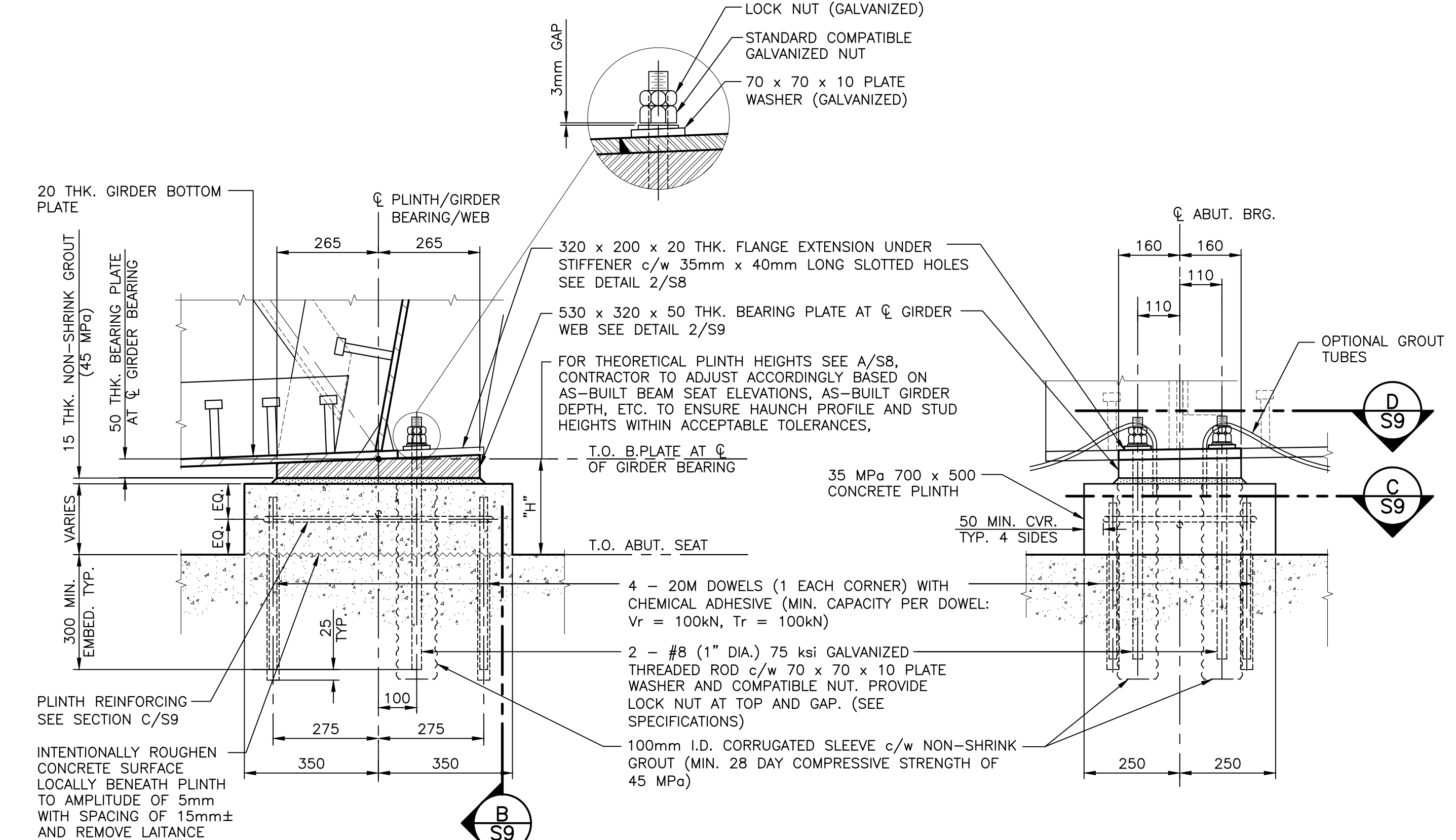
designed WADE POTTIE conçu
date JANUARY, 2020
drawn WAYNE MORROW dessiné
date JANUARY, 2020
approved ROBBIE FRASER approuvé
date
Tender
PWSC Project Manager Administrateur de projets TPSC
project number no. du projet
drawing no. no. du dessin

S8



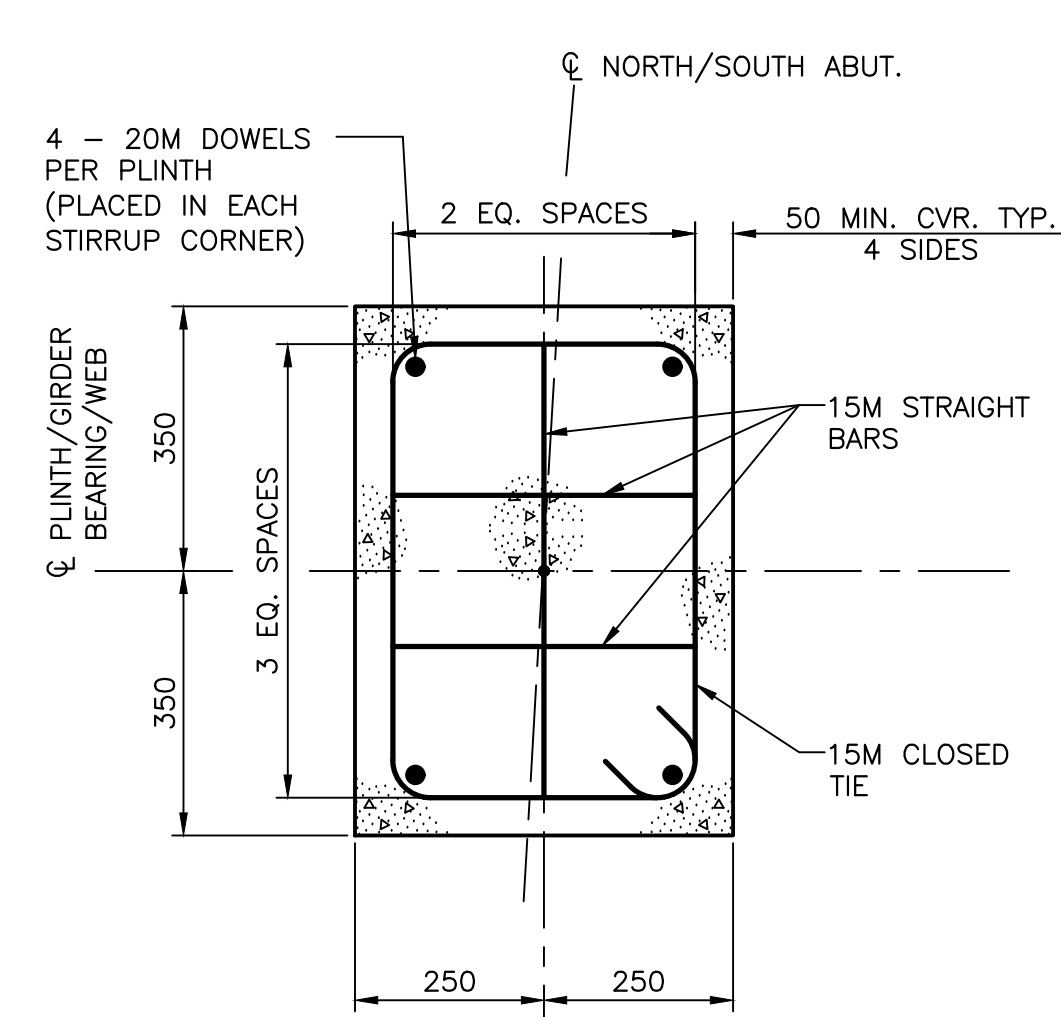
DETAIL - BEARING LAYOUT PLAN AND ELEVATIONS
- HATCH AND DRAIN LOCATION PLAN
SCALE: 1:40
0mm 1mm 2mm 3mm 4mm 5mm

LEGEND:
T.O. B. PLATE --- ELEVATION TOP OF BEARING
PLATE AT CENTERLINE PLATE

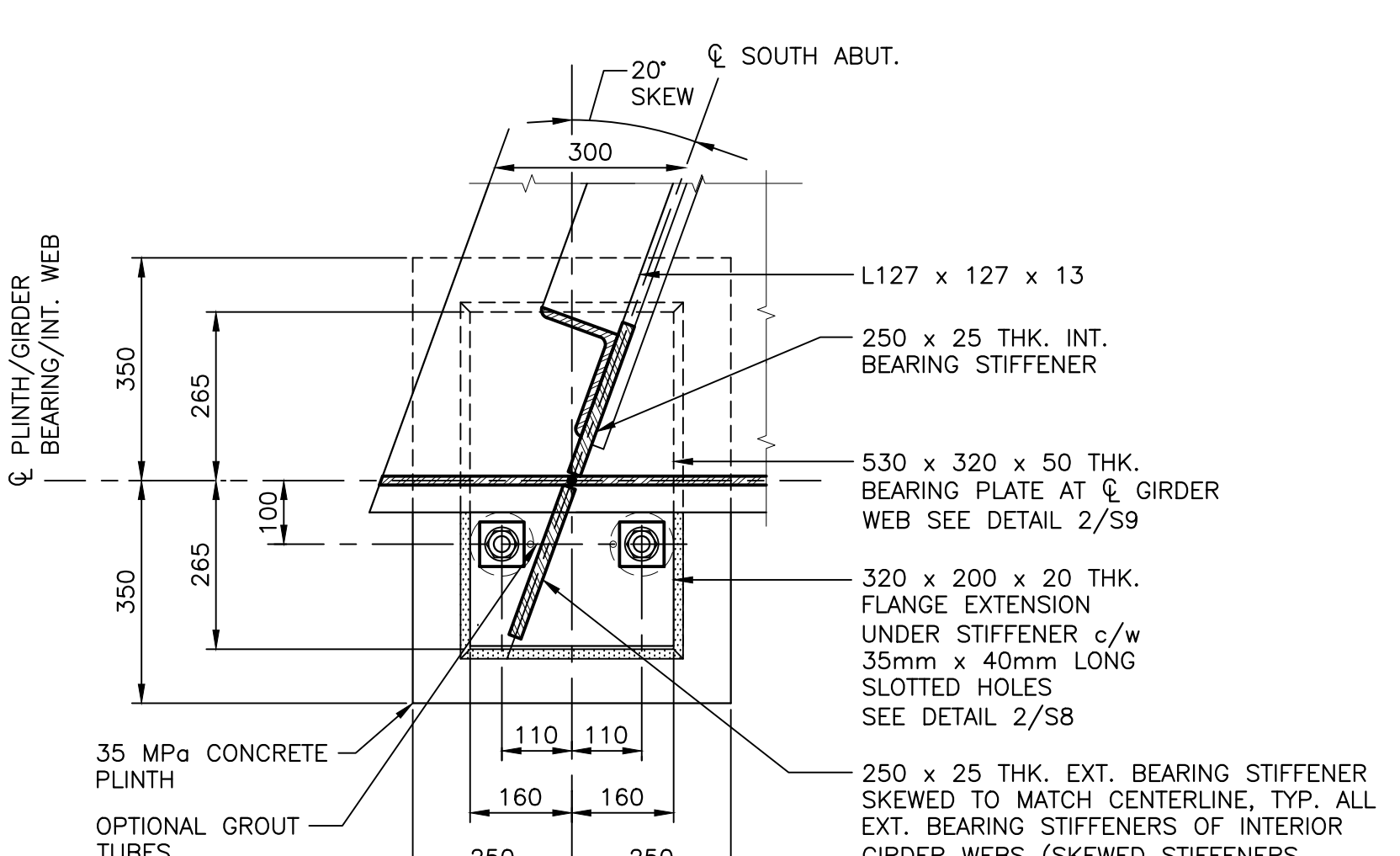


SECTION - GIRDER BEARING
SCALE: 1:10
0mm 100 200 300 400 500 600 700 800 900 1000mm

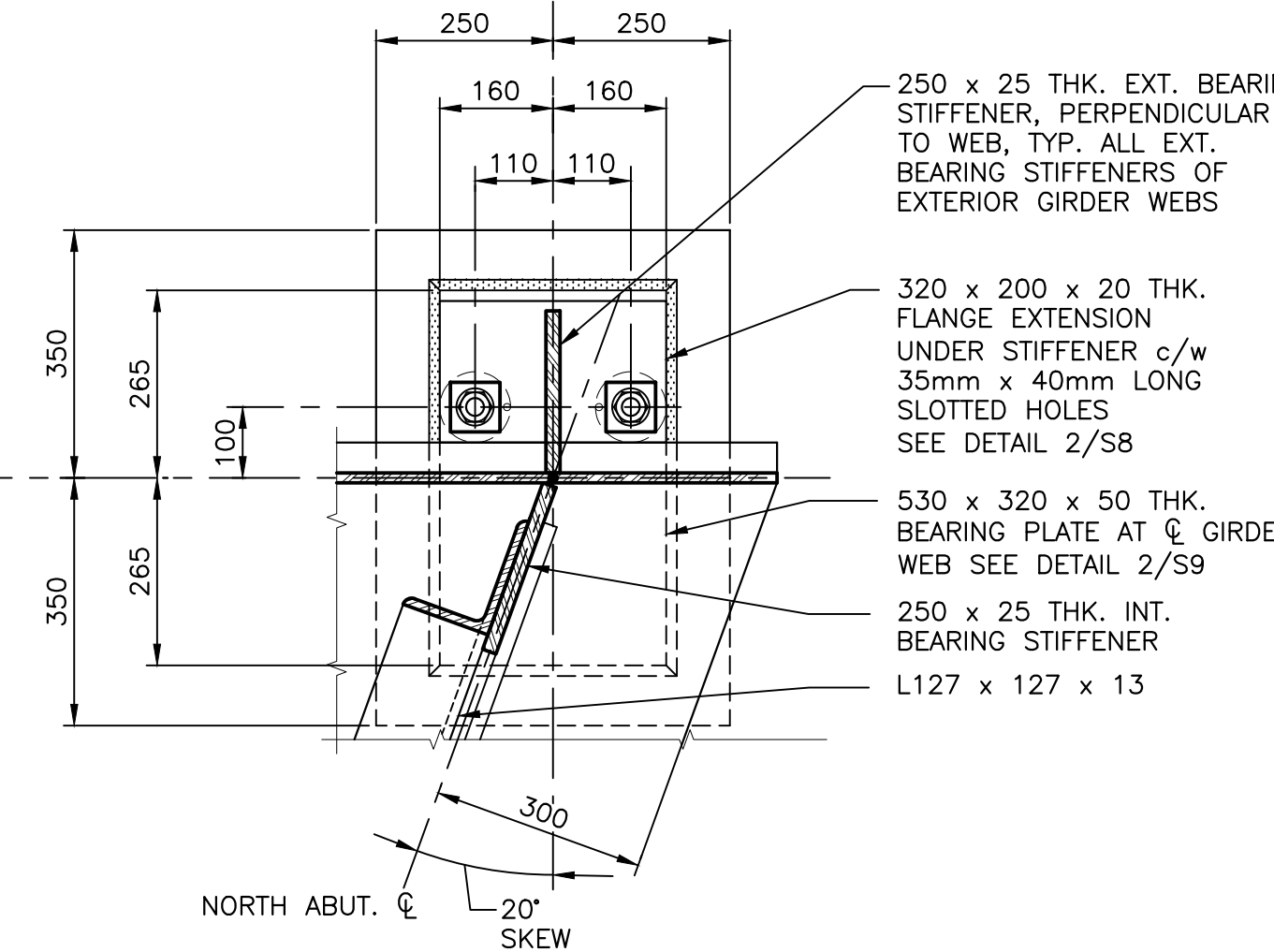
SECTION - GIRDER BEARING
SCALE: 1:10
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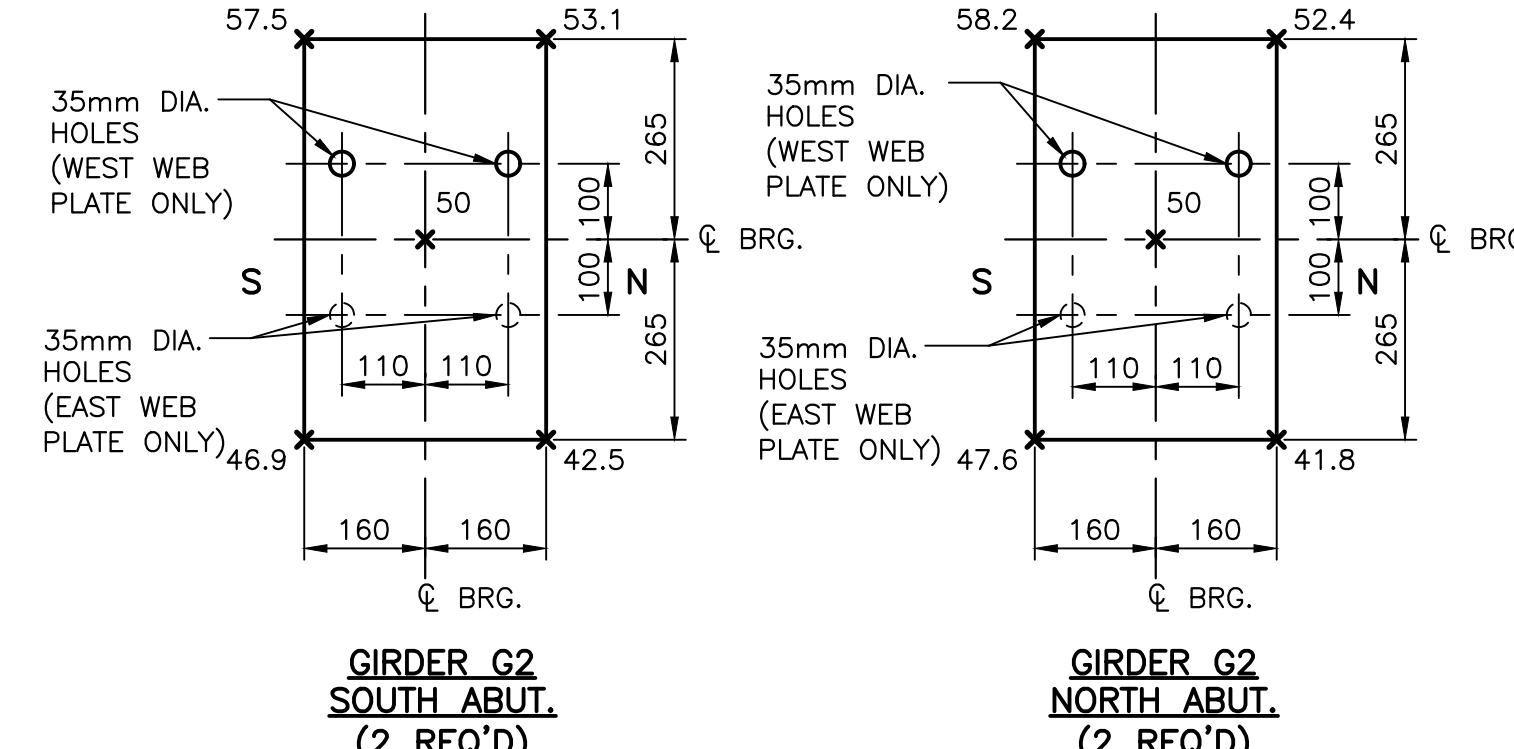
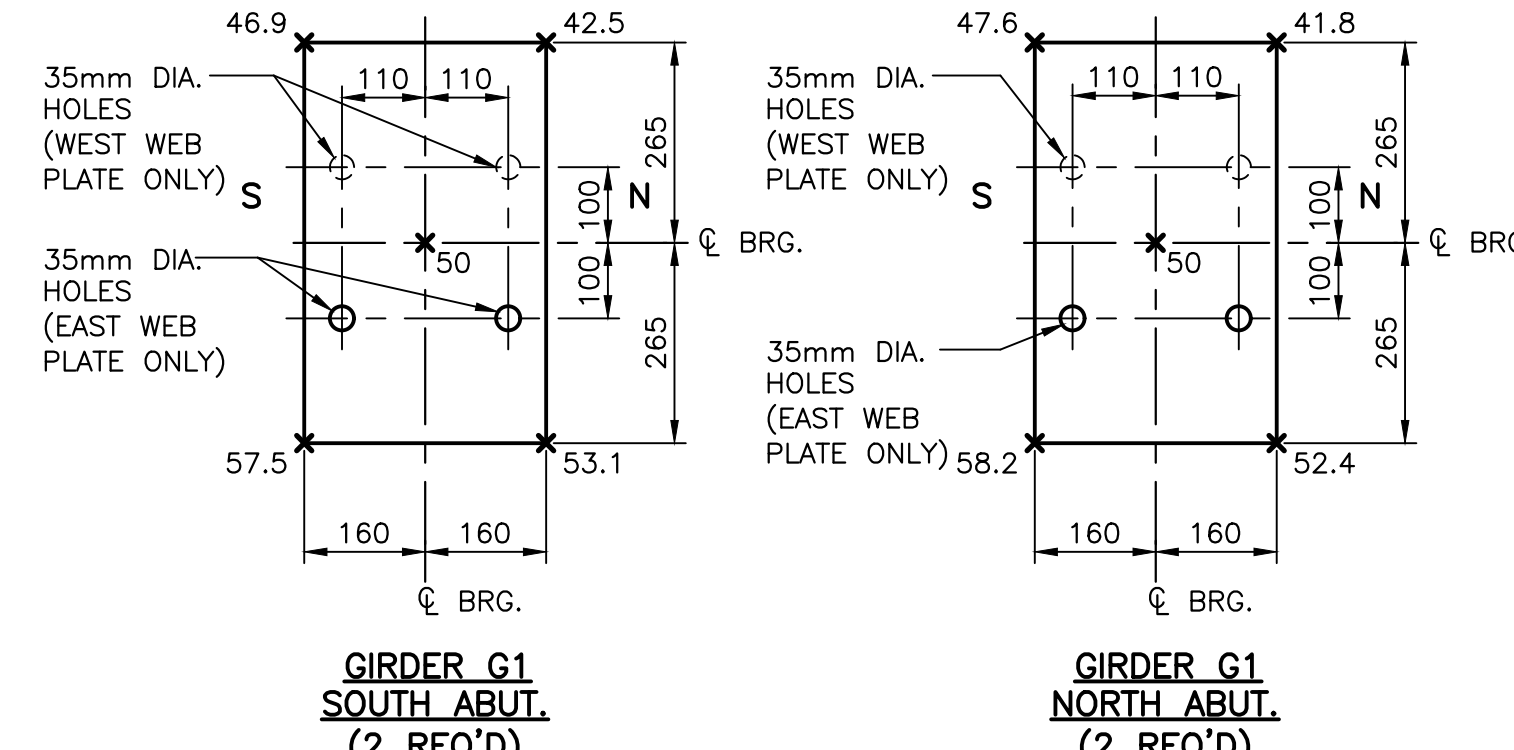
SECTION - TYPICAL PLINTH REINFORCING
SCALE: 1:10
0mm 100 200 300 400 500 600 700 800 900 1000mm



PLINTH GEOMETRY AT
INTERIOR WEBS
(G1E AND G2W)
SCALE: 1:10
0mm 100 200 300 400 500 600 700 800 900 1000mm



PLINTH GEOMETRY AT
EXTERIOR WEBS
(G1W AND G2E)
SCALE: 1:10
0mm 100 200 300 400 500 600 700 800 900 1000mm



NOTE:
BEARING PLATE, INCLUDING BOLT HOLES SHALL BE COATED TO
ENSURE GALVANIC ISOLATION BETWEEN ANCHOR BOLTS AND PLATE.

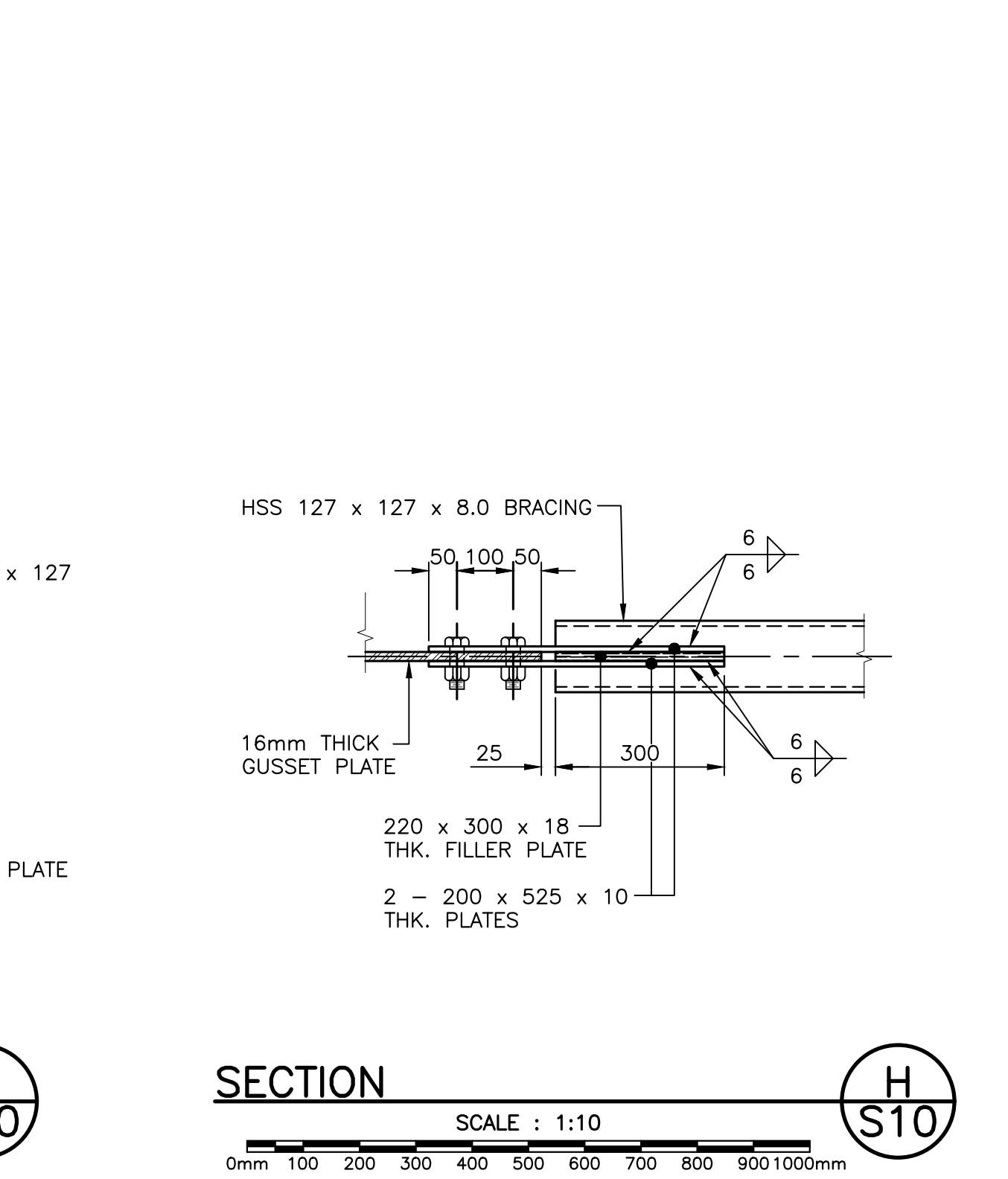
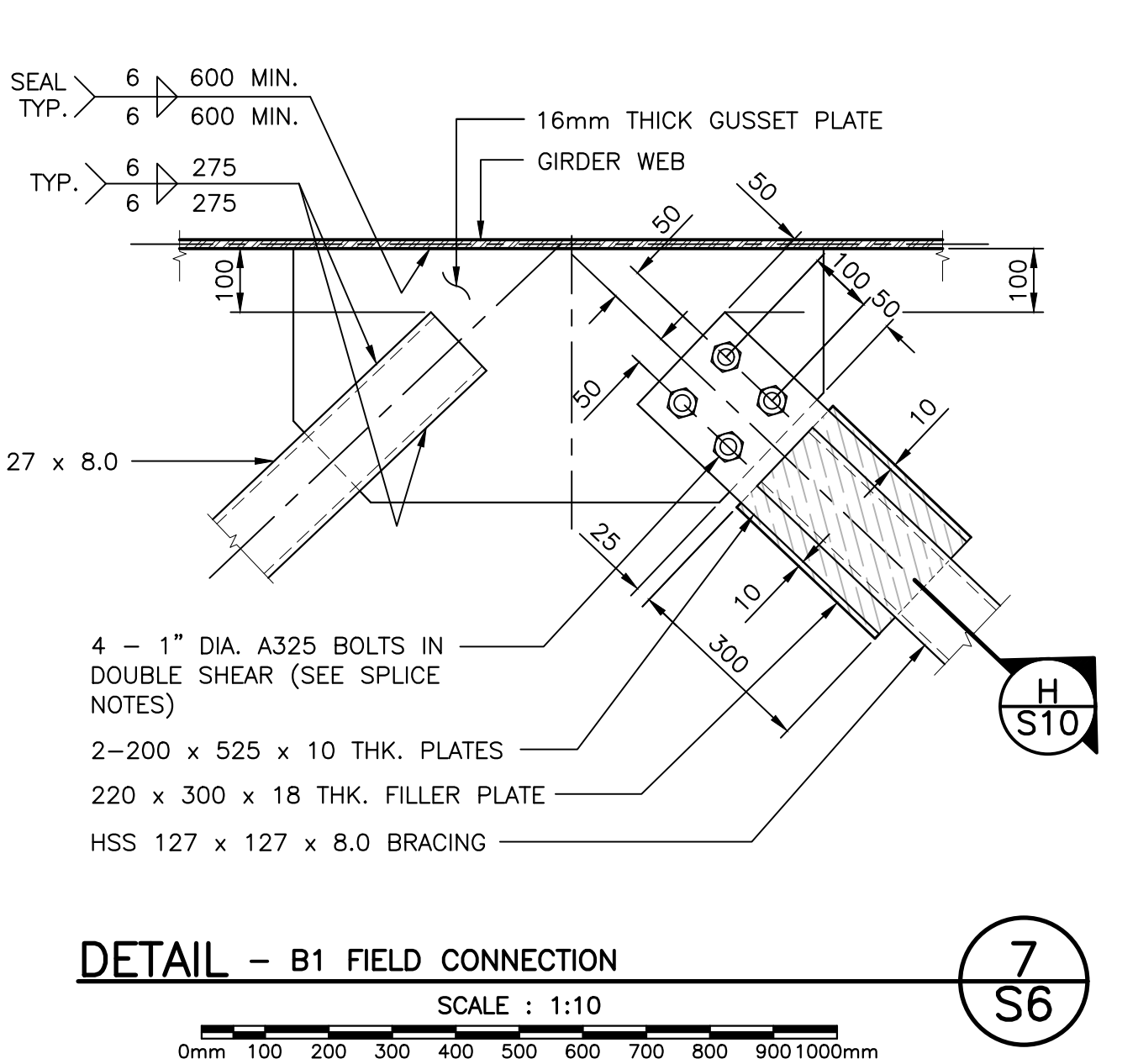
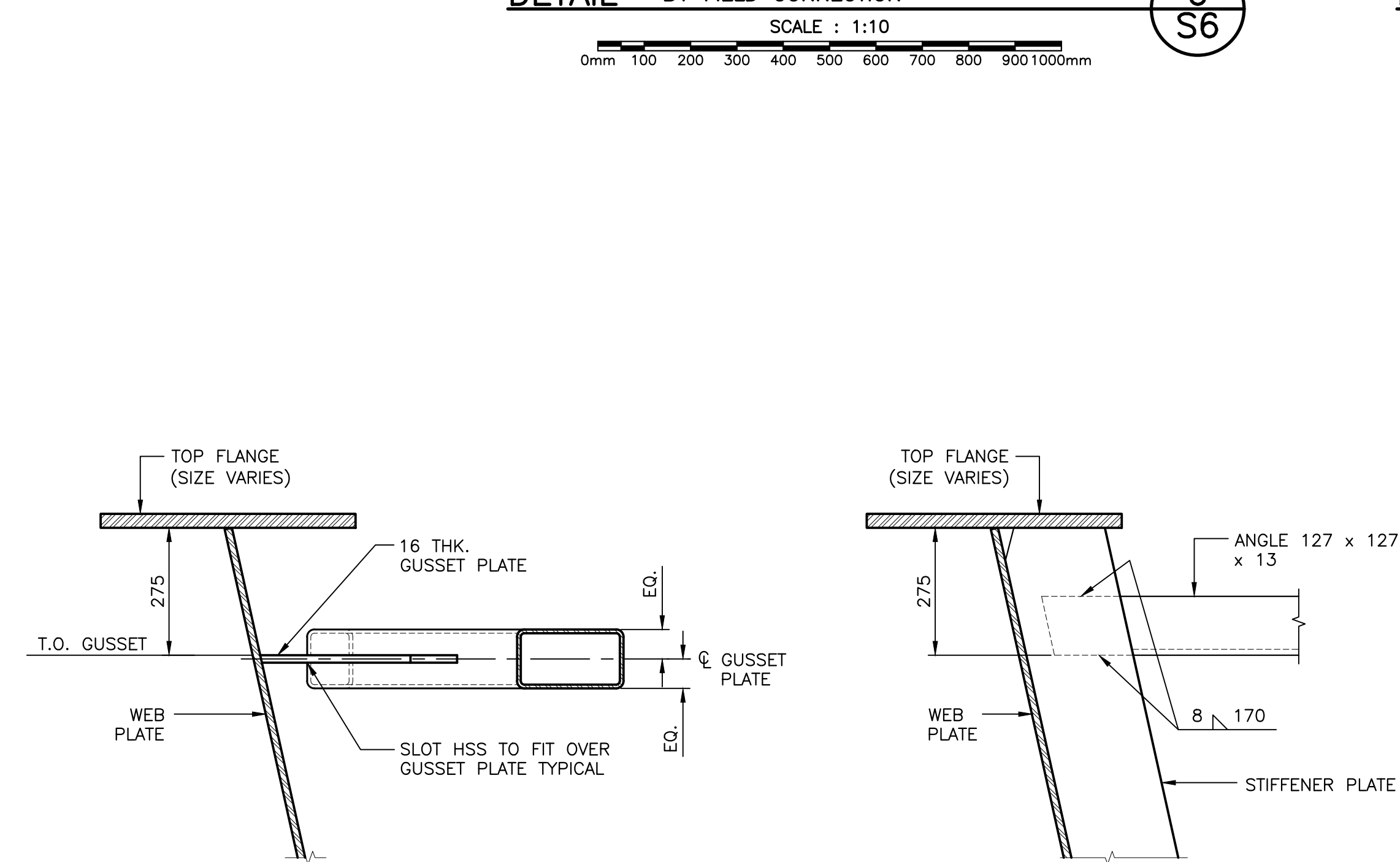
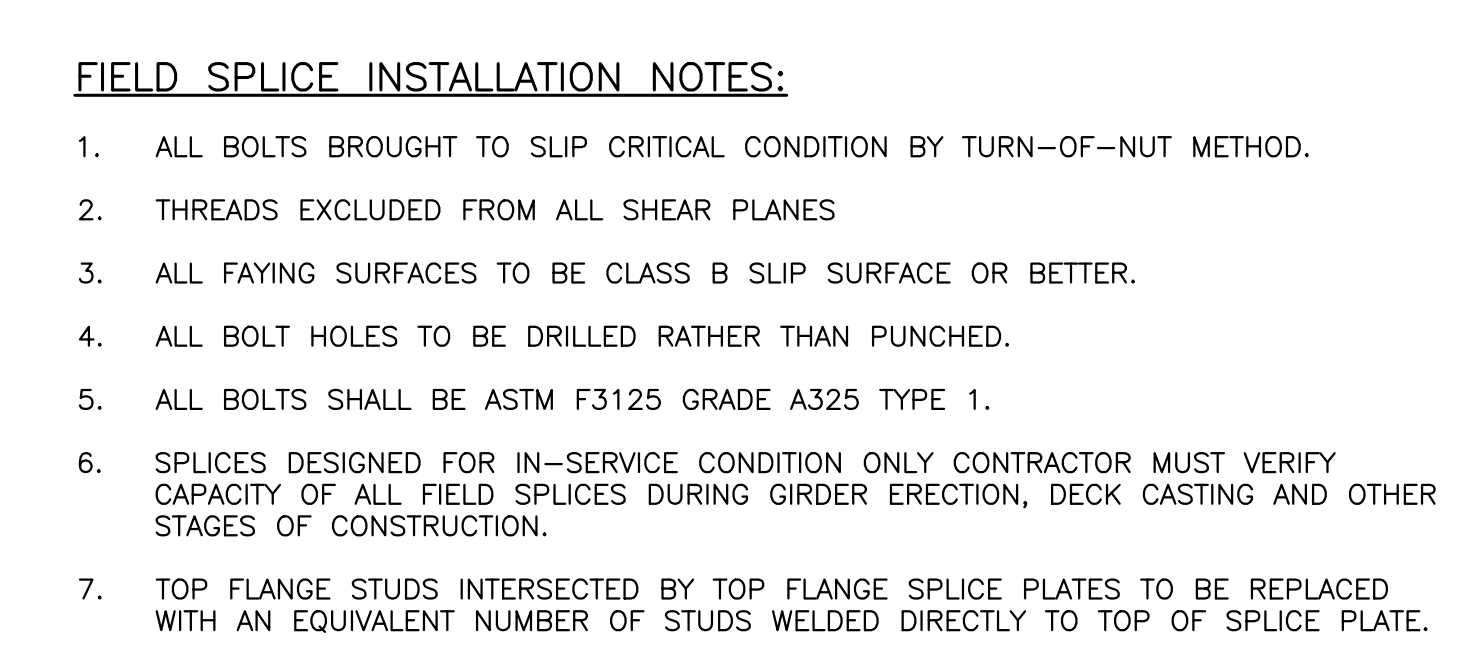
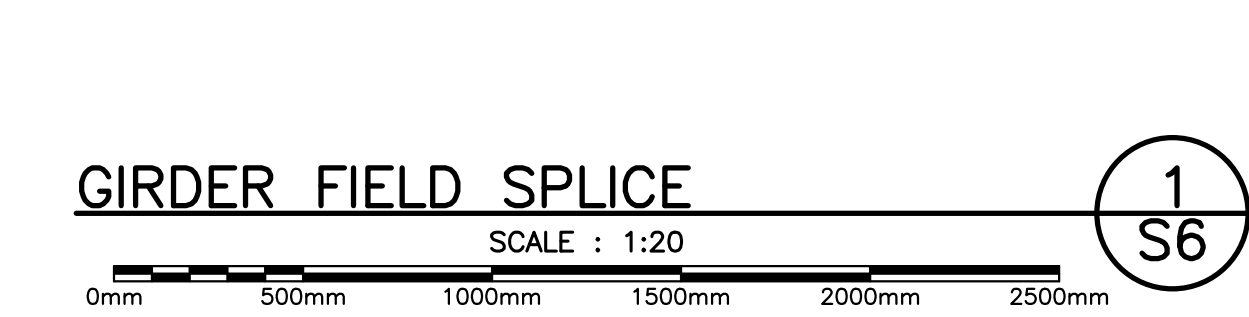
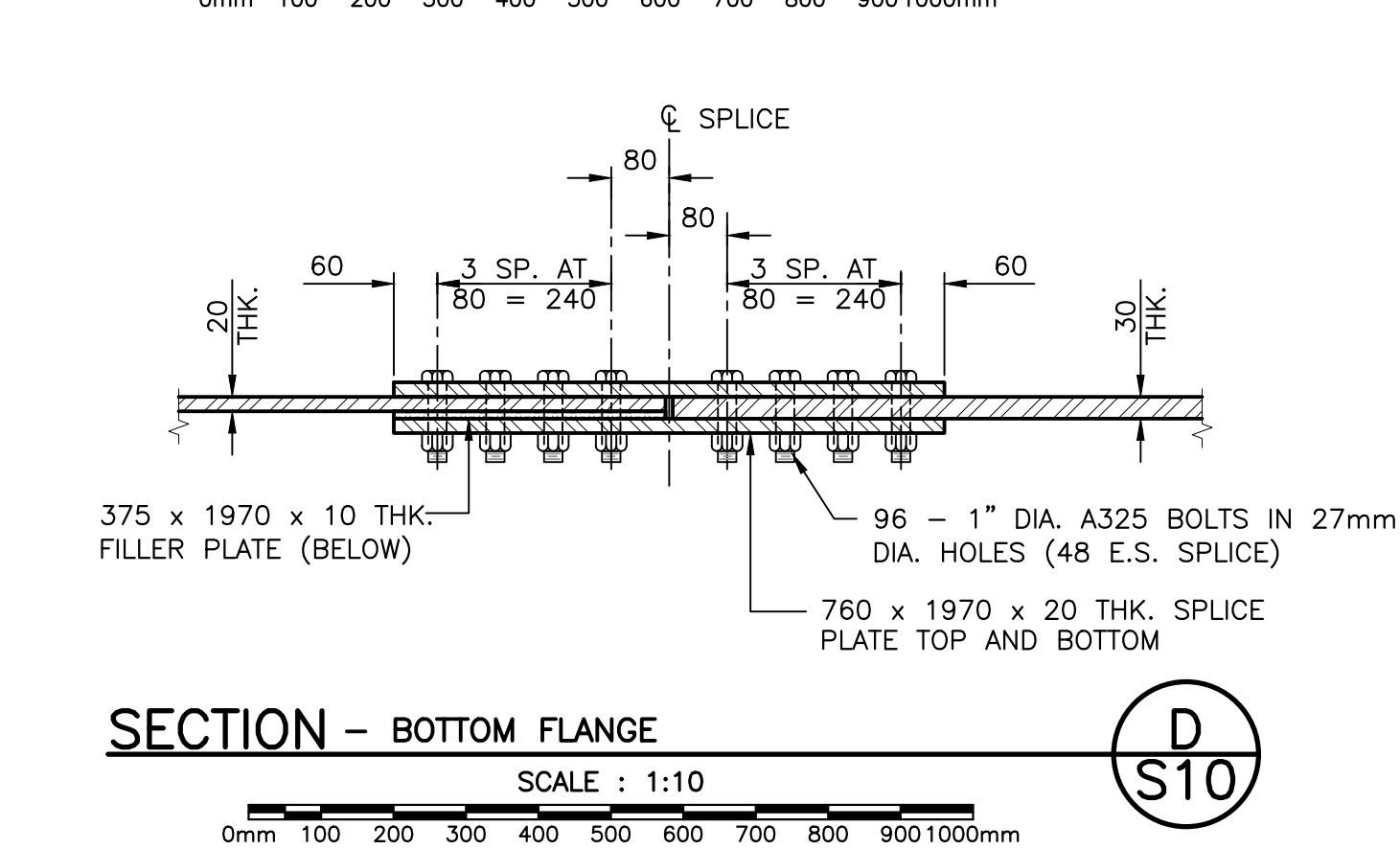
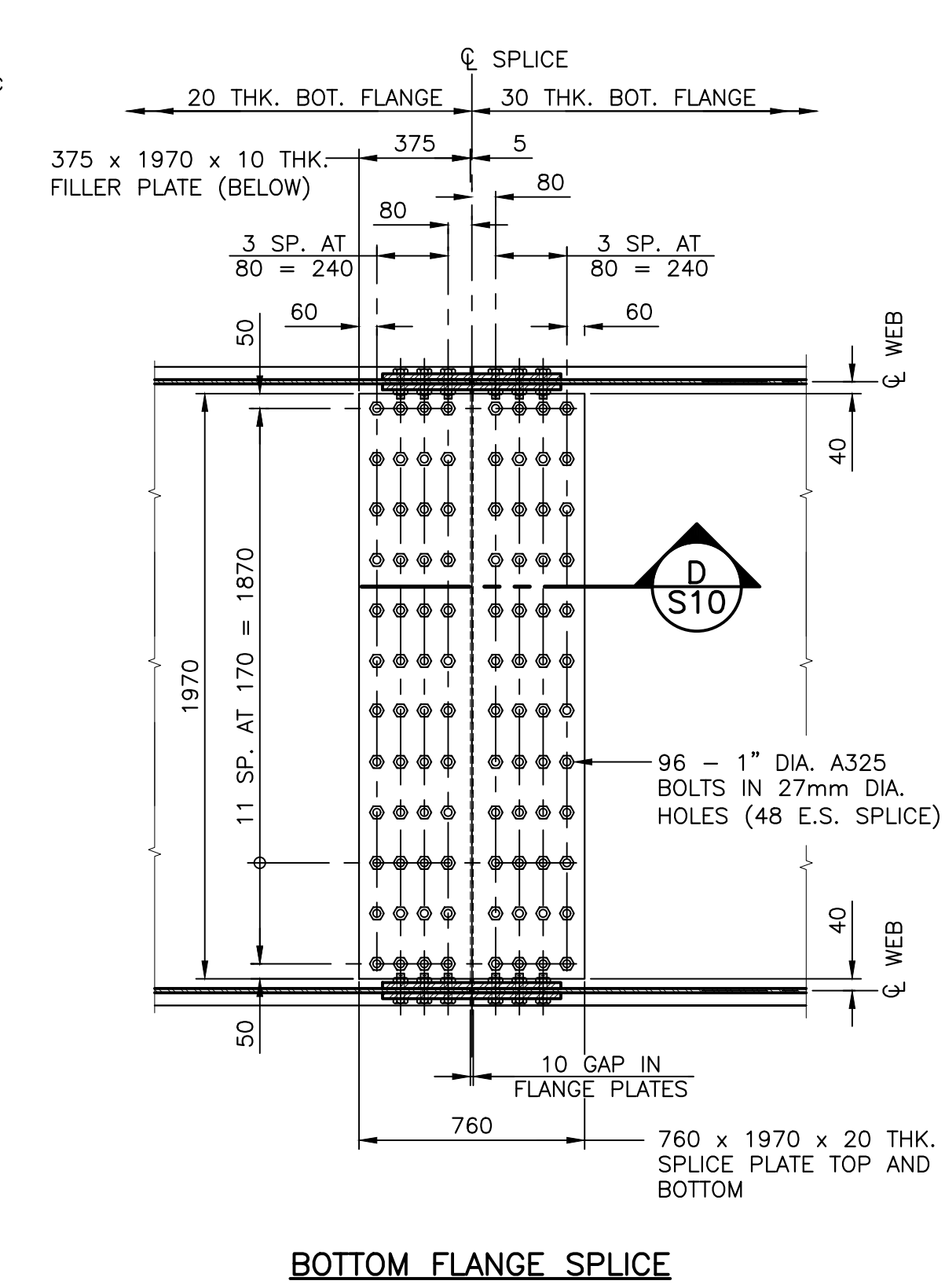
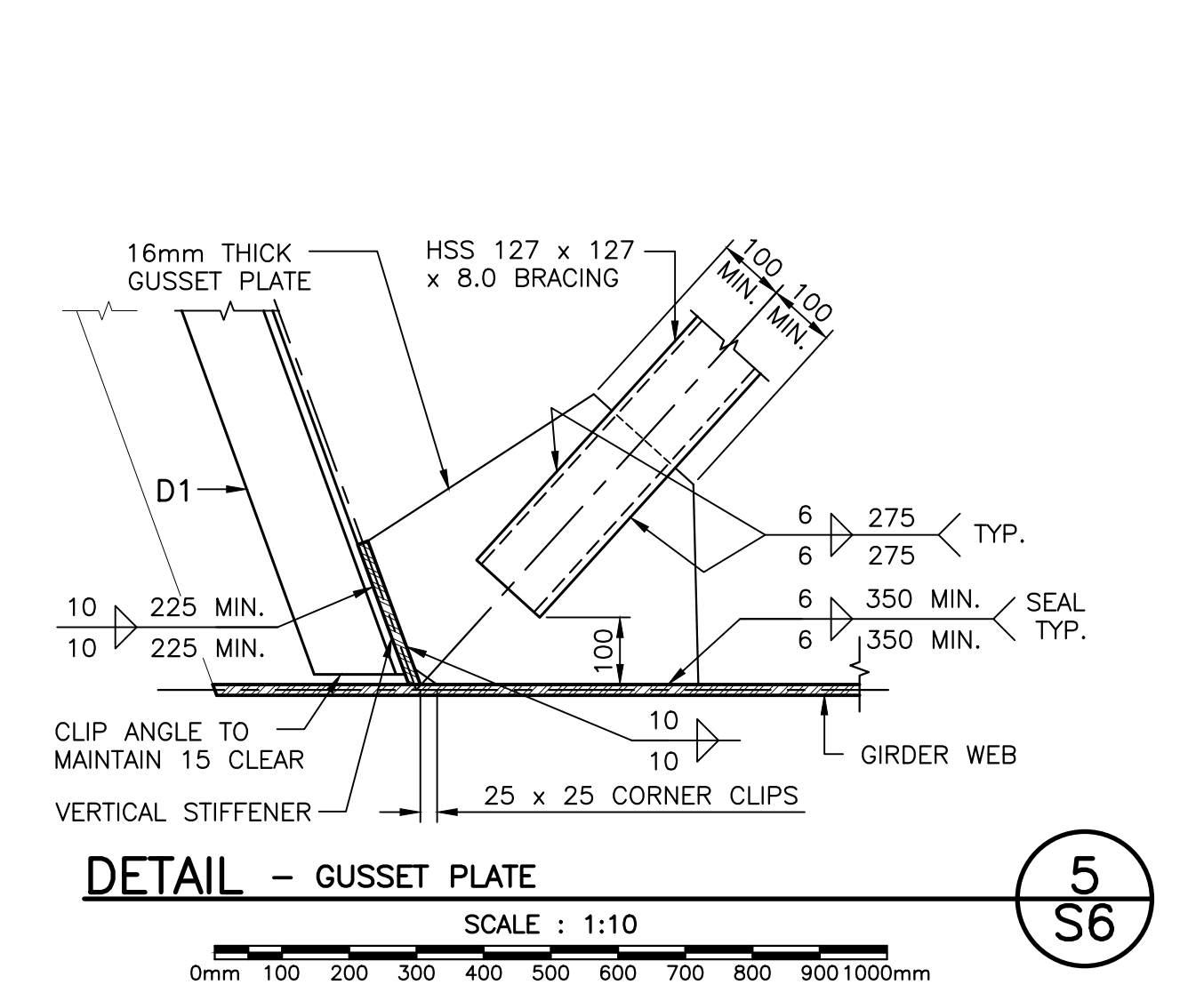
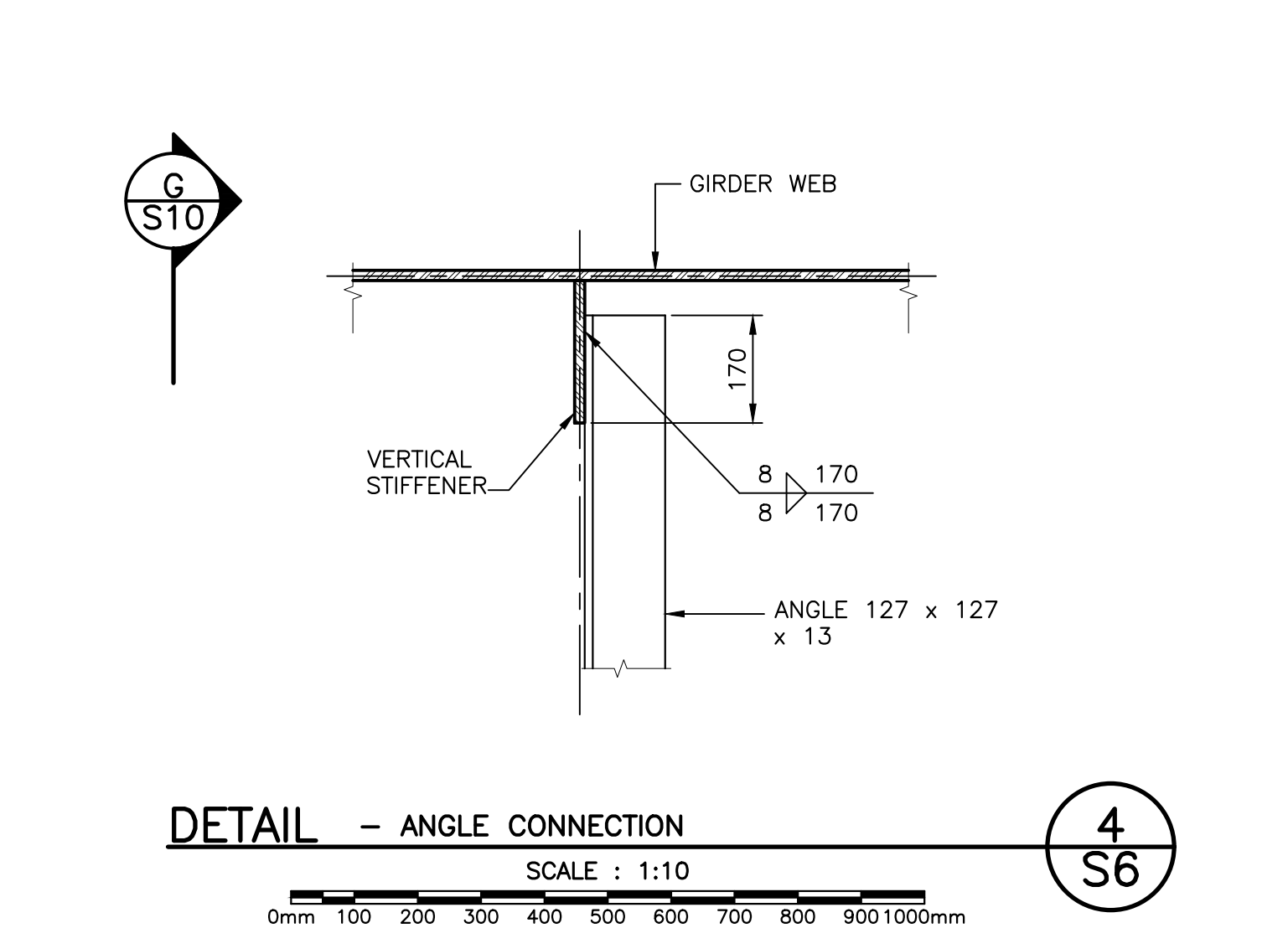
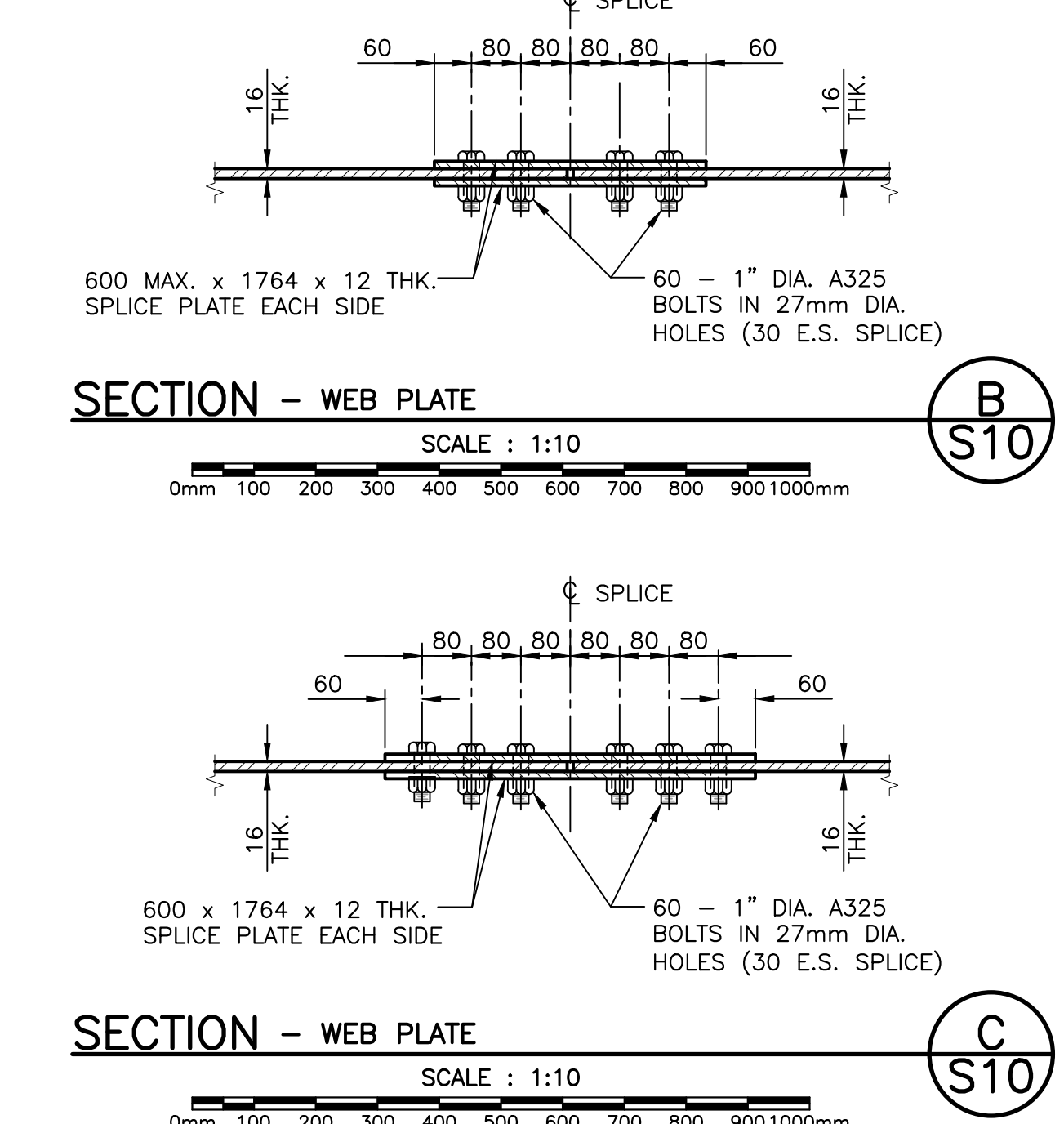
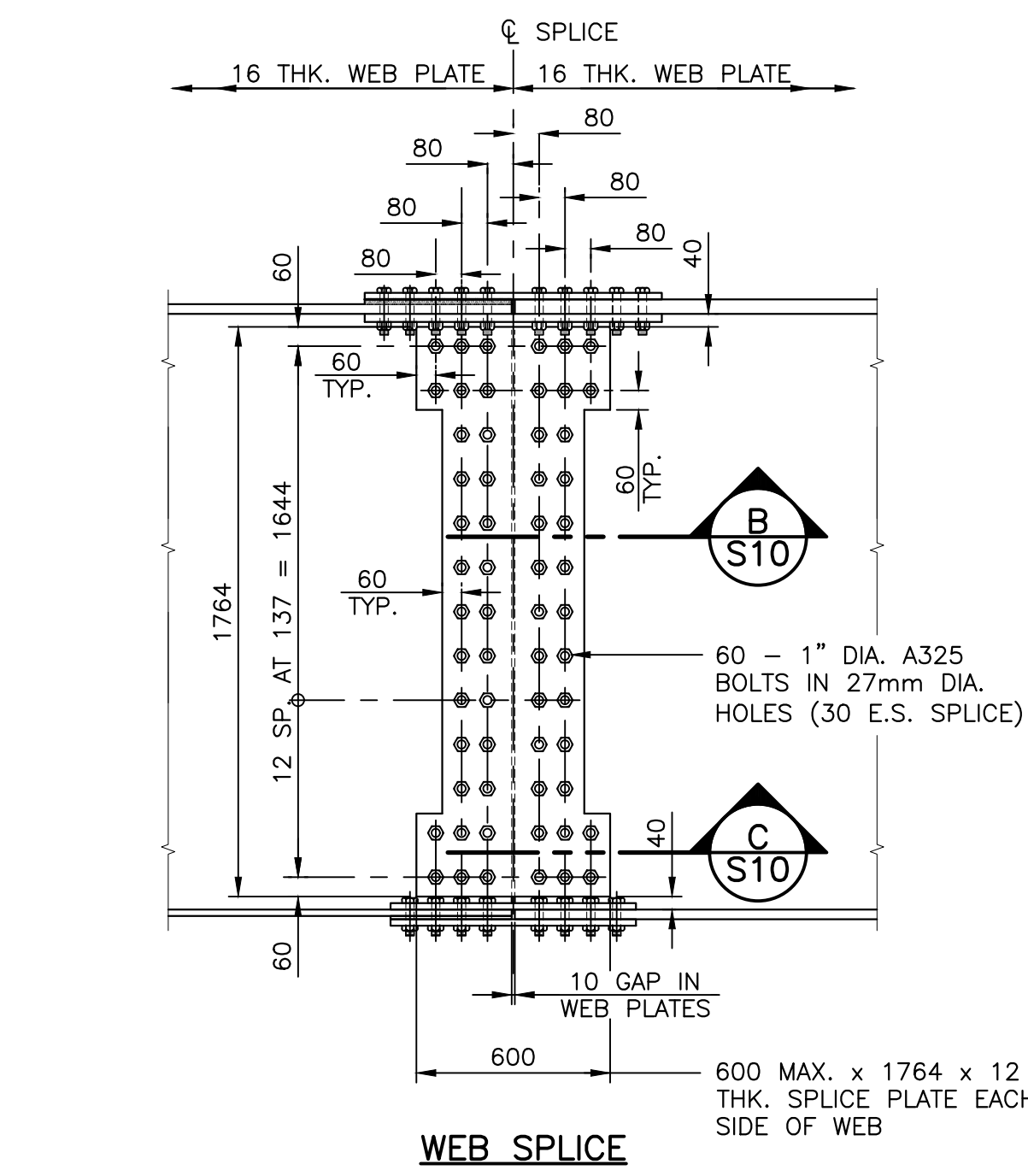
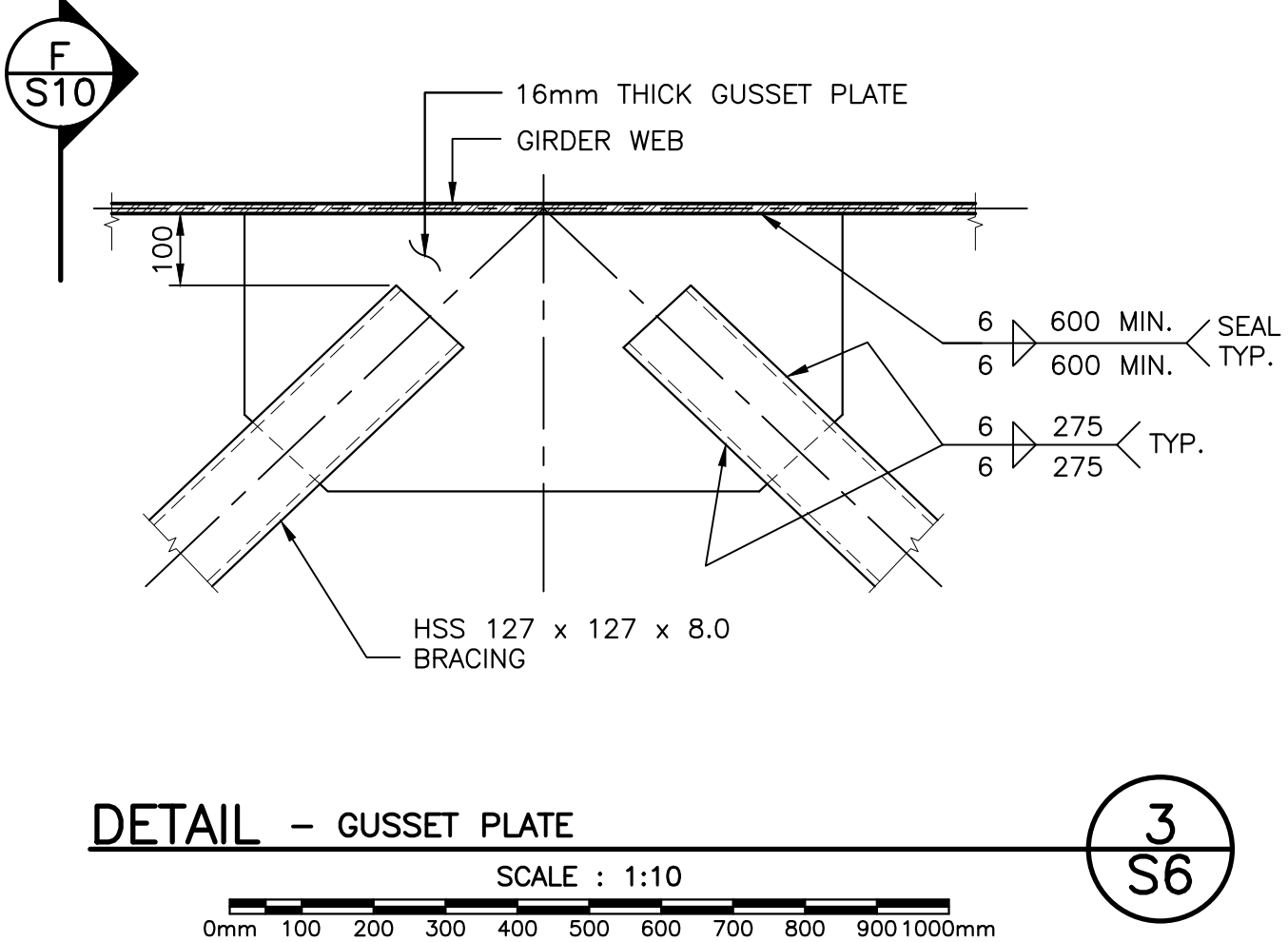
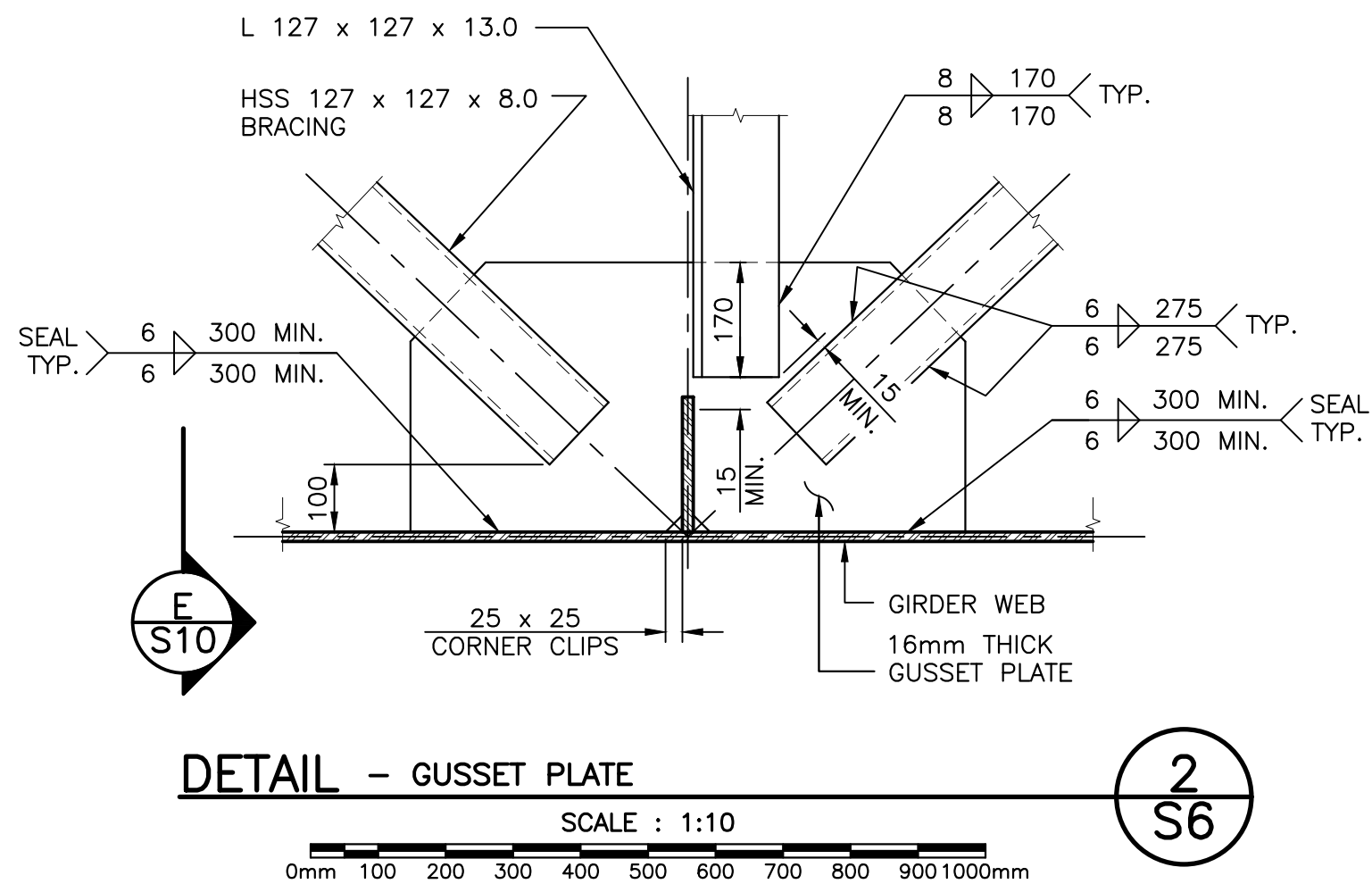
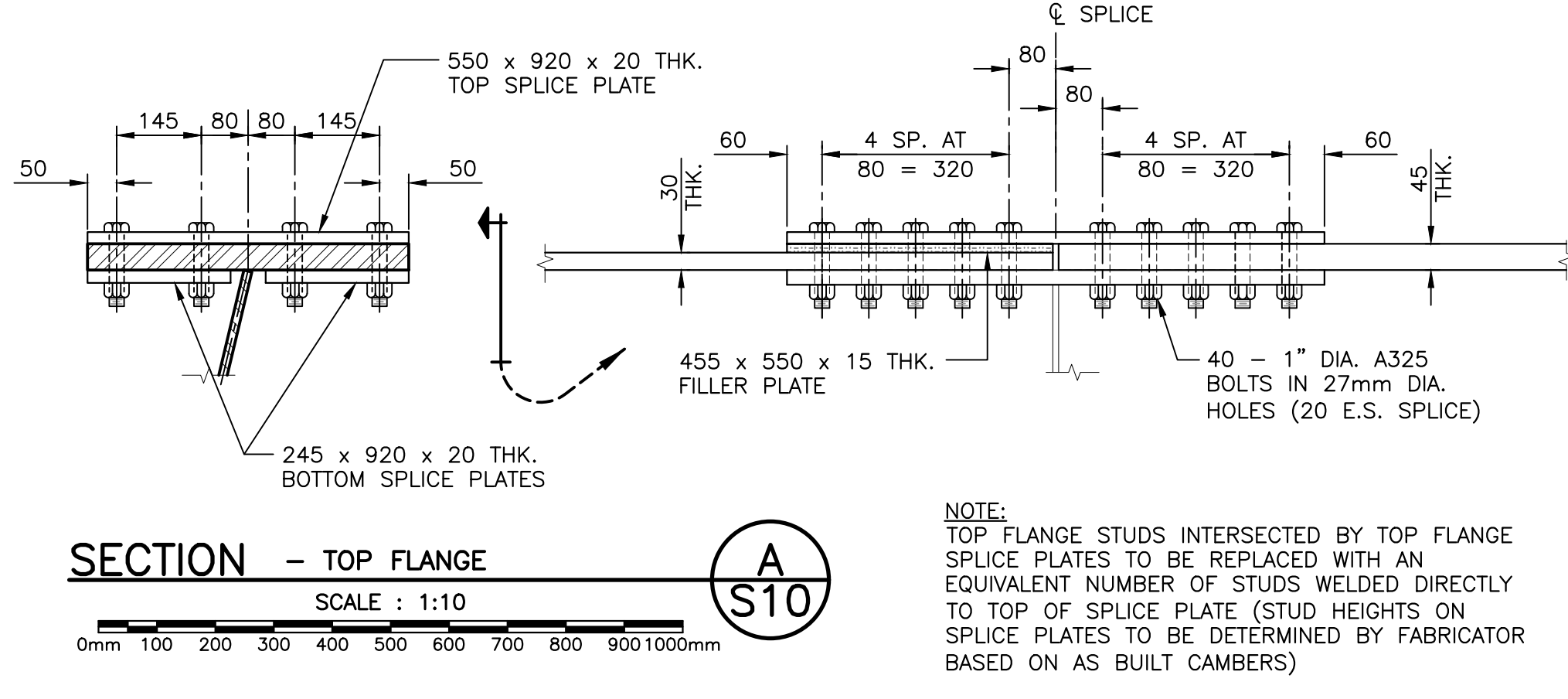
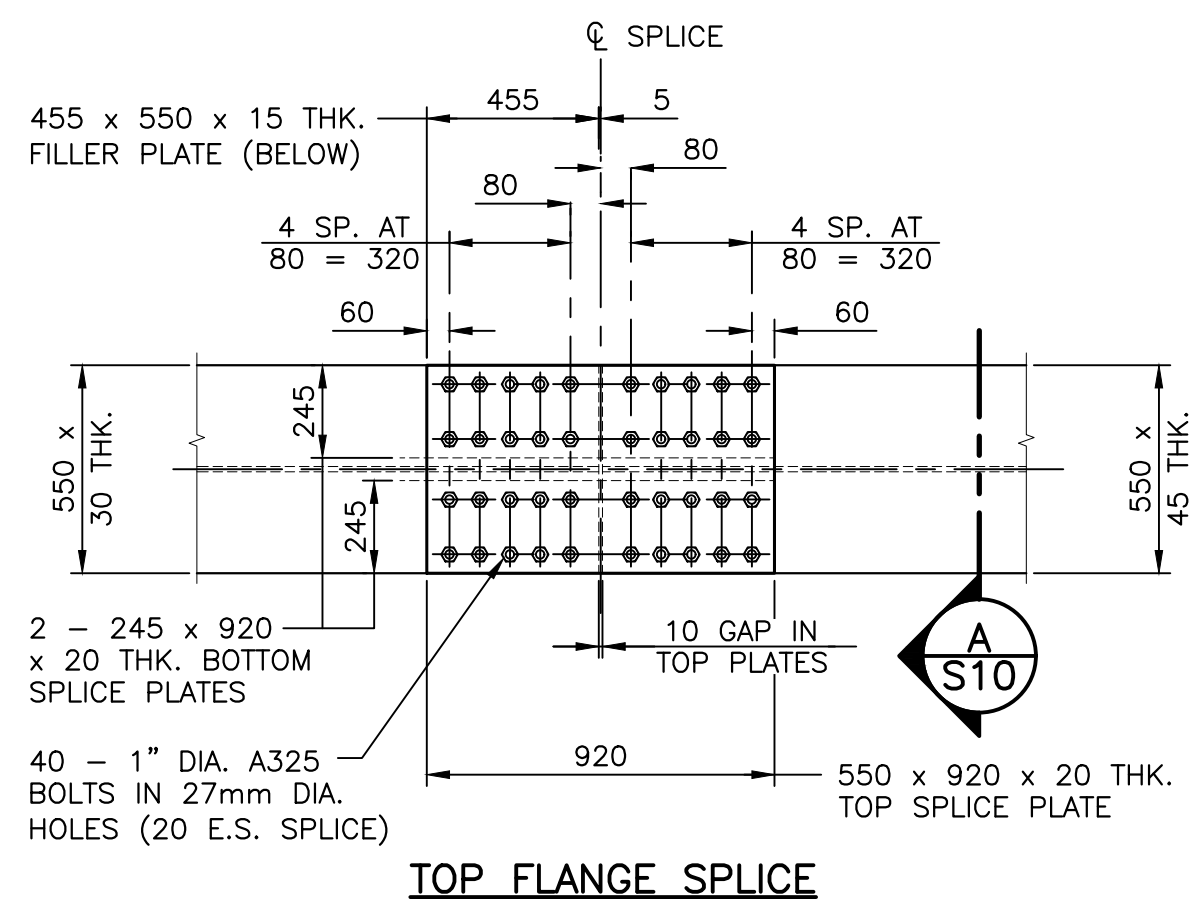
DETAIL - BEARING PLATE
SCALE: 1:10
0mm 100 200 300 400 500 600 700 800 900 1000mm

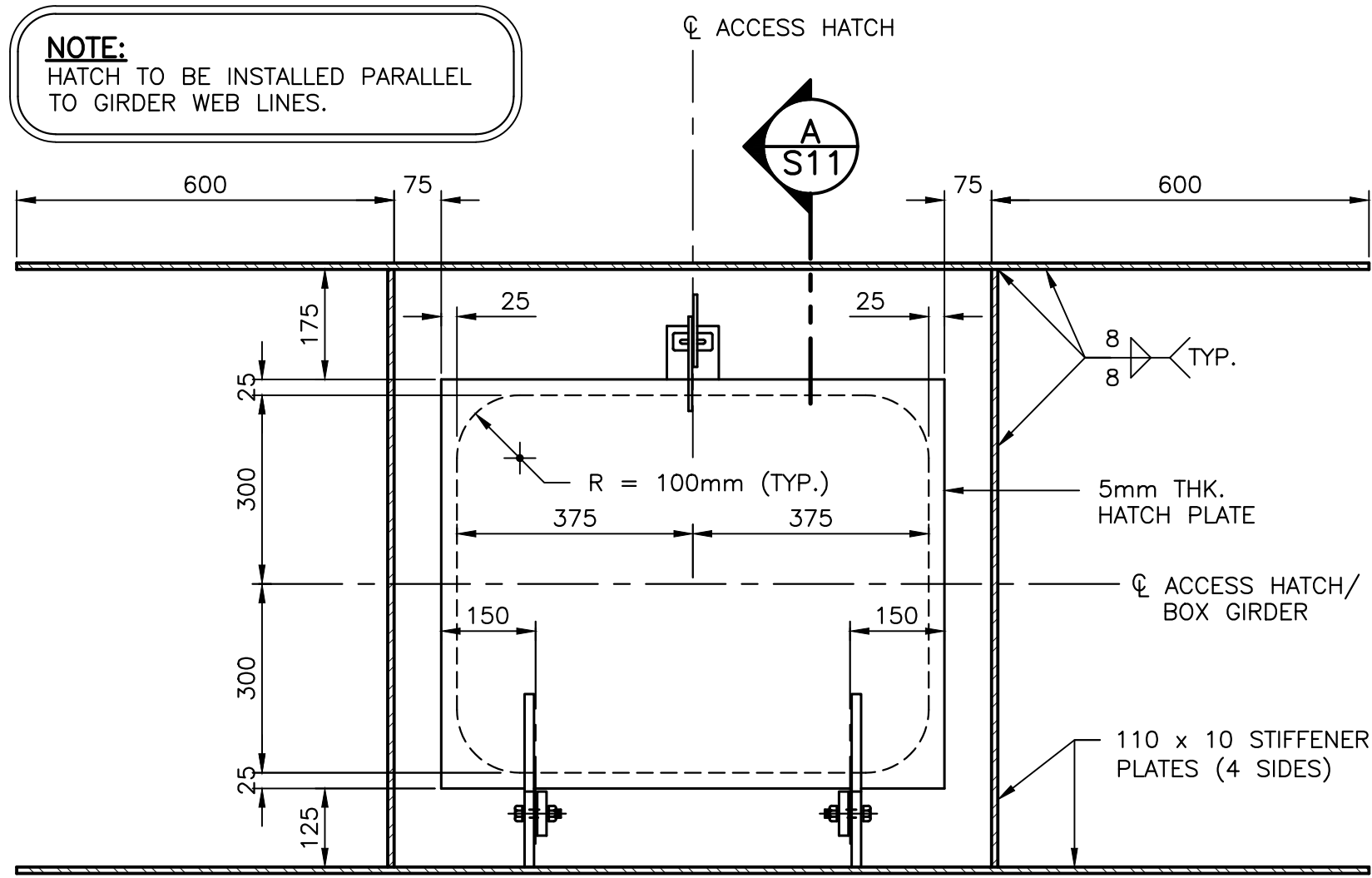
BEARING NOTES:

- CORRUGATED SLEEVES PROVIDE A FIT-UP TOLERANCE OF ± 35 mm BETWEEN THE BOX GIRDER BOTTOM FLANGE AND THE ANCHOR BOLTS.
- CAREFUL COORDINATION IS REQUIRED BETWEEN GENERAL CONTRACTOR AND STEEL FABRICATOR TO ENSURE CORRECT FIT-UP OF AS-BUILT BOX GIRDER WITH AS-BUILT ABUTMENT GEOMETRY (IN ELEVATION AND PLAN).
- ALL STEEL IN BEARING ASSEMBLES SHALL BE HOT DIPPED GALVANIZED (I.E. THREADED RODS, DOWELS, ALL NUTS AND WASHER PLATES).

GIRDER INSTALLATION PROCEDURE:

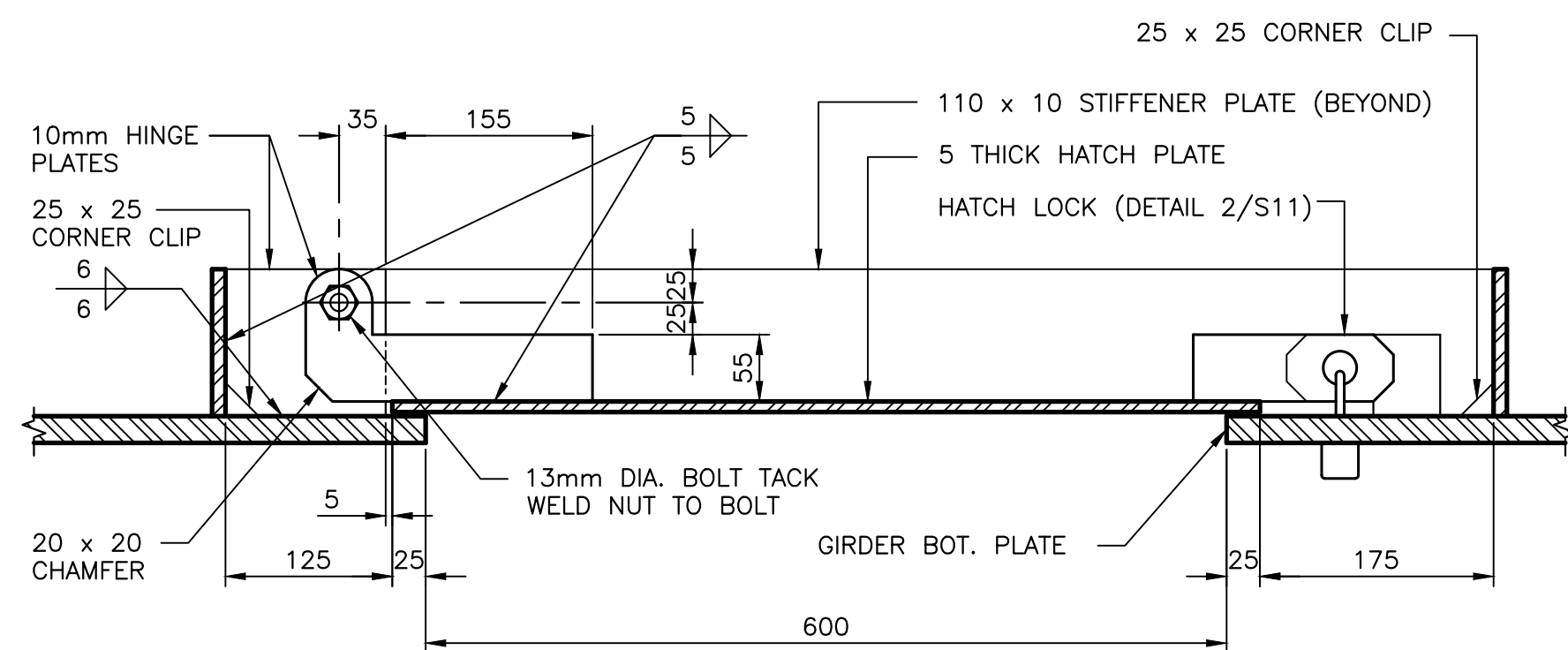
- CORRUGATED STEEL SLEEVE TO BE CUT OFF AT TOP OF CONCRETE PLINTH (PROVIDED FOR ANCHOR INSTALLATION TOLERANCE).
- BEARING PLATE TO BE SHIMMED AND HELD IN PLACE UNTIL GROUTING IS COMPLETE.
- DROP THREADED BARS INTO CORRUGATED STEEL SLEEVE THEN INSTALL GIRDER OVER PROTRUDING THREADED BARS. PLACE GIRDER IN PROPER/FINAL POSITION.
- FILL SLEEVES WITH GROUT TO TOP OF BEARING PLATE ENSURING THAT ALL VOLUME BETWEEN BEARING PLATE AND CONCRETE PLINTH IS FILLED WITH GROUT. IMMEDIATELY FOLLOW BY INSTALLING PLATE WASHERS AND TWO TOP NUTS. THREAD FIRST NUT UNTIL NUT IS 3mm CLEAR/ABOVE PLATE WASHER SITTING ON GIRDER BOTTOM PLATE THEN TIGHTEN SECOND LOCK NUT TO BOTTOM NUT TO MAINTAIN 3mm CLEARANCE.





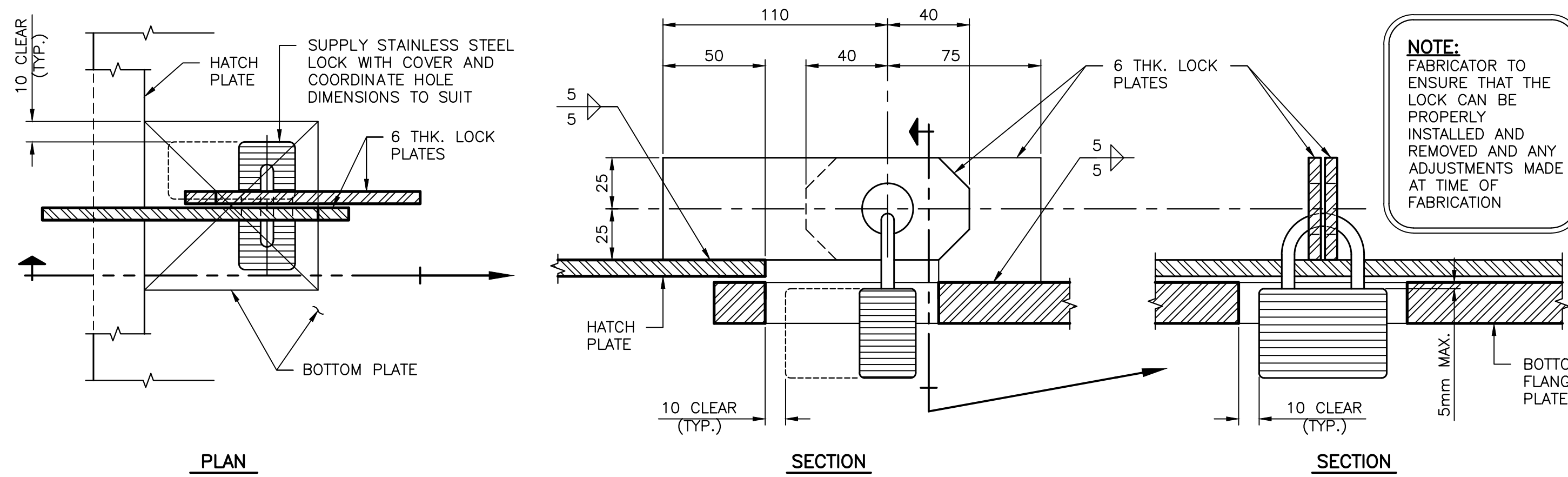
DETAIL - ACCESS HATCH PLAN
SCALE : 1:10
0mm 100 200 300 400 500 600 700 800 900 1000mm

1
S6



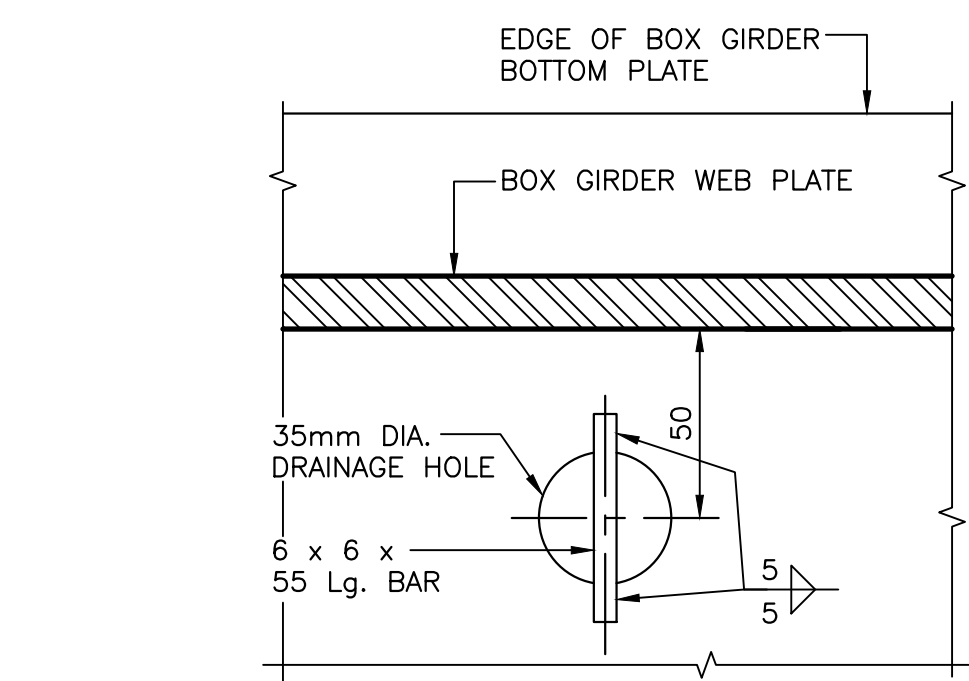
SECTION
SCALE : 1:5
0mm 100mm 200mm 300mm 400mm 500mm

A
S11



DETAIL - ACCESS HATCH LOCK
SCALE : 1:2
0mm 50mm 100mm 150mm 200mm 250mm

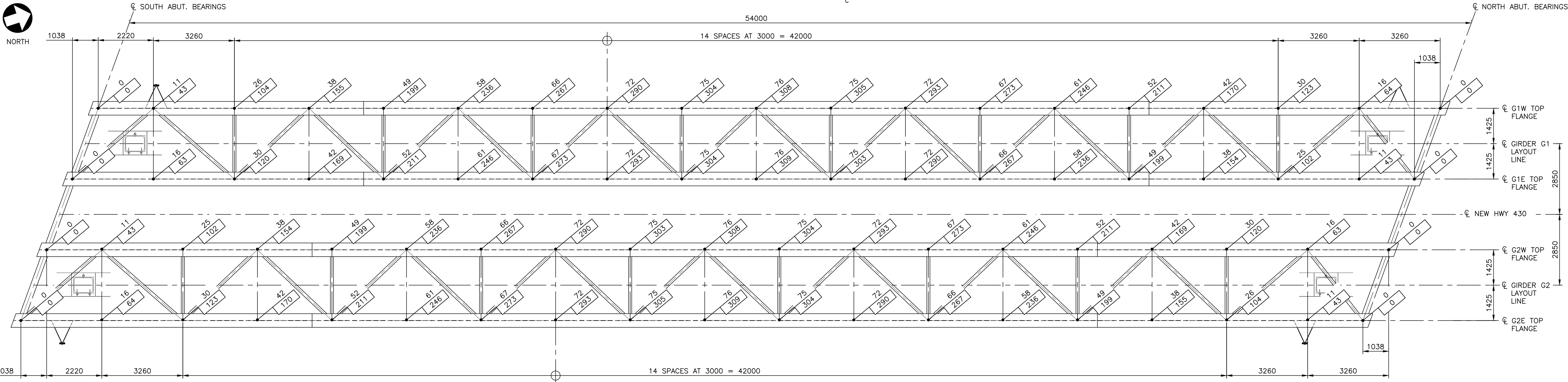
2
S11



DETAIL - GIRDER DRAINAGE HOLE
SCALE : 1:2
0mm 50mm 100mm 150mm 200mm 250mm

3
S9

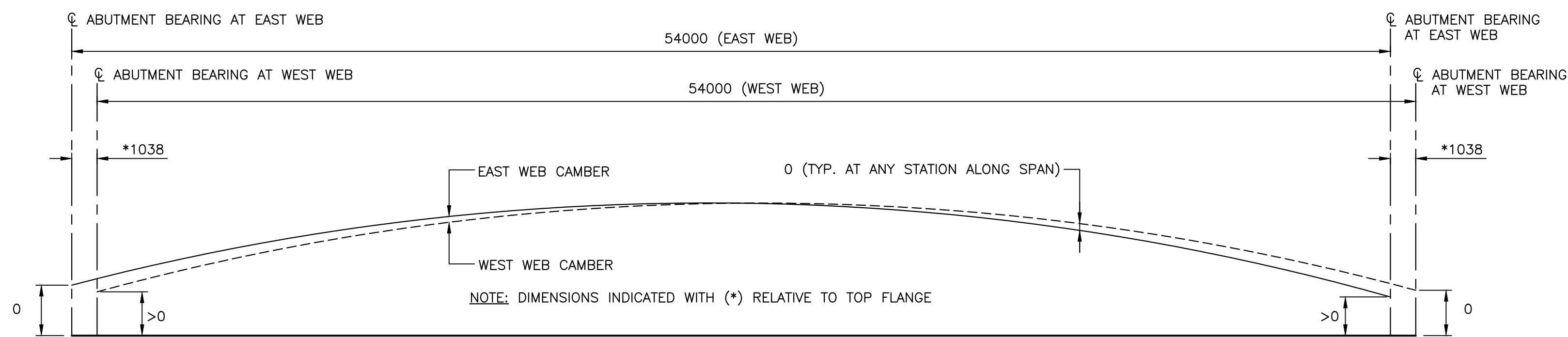
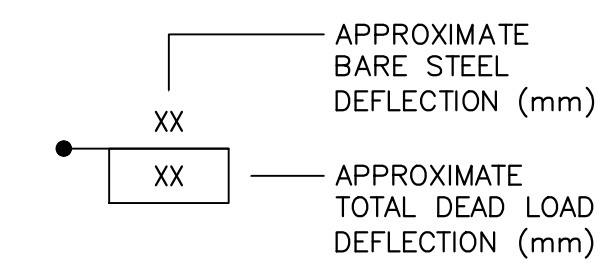
0	ISSUED FOR TENDER	JULY 19 2021
revisions		date
project	WESTERN BROOK BRIDGE REPLACEMENT GROS MORNE NATIONAL PARK	
drawing	GIRDER MISCELLANEOUS DETAILS	
designed	WADE POTTIE	conçu
date	JANUARY, 2020	
drawn	WAYNE MORROW	dessiné
date	JANUARY, 2020	
approved	ROBBIE FRASER	approuvé
date		
Tender		Soumission
PWOSC Project Manager	Administrateur de projets TPSC	
project number	no. du projet	
drawing no.	no. du dessin	



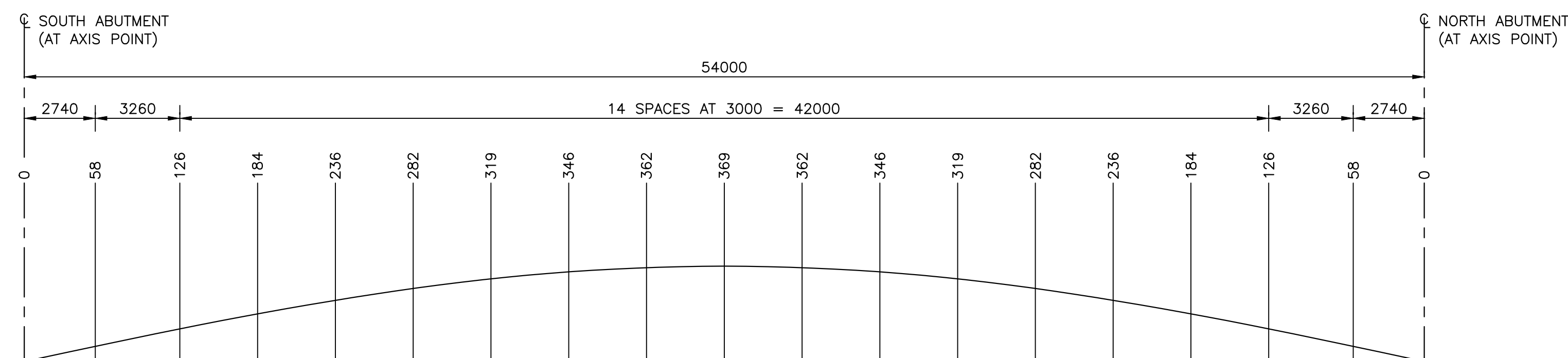
DEAD LOAD DEFLECTION DIAGRAM



SELF WEIGHT AND TOTAL DEAD LOAD DEFLECTIONS ARE GIVEN ALONG WEB LINE (TOP OF WEB) WITH POSITION ALONG SPAN AS INDICATED ON PLAN, RELATIVE TO CENTERLINE OF INDIVIDUAL WEB.



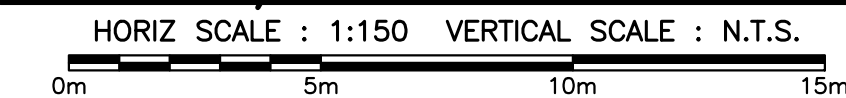
WEB CUT COMPARISON DIAGRAM



BOX GIRDER WEB CUTS (TYPICAL GIRDER G1 AND G2)

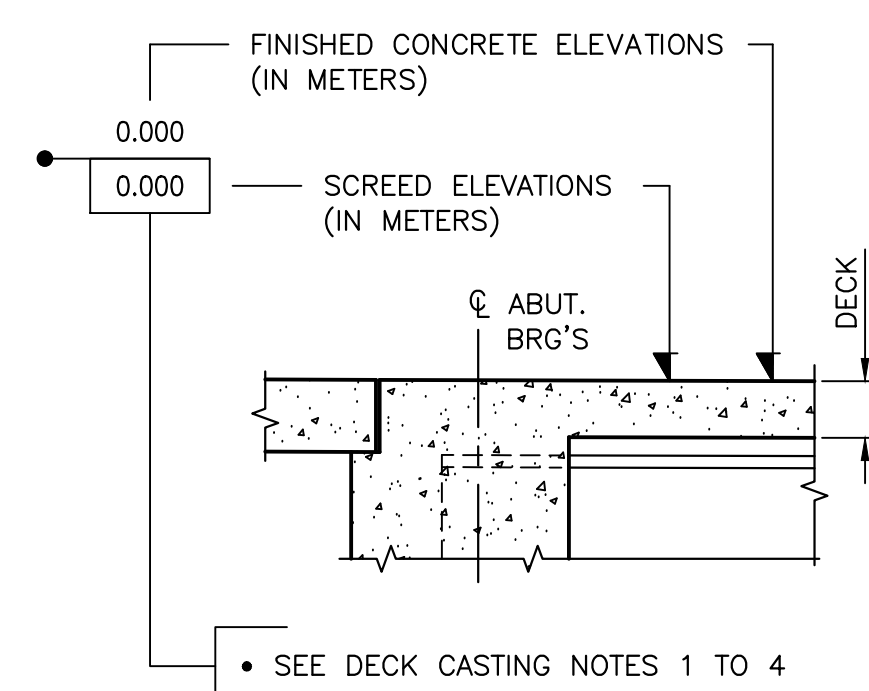
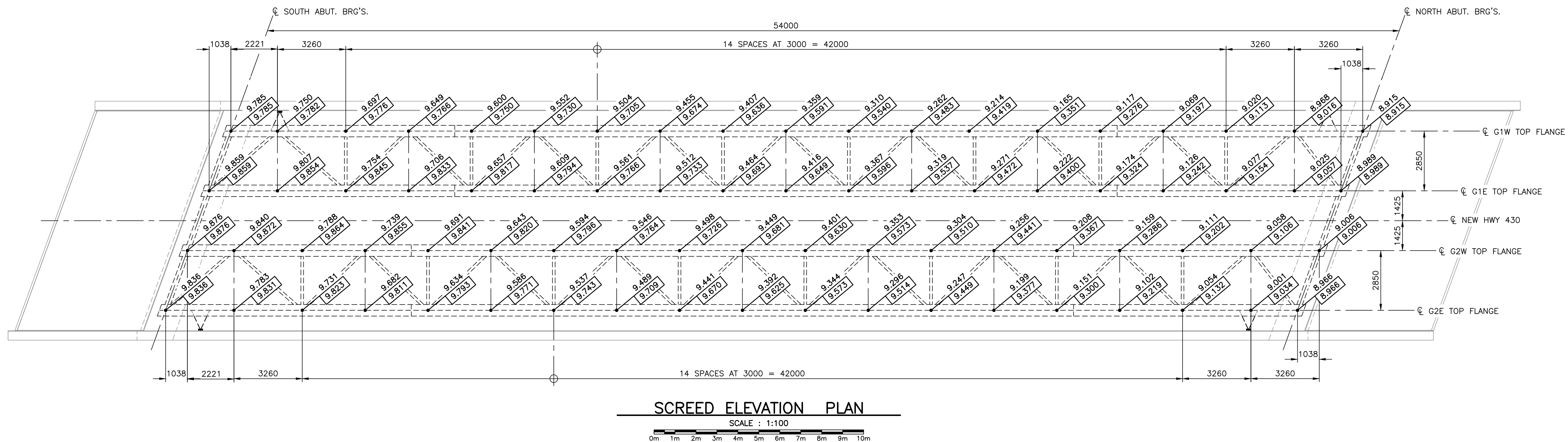
NOTE: WEB CUT DIAGRAMS ARE RELATIVE TO CENTERLINE BOX WITH WEBS CAMBERED BY SAME AMOUNT AT EACH LOCATION (STATION) ALONG SPAN TO ENSURE FIT-UP OF BOTTOM PLATE TO U/S WEBS (REFER TO WEB CUT COMPARISON DIAGRAM).

CAMBER/WEB CUT DIAGRAMS



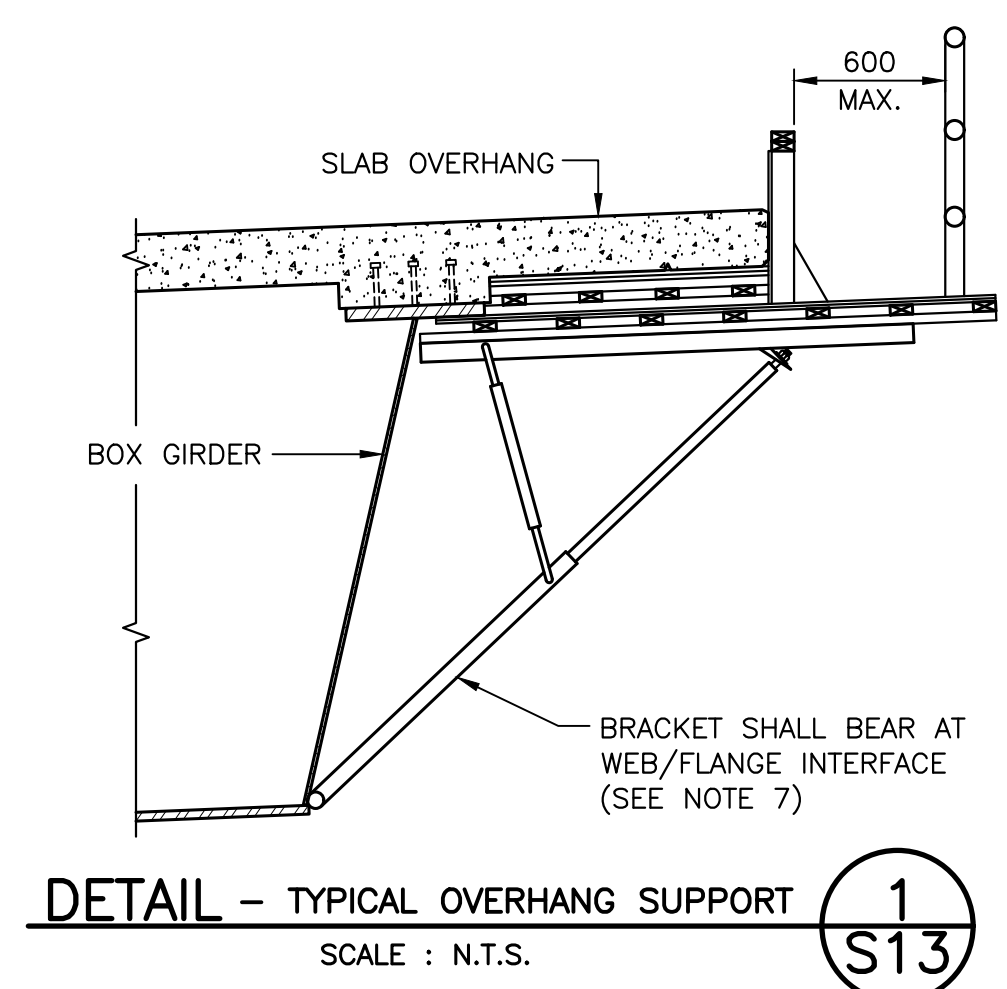
0	ISSUED FOR TENDER	JULY 19, 2021
revisions		date
project	WESTERN BROOK BRIDGE REPLACEMENT GROS MORNE NATIONAL PARK	
drawing	GIRDER CAMBER/ WEB CUT DIAGRAMS	


designed	WADE POTTIE	conçu
date	JANUARY, 2020	
drawn	WAYNE MORROW	dessiné
date	JANUARY, 2020	
approved	ROBBIE FRASER	approuvé
date		
Tender		Soumission
PWOSC Project Manager	Administrateur de projets TP50C	
project number	no. du projet	
drawing no.	no. du dessin	

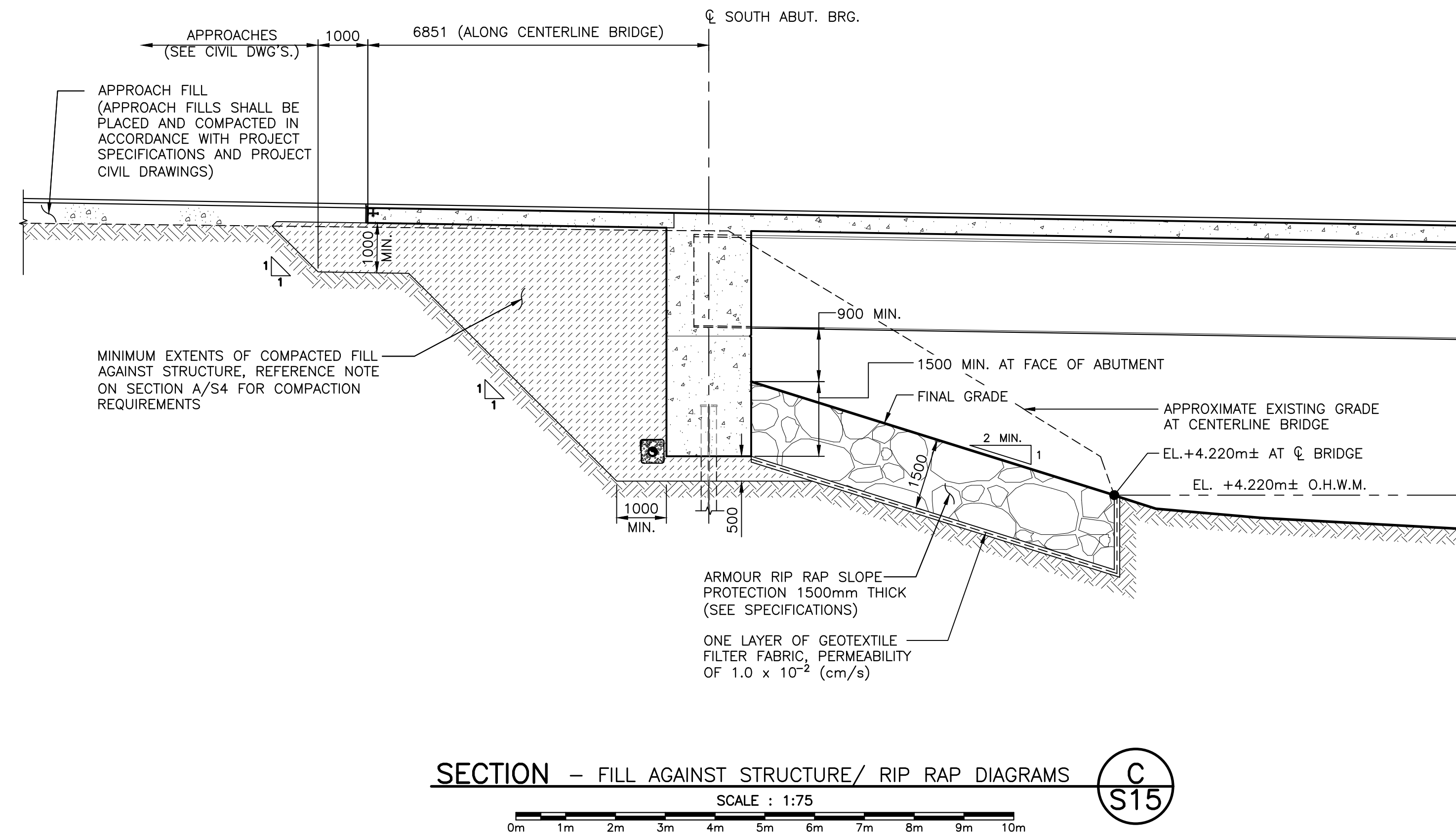
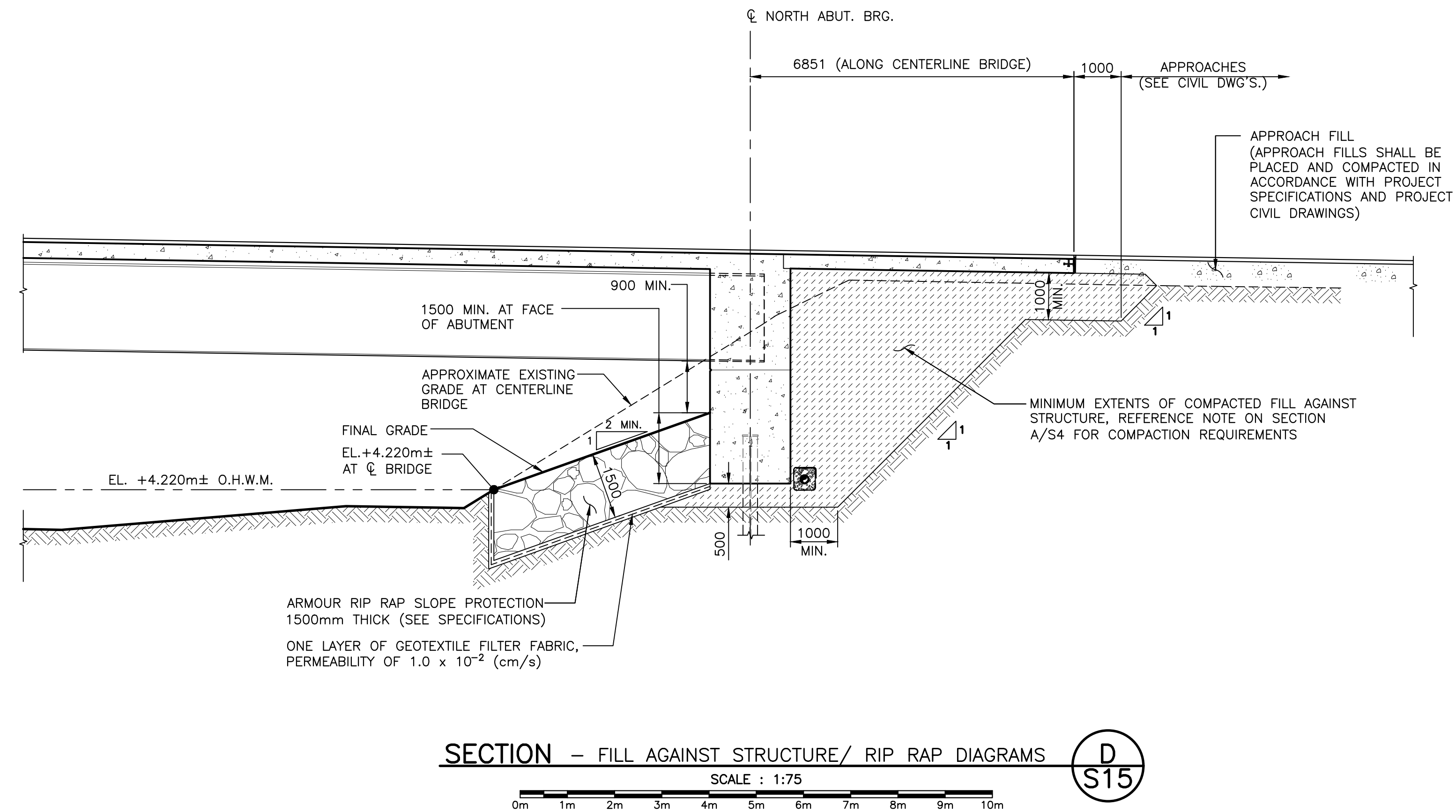
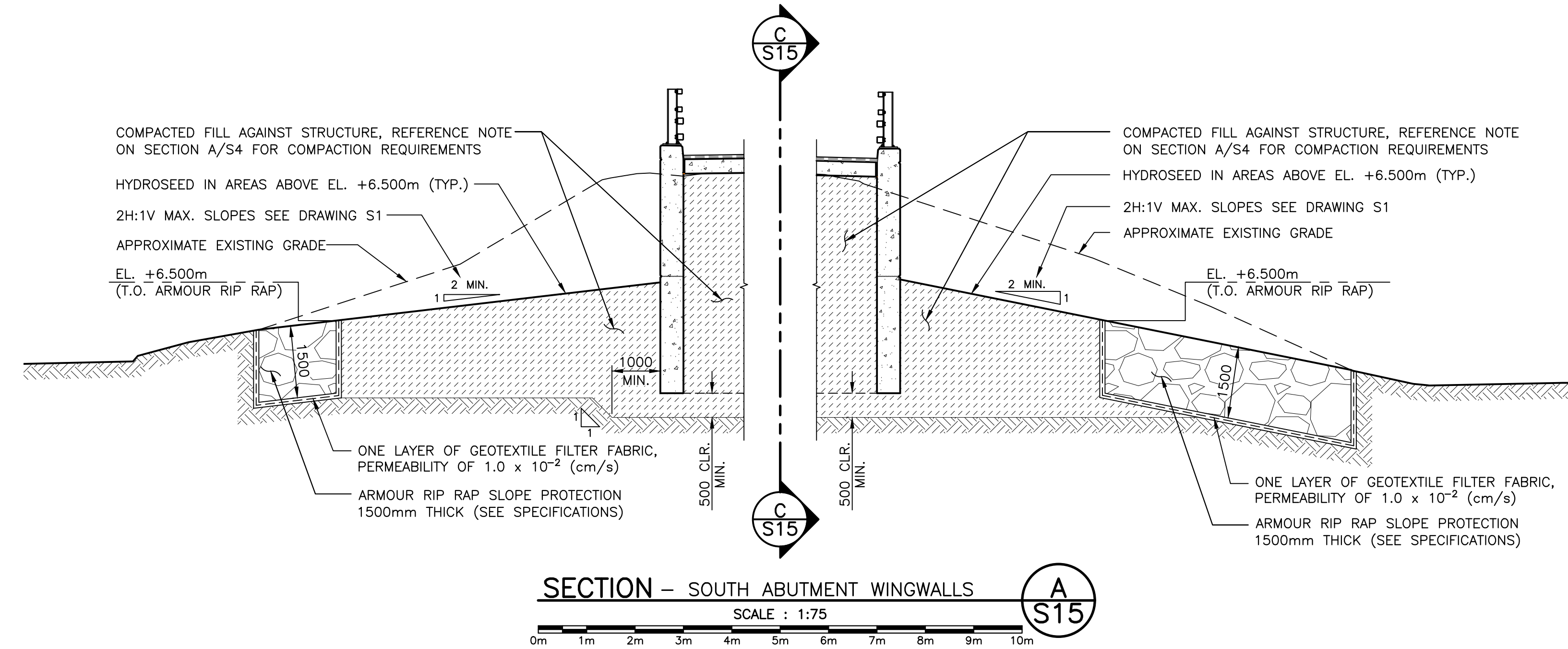
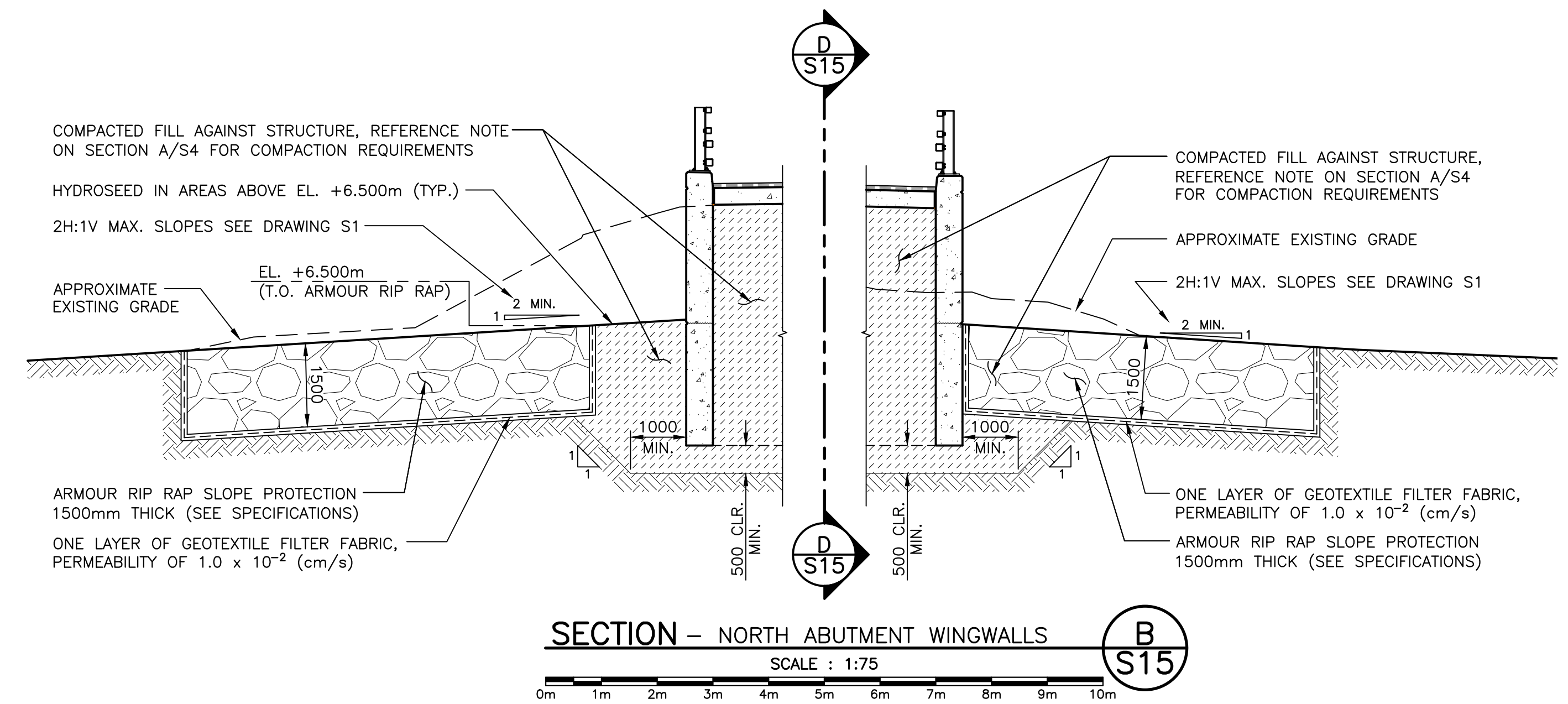


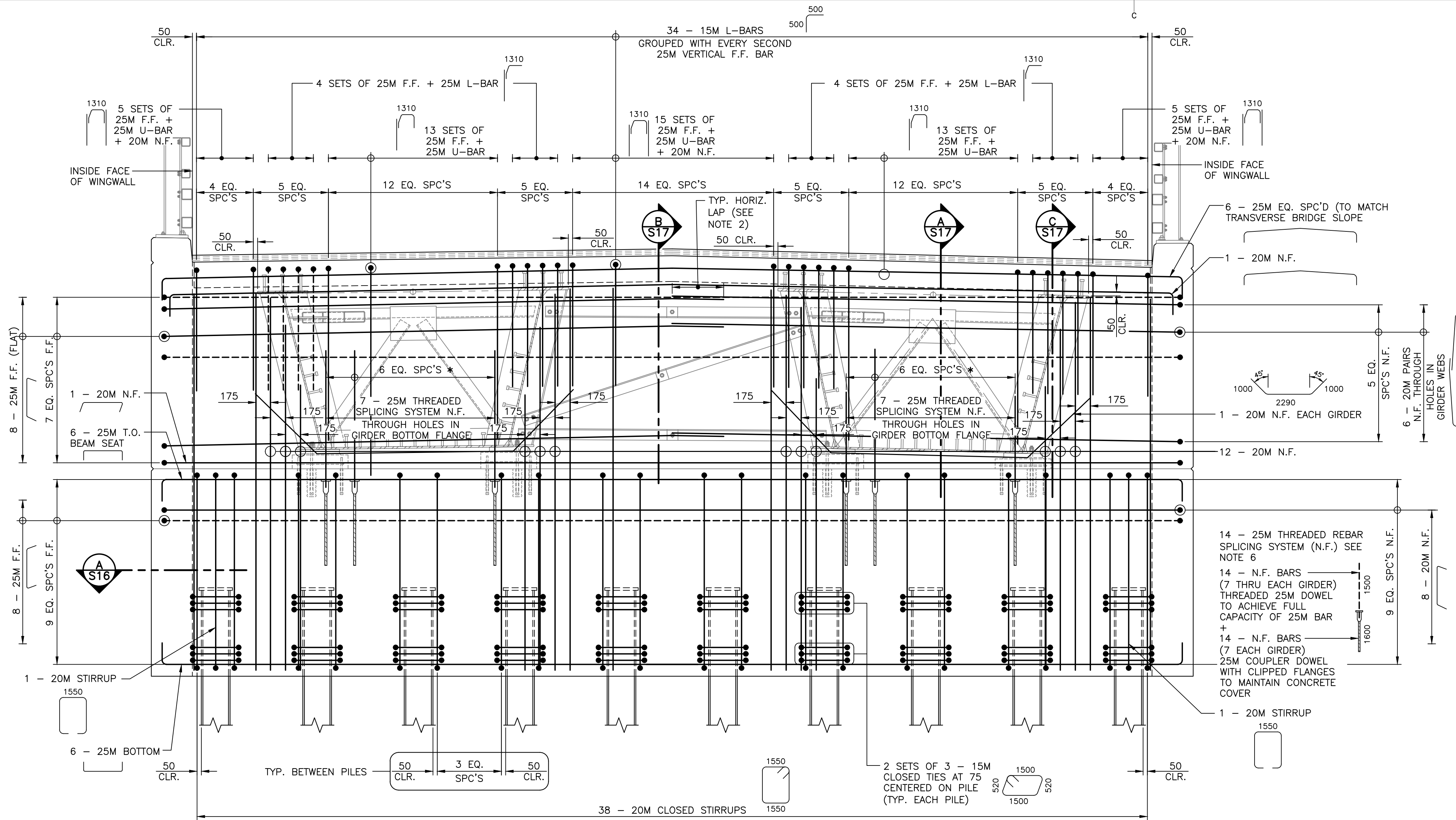
1. SCREED ELEVATIONS ARE BASED ON THE DECK PROFILE ELEVATIONS PRIOR TO PLACING CONCRETE DECK, CURBS, BARRIERS, HAUNCHES AND ASPHALT SURFACE.
2. IT IS ASSUMED THAT THE ENTIRE DECK IS CAST MONOLITHICALLY. IF DECK IS POURED IN SEGMENTS, THE GIVEN SCREED ELEVATIONS ARE NOT VALID AND THE CONTRACTOR MUST REVISE THE SCREED ELEVATIONS ACCORDINGLY.
3. IT IS ALSO ASSUMED THAT ENTIRE DECK IS CAST AND REACHES 35 MPa PRIOR TO CASTING CURBS, INSTALLING BARRIERS AND WATERPROOFING AND PAVING DECK.
4. CASTING SEQUENCE DURING MONOLITHIC DECK CASTING OPERATION: PLACE CONCRETE IN ALL AREAS OF DECK PRIOR TO CASTING INTEGRAL ABUTMENTS. TO ACHIEVE THIS, LEAVE 3m OF DECK AT EACH END OF BRIDGE/ADJACENT TO ABUTMENTS UNTIL CONCRETE IS PLACED IN REMAINDER OF DECK.
5. DECK SHALL NOT BE CAST SHOULD WINDS EXCEEDING 100 km/h (3 SEC. GUST) BE FORECAST OR ANTICIPATED DURING DECK CASTING OPERATIONS OR WITHIN 24 HOURS OF COMPLETION OF DECK CASTING.

1. ALL DECK FORMWORK SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER LICENSED TO PRACTICE IN NEWFOUNDLAND AND LABRADOR.
2. THE OVERHANG BRACKETS SHALL BE ORIENTED AS INDICATED UNLESS AN ALTERNATE DETAIL IS ACCEPTED IN WRITING BY THE DEPARTMENTAL REPRESENTATIVE.
3. OVERHANG BRACKET SPACING SHALL BE LESS THAN OR EQUAL TO 1200mm o.c.
4. MAXIMUM UNFACTORED SCREED LOAD PER SIDE OF BRIDGE ASSUMED TO BE 32.7kN.
5. DECK CASTING SHALL PROGRESS IN A BALANCED FASHION BY ESSENTIALLY BALANCING THE WET CONCRETE WEIGHT ON EACH SIDE OF THE GIRDERS. TO ACCOMPLISH THIS, CASTING SHALL PROGRESS ACROSS THE FULL WIDTH OF THE BRIDGE DECK WITH CASTING ADVANCING A MINIMUM OF 3.0m ON ONE SIDE OF A GIRDER VERSUS THE OTHER SIDE OF THE SAME GIRDER. THIS BALANCED PROCEDURE IS ESSENTIAL TO PREVENT EXCESSIVE UNBALANCED LOADS/TORSIONS IN GIRDERS.
6. CONTRACTOR TO ENSURE GIRDER STABILITY DURING ALL PHASES OF CONSTRUCTION.
7. AS INDICATED IN DETAIL 1/513 THE BOTTOM BEARING POINT OF THE SLAB OVERHANG BRACKET SHALL BE ORIENTATED NO HIGHER THAN 25mm ABOVE THE EXTERIOR GIRDER BOTTOM FLANGE/WEB INTERLINE DURING DECK CASTING.

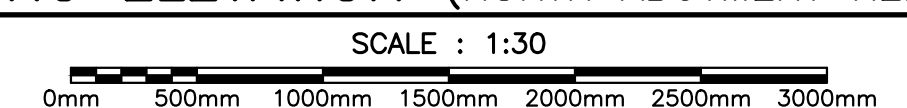


0	ISSUED FOR TENDER	JULY 19 2021
revisions		date
project	project	
<p>WESTERN BROOK BRIDGE REPLACEMENT</p> <p>GROS MORNE NATIONAL PARK</p>		
drawing	dessin	
<p>DECK PLAN AND SCREED ELEVATIONS</p>		
designed	WADE POTTIE	conçu
date	JANUARY, 2020	
drawn	WAYNE MORROW	dessiné
date	JANUARY, 2020	
approved	ROBBIE FRASER	approuvé
date		
Tender	Soumission	
TPWCSG Project Manager		Administrateur de projets TPWCSG
project number	no. du projet	
drawing no.	no. du dessin	

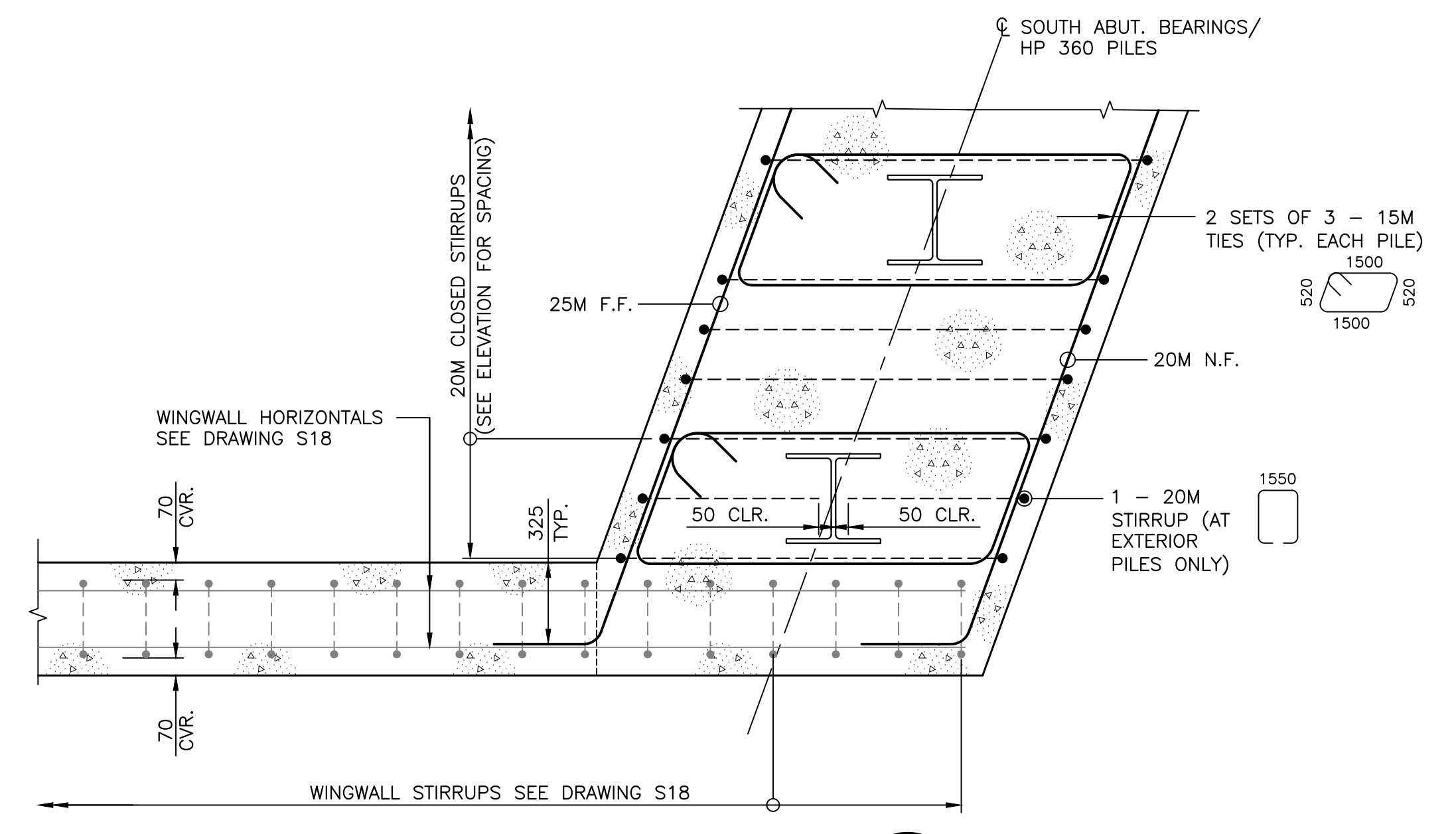




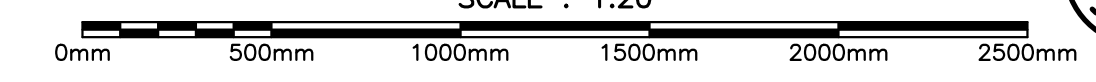
SOUTH ABUTMENT REINFORCING ELEVATION (NORTH ABUTMENT REINFORCING OPPOSITE HAND - SIMILAR)



***NOTE:**
COORDINATE WITH GIRDER FABRICATOR TO ENSURE BAR COUPLERS CAST INTO BEAM SEAT ARE IN LINE WITH HOLES IN GIRDER BOTTOM FLANGES (LONGITUDINALLY AND TRANSVERSELY)



SECTION (PLAN SECTION)
SCALE : 1:20



- NOTES:**
- ALL HORIZONTAL BARS TO HAVE STANDARD HOOKS AT END IN EACH WINGWALL. TURN ALL HOOKED HORIZONTAL BARS INTO WINGWALLS IN SUCH A MANNER AS TO MAINTAIN ALL COVERS AND AVOID CLASH WITH WINGWALL REINFORCING. REFERENCE PROJECT SPECIFICATIONS FOR REQUIRED BAR HOOK BEND DIAMETERS.
 - MINIMUM HORIZONTAL BAR LAPS: (U.N.O.)
15M BARS - 600mm
20M BARS - 800mm
25M BARS - 1200mm
 - MINIMUM VERTICAL BAR LAPS: (U.N.O.)
15M BARS - 600mm
20M BARS - 600mm
25M BARS - 900mm
 - VERTICAL BAR LAPS TO BE DETAILED TO INCLUDE ADDITIONAL 100mm OF LAP OVER THAT INDICATED IN NOTE 3 TO ACCOUNT FOR POTENTIAL ADJUSTMENTS/ALTERATIONS IN DECK DUE TO AS-BUILT CONDITIONS (e.g. 20M VERTICAL BARS SHALL BE DETAILED FOR 700mm LAP RATHER THAN 600mm).
 - ALL HORIZONTAL BARS TO EXTEND 325mm PAST I.F. OF WINGWALL.
 - BAR COUPLERS TO ACHIEVE FULL CAPACITY OF INDICATED BAR.
 - ALL REINFORCING TO BE GALVANIZED AFTER FABRICATION. CARE SHALL BE TAKEN WHEN HANDLING GALVANIZED BARS NOT TO DAMAGE COATINGS.
 - A CLEAR SPACING OF 30mm MINIMUM SHALL BE PROVIDED BETWEEN ALL GALVANIZED AND BLACK STEEL COMPONENTS, OTHERWISE THE GALVANIZED BAR SHALL BE WRAPPED IN DENSO TAPE LOCALLY AT CONTACT POINT TO AVOID CONTACT BETWEEN DISSIMILAR METALS.

REINFORCING LEGEND:

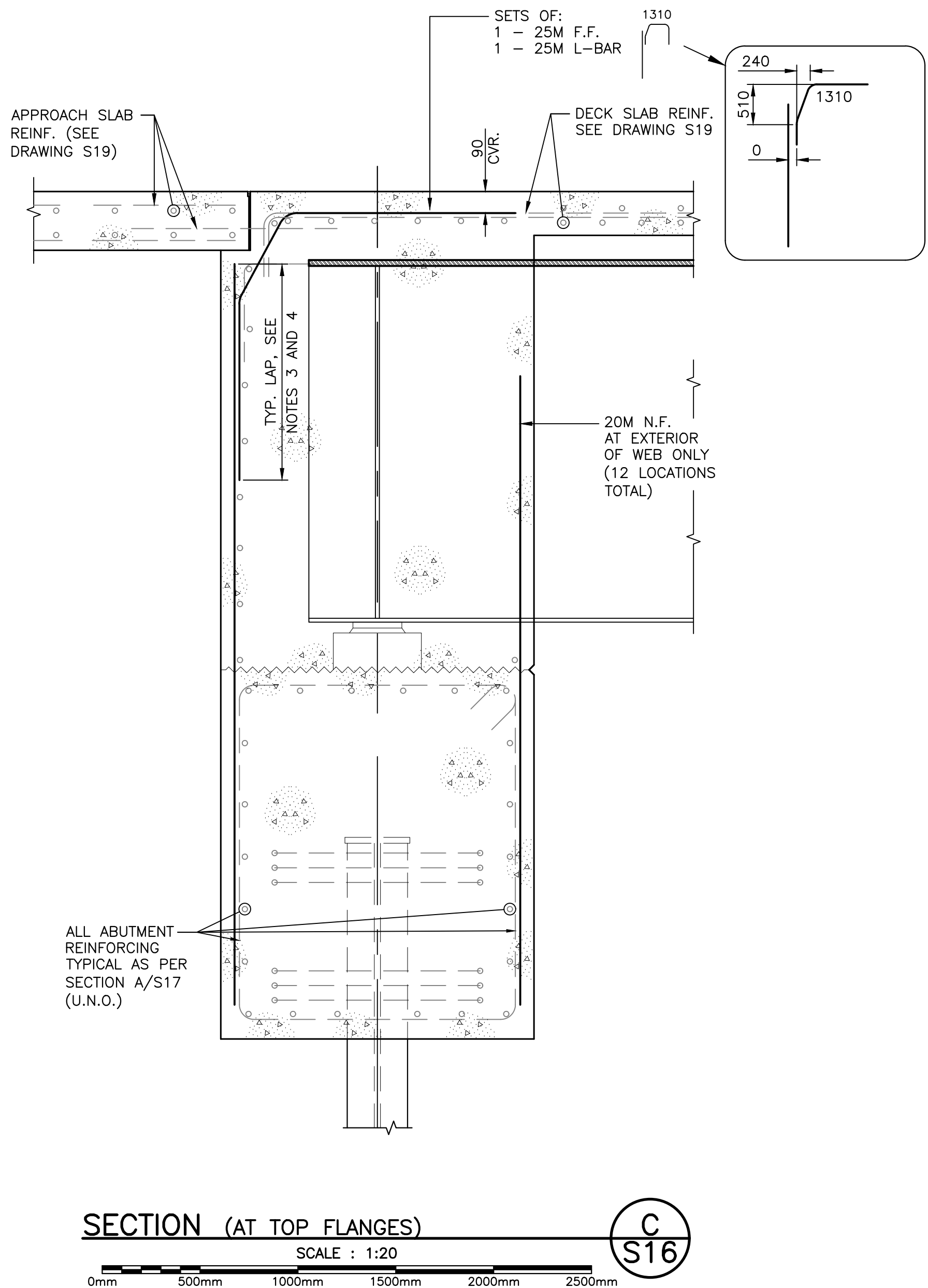
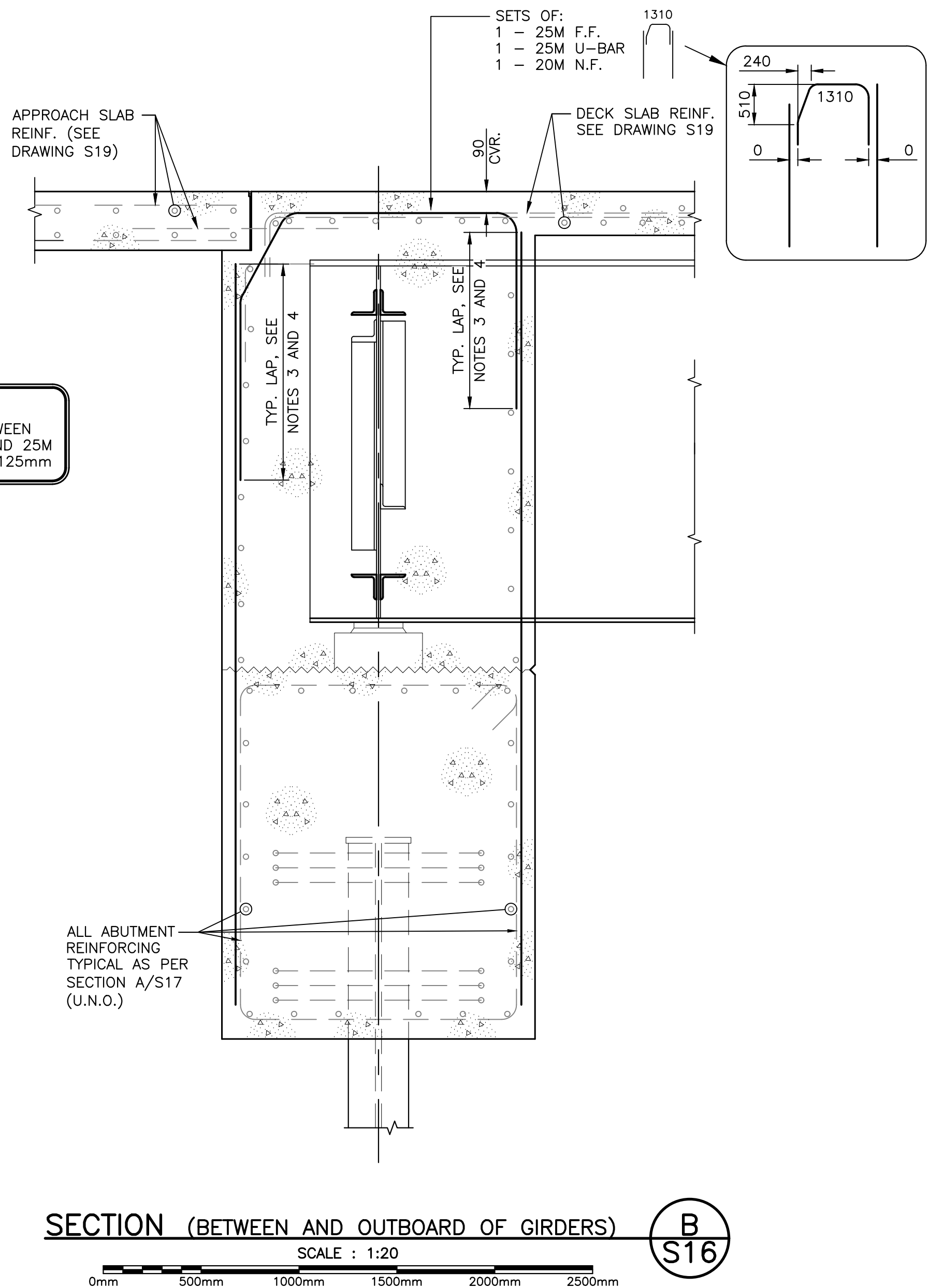
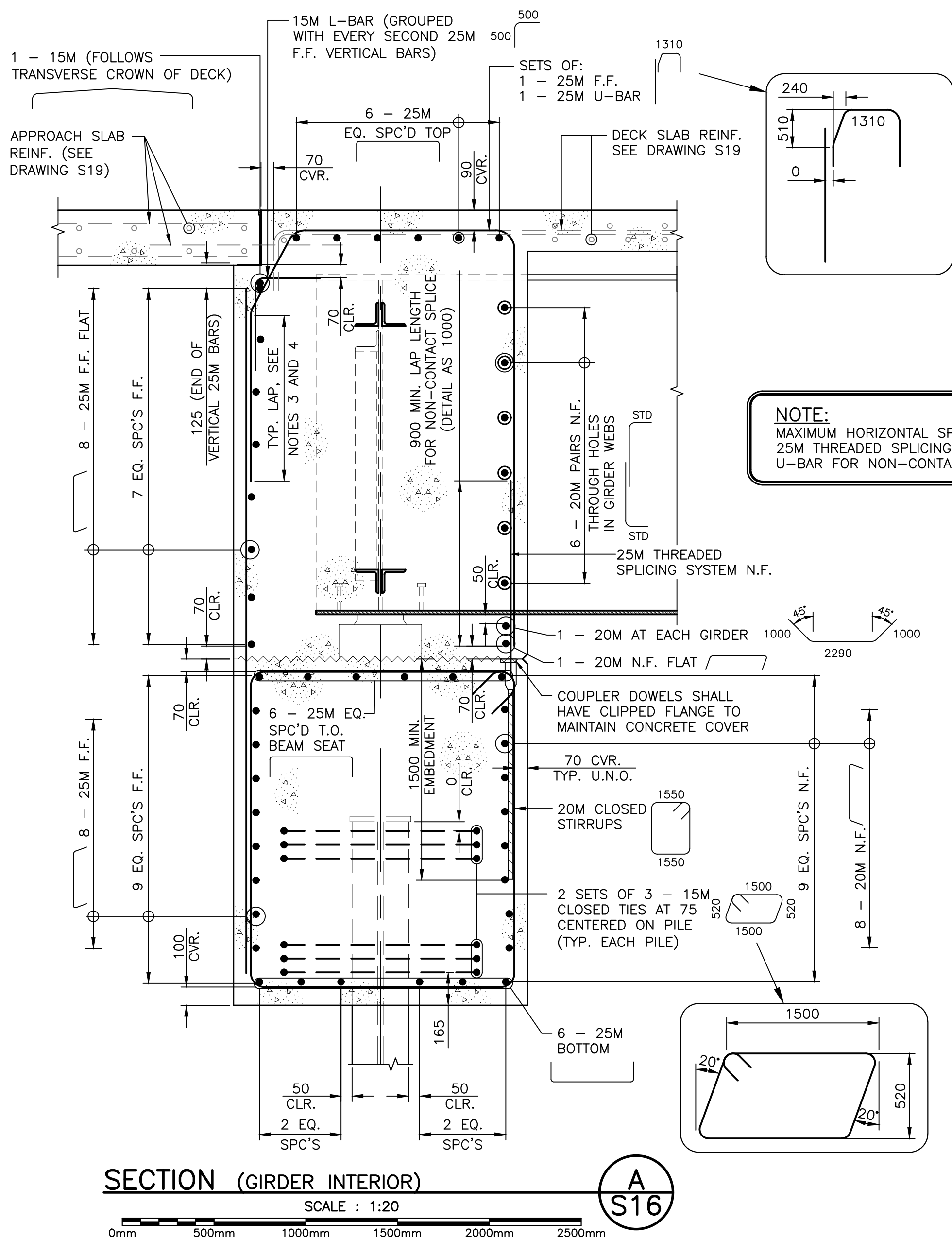
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N.F.F. ——— NEAR FAR FACE	E.A. ——— EAST ABUTMENT
F.F.F. ——— FAR FAR FACE	N.E. ——— NORTH EAST WINGWALL
E.F. ——— EACH FACE	N.W. ——— NORTH WEST WINGWALL
E.W. ——— EACH WAY	S.E. ——— SOUTH EAST WINGWALL
I.F. ——— INSIDE FACE	S.W. ——— SOUTH WEST WINGWALL
O.F. ——— OUTSIDE FACE	N.C. ——— NORTH CURB
I.C. ——— IN CENTER	S.C. ——— SOUTH CURB
T.U.L. ——— TOP UPPER LAYER	CVR. ——— COVER
T.L.L. ——— TOP LOWER LAYER	CLR. ——— CLEAR
B.U.L. ——— BOTTOM UPPER LAYER	

0 ISSUED FOR TENDER JULY 19 2021
revisions date
project WESTERN BROOK BRIDGE REPLACEMENT
GROS MORNE NATIONAL PARK
drawing design

ABUTMENT REINFORCING ELEVATION AND SECTIONS

designed WADE POTTIE conçu
date JANUARY, 2020
drawn WAYNE MORROW dessiné
date JANUARY, 2020
approved ROBBIE FRASER approuvé
date
Tender Soumission
PWSC Project Manager Administrateur de projets TPSC
project number no. du projet
drawing no. no. du dessin

S16

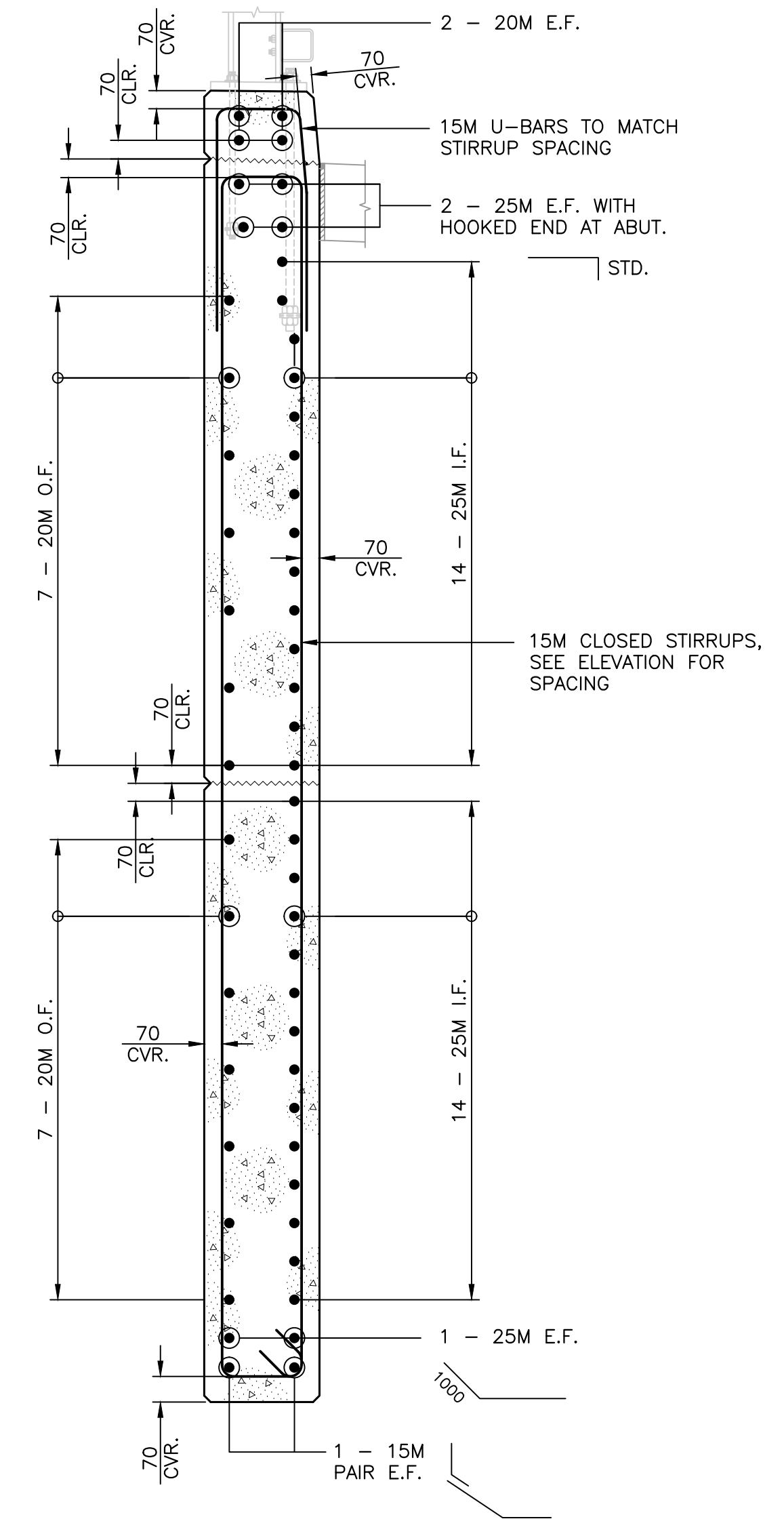


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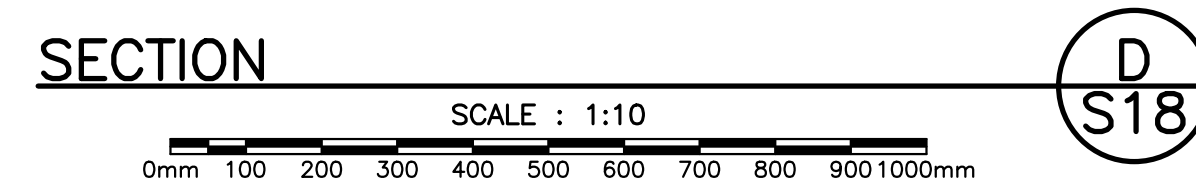
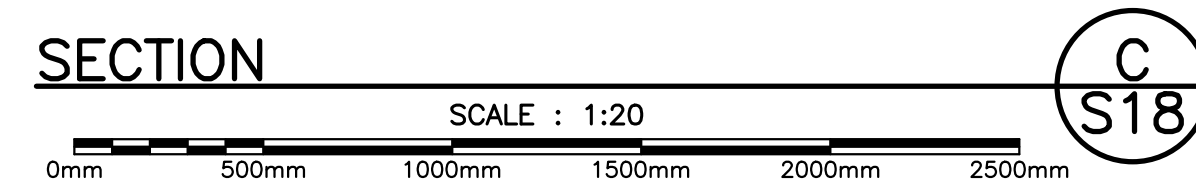
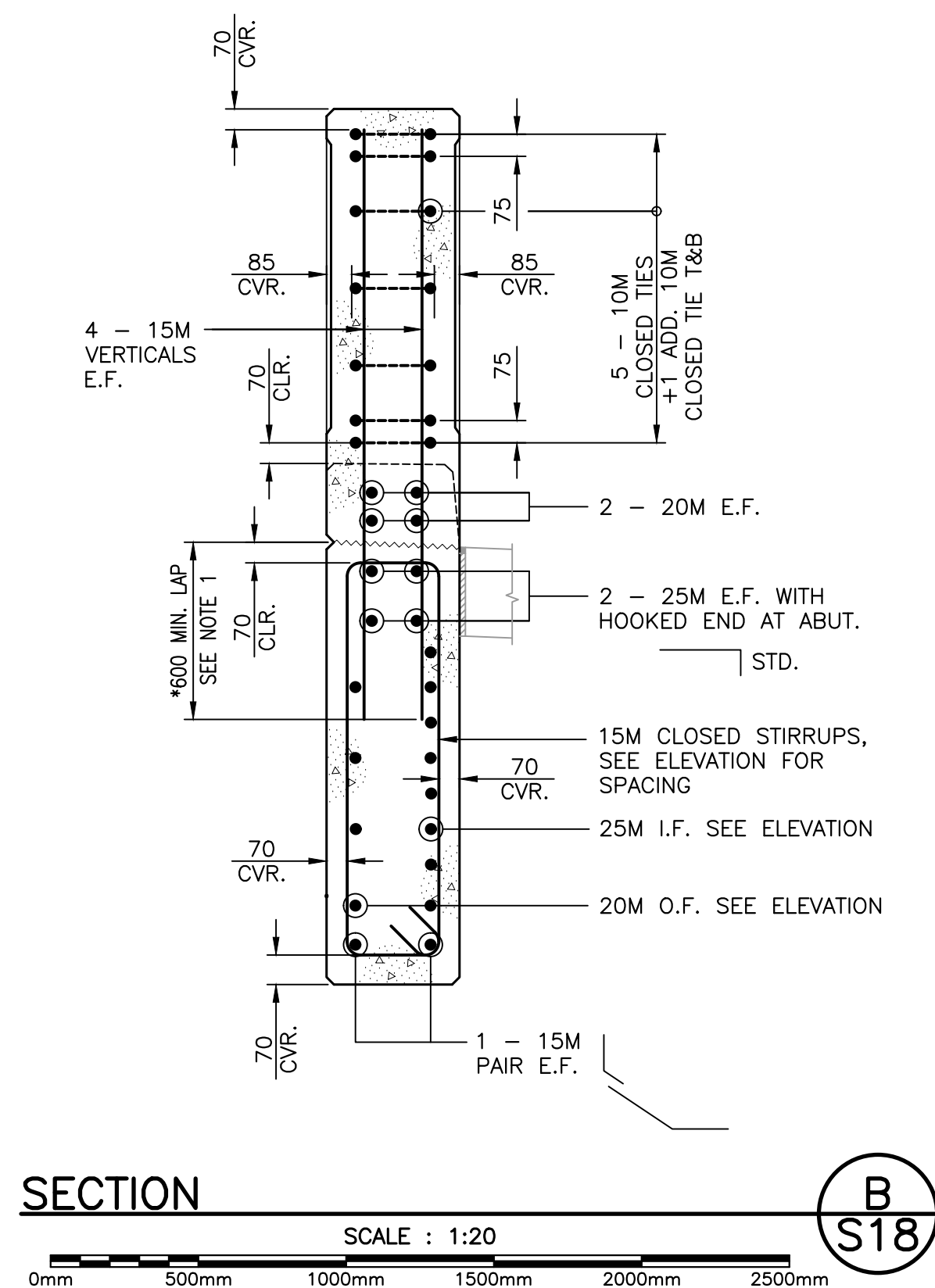
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REINFORCING LEGEND:

N.F. ——— NEAR FACE	B.L.L. ——— BOTTOM LOWER LAYER
F.F. ——— FAR FACE	W.A. ——— WEST ABUTMENT
N.F.F. ——— NEAR FAR FACE	E.A. ——— EAST ABUTMENT
F.F.F. ——— FAR FAR FACE	N.E. ——— NORTH EAST WINGWALL
E.F. ——— EACH FACE	N.W. ——— NORTH WEST WINGWALL
E.W. ——— EACH WAY	S.E. ——— SOUTH EAST WINGWALL
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O.F. ——— OUTSIDE FACE	N.C. ——— NORTH CURB
I.C. ——— IN CENTER	S.C. ——— SOUTH CURB
T.U.L. ——— TOP UPPER LAYER	CVR. ——— COVER
T.L.L. ——— TOP LOWER LAYER	CLR. ——— CLEAR
B.U.L. ——— BOTTOM UPPER LAYER	

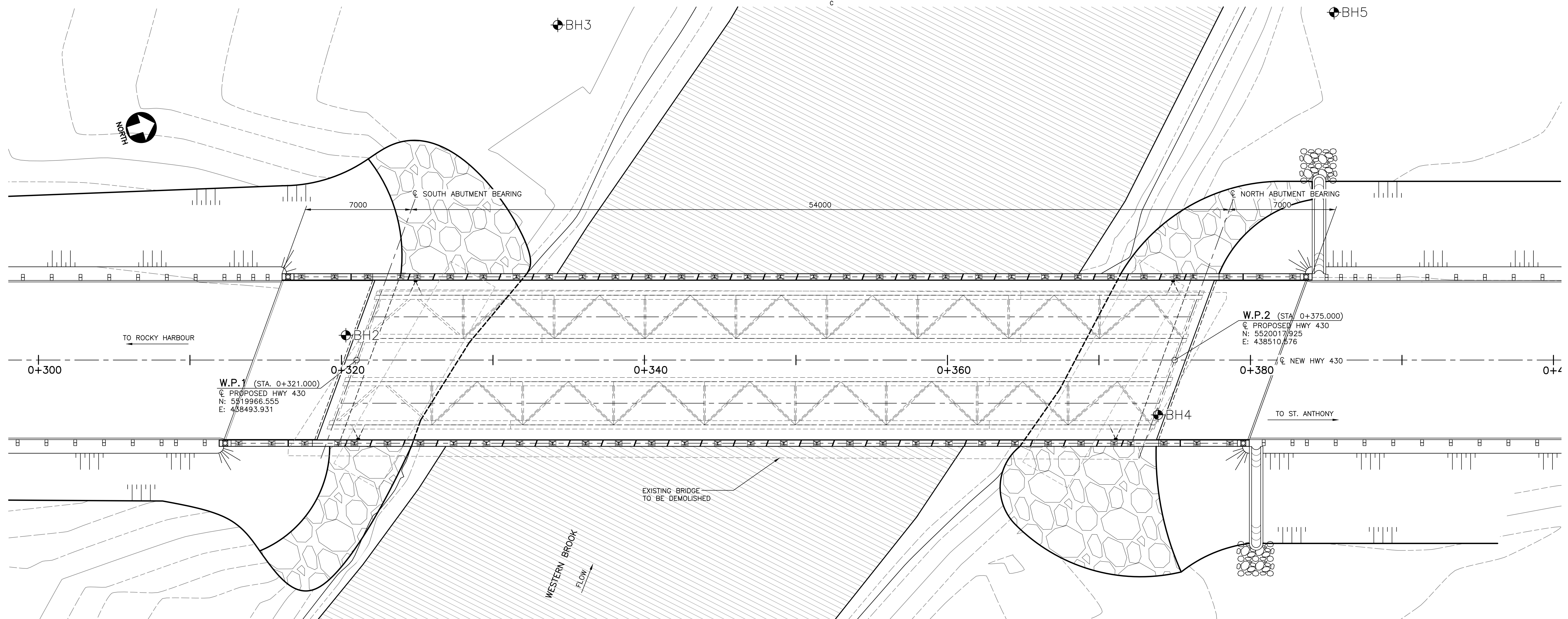


TYPICAL WINGWALL ELEVATION – SOUTHEAST SHOWN (OTHERS SIMILAR)



- | | | | |
|--------------|--------------------|--------------|---------------------|
| N.F. _____ | NEAR FACE | B.L.L. _____ | BOTTOM LOWER LAYER |
| F.F. _____ | FAR FACE | W.A. _____ | WEST ABUTMENT |
| N.F.F. _____ | NEAR FAR FACE | E.A. _____ | EAST ABUTMENT |
| F.F.F. _____ | FAR FAR FACE | N.E. _____ | NORTH EAST WINGWALL |
| E.F. _____ | EACH FACE | N.W. _____ | NORTH WEST WINGWALL |
| E.W. _____ | EACH WAY | S.E. _____ | SOUTH EAST WINGWALL |
| I.F. _____ | INSIDE FACE | S.W. _____ | SOUTH WEST WINGWALL |
| O.F. _____ | OUTSIDE FACE | N.C. _____ | NORTH CURB |
| I.C. _____ | IN CENTER | S.C. _____ | SOUTH CURB |
| T.U.L. _____ | TOP UPPER LAYER | CVR. _____ | COVER |
| T.L.L. _____ | TOP LOWER LAYER | CLR. _____ | CLEAR |
| B.U.L. _____ | BOTTOM UPPER LAYER | | |

drawing no.	no. du dessin
S18	



BRIDGE PLAN

SCALE : 1:125
0m 1 2 3 4 5 6 7 8 9 10m

PAGE 1 OF 3

HARBOURSIDE
Geotechnical Consultants

BOREHOLE RECORD **BH02**

CLIENT: PARKS CANADA AGENCY
LOCATION: WESTERN BROOK BRIDGE, ROUTE 430, GROS MORNE NATIONAL PARK, NL
DATES: BORING 2019-12-11
PROJECT No. 193132
DATUM CGVD28
BH SIZE HW/HQ

DEPTH (m)	ELEVATION (m)	SOIL/BEDROCK DESCRIPTION	GRAPHIC LOG	WATER LEVEL	TYPE	NUMBER	REC. SOIL (mm)	REC. ROCK (%)	BLOWS / 150 mm (N60)	WATER CONTENT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	OTHER TESTS	UNDRAINED SHEAR STRENGTH - kPa
0.26	6.26	ASPHALT			SS	1	275	24-31-32 (63)	45					
0.75	5.75	FILL: grey silty sand with gravel			SS	2	325	8-22-15 (35)	17					
1.12	5.38	FILL: brown gravel with silt and sand			SS	3	275	11-12-12 (24)	17					
1.50	4.99				SS	4	250	13-11-19 (30)	21					
2.25	4.24	Compact dark brown silty SAND with gravel to SAND with silt and gravel			SS	5	200	12-23-18 (61)	36					
2.75	3.74	Soft to stiff brown SILT with sand			SS	6	175	13-12-11 (22)	11					
3.25	3.24	Soft to firm grey sandy lean CLAY to sandy CLAY with gravel			SS	7	100	5-7-7-4 (11)						
3.75	2.74				SS	8	25	2-2-1-2 (3)						
4.25	2.24				ST	9	375	PUSH						
4.75	1.74				SS	10	600	2-2-4-3 (6)						
5.25	1.24				ST	11	575	PUSH						
5.75	0.74	Stiff to very stiff grey clayey SAND with gravel - with occasional cobbles			SS	12	0	5-6-7-9 (13)						
6.25	0.24				SS	13	0	3-5-7-8 (12)						
6.75	-0.26				SS	14	175	8-8-8-12 (16)						
7.25	-0.76				SS	15	175	15-10-13 (23)						
7.75	-1.26				SS	16	225	10-13-16 (29)						

(Continued Next Page)

PAGE 2 OF 3

HARBOURSIDE
Geotechnical Consultants

BOREHOLE RECORD **BH02**

CLIENT: PARKS CANADA AGENCY
LOCATION: WESTERN BROOK BRIDGE, ROUTE 430, GROS MORNE NATIONAL PARK, NL
DATES: BORING 2019-12-11
PROJECT No. 193132
DATUM CGVD28
BH SIZE HW/HQ

DEPTH (m)	ELEVATION (m)	SOIL/BEDROCK DESCRIPTION	GRAPHIC LOG	WATER LEVEL	TYPE	NUMBER	REC. SOIL (mm)	REC. ROCK (%)	BLOWS / 150 mm (N60)	WATER CONTENT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	OTHER TESTS	UNDRAINED SHEAR STRENGTH - kPa
8.25	-1.76	Stiff to very stiff grey clayey SAND with gravel - with occasional cobbles (continued)			SS	17	175	15-22-13 (29)	12					
8.75	-2.26				SS	18	500	13-13-16 (29)	14					
9.25	-2.76				SS	19	0	6-9-15-13 (24)						
9.75	-3.26				SS	20	500	14-5-12 (20)						
10.25	-3.76				SS	21	0	9-8-9-12 (17)						
10.75	-4.26				SS	22	475	8-7-8-11 (15)						
11.25	-4.76				SS	23	300	6-4-6-7 (10)						
11.75	-5.26	Firm to stiff grey sandy lean CLAY to clayey SAND			SS	24	200	6-7-7-28 (14)						
12.25	-5.76				SS	25	175	5-4-5-8 (9)						
12.75	-6.26				SS	26	100	5-6-7-7 (13)						
13.25	-6.76				SS	27	175	8-3-2-5 (5)						

(Continued Next Page)

PAGE 3 OF 3

HARBOURSIDE
Geotechnical Consultants

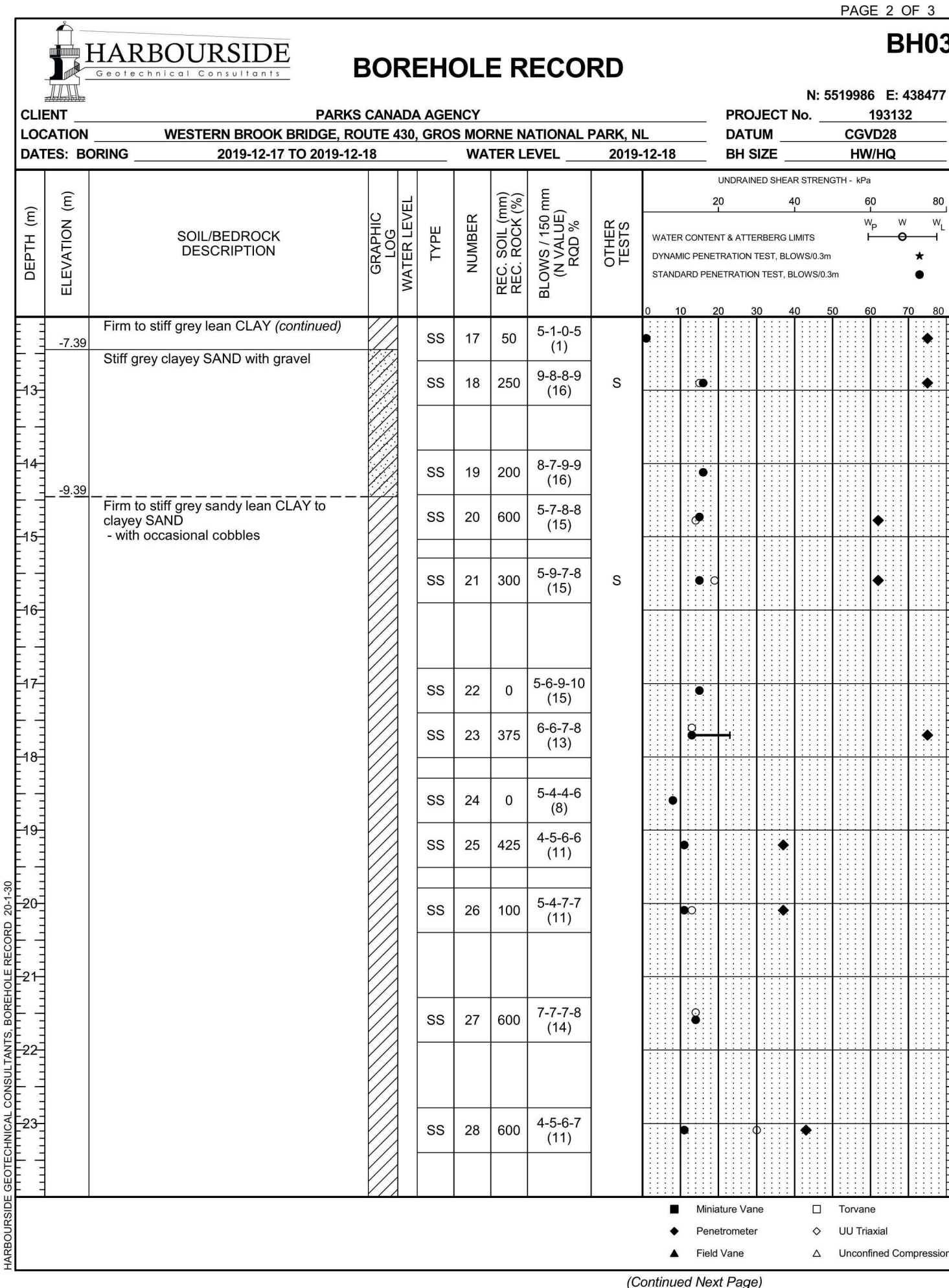
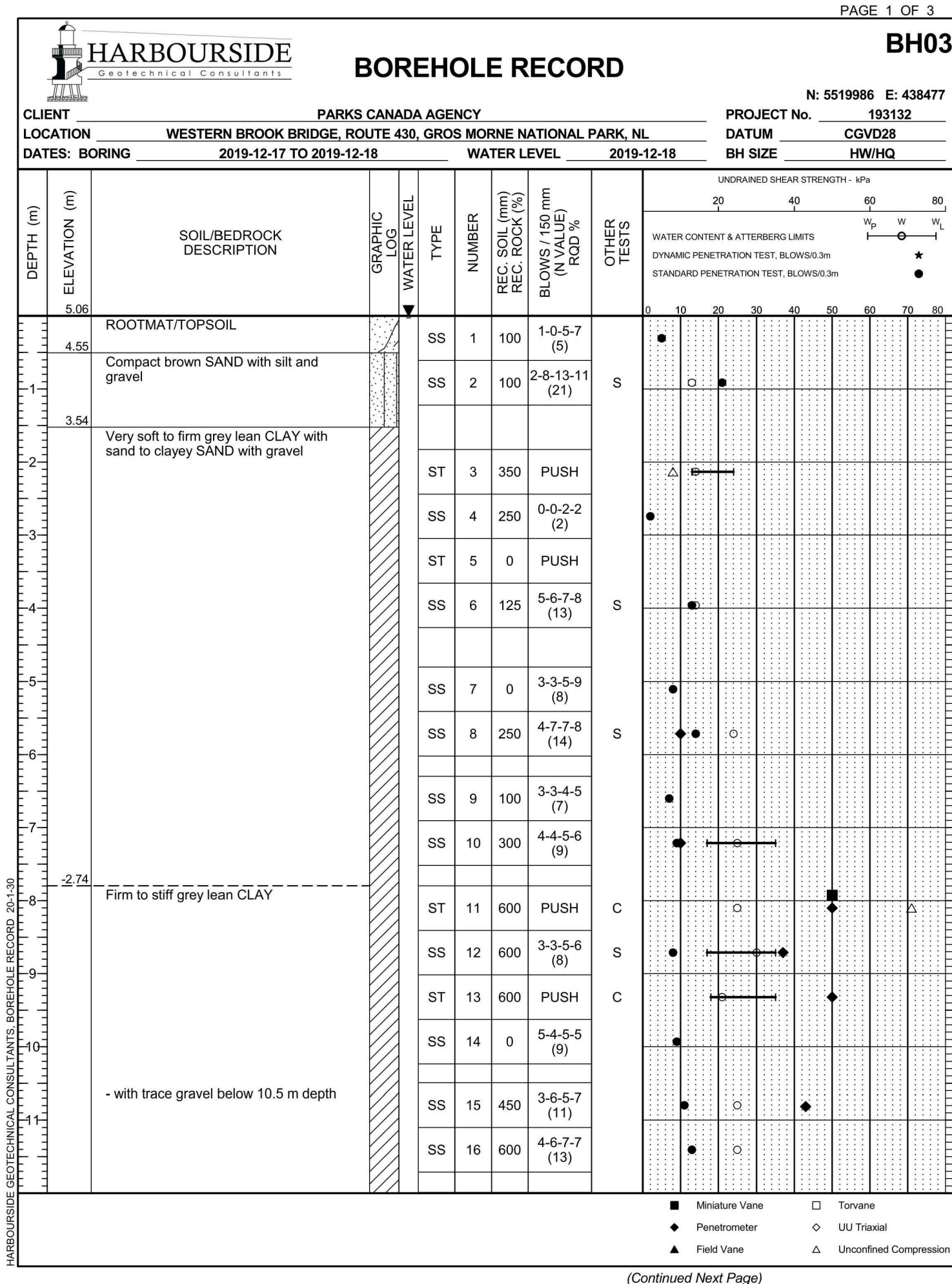
BOREHOLE RECORD **BH02**

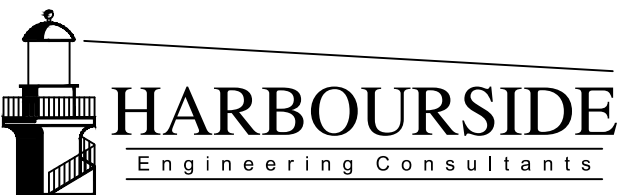
CLIENT: PARKS CANADA AGENCY
LOCATION: WESTERN BROOK BRIDGE, ROUTE 430, GROS MORNE NATIONAL PARK, NL
DATES: BORING 2019-12-11
PROJECT No. 193132
DATUM CGVD28
BH SIZE HW/HQ

DEPTH (m)	ELEVATION (m)	SOIL/BEDROCK DESCRIPTION	GRAPHIC LOG	WATER LEVEL	TYPE	NUMBER	REC. SOIL (mm)	REC. ROCK (%)	BLOWS / 150 mm (N60)	WATER CONTENT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	OTHER TESTS	UNDRAINED SHEAR STRENGTH - kPa
13.75	-7.26	Very dense grey silty SAND with gravel (continued)			SS	28	175	24-32-50 (50)						
14.25	-7.76	-350 mm boulder at 24.1 m below surface			HQ	29	81%	22%						
14.75	-8.26	Very poor to excellent quality grey LIMESTONE - slightly weathered to fresh			HQ	30	100%	100%						
15.25	-8.76				HQ	31	100%	90%						
15.75	-9.26	End of borehole - standpipe installed												

(Continued Next Page)

0	ISSUED FOR TENDER	JULY 19 2021
revisions		date
project	WESTERN BROOK BRIDGE REPLACEMENT	
	GROS MORNE NATIONAL PARK	
drawing		design
BOREHOLE LOGS SHEET 1 of 3		
designed	WADE POTTIE	conqu
date	JANUARY, 2020	
drown	WAYNE MORROW	dessiné
date	JANUARY, 2020	
approved	ROBBIE FRASER	approuvé
date		
Tender		Soumission
PWGC Project Manager	Administrateur de projets TPSC	
project number		no. du projet
drawing no.		no. du dessin
S20		





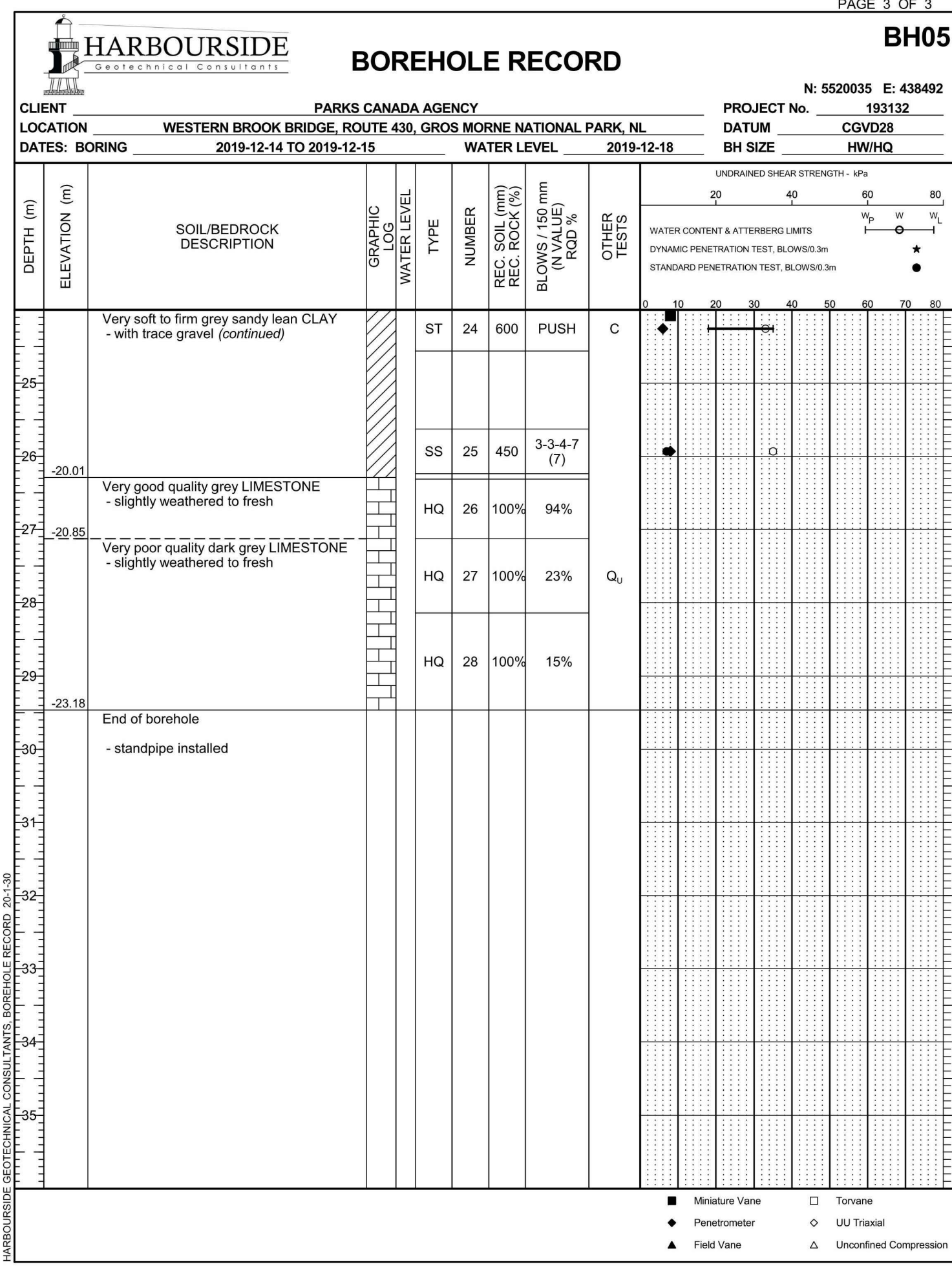
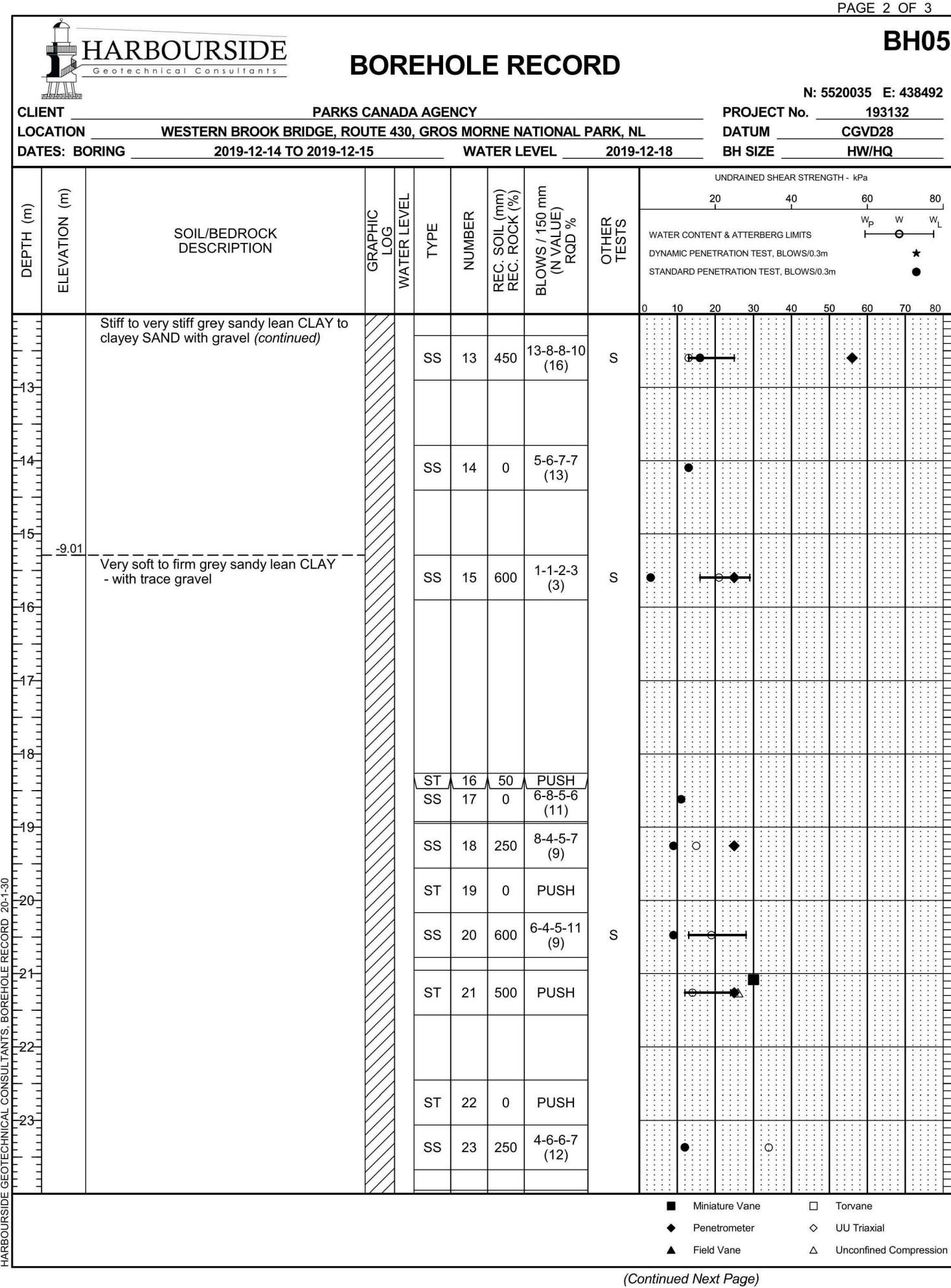
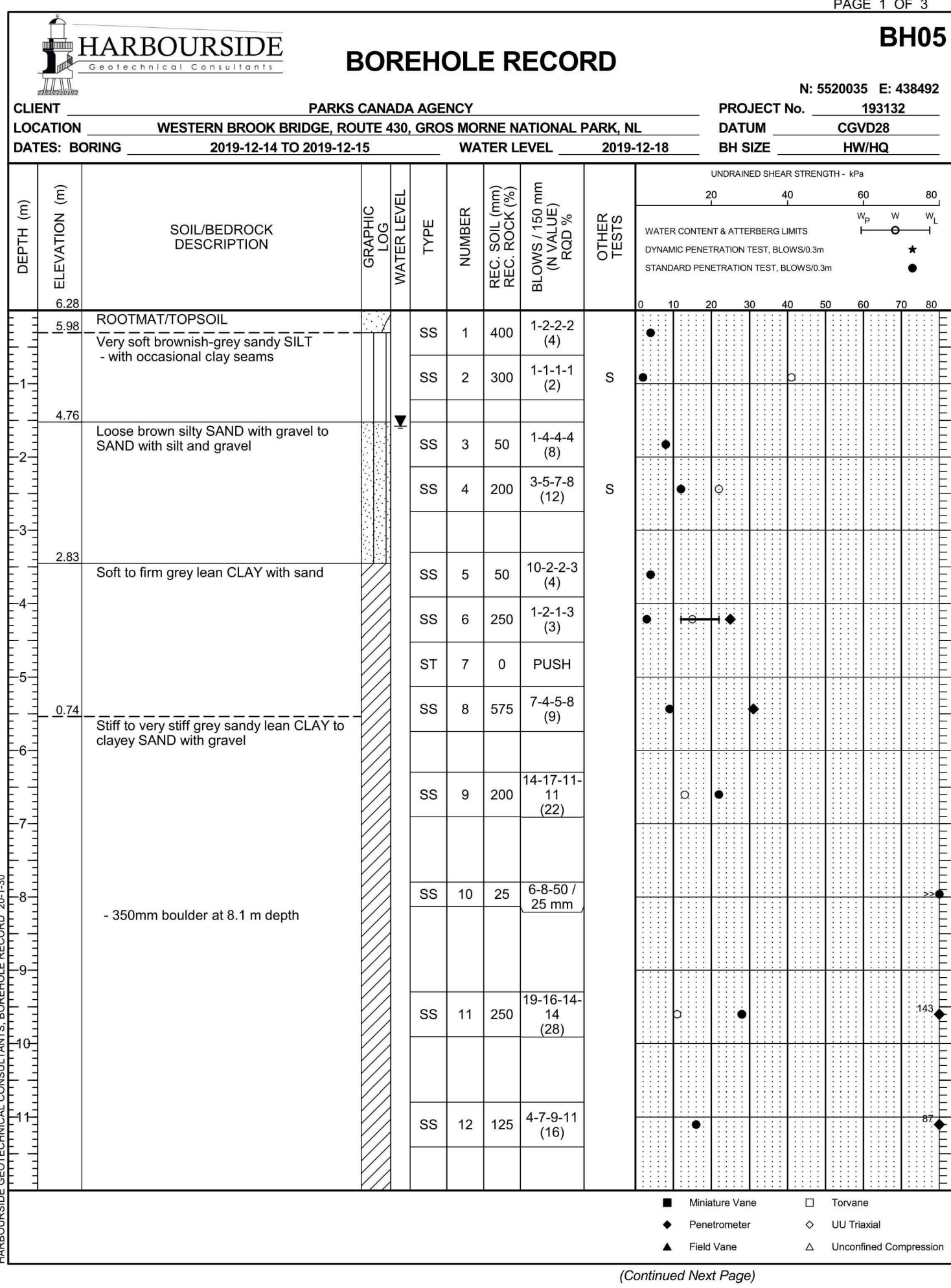
PROVINCE OF NEWFOUNDLAND AND LABRADOR
PEEL
PERMIT HOLDER
This Permit Allows
HARBORSID ENGINEERING CONSULTANTS
To practice Professionally, Engineering
in Newfoundland and Labrador
Permit No. as Issued by PCL 334
which is valid for the year 2021

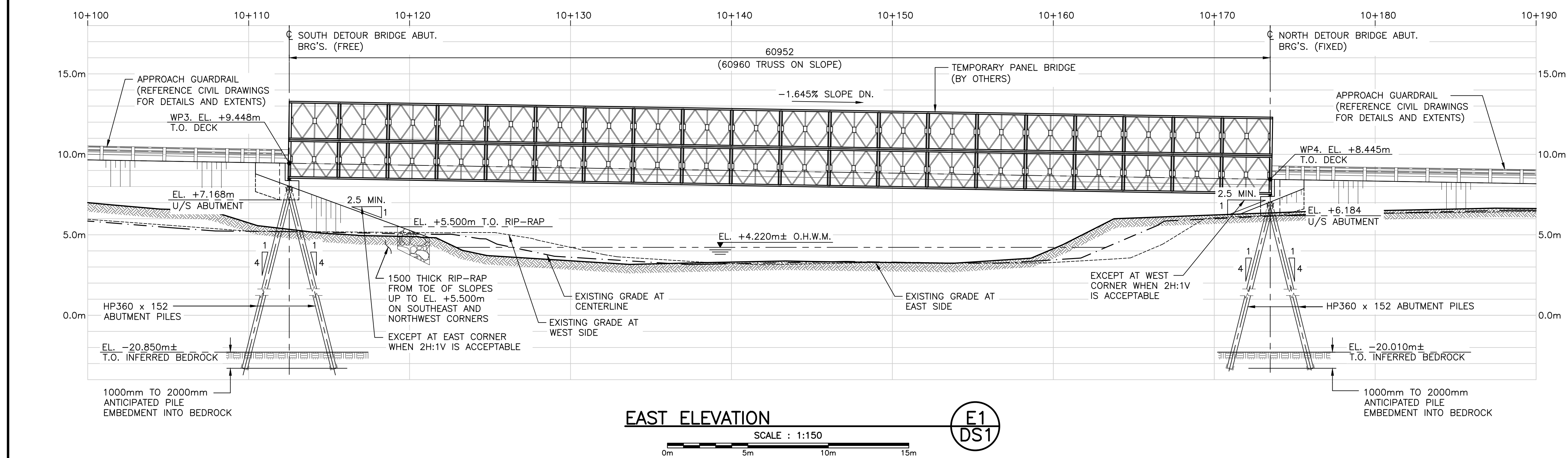
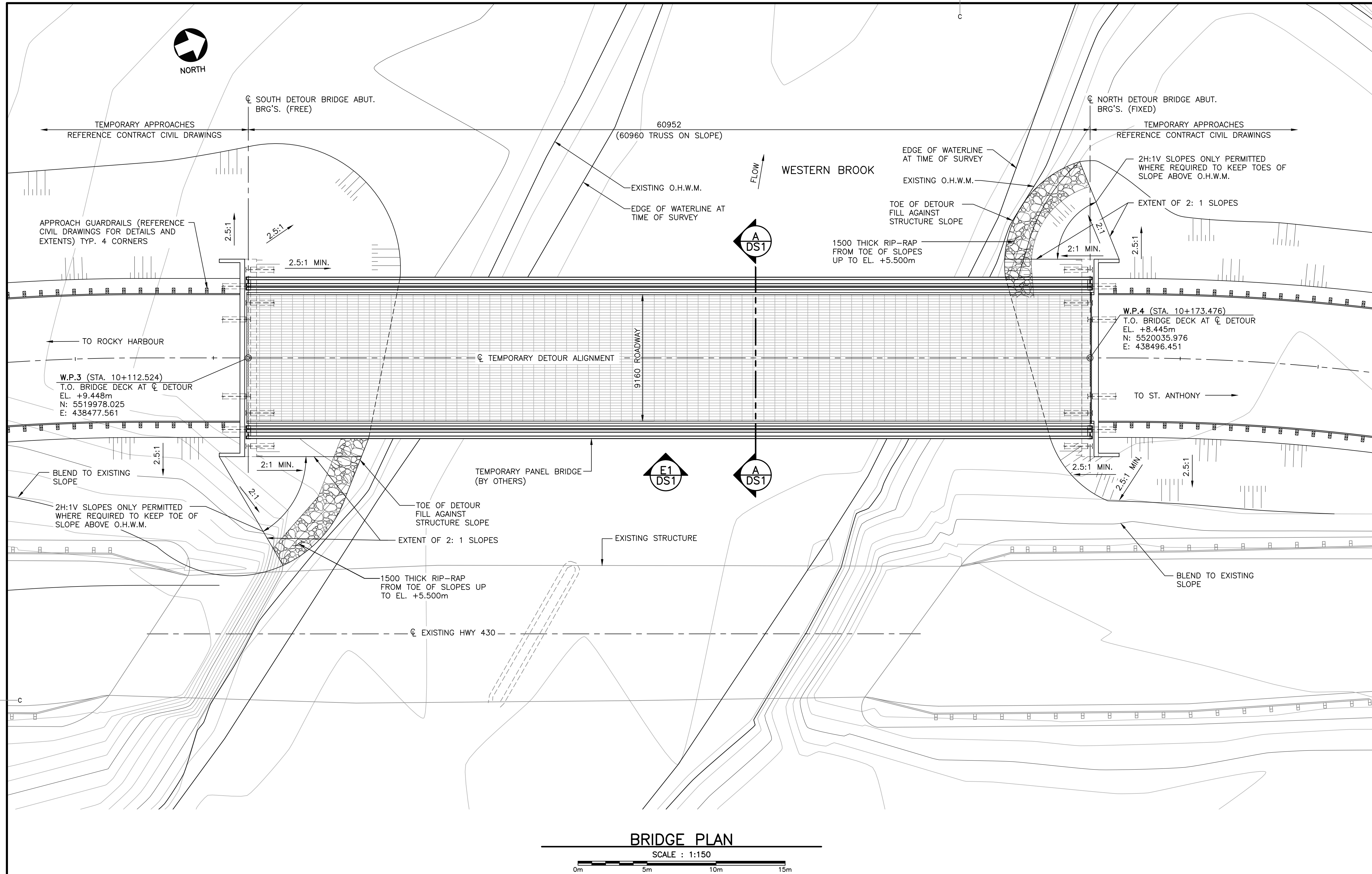


0 ISSUED FOR TENDER JULY 19 2021
revisions date
project WESTERN BROOK BRIDGE REPLACEMENT
GROS MORNE NATIONAL PARK
drawing design

BOREHOLE LOGS
SHEET 3 of 3

designed WADE POTTIE conçu
date JANUARY, 2020
drawn WAYNE MORROW dessiné
date JANUARY, 2020
approved ROBBIE FRASER approuvé
date
Tender Soumission
PWSC Project Manager Administrateur de projets TPSGC
project number no. du projet
drawing no. no. du dessin
S22

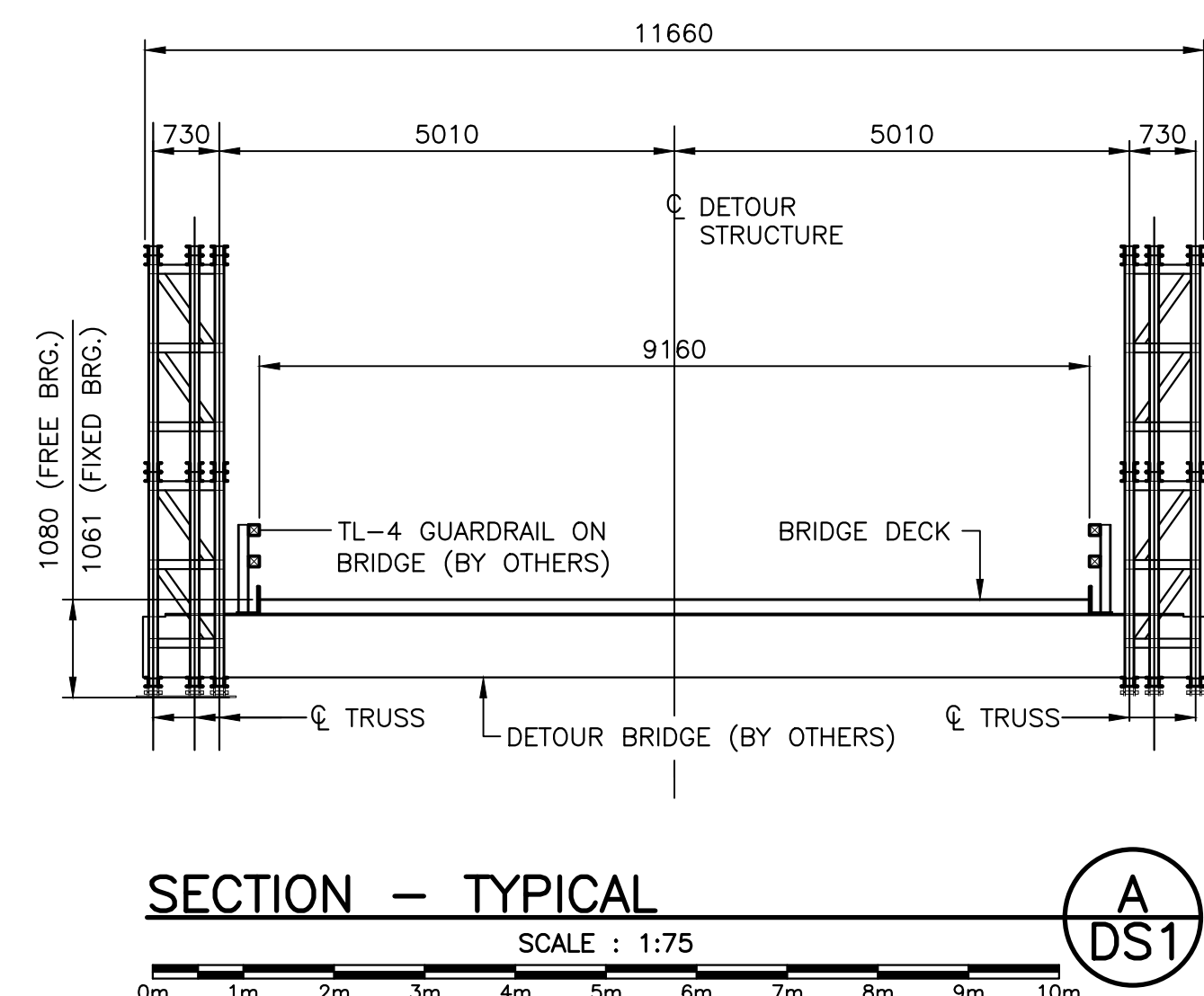


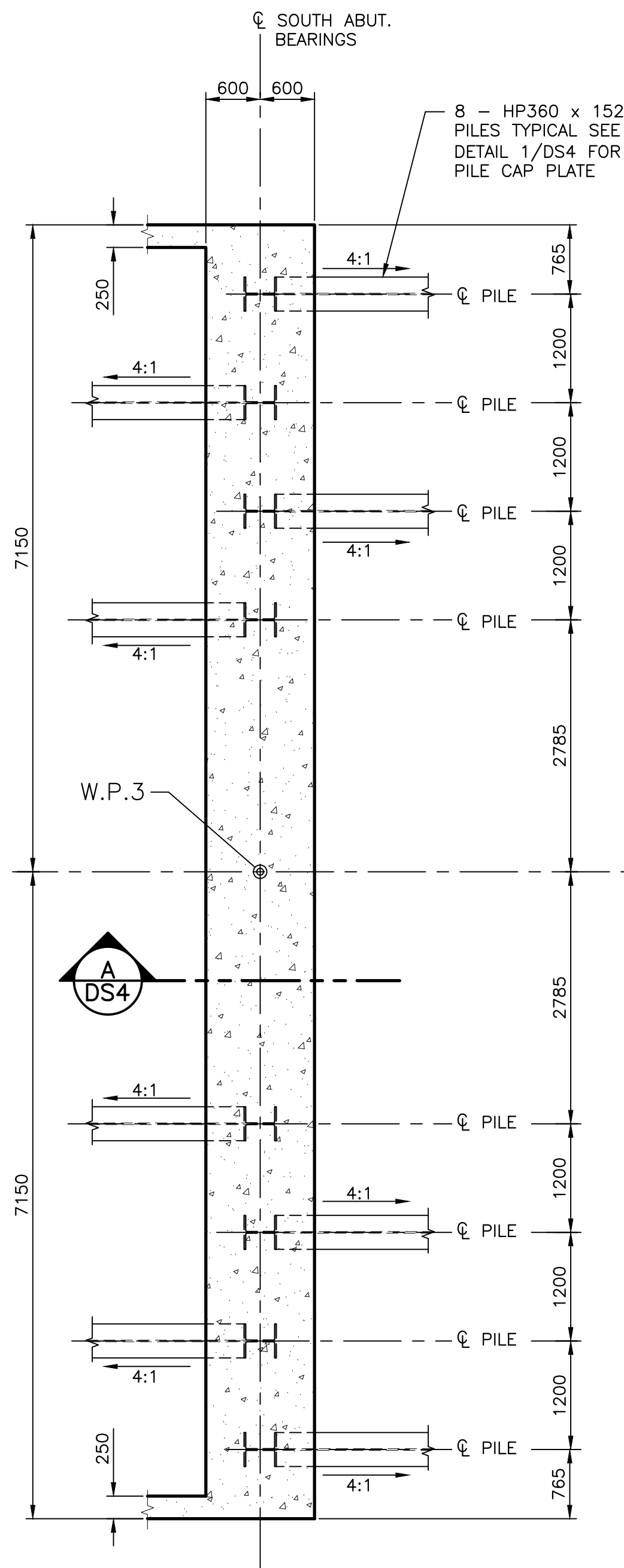


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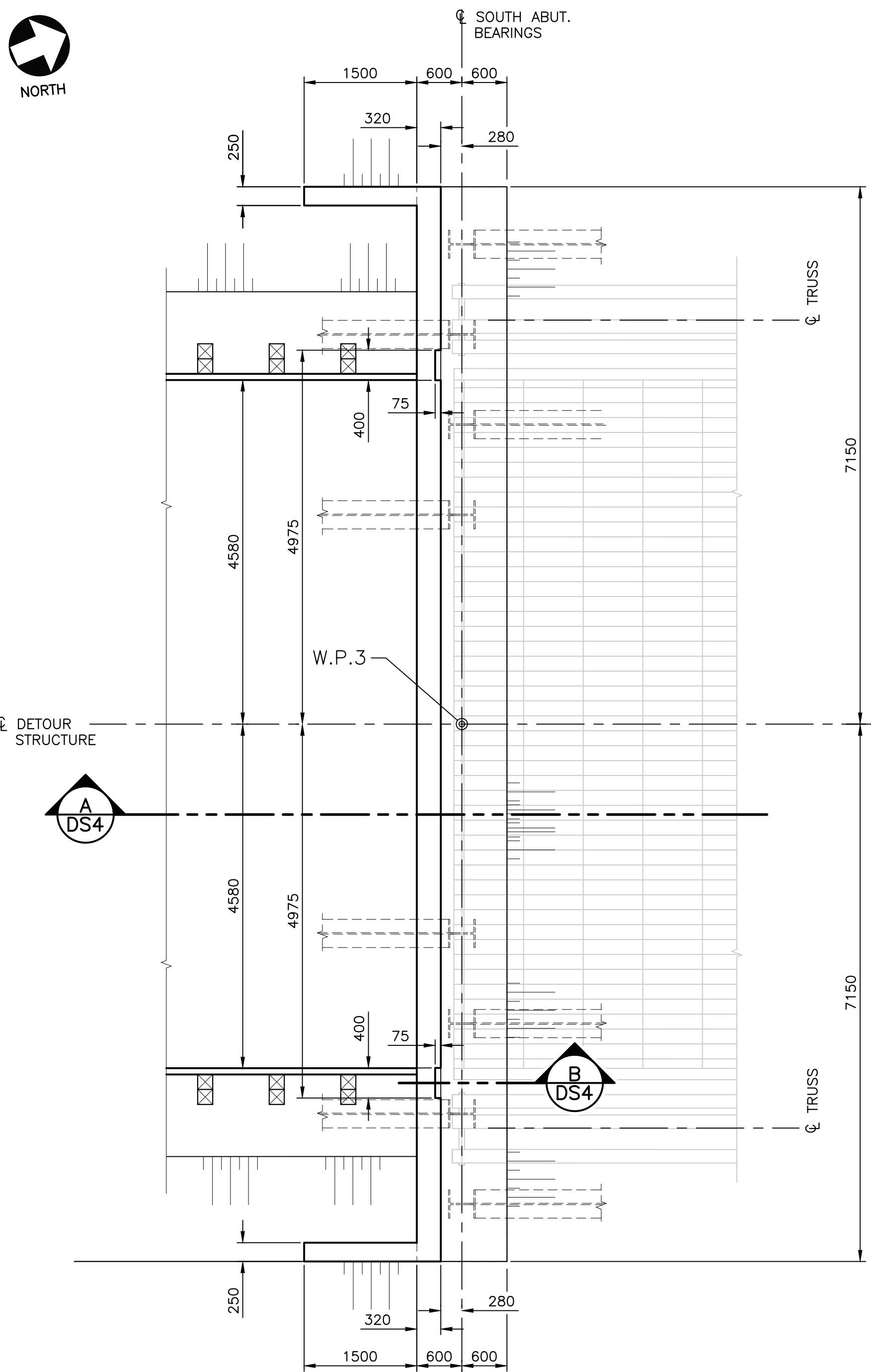
- DETOUR STRUCTURE TO BE SUPPLIED BY PARKS CANADA FOR 2 LANE TRAFFIC WITH A LENGTH OF 60.960m. ASSEMBLY DRAWINGS ARE TO BE PROVIDED BY CONTRACTOR TO PARKS CANADA FOR 60.960m PANEL BRIDGE. CONTRACTOR TO FOLLOW ALL REQUIREMENTS OF SPECIFICATIONS FOR HANDLING, ASSEMBLY, DISASSEMBLY, FINAL INSPECTION OF ALL COMPONENTS AND FINAL STORAGE AT THE COMPLETION OF WESTERN BROOK BRIDGE.
- PANEL BRIDGE, INCLUDING ALL COMPONENTS, SHALL REMAIN THE PROPERTY OF PARKS CANADA AFTER CONSTRUCTION. CONTRACTOR RESPONSIBLE TO RETURN ALL COMPONENTS IN GOOD WORKING ORDER TO PARKS CANADA COMPOUND IN ROCKY HARBOUR AT END OF CONSTRUCTION (SEE SPECIFICATIONS).
- PANEL BRIDGE NORTH AND SOUTH ABUTMENT TO BE FOUNDED ON PILES DRIVEN TO REFUSAL IN BEDROCK.
- ALIGNMENT INFORMATION AS PER HARBOURSIDE TRANSPORTATION CONSULTANTS CIVIL DRAWINGS. SURVEY INFORMATION PROVIDED BY DESIGN POINT ENGINEERING AND SURVEYING. SURVEY REFERENCED HORIZONTALLY TO UTM ZONE 21 NORTH NAD83 (CSRS) AND VERTICALLY TO CGVD28. (SEE SPECIFICATIONS) CONTROL IS DERIVED FROM STATIC GNSS OBSERVATIONS ON AN IRON BAR (POINT 1000) HAVING COORDINATES OF:
N = 5519990.091M
E = 4384437.178M
EL = 6.331M
- BRIDGE GEOMETRY AND BEARING REACTIONS USED FOR ABUTMENT DESIGN BASED ON ALGONQUIN BRIDGE DRAWINGS DATED JULY 12, 2017 AND SK200 DATED 2020 APRIL 3. ANY DISCREPANCIES BETWEEN DRAWINGS AND AS-BUILT GEOMETRY SHALL BE REPORTED TO THE ENGINEER. CONTRACTOR TO CONFIRM GEOMETRY PRIOR TO CONSTRUCTION.
- DESIGN LIVE LOADING: CL-625

CL - 625 TRUCK LOADING
CL - 625 LANE LOADING
AXLE LOADS 9 kN/m
- ALL DIMENSIONS SHOWN IN MILLIMETERS (mm) AND ELEVATIONS IN METERS (m).
- ENVIRONMENTAL MITIGATION MEASURES TO MEET REQUIREMENTS OF DEPARTMENTAL REPRESENTATIVE AND DFO, BY CONTRACTOR.
- SURVEY AND LAYOUT RESPONSIBILITY OF CONTRACTOR.
- CONSTRUCTION SHALL BE CARRIED OUT PER CAN/CSA-S6-19 WITH LATEST REVISIONS.
- DESIGN OF BRIDGE SUPERSTRUCTURE, BEARING ASSEMBLIES AND EXPANSION/DECK JOINTS IS THE RESPONSIBILITY OF OTHERS.
- UNFACTORED BEARING REACTIONS PER CORNER:
DEAD LOAD = 570 kN
LIVE LOAD = 731 kN
BRAKING LOAD (NORTH ABUTMENT) = 118kN
- FOUNDATION DESIGN BASED ON INFORMATION PROVIDED IN HARBOURSIDE GEOTECHNICAL REPORT 193132R1 DATED MARCH 23, 2020.
- REFER TO DRAWING DS2 FOR PILE NOTES AND C.I.P. CONCRETE NOTES.
- ALL ROOTMAT, TOPSOIL, AND DELETERIOUS MATERIALS (e.g. SOFT LOOSE SOILS, OR SOILS CONTAINING A SIGNIFICANT PROPORTION OF ORGANIC MATERIAL) SHOULD BE REMOVED FROM BELOW THE FOOTPRINT OF THE PILE CAP, STRUCTURAL FILLS, AND APPROACH FILLS TO EXPOSE THE IN-SITU SAND AND GRAVEL FILL OR NATIVE SAND, SILT AND GRAVEL.
AFTER REMOVAL OF THE REQUIRED MATERIALS, THE EXPOSED SOIL SURFACE SHOULD BE RE-GRADED, COMPACTED, AND TESTED (PROOF ROLLED) WITH LOADED TANDEM TRUCK OR LARGE VIBRATORY ROLLER UNDER THE SUPERVISION OF QUALIFIED GEOTECHNICAL PERSONNEL PRIOR TO FILL PLACEMENT. ANY SOFT AREAS OR YIELDING MATERIAL WITH DEFLECTIONS GREATER THAN 20mm WITHIN THE SUBGRADE SHOULD BE REMOVED AND REPLACED WITH APPROVED FILL.
- ALL SLOPES SHALL BE 2.5H:1V MAXIMUM EXCEPT AT SOUTHEAST AND NORTHWEST CORNERS WHERE SLOPE MAY BE STEEPENED TO 2.0H:1V MAXIMUM ONLY AS REQUIRED TO KEEP TOES OF SLOPES ABOVE THE O.H.W.M.
- ANY DISCREPANCIES BETWEEN FIELD CONDITION AND THE DRAWINGS SHALL BE BROUGHT TO THE ENGINEERS ATTENTION PRIOR TO PROCEEDING WITH CONSTRUCTION.
- THE CONTRACTOR SHALL COORDINATE JACKING OF THE DETOUR STRUCTURE WITH THE DETOUR STRUCTURE SUPPLIER AND PROVIDE JACKING SUPPORT POINTS AS REQUIRED.

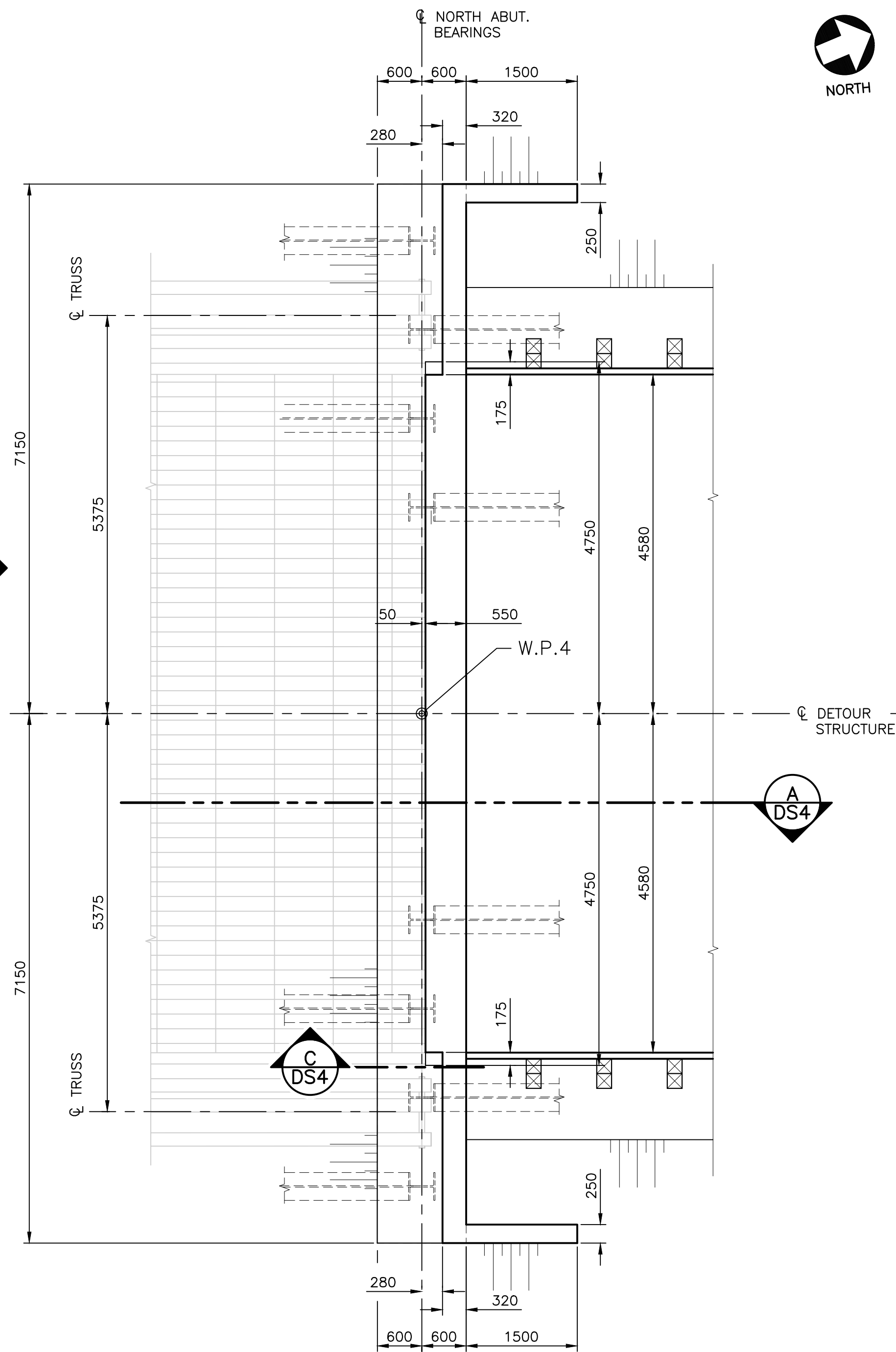




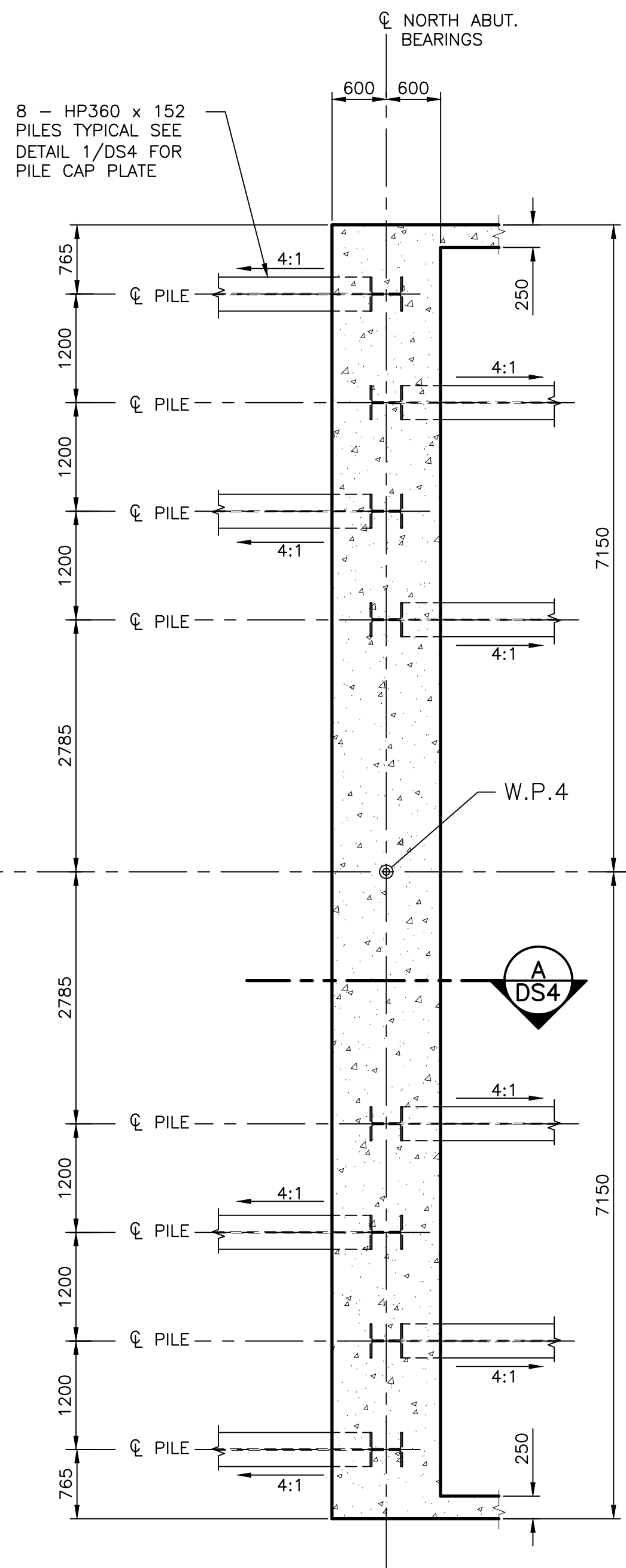
SOUTH ABUTMENT PILE PLAN



SOUTH ABUTMENT TOP PLAN



NORTH ABUTMENT TOP PLAN



NORTH ABUTMENT PILE PLAN

PILE NOTES:

- PILE MATERIAL
 - STEEL H-PILES (HP360 x 152), $F_y = 350 \text{ MPa}$ (MIN.) TO CSA G40.20/G40.21 (350W)
 - ALL PILE SPLICES SHALL BE FULL STRENGTH WELDED CONNECTIONS (LIMIT 1 SPLICE PER PILE)
 - CAP PLATE, $F_y = 350 \text{ MPa}$ MINIMUM (SEE DETAIL 1/DS4) TO CSA G40.20/G40.21 (350W)
 - WELDING MATERIAL TO CSA G40.1 - LATEST EDITION
 - WELDING TO BE IN ACCORDANCE TO CSA W59 - LATEST EDITION
- PILE SET CRITERIA AS PER HARBOURSIDE GEOTECHNICAL CONSULTANTS REPORT No.193132R1 DATED MARCH 23, 2020
 - RATED HAMMER ENERGY TO BE A MINIMUM OF 450 J/cm^2 OF STEEL CROSS-SECTIONAL AREA .
 - REFUSAL TAKEN AS PILE PENETRATION OF LESS THAN 25mm FOR 15 BLOWS FOR FOUR CONSECUTIVE SETS. ALL PILES SHALL BE DRIVEN TO REFUSAL IN BEDROCK, DRILLING OR VIBRATION METHODS ARE NOT TO BE USED FOR PILE INSTALLATION.
 - ALL PILES SHALL BE DRIVEN WITH A PROTECTIVE H-PILE DRIVING SHOE. ALL POINTS SHALL MATCH PILE SIZE AND SHALL BE WELDED TO PILE TIP AS PER MANUFACTURERS RECOMMENDATIONS. PILE TIP DETAILS SHALL BE FORWARDED TO THE ENGINEER FOR REVIEW AND ACCEPTANCE PRIOR TO DRIVING PILES.
 - THE REQUIRED FACTORED PILE COMPRESSION LOAD OF 1360 kN SHALL BE CONFIRMED BY PDA TESTING ON AT LEAST ONE REPRESENTATIVE PILE. REFERENCE PROJECT GEOTECHNICAL REPORT FOR PDA TESTING REQUIREMENTS. MAXIMUM PILE TENSION IS 300 kN AT ULS.
 - FULL TIME INSPECTION SHALL BE UNDERTAKEN DURING PILE DRIVING AND COMPLETE DRIVING RECORDS SHALL BE KEPT.
 - DYNAMIC MONITORING SHALL BE PERFORMED ON A MINIMUM OF TWO (2) PILES AT THE END OF INITIAL DRIVE AND AT THE BEGINNING OF RESTRIKE AT EACH ABUTMENT.
 - A MINIMUM OF TWO (2) PILES AT EACH ABUTMENT SHALL BE RE-TAPPED A MINIMUM OF 24 HOURS AFTER INITIAL DRIVING REFUSAL IF RELAXATION OCCURS, ALL PILES SHALL BE RE-DRIVEN TO REFUSAL CRITERIA AND THE CYCLE REPEATED UNTIL THE REFUSAL CRITERIA IS MAINTAINED DURING SUBSEQUENT RE-TAPS.

C.I.P. CONCRETE NOTES:

- ALL EXPOSED CORNERS OF CONCRETE TO HAVE 25mm CHAMFERS.
- LOCATION OF CONSTRUCTION JOINTS AND SEQUENCE OF CONCRETE PLACEMENT TO BE APPROVED BY HEC.
- CAST-IN-PLACE CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS:
 - ABUTMENTS AND PILECAPS 35 MPa WITH 20mm MAX. AGGREGATE SIZE AND $6\% \pm 1\%$ AIR ENTRAINMENT (AIR Voids SPACING REQUIREMENTS AS PER DIV. 5, SECTION 18 OF DEPARTMENTS STD. SPEC'S), MAX. WATER-CEMENT RATIO 0.35.
- CONCRETE COVER TO REINFORCING STEEL AS NOTED ON DRAWINGS.
- CONCRETE METHODS AS PER CONTRACT SPECIFICATIONS, AND METHODS FOR TEST OF CONCRETE TO CAN/CSA-A23.2.
- REINFORCING STEEL TO BE GRADE 400W DEFORMED BARS TO PROJECT SPECIFICATION WITH YIELD STRENGTH OF 400 MPa (WELDABLE).
- ALL REINFORCEMENT TO BE INSPECTED BY THE ENGINEER PRIOR TO CLOSING, FORMWORK OR PLACING CONCRETE.
- COMPACTING IMMEDIATELY ADJACENT TO BACK WALL SHALL BE ACCOMPLISHED WITH LIGHT COMPACTING EQUIPMENT. MODERATE COMPACTING WITH A TRENCH ROLLER IN 200MM LIFTS ELSEWHERE (ALL COMPACTION SHALL BE TO 100% STD. PROCTOR DENSITY). WHEEL LOADS/Crane TRACKS SHALL BE KEPT 5.0M CLEAR OF ABUTMENT BACKWALLS DURING ERECTION OF SUPERSTRUCTURE AND ALL PHASES OF CONSTRUCTION.
- EACH PHASE OF WORK TO BE INSPECTED BY THE ENGINEER PRIOR TO PROCEEDING TO THE NEXT PHASE OF WORK.
- BACKFILL IMMEDIATELY BEHIND ABUTMENTS AND AROUND PIER PILE CAPS TO BE "FILL AGAINST STRUCTURE" MATERIAL AS PER PROJECT SPECIFICATIONS.
- LOCATION OF ANCHOR BOLTS FOR TEMPORARY BRIDGE BEARINGS SHALL BE VERIFIED WITH THE TEMPORARY BRIDGE MANUFACTURER.
- BENT REINFORCING BAR TYPES REFER TO R.S.I.C. REINFORCING STEEL MANUAL OF STANDARD PRACTICE. TYPICAL BAR BENDS EXCEPT BAR BEND DIAMETERS AS PER PROJECT SPECIFICATION (U.N.O.).
- MISCELLANEOUS STEEL TO CAN/CSA-G40.21-350W.
- WELDING TO CAN/CSA-W59 (LATEST EDITION).

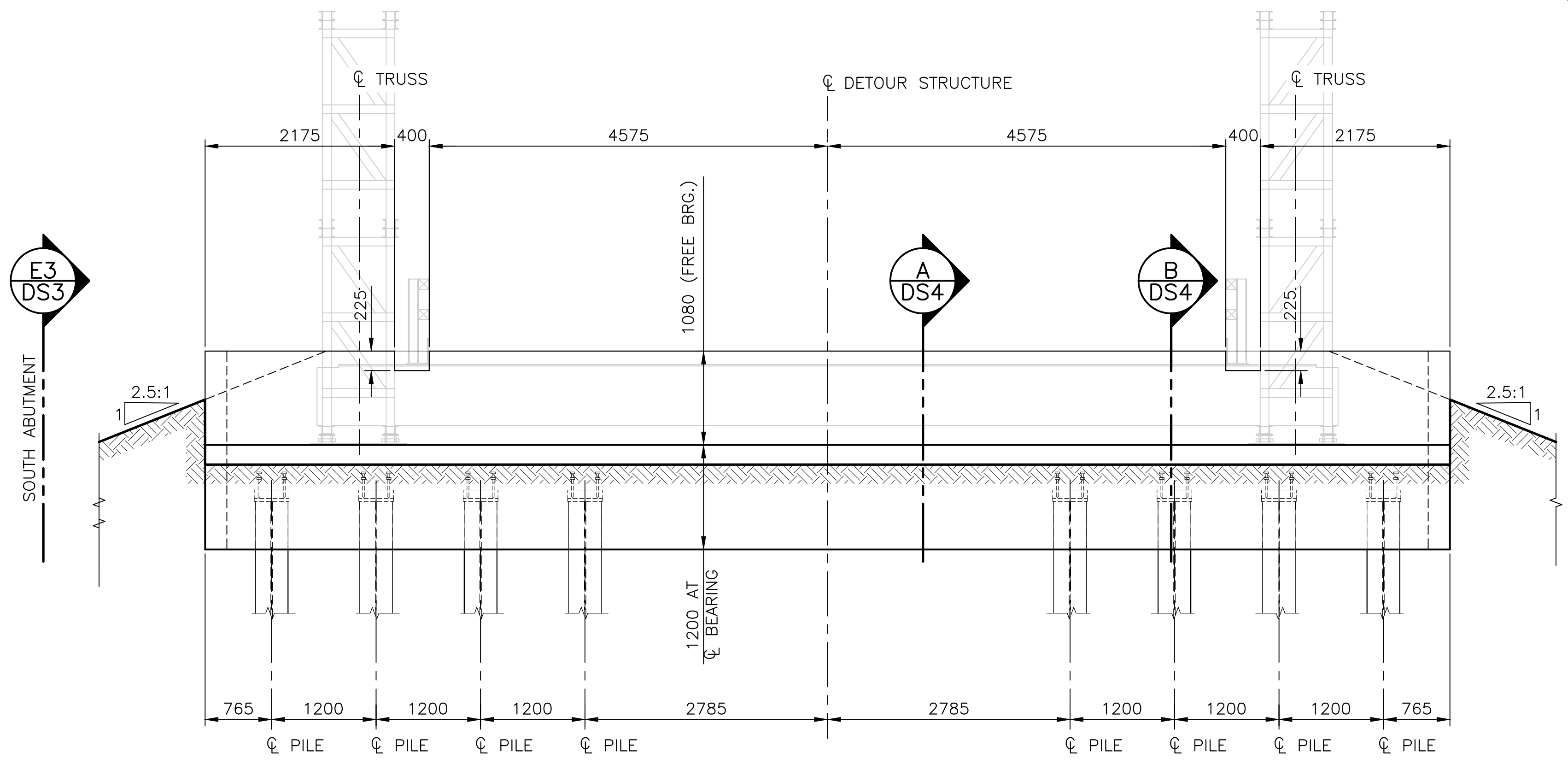
0	ISSUED FOR TENDER	JULY 19 2021
revisions		date

project WESTERN BROOK BRIDGE REPLACEMENT
GROS MORNE NATIONAL PARK

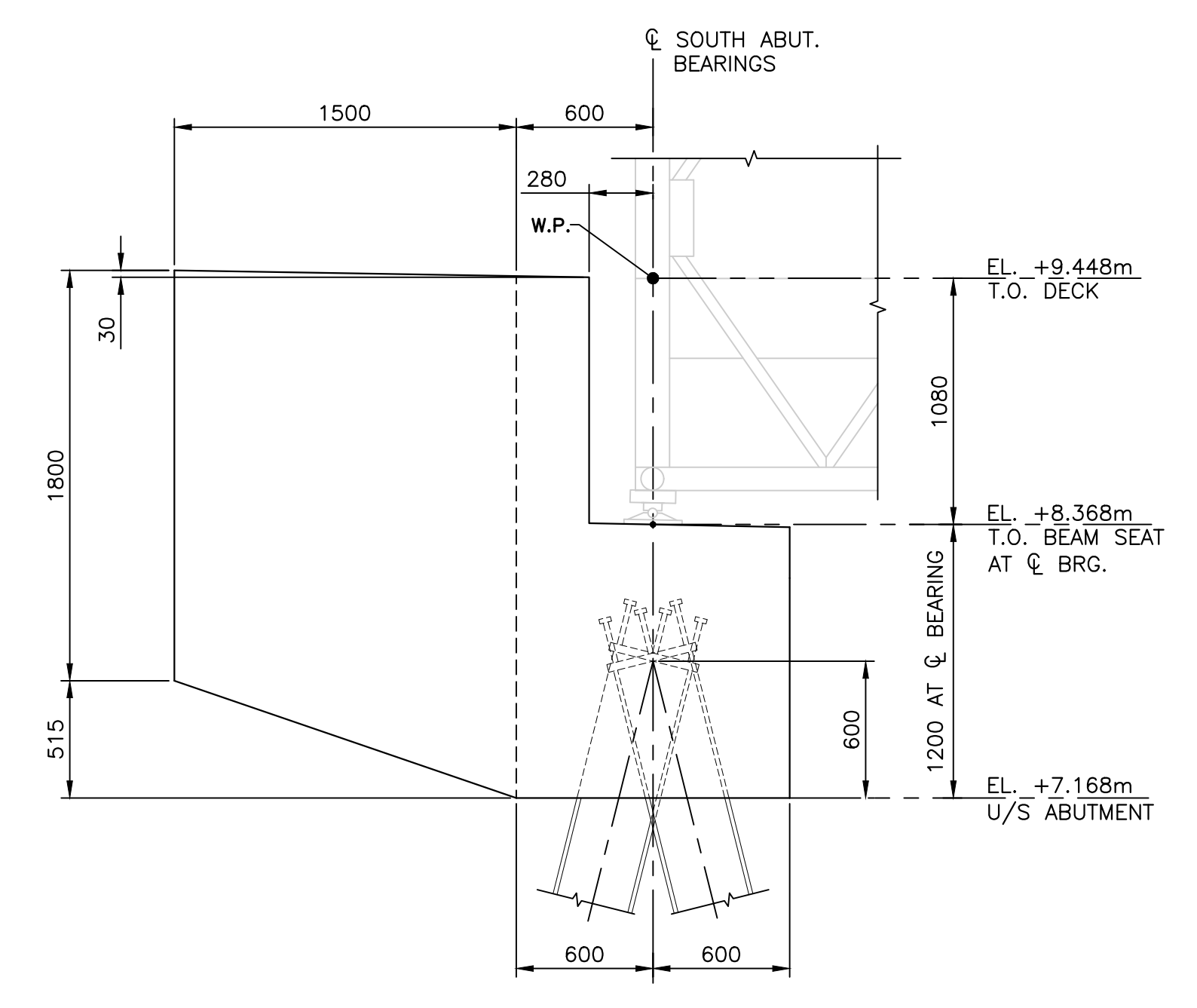
drawing design
ABUTMENT PLANS AND ELEVATION DETOUR STRUCTURE

designed THOMAS McNUTT	conçu
date APRIL 2020	
drawn WAYNE MORROW	dessiné
date APRIL 2020	
approved ROBBIE FRASER	approuvé
date APRIL 2020	
Tender	Soumission
PWOSC Project Manager	Administrateur de projets TP50C
project number	no. du projet
drawing no.	no. du dessin

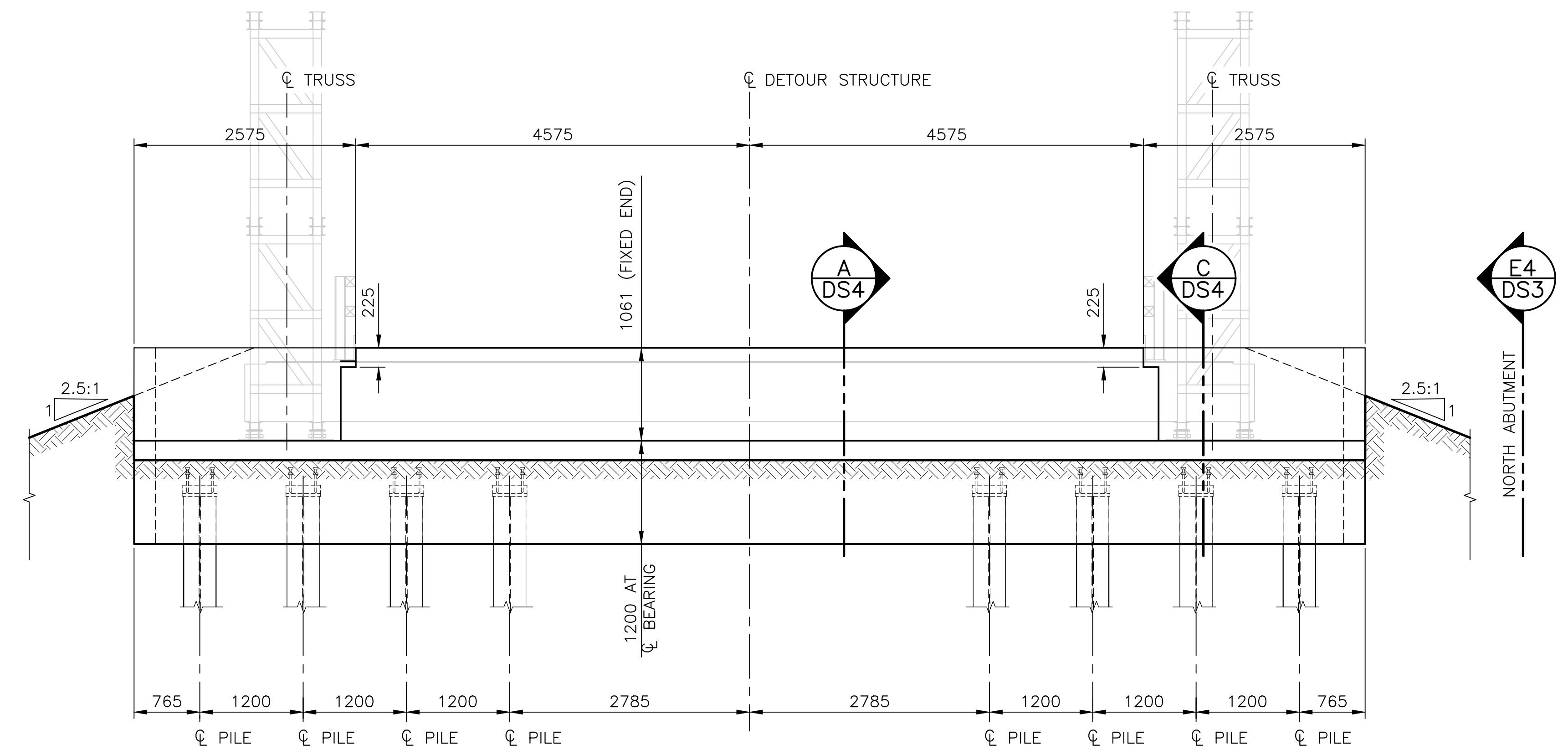
DS2



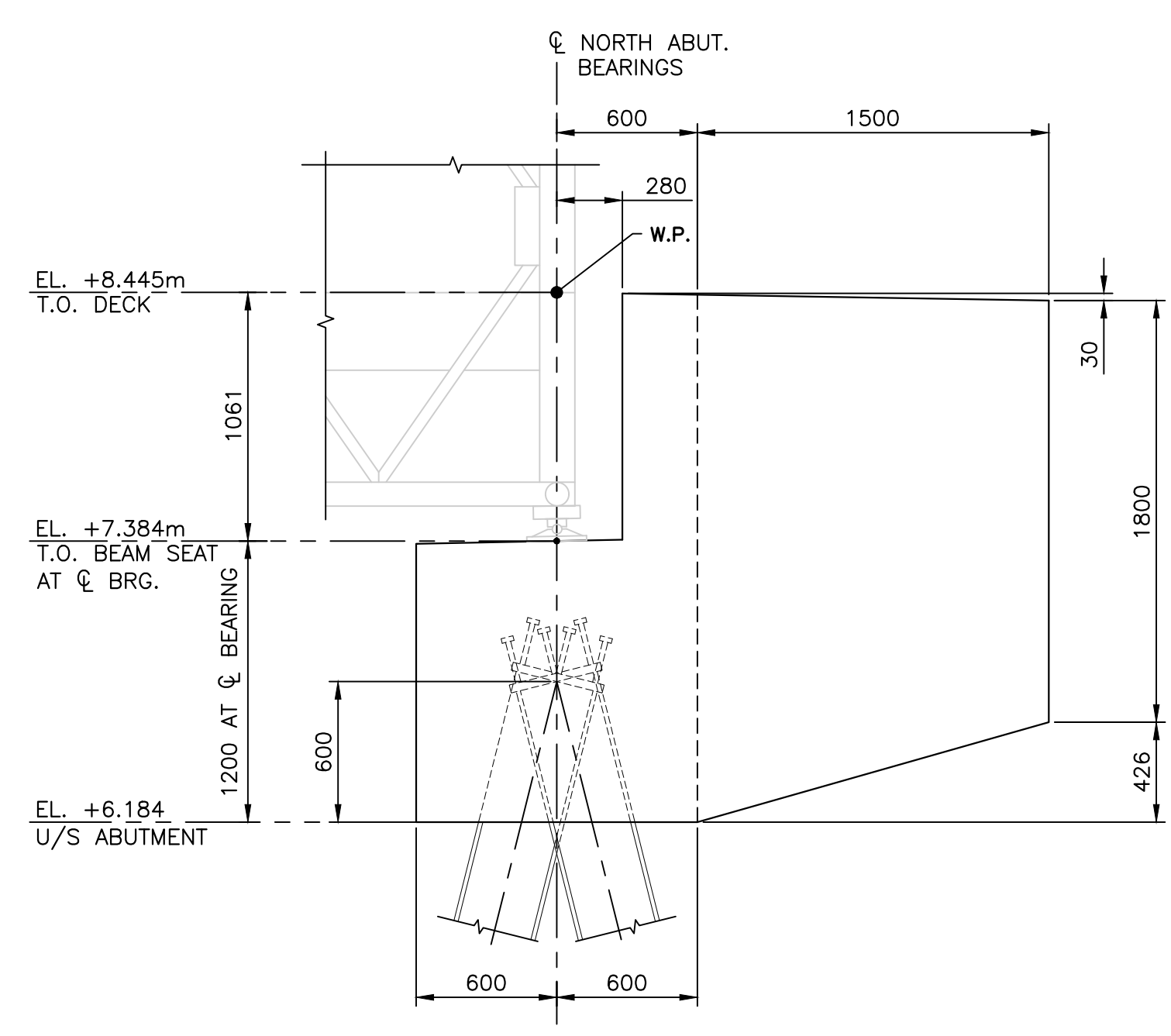
SOUTH ABUTMENT ELEVATION
SCALE : 1:50
E1 DS2



SOUTH ABUTMENT WINGWALL
SCALE : 1:25
E3 DS2

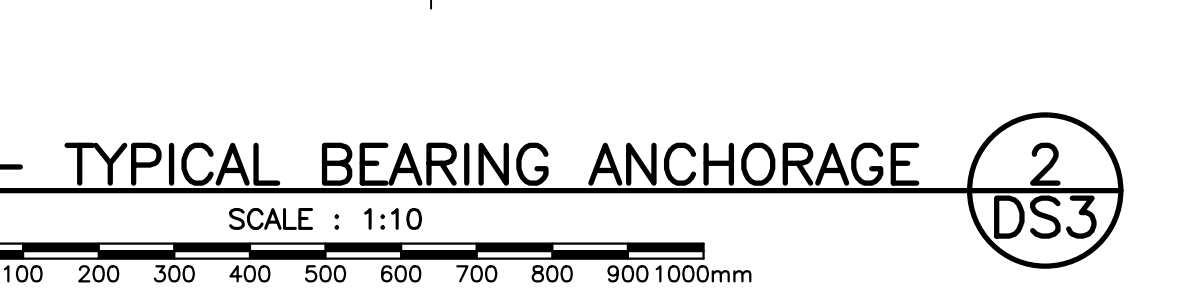
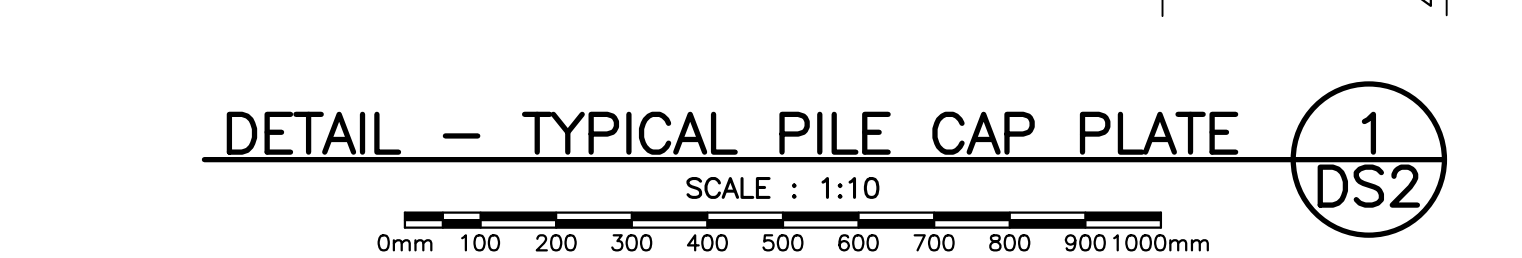
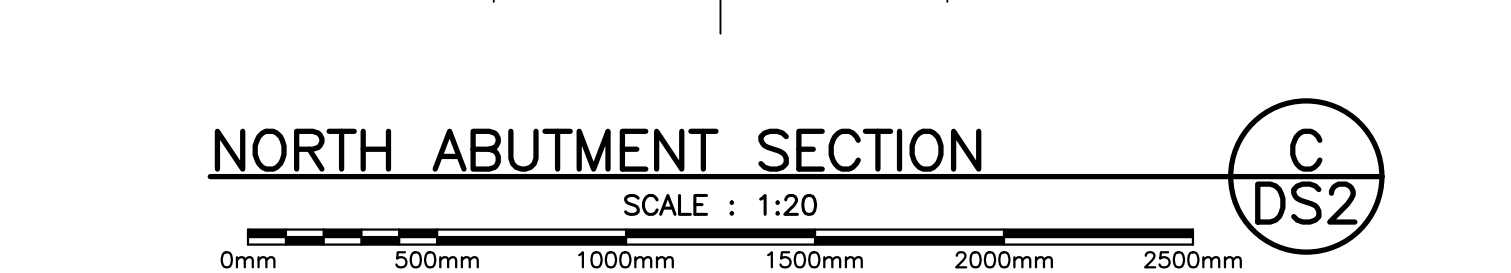
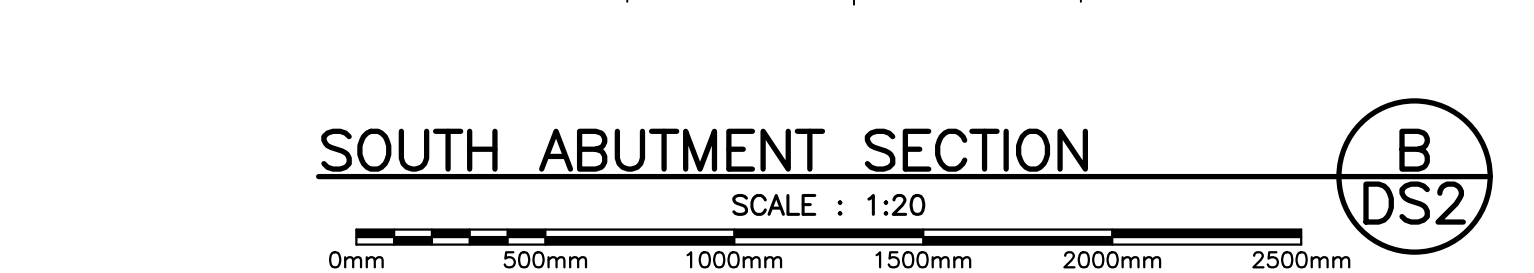
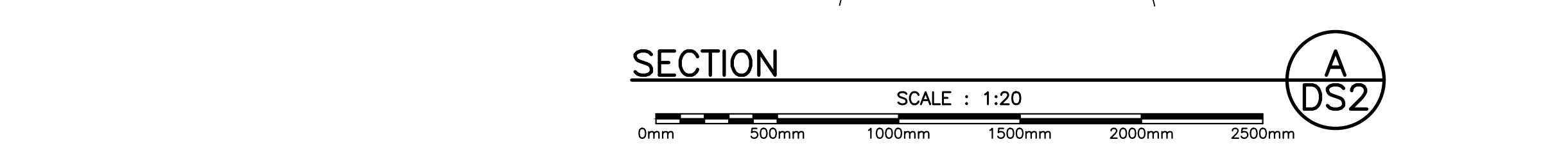


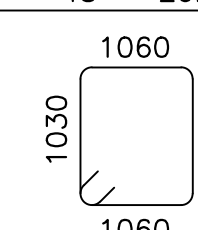
NORTH ABUTMENT ELEVATION
SCALE : 1:50
E2 DS2



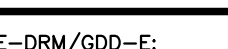
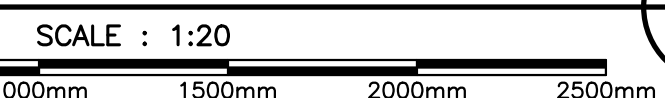
NORTH ABUTMENT WINGWALL
SCALE : 1:25
E4 DS2

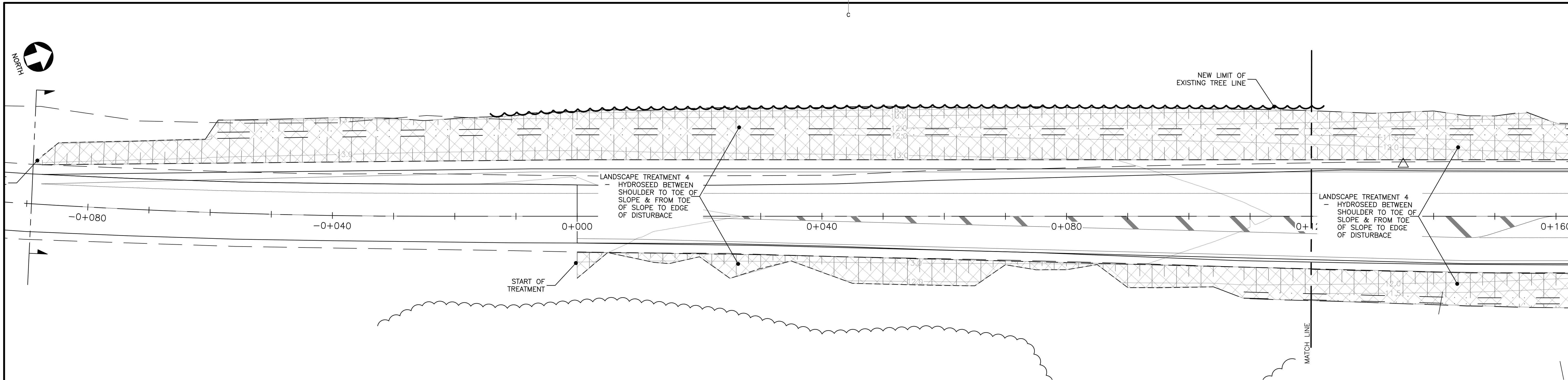
** - TO BE CONFIRMED WITH PANEL BRIDGE MANUFACTURER PRIOR TO CONSTRUCTION. ADJUST TABLE AS REQUIRED IF NECESSARY.

DS4

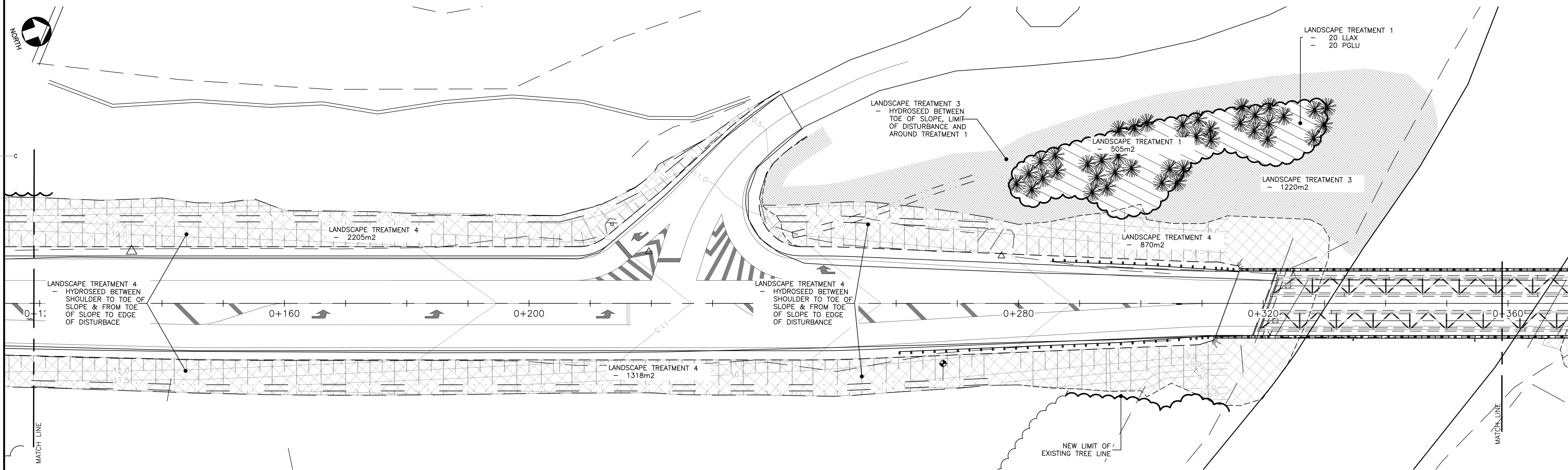


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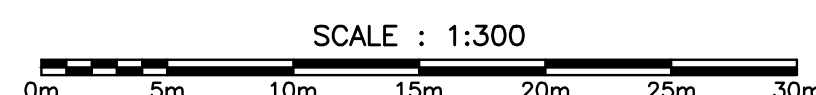




PLAN — PROPOSED NEW ROUTE (-0+080 TO 0+120)



PLAN — PROPOSED NEW ROUTE (0+120 TO 0+360)



LANDSCAPE TREATMENT 1 — REINSTATED TREE PLANTING
PLACE MINIMUM OF 750MM THICK OF ORGANIC MATERIAL.
PLANT TREES IN PLANTING HOLES WITH NEW PLANTING SOIL.
PLANT TREES RANDOMLY FOLLOWING ROUGH SPACING
INDICATED IN TABLE.
HYDROSEED EXPOSED SURFACE.

LANDSCAPE TREATMENT 2 — REINSTATED TREE PLANTING
PLACE MINIMUM OF 450MM THICK OF ORGANIC MATERIAL.
PLANT TREES IN PLANTING HOLES WITH NEW PLANTING SOIL.
PLANT TREES RANDOMLY & GROUPINGS OF 3-8 TREES.
HYDROSEED EXPOSED SURFACE.

LANDSCAPE TREATMENT 3 — HYDROSEEDING & TOPSOIL
PLACE MINIMUM OF 150MM THICK OF ORGANIC MATERIAL.
HYDROSEED EXPOSED SURFACE.

LANDSCAPE TREATMENT 4 — HYDROSEEDING ONLY
HYDROSEED EXPOSED SURFACE.

LEGEND		
EXISTING		PROPOSED
	EDGE OF ASPHALT	
	EDGE OF GRAVEL/SHOULDER	
	GUIDERAIL	
	TREELINE	
	SHRUB/GRASS LINE	
	LANDSCAPE TREATMENT 1	
	LANDSCAPE TREATMENT 2	
	LANDSCAPE TREATMENT 3	
	LANDSCAPE TREATMENT 4	
	DITCHLINE	
	SHORELINE/WATERCOURSE	
	POWER POLE c/w GUYWIRE	
	SIGN	
	CULVERT	
	OVERHEAD WIRE	

Parcs
Canada

Parks
Canada

HARBOURSIDE
Engineering Consultants

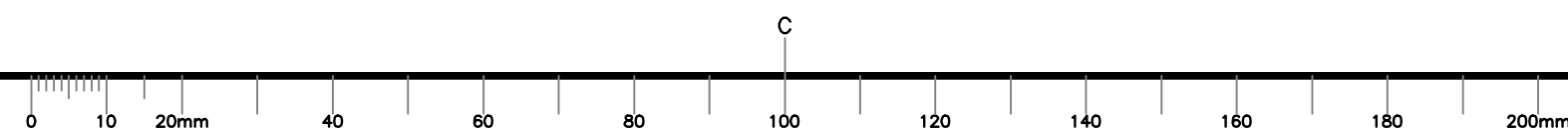
Vollick McKee Petersmann

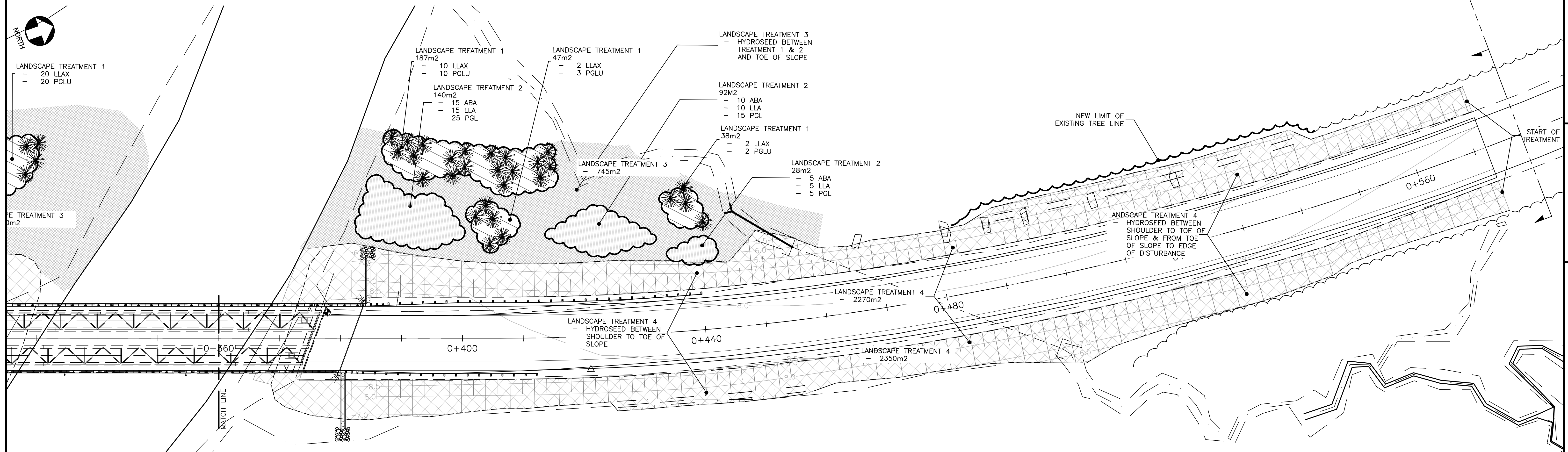
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21-JUL 19
JAMES W. D.
MCKEE

1	ISSUED FOR TENDER	19 JUL 2021
revisions		date
project	WESTERN BROOK BRIDGE REPLACEMENT	
drawing	GROS MORNE NATIONAL PARK	
designed	JAMES MCKEE	conçu
date	JULY 2021	
drawn	MELISSA BARKER/JAMES MCKEE	dessiné
date	JULY 2021	
approved	JAMES MCKEE	approuvé
date	JULY 2021	
Tender		Soumission
PWGSC Project Manager	Administrateur de projets TP50C	
project number	182009	no. du projet
drawing no.	L01	no. du dessin

E-DRM/GDD-E:





PLAN — PROPOSED NEW ROUTE (0+360 TO 0+560)

SCALE: 1:500
0m 10m 20m 30m 40m 50m

FULL PLANTING SCHEDULE								
CODE	QTY	BOTANICAL NAME	COMMON NAME	SIZE	COND.	SPACING	STAKING	REMARKS
ABA	30	Abies balsamiae	Balsam Fir	80cm	CG#3	1000 o.c.	—	Spaced randomly
LLA	30	Larix laricina	Tamarack	100cm	CG#3	1000 o.c.	—	Spaced randomly
LLAX	34	Larix laricina	Tamarack	200cm	W.B.	3000 o.c.	GUYED	
PGL	45	Picea glauca	White Spruce	80cm	CG#3	1000 o.c.	—	Spaced randomly
PGLU	35	Picea glauca	White Spruce	150cm	W.B.	3000 o.c.	GUYED	

NOTE: SUBSTITUTIONS TO PLANTS AS SPECIFIED ABOVE ARE NOT ACCEPTABLE UNLESS WRITTEN PERMISSION HAS BEEN OBTAINED FOR SPECIES / VARIETY, SIZE, QUANTITY &/OR CONDITION FROM DEPARTMENTAL REPRESENTATIVE

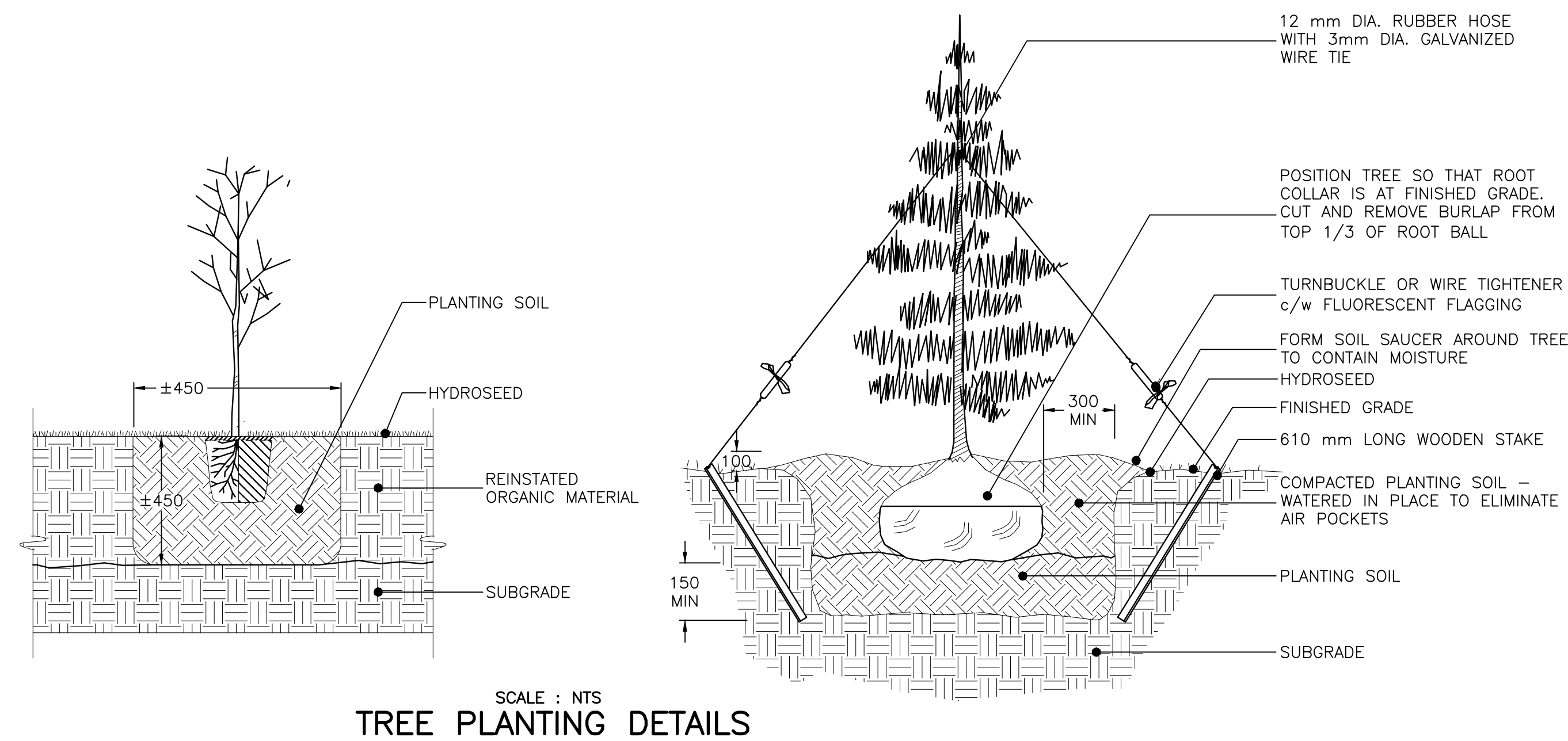
LANDSCAPE TREATMENT 1 — REINSTATED TREE PLANTING
PLACE MINIMUM OF 750MM THICK OF ORGANIC MATERIAL.
PLANT TREES IN PLANTING HOLES WITH NEW PLANTING SOIL.
PLANT TREES RANDOMLY FOLLOWING ROUGH SPACING INDICATED IN TABLE.
HYDROSEED EXPOSED SURFACE.

LANDSCAPE TREATMENT 2 — REINSTATED TREE PLANTING
PLACE MINIMUM OF 450MM THICK OF ORGANIC MATERIAL.
PLANT TREES IN PLANTING HOLES WITH NEW PLANTING SOIL.
PLANT TREES RANDOMLY & GROUPINGS OF 3-8 TREES.
HYDROSEED EXPOSED SURFACE.

LANDSCAPE TREATMENT 3 — HYDROSEEDING & TOPSOIL
PLACE MINIMUM OF 150MM THICK OF ORGANIC MATERIAL.
HYDROSEED EXPOSED SURFACE.

LANDSCAPE TREATMENT 4 — HYDROSEEDING ONLY
HYDROSEED EXPOSED SURFACE.

LEGEND		
EXISTING		PROPOSED
— — — — —	EDGE OF ASPHALT	— — — — —
— — — — —	EDGE OF GRAVEL/SHOULDER	— — — — —
— — — — —	GUIDERAIL	— — — — —
— — — — —	TREELINE	— — — — —
— — — — —	SHRUB/GRASS LINE	— — — — —
— — — — —	LANDSCAPE TREATMENT 1	— — — — —
— — — — —	LANDSCAPE TREATMENT 2	— — — — —
— — — — —	LANDSCAPE TREATMENT 3	— — — — —
— — — — —	LANDSCAPE TREATMENT 4	— — — — —
— — — — —	DITCHLINE	— — — — —
— — — — —	SHORELINE/WATERCOURSE	— — — — —
— — — — —	POWER POLE c/w GUYWIRE	— — — — —
— — — — —	SIGN	— — — — —
— — — — —	CULVERT	— — — — —
— — — — —	OVERHEAD WIRE	— — — — —

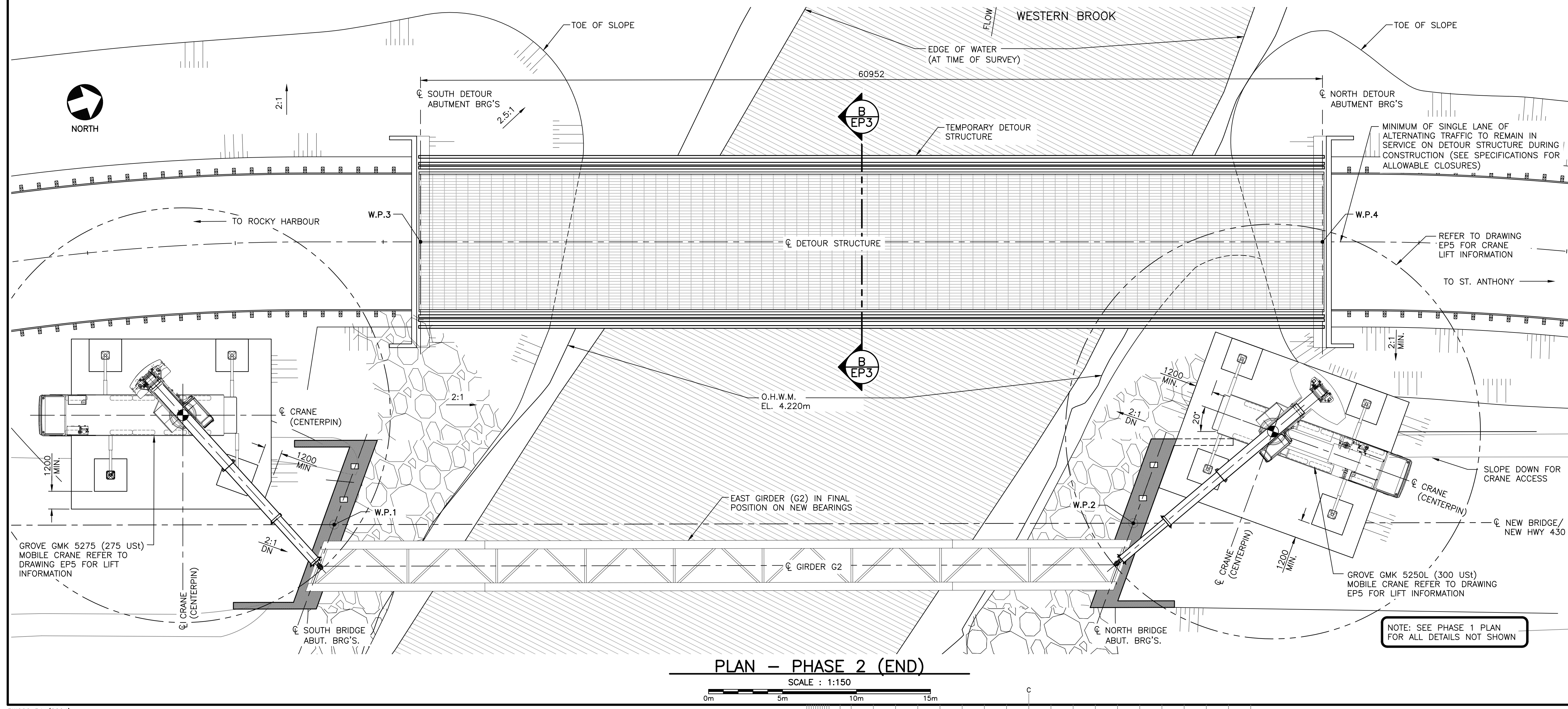
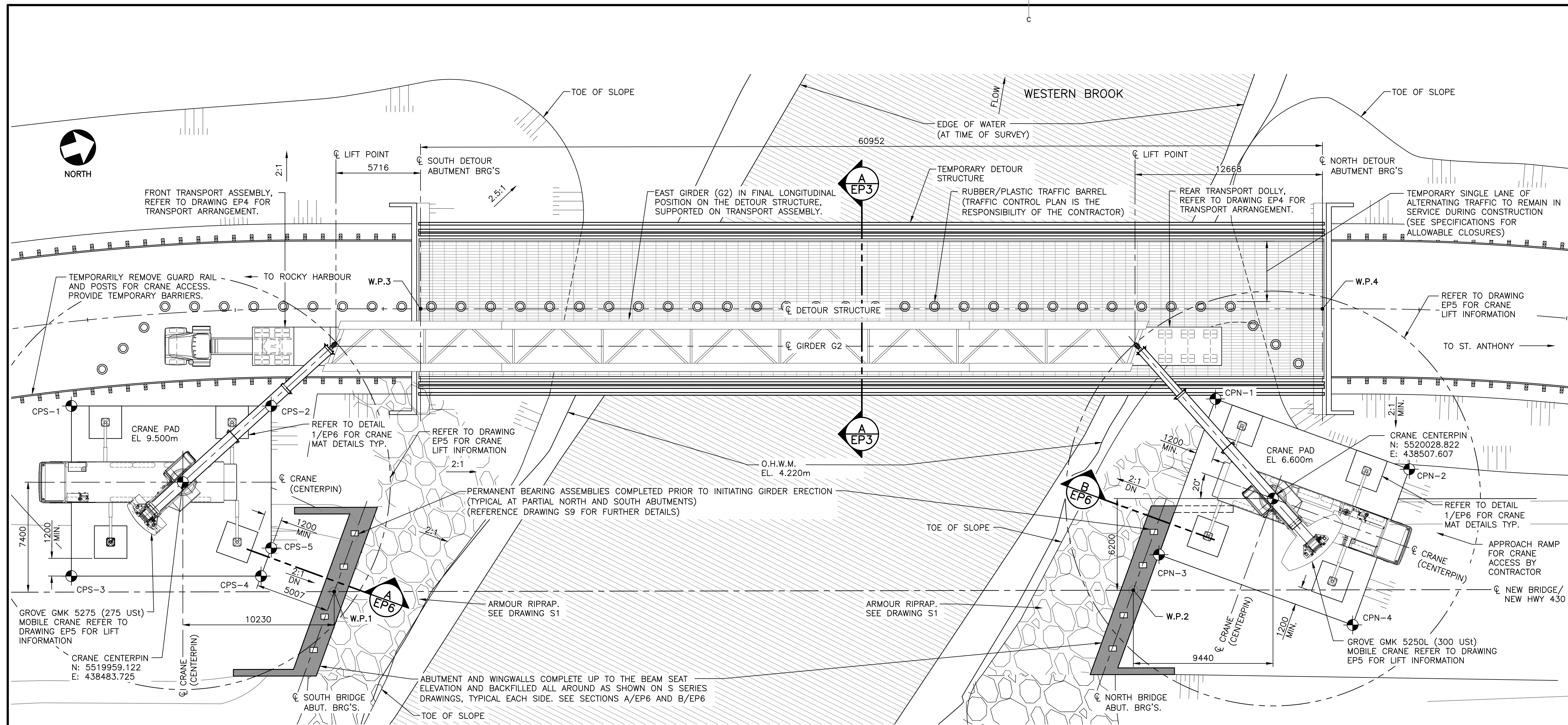


Association des architectes-paysagistes des provinces atlantiques
21-JUL 19 2021
JAMES W. D. MCKEE

1	ISSUED FOR TENDER	19 JUL 2021
revisions		date
project	WESTERN BROOK BRIDGE REPLACEMENT	
	GROS MORNE NATIONAL PARK	

LANDSCAPE FINISH TREATMENT PLAN

designed	JAMES MCKEE	conçu
date	JULY 2021	
drawn	MELISSA BARKER/JAMES MCKEE	dessiné
date	JULY 2021	
approved	JAMES MCKEE	approuvé
date	JULY 2021	
Tender		Soumission
PWGSC Project Manager	Administrateur de projets TPSGC	
project number	182009	no. du projet
drawing no.	L02	no. du dessin



NOTES:

1. REFER TO DRAWING EP3 FOR GENERAL NOTES.

PHASE 1 PROCEDURE:

1. INSTALL NEW BRIDGE ABUTMENTS, BACKFILLING AND FINAL SLOPE TREATMENTS COMPLETED UP TO THE BEAM SEAT ELEVATIONS, COMPLETE CRANE PADS IN ACCORDANCE WITH CRANE PAD REQUIREMENTS.
2. MOBILIZE GROVE GMK 5250L (300USL) MOBILE CRANE ON THE NORTH CRANE PAD AND THE GROVE GMK 5275 (275USL) MOBILE CRANE ON THE SOUTH CRANE PAD. POSITION AS INDICATED ON THE DRAWINGS. ALL CRANE OUTRIGGERS SHALL BE FULLY EXTENDED AND SUPPORTED ON CRANE MATS AS DETAILED ON DRAWING EP6. TRAFFIC CONTROL MEASURES FOR THE MOBILIZATION AND INSTALLATION OF BOTH CRANE PADS AND THE CRANES ARE THE RESPONSIBILITY OF THE CONTRACTOR. REFERENCE NOTES ON EP4/EP5 AND PROJECT SPECIFICATIONS FOR CRANE/MATERIAL ERECTION PHASING SUBSTITUTION.
3. ASSEMBLE FULL LENGTH OF EAST GIRDER (G2) AND POSITION ON THE TRANSPORTER ASSEMBLY (TRUCK, JEEP AND REAR SUPPORT DOLLY) IN ACCORDANCE WITH DRAWING EP4. GIRDER ASSEMBLY IN LAY DOWN AREA IS THE RESPONSIBILITY OF THE CONTRACTOR. GIRDER SHALL BE FULLY ASSEMBLED BEFORE MOVING ONTO DETOUR STRUCTURE.
4. WITH TRAFFIC CONTROL MEASURES IN PLACE (BY CONTRACTOR), TEMPORARILY CLOSE THE EXISTING LANES TO TRAFFIC FROM THE OFF-SITE GIRDER ASSEMBLY YARD TO THE FAR SIDE OF THE TEMPORARY DETOUR STRUCTURE. TRAFFIC CONTROL PLAN IS THE RESPONSIBILITY OF THE CONTRACTOR (SEE SPECIFICATIONS FOR ALLOWABLE CLOSURE TIMES/PROCEDURES).
5. WITH BOTH LANES CLOSED TO TRAFFIC AS INDICATED IN NOTE 4, TRANSPORT FULLY ASSEMBLED EAST GIRDER (G2) ACROSS THE TEMPORARY DETOUR STRUCTURE INTO ITS FINAL LONGITUDINAL POSITION AS INDICATED. ENSURE PROPER TRANSVERSE GIRDER ALIGNMENT (EAST-WEST) ON THE TEMPORARY DETOUR STRUCTURE AS INDICATED ON SECTION A/EP3. TRANSPORT SPEED ACROSS THE TEMPORARY DETOUR STRUCTURE SHALL NOT EXCEED 5 km/hr. DRAWINGS SHOW TRANSPORTER FACING SOUTH, TRANSPORTER MAY ALTERNATIVELY FACE NORTH DEPENDING ON CONTRACTOR'S LAY DOWN LOCATION AND PROCEDURES.
6. RE-CONFIGURE TRAFFIC CONTROL MEASURES LOCALLY AT THE TEMPORARY DETOUR STRUCTURE TO ESTABLISH A SINGLE LANE OF ALTERNATING TRAFFIC INDICATED ON SECTION A/EP3 (THIS INCLUDES INSTALLATION OF TEMPORARY TRAFFIC DRUMS AS REQUIRED). TRAFFIC CONTROL PLAN IS THE RESPONSIBILITY OF THE CONTRACTOR.
7. WITH GIRDER IN A STATIC CONDITION IN ITS FINAL LONGITUDINAL AND TRANSVERSE POSITIONS ON THE TEMPORARY DETOUR STRUCTURE AND TRAFFIC CONTROL MEASURES IN PLACE, A SINGLE LANE OF TRAFFIC MAY BE RE-OPENED ACROSS THE EXISTING STRUCTURE AS INDICATED ON SECTION A/EP3. MAXIMUM ALLOWABLE TIME TO COMPLETE STEPS 4-7 SHALL BE AS PER THE PROJECT SPECIFICATIONS.
8. CONNECT CRANES TO GIRDER AT LOCATIONS INDICATED ON DRAWING EP5.
9. END OF PHASE 1.

PHASE 2 PROCEDURE:

1. WITH CONNECTIONS COMPLETED BETWEEN THE GIRDER AND THE NORTH/SOUTH CRANES, SIMULTANEOUSLY ENGAGE CRANES AND REMOVE SLACK FROM RIGGING (STRAIN COMPATIBLE CONDITION).
2. FULLY CLOSE THE TEMPORARY DETOUR STRUCTURE TO TRAFFIC (SEE SPECIFICATIONS FOR ALLOWABLE CLOSURE TIMES/PROCEDURES).
3. DISCONNECT GIRDER FROM THE TRANSPORTER ASSEMBLY AT THE JEEP AND DOLLY SUPPORTS.
4. SIMULTANEOUSLY RAISE BOTH ENDS OF THE GIRDER AND TRANSFER LOAD FROM THE TRANSPORTER ASSEMBLY TO THE CRANES.
5. IN A SLOW AND CONTROLLED MANNER, PLACE THE EAST GIRDER (G2) INTO FINAL POSITION ON THE COMPLETED EAST GIRDER BEARINGS ON THE NEW BRIDGE ABUTMENTS AS INDICATED. CRANE OPERATORS SHALL ENSURE THE CRANE LINES ARE PLUMB AND MAXIMUM LIFT RADII ARE NOT EXCEEDED AT ANY TIME DURING THE LIFT.
6. RE-OPEN SINGLE LANE OF ALTERNATING TRAFFIC ON THE TEMPORARY DETOUR STRUCTURE.
7. SIMULTANEOUSLY TRANSFER THE GIRDER LOAD FROM THE MOBILE CRANES TO THE NORTH AND SOUTH ABUTMENT BEARING ASSEMBLIES. DISCONNECT THE GIRDER FROM THE CRANES.
8. REMOVE TRANSPORTER ASSEMBLY FROM THE TEMPORARY DETOUR STRUCTURE.
9. REMOVE TRAFFIC CONTROL MEASURES FROM THE EXISTING BRIDGE STRUCTURE AND RE-OPEN TO TWO LANES OF TRAFFIC IN NORTHBOUND AND SOUTHBOUND LANES ACROSS THE EXISTING BRIDGE.
10. END OF PHASE 2.

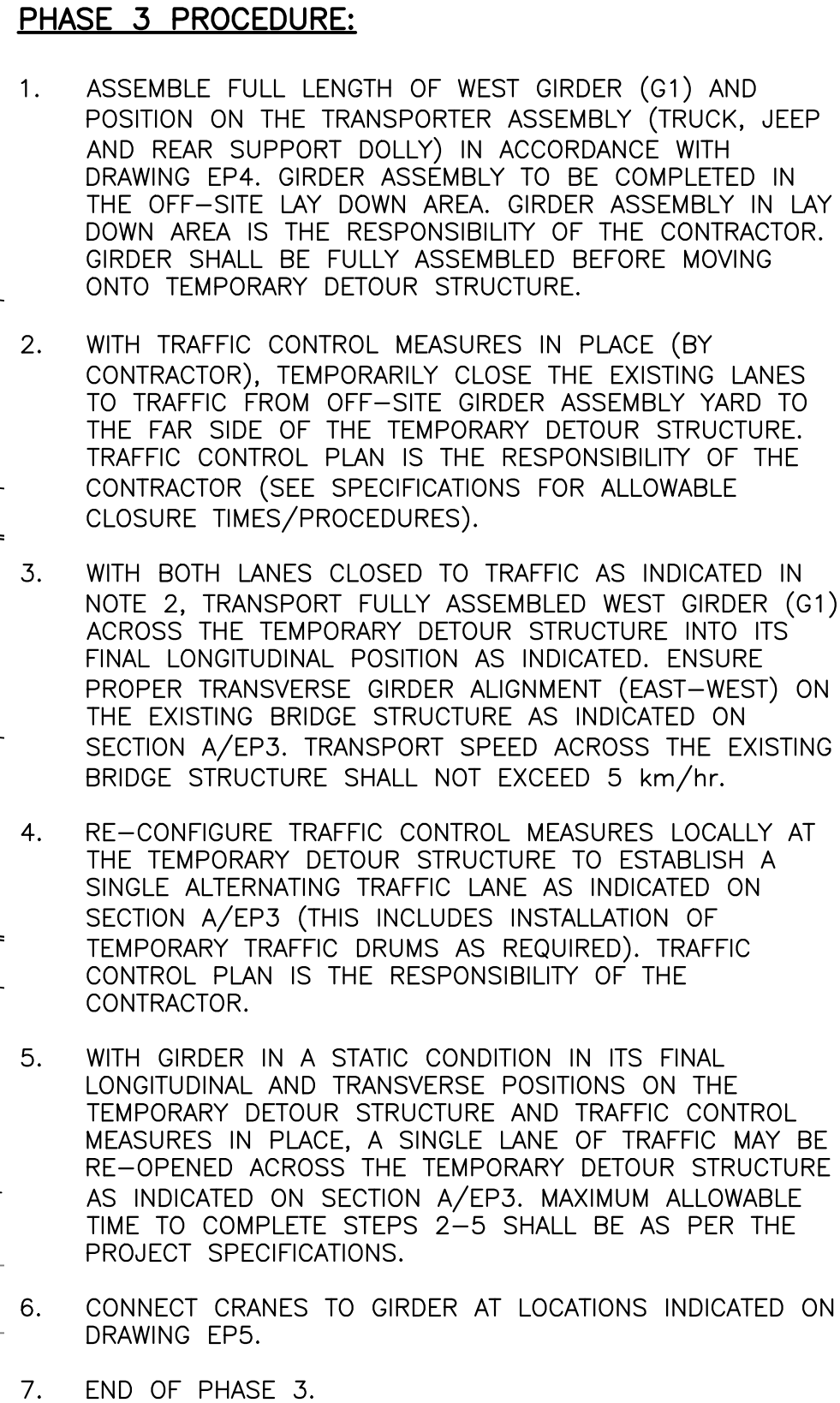
SOUTH CRANE PAD COORDINATES

LOCATION	NORTHING (m)	EASTING (m)
CPS-1	5519953.556	438476.493
CPS-2	5519966.396	438480.679
CPS-3	5519949.991	438487.427
CPS-4	5519962.179	438491.400
CPS-5	5519963.419	438489.810


NORTH CRANE PAD COORDINATES

LOCATION	NORTHING (m)	EASTING (m)
CPN-1	5520027.217	438500.004
CPN-2	5520038.190	438508.579
CPN-3	5520020.320	438508.828
CPN-4	5520031.293	438517.404

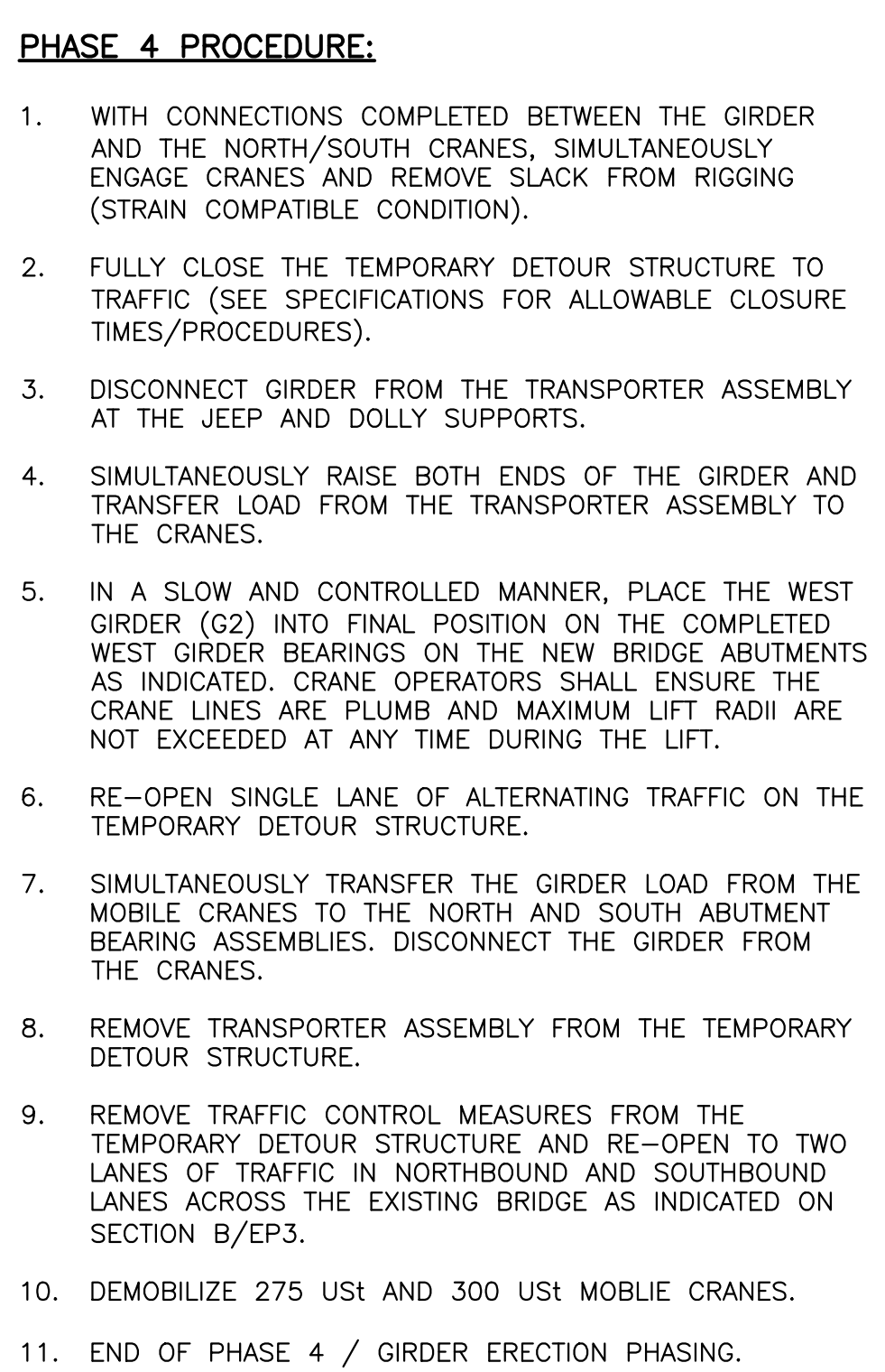
NOTE: SEE PHASE 1 PLAN FOR ALL DETAILS NOT SHOWN



SCALE : 1:150



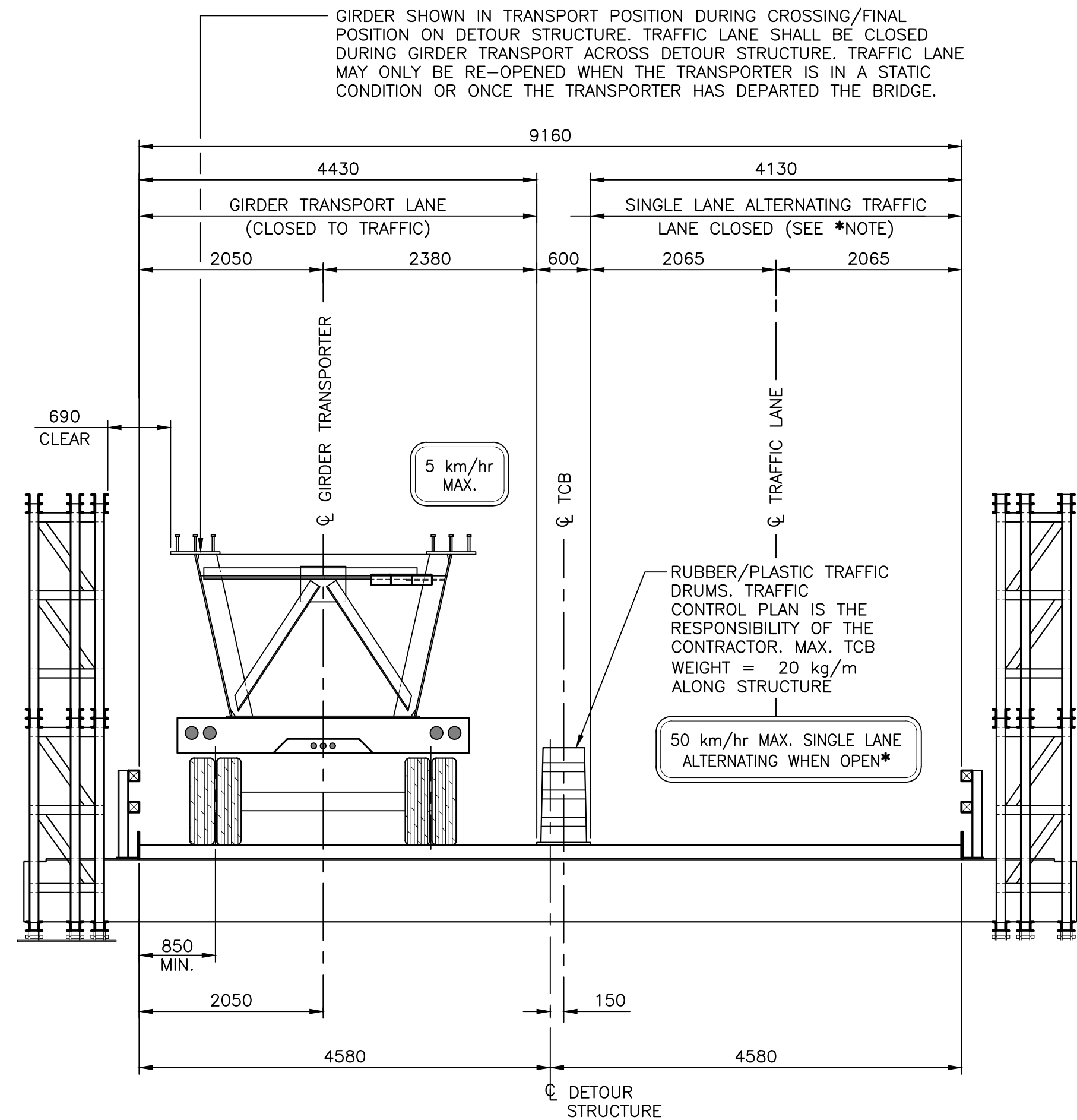
0m 5m 10m 15m



SCALE : 1:150



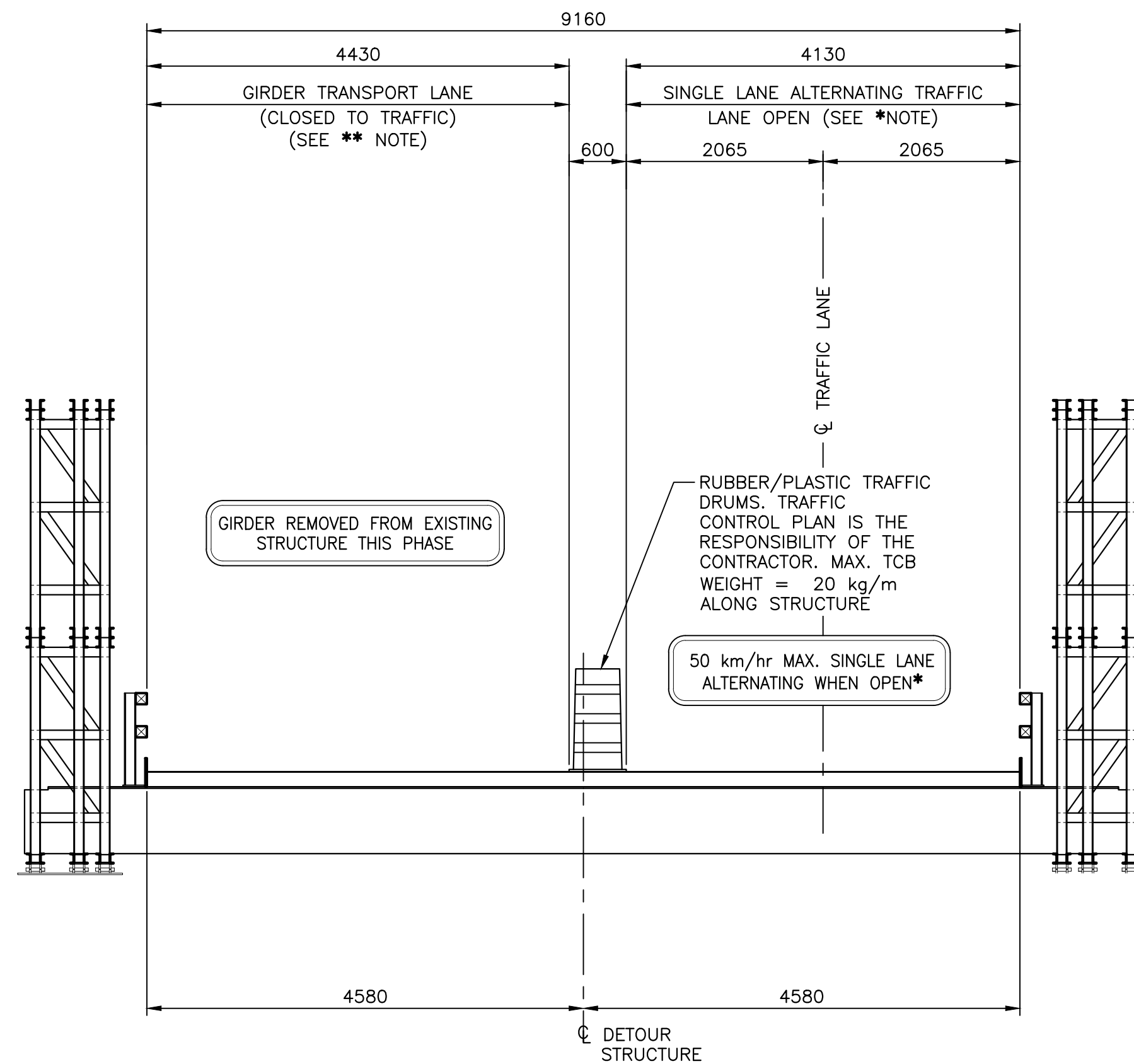
A horizontal scale bar with alternating black and white segments. It is marked with '0m' at the left end, '5m' at the first major tick, '10m' at the second major tick, and '15m' at the right end.



SECTION PHASE 1 AND PHASE 3

SCALE : 1:50

A
EP1



SECTION PHASE 2 AND PHASE 4

SCALE : 1:50

B
EP1

GIRDER ERECTION GENERAL NOTES:

- DESIGN, FABRICATION AND CONSTRUCTION AS PER THE REQUIREMENTS OF CAN/CSA S6-19.
- DIMENSIONS IN mm UNLESS NOTED OTHERWISE. ELEVATIONS IN METERS (m).
- THE GIRDER ERECTION DESIGN (INCLUDING THE CRANE LIFTS) IS BASED ON A MAXIMUM GUSTING WIND SPEED OF 30 km/hr. CONTRACTOR SHALL MONITOR WEATHER FORECAST AND NOT PROCEED WITH GIRDER ERECTION WHEN WIND GUSTS ARE FORECAST TO EXCEED 30 km/hr (FOR ANY INTERVAL EXCEEDING 3 SECONDS). IF WIND CONDITIONS CHANGE DURING ERECTION AND APPROACH OR EXCEED THE 30 km/hr THRESHOLD, GIRDER SHALL BE REMOVED FROM THE DETOUR BRIDGE STRUCTURE BY MEANS OF THE TRANSPORTER ASSEMBLY OR BE PLACED ON THE FINAL ABUTMENT BEARINGS AT THE NORTH AND SOUTH ABUTMENTS OF THE NEW STRUCTURE (RELEASED FROM THE CRANES).
- CARE SHALL BE TAKEN DURING ERECTION TO MINIMIZE IMPACT (DYNAMIC) EFFECTS.
- PERMANENT BEARING INSTALLATION PROCEDURE IS THE RESPONSIBILITY OF THE CONTRACTOR.
- STABILITY OF THE CRANES DURING ERECTION IS THE RESPONSIBILITY OF THE CONTRACTOR. RIGGING AND CONNECTIONS TO THE GIRDER ARE THE RESPONSIBILITY OF THE CONTRACTOR. REFER TO DRAWING EP5 FOR THE GIRDER LIFT LOCATIONS AND CRANE LIFT INFORMATION. REFER TO EP DRAWINGS FOR CRANE PLACEMENT AND CRANE MAT / OUTRIGGER BEARING PAD REQUIREMENTS.
- SECONDARY CRANE REQUIREMENTS TO FACILITATE INSTALLATION OF MISCELLANEOUS ITEMS NOT COVERED SPECIFICALLY IN THE ERECTION PHASING DRAWINGS, INCLUDING THE GIRDER ASSEMBLY, ARE THE RESPONSIBILITY OF THE CONTRACTOR.
- THE NEW ABUTMENTS HAVE NOT BEEN REVIEWED FOR SURCHARGE EFFECTS FROM LARGE MACHINERY OR SECONDARY CRANES / BOOM TRUCKS BEYOND WHAT IS INDICATED WITHIN THE GIRDER ERECTION PACKAGE (I.E. THE 220t AND 250t MOBILE CRANES WITH THEIR RESPECTIVE POSITIONING AS INDICATED). ENSURE ALL LARGE MACHINERY OR SECONDARY CRANES / BOOM TRUCKS ARE KEPT OUT OF THE INFLUENCE ZONE OF THE ABUTMENT AT ALL TIMES UNLESS OTHERWISE APPROVED BY THE DEPARTMENTAL REPRESENTATIVE.
- STABILITY OF THE GIRDER SEGMENTS DURING TRANSPORT IS THE RESPONSIBILITY OF THE CONTRACTOR. REFER TO DRAWING EP4 FOR ASSUMED GIRDER TRANSPORT ARRANGEMENT AND RESULTING AXLE LOADS. CONTRACTOR TO SUBMIT ACTUAL TRANSPORTER DETAILS TO THE DEPARTMENTAL REPRESENTATIVE FOR REVIEW AND APPROVAL PRIOR TO PROCEEDING WITH GIRDER ERECTION (REFER TO PROJECT SPECIFICATIONS FOR SUBMITTAL SCHEDULE).
- TRAFFIC CONTROL PLAN IS THE RESPONSIBILITY OF THE CONTRACTOR.
- THE DETOUR BRIDGE STRUCTURE HAS BEEN REVIEWED BY HEC AND THE BRIDGE SUPPLIER TO RESIST AND TRANSFER LOADS RESULTING FROM THE GIRDER ERECTION SEQUENCE AS DETAILED IN THE EP SERIES DRAWINGS. ANY CHANGES TO THE PROPOSED CRANE / GIRDER TRANSPORTER CONFIGURATIONS SHALL REQUIRE REVIEW AND APPROVAL BY THE DEPARTMENTAL REPRESENTATIVE. REFER TO PROJECT SPECIFICATIONS FOR MORE INFORMATION.
- TRANSPORT SPEED OF THE FULLY ASSEMBLED GIRDER ACROSS THE TEMPORARY DETOUR STRUCTURE SHALL NOT EXCEED 5 km/hr.
- A DEPARTMENTAL REPRESENTATIVE SHALL COMPLETE A SITE VISIT TO VERIFY THE CRANE PADS AND CRANE SET-UPS HAVE BEEN COMPLETED IN ACCORDANCE WITH THE CONTRACT DRAWINGS PRIOR TO INITIATING GIRDER ERECTION PHASING.
- A DEPARTMENTAL REPRESENTATIVE SHALL BE PRESENT ON SITE DURING PHASE 1/EP1 AND PHASE 3/EP2 TO CONFIRM THE POSITIONING OF THE GIRDER TRANSPORT ACROSS THE TEMPORARY DETOUR.
- DO NOT SCALE FROM DRAWINGS.
- REFER TO S SERIES DRAWINGS FOR NEW BRIDGE STRUCTURE.
- REFER TO DS SERIES DRAWINGS FOR DETOUR STRUCTURE.
- THE TEMPORARY DETOUR STRUCTURE SHALL BE FULLY CLOSED TO TRAFFIC WHILE LIFTING GIRDERS OFF THE GIRDER TRANSPORTER. REFER TO DRAWINGS EP1 AND EP2 FOR DETAILS.

0	ISSUED FOR TENDER	JULY 19 2021
revisions		date

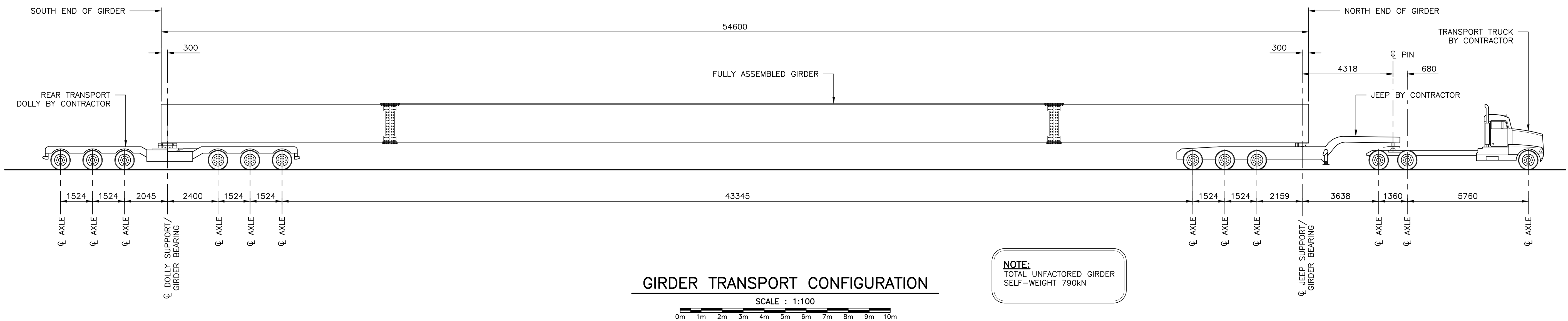
project WESTERN BROOK
BRIDGE REPLACEMENT
GROS MORNE
NATIONAL PARK

drawing design
GIRDER ERECTION
DETOUR STRUCTURE
SECTIONS AND
GENERAL NOTES

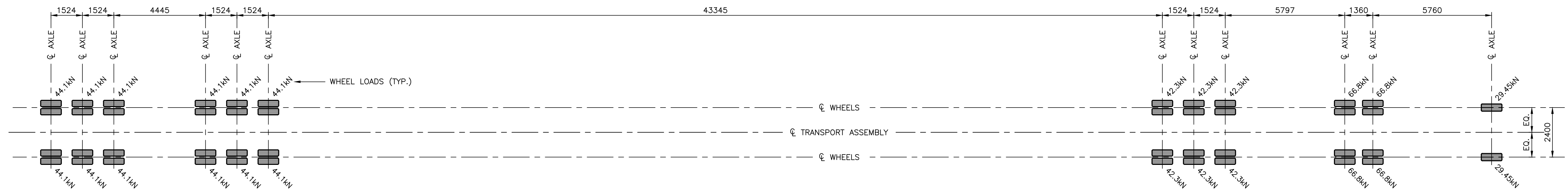
designed	THOMAS McNUTT	conçu
date	JANUARY 2021	
drawn	MIKE HARDY	dessiné
date	JANUARY 2021	
approved	ROBBIE FRASER	approuvé
date	JANUARY 2021	
Tender		Soumission

PWGSC Project Manager	Administrateur de projets TP50C
project number	no. du projet
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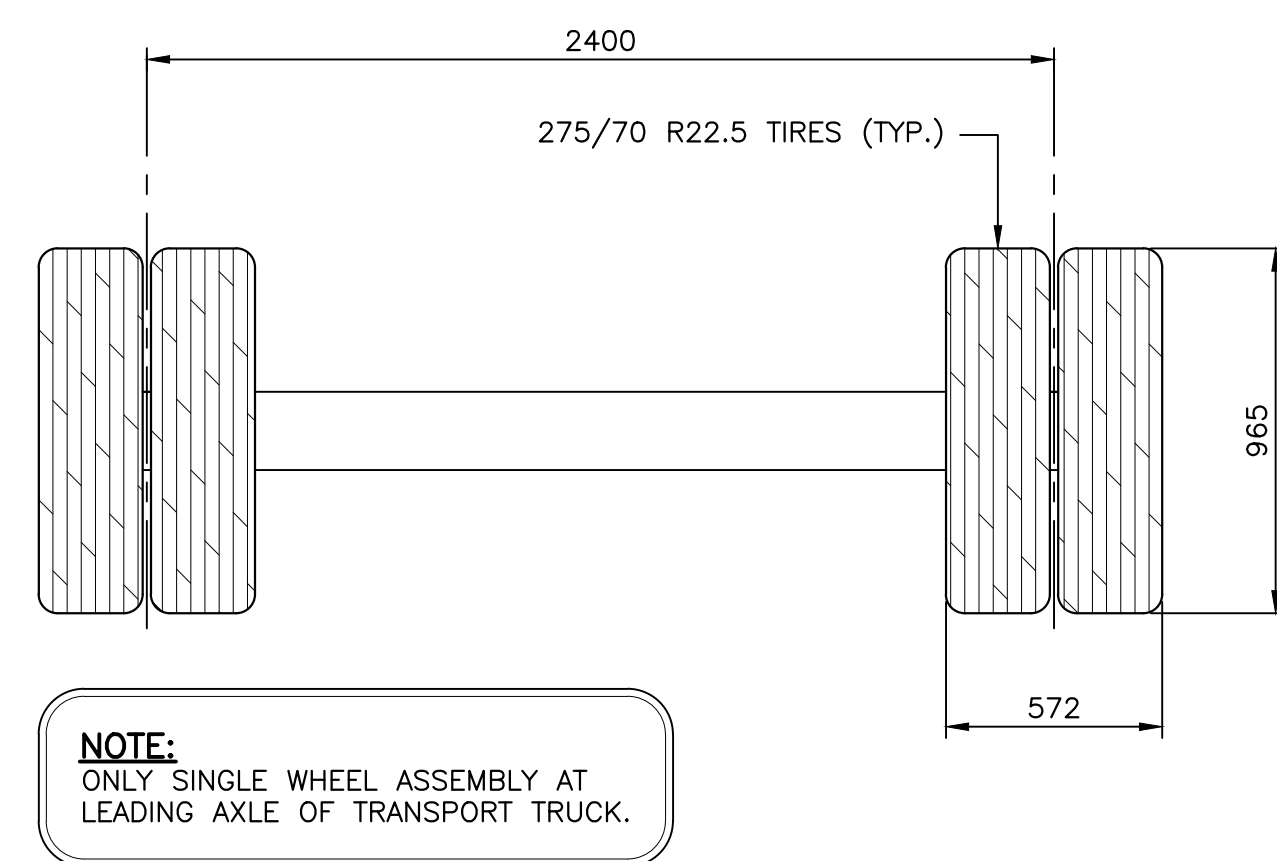
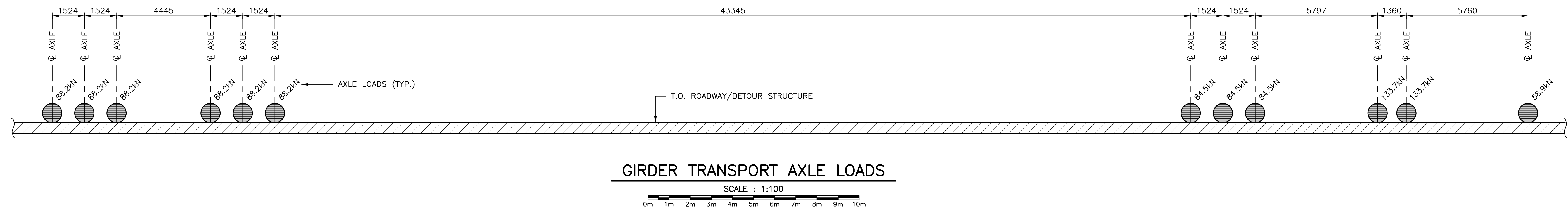
drawing no. EP3
no. du dessin



DESIGN GIRDER
TRANSPORTER
LOADING PLAN



DESIGN GIRDER
TRANSPORTER
LOADING
ELEVATION



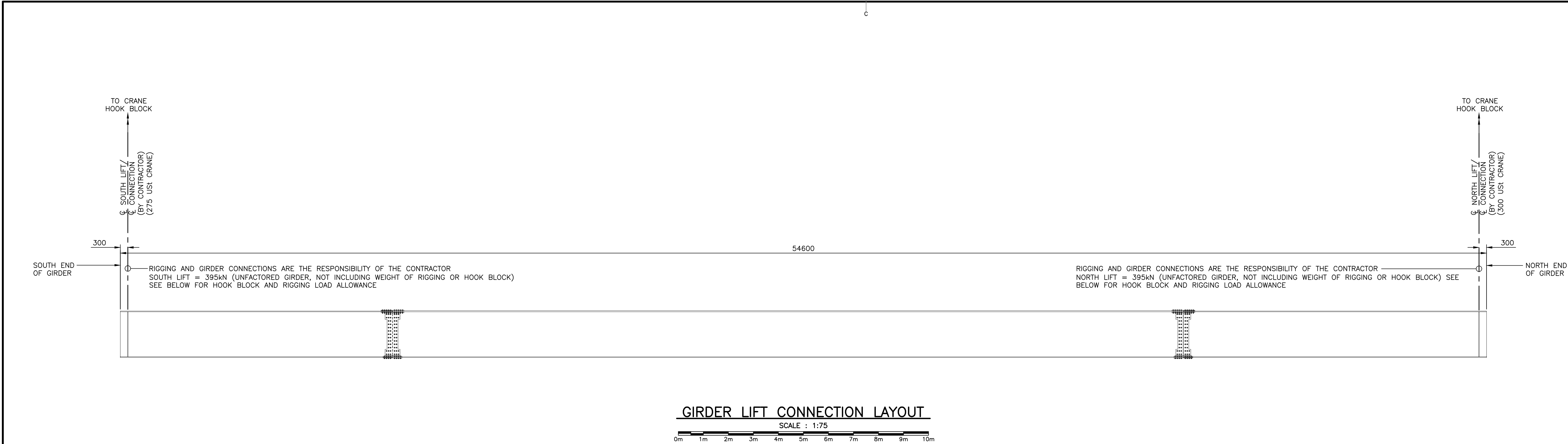
AXLE/TIRE DIMENSIONS (TYP.)



ASSUMED WEIGHT (ALL LOADS UNFACTORED)	
TOTAL GIRDER SELF-WEIGHT	790 kN
TRANSPORT TRUCK	118 kN
JEEP	67 kN
REAR TRANSPORT DOLLY	134 kN
TOTAL	1109 kN

NOTES:

- REFER TO DRAWING EP3 FOR GIRDER ERECTION GENERAL NOTES.
- GIRDER TRANSPORT AXLE LOADS INCLUDE THE GIRDER SELF-WEIGHT AND THE SELF-WEIGHT OF THE DOLLY / JEEP / TRUCK CONFIGURED AS SHOWN ON THE DRAWINGS. THE CONTRACTOR MAY OPT FOR AN ALTERNATE TRANSPORTER CONFIGURATION, HOWEVER, THE LOAD CONFIGURATION SHALL NOT INDUCE LOAD EFFECTS (BOTH LOCALLY AND GLOBALLY) ON THE EXISTING STRUCTURE THAT ARE MORE SEVERE THAN THOSE INDUCED BY THE LOAD CONFIGURATION ILLUSTRATED ON THE DRAWINGS. REFER TO PROJECT SPECIFICATIONS FOR TRANSPORTER ALTERNATE SUBMITTAL REQUIREMENTS.
- THE CONTRACTOR SHALL SUBMIT THE FOLLOWING INFORMATION RELATING TO THE TRANSPORTER TO THE DEPARTMENTAL REPRESENTATIVE FOR APPROVAL:
 - THE AXLE SPACING OF THE DOLLY, JEEP, AND TRUCK AND GEOMETRY OF THE GIRDER SUPPORT LOCATIONS ON THE TRANSPORTER.
 - THE ACTUAL SELF WEIGHT AXLE LOADS FOR THE DOLLY, JEEP, AND TRUCK. THE SELF WEIGHT AXLE LOADS SHALL BE DETERMINED BY A WEIGH SCALE.
 - THEORETICAL AXLE LOADS WITH THE GIRDER LOADED ON THE TRANSPORTER.
- THE DEPARTMENTAL REPRESENTATIVE SHALL INSPECT THE PLACEMENT OF THE GIRDER ON THE TRANSPORTER FOR CONFORMANCE WITH THE CONTRACT DRAWINGS.
- THE STABILITY OF THE FULLY ASSEMBLED GIRDER TRANSPORT IS THE RESPONSIBILITY OF THE CONTRACTOR.



GIRDER LIFT CONNECTION LAYOUT
SCALE : 1:75

GROVE GMK-5250L 300 USL MOBILE CRANE	
OUTRIGGERS FULLY EXTENDED, FULL COUNTERWEIGHT	176,300 lbs
MAXIMUM RADIUS	46' (14021mm)
MAXIMUM LIFT	94,210 lbs (420 kN) TOTAL WHICH INCLUDES 5,640 lbs (25.1 kN) ALLOWANCE FOR HOOK BLOCK AND RIGGING
BOOM LENGTH	77.4' (23591mm)
BOOM CONFIGURATION	56-53-0-0-0-0
CRANE CHART CAPACITY	106,000 lbs (473 kN) WITH 30 KM/HR WINDS
% UTILIZATION	88% OF CRANE CHART VALUES WITH 30 KM/HR WINDS
MAX. OUTRIGGER LOAD	212,200 lbs (946 kN)
GROVE GMK-5275 275 USL MOBILE CRANE	
OUTRIGGERS FULLY EXTENDED, FULL COUNTERWEIGHT	169,700 lbs
MAXIMUM RADIUS	46' (14021mm)
MAXIMUM LIFT	94,210 lbs (420 kN) TOTAL WHICH INCLUDES 5,640 Lbs (25.1 kN) ALLOWANCE FOR HOOK BLOCK AND RIGGING
BOOM LENGTH	104.2' (31760mm)
BOOM CONFIGURATION	50-50-50-50-0-0
CRANE CHART CAPACITY	103,800 lbs (463 kN) WITH 30 KM/HR WINDS
% UTILIZATION	91% OF CRANE CHART VALUES WITH 30 KM/HR WINDS
MAX. OUTRIGGER LOAD	190,847 lbs (851 kN)

NOTE: REFER TO ERECTION PHASES 1 THRU 4 ON DRAWINGS EP1 TO EP2 FOR APPLICABLE CRANE LIFT INFORMATION LISTED ABOVE.

ERECTION PHASING CRANE LIFT INFORMATION

NOTES:

1. REFER TO DRAWING EP3 FOR GENERAL GIRDER ERECTION NOTES.
2. RIGGING AND GIRDER CONNECTIONS ARE THE RESPONSIBILITY OF THE CONTRACTOR, INCLUDING ALL STRENGTH AND STABILITY CHECKS OF THE GIRDER AS A RESULT OF THE ANTICIPATED LOADING AND CONNECTION DETAILS, AND ALL GEOMETRIC CHECKS TO ENSURE NON CONFLICTS WITH INTERNAL GIRDER STIFFENING OR BRACING.
3. CRANE OPERATOR IS RESPONSIBLE FOR CONFIRMING THE CRANE CAPACITIES PRIOR TO ERECTION BASED ON THE LIFT LOADS AND RADII IDENTIFIED ON DRAWING EP5.
4. CRANE OPERATOR SHALL ENSURE CRANE CAPACITIES ARE NOT EXCEEDED AT ANY TIME DURING GIRDER ERECTION.
5. CRANE OPERATOR SHALL ENSURE MAXIMUM OUTRIGGER LOADS ARE NOT EXCEEDED AT ANY POINT DURING THE GIRDER ERECTION.
6. ALTERNATE CRANE SIZE PROPOSALS SHALL BE SUBMITTED TO THE DEPARTMENTAL REPRESENTATIVE FOR APPROVAL PRIOR TO GIRDER ERECTION. REFER TO PROJECT SPECIFICATIONS FOR PROPOSED ALTERNATES AND SCHEDULE FOR SUBMITTALS. THIS SUBMITTAL SHALL PROVE STABILITY OF SLOPES, AVOIDANCE OF SURCHARGING OF ABUTMENTS AND WINGWALLS, CRANE CAPACITY REQUIREMENTS AND CONFIRMATION THAT ALTERNATE CRANES SATISFY ALL SITE GEOMETRIC CONSTRAINTS.

0	ISSUED FOR TENDER	JULY 19 2021
revisions		date
project	WESTERN BROOK BRIDGE REPLACEMENT	
	GROS MORNE NATIONAL PARK	

drawing	design
GIRDER ERECTION	
GIRDER LIFT CONNECTION LOCATIONS AND CRANE LIFT INFORMATION	

designed	THOMAS McNUTT	conçu
date	JANUARY 2021	
drawn	MIKE HARDY	dessiné
date	JANUARY 2021	
approved	ROBBIE FRASER	approuvé
date	JANUARY 2021	
Tender	Soumission	
PWOSC Project Manager	Administrateur de projets TP50C	
project number	no. du projet	
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drawing no.	no. du dessin	
	EP5	

0	ISSUED FOR TENDER	JULY 19, 2021
revisions		date

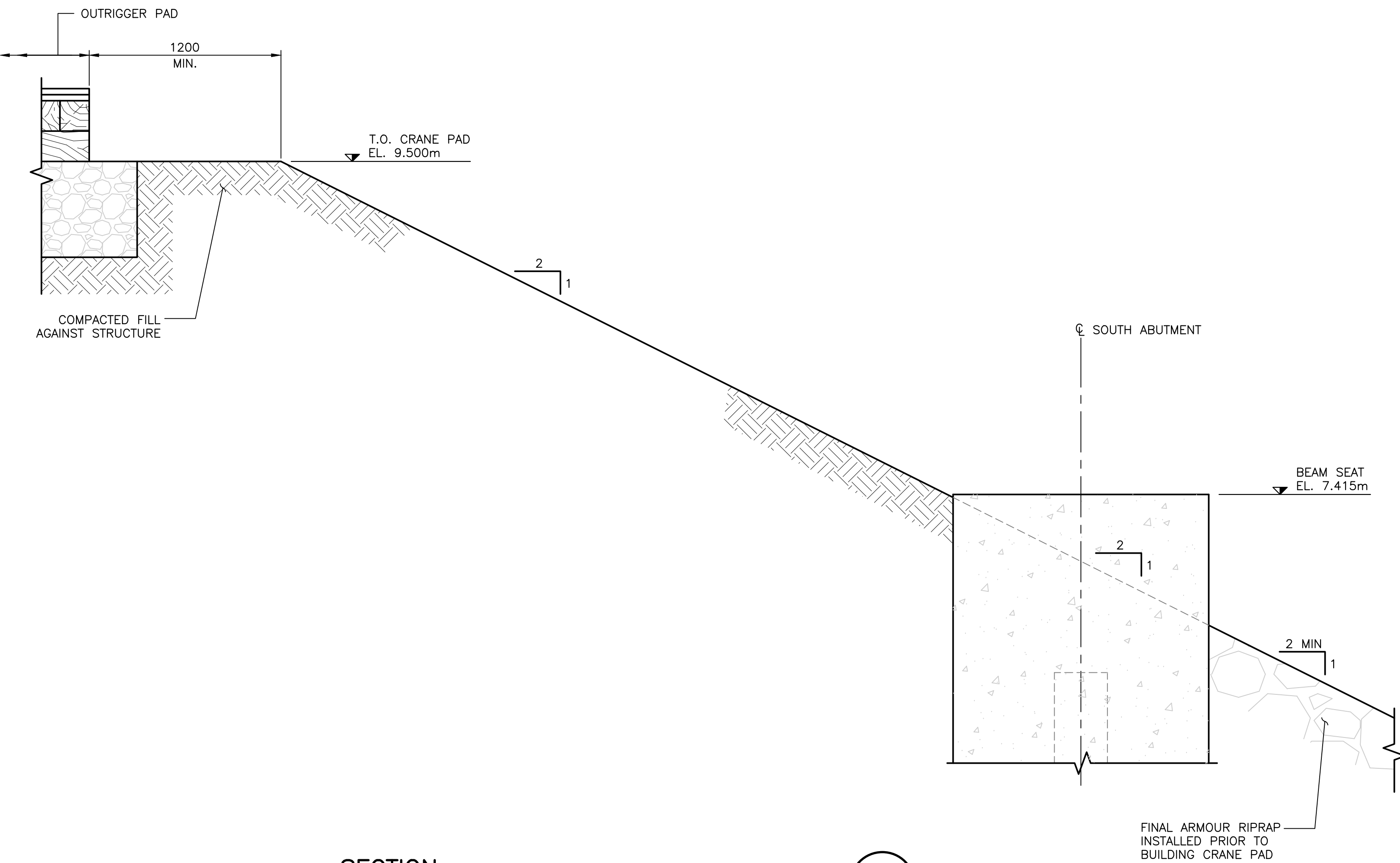
project WESTERN BROOK
BRIDGE REPLACEMENT
GROS MORNE
NATIONAL PARK

drawing design
GIRDER ERECTION
CRANE MATS

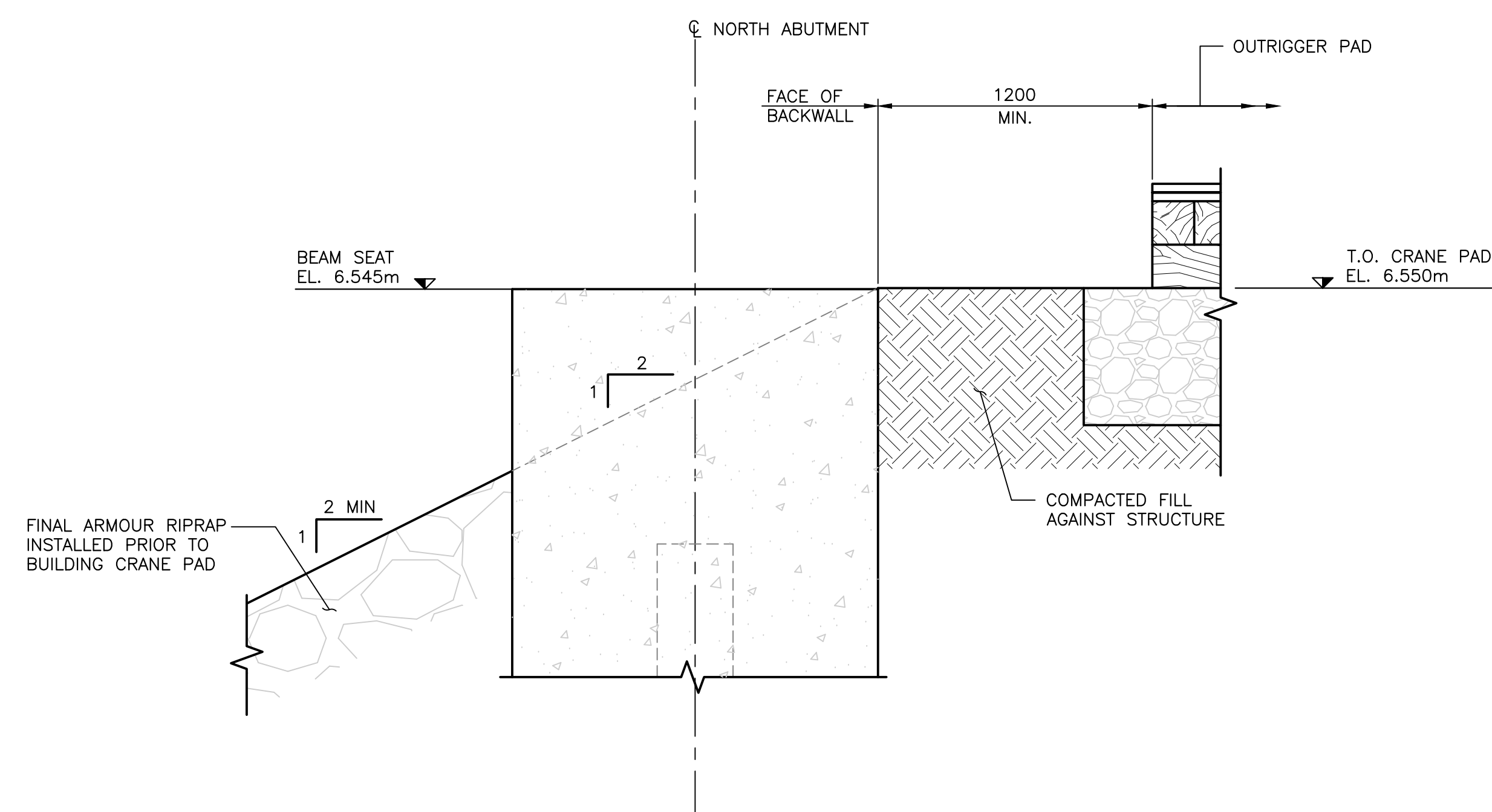
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date	JANUARY 2021	
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Tender	Soumission
PWGS Project Manager	Administrateur de projets TP50C
project number	no. du projet

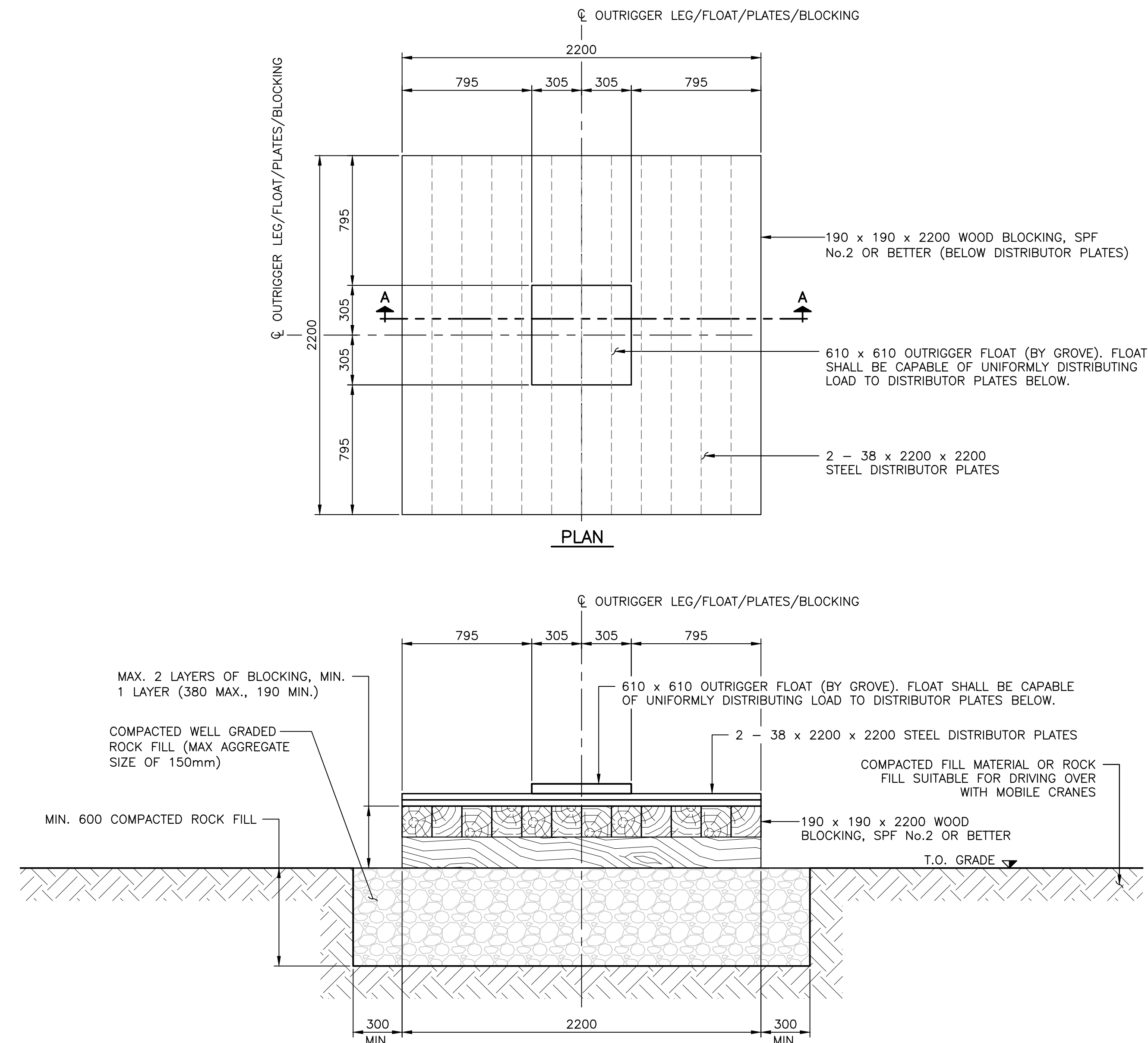
drawing no. EP6
no. du dessin



SECTION - SOUTH OUTRIGGER PAD
SCALE : 1:20
0mm 500mm 1000mm 1500mm 2000mm 2500mm
A
EP1



SECTION - NORTH OUTRIGGER PAD
SCALE : 1:20
0mm 500mm 1000mm 1500mm 2000mm 2500mm
B
EP1



DETAIL - MOBILE CRANE OUTRIGGER PADS/BLOCKING
SCALE : 1:20
0mm 500mm 1000mm 1500mm 2000mm 2500mm
1
EP1

CRANE MAT NOTES:

- REFER TO DRAWING EP3 FOR GENERAL NOTES.
- STABLE SLOPES, OUTRIGGER PADS, GRANULAR MATERIAL THICKNESS REQUIREMENTS, GRANULAR MATERIAL SELECTION AND COMPACTION REQUIREMENTS HAVE BEEN DEVELOPED BASED ON THE MAXIMUM ANTICIPATED OUTRIGGER LOADS RESULTING FROM THE GIRDER LIFTS AS INDICATED ON DRAWING EP5.
- THE GEOTECHNICAL REQUIREMENTS IDENTIFIED WITHIN THE EP SERIES DRAWINGS ARE AS SPECIFIED BY HARBOURSIDE GEOTECHNICAL CONSULTANTS IN THE LETTER DATED APRIL 15, 2021.
- CRANE SUPPLIER IS RESPONSIBLE FOR ENSURING THE MAXIMUM OUTRIGGER LOADS SPECIFIED ON DRAWINGS EP5 ARE NOT EXCEEDED DURING CONSTRUCTION.
- THE CONTRACTOR IS RESPONSIBLE FOR THE SUPPLY AND INSTALLATION OF THE 610X610 OUTRIGGER 'FLOATS' AS INDICATED FOR THE MOBILE CRANES. THE CONTRACTOR SHALL ENSURE THE 'FLOATS' HAVE BEEN DESIGNED TO RESIST AND TRANSFER OUTRIGGER LOADS TO THE BEARING PLATE BELOW.
- 38 mm THICK STEEL BEARING PLATES TO BE GRADE 300W OR BETTER.
- TIMBER MATS TO BE SPF GRADE No. 2 OR BETTER.
- ALTERNATE CRANE MAT PROPOSALS BY CONTRACTOR SHALL BE SUBMITTED TO ENGINEER FOR WRITTEN APPROVAL A MINIMUM OF 7 DAYS PRIOR TO GIRDER ERECTION.
- EDGE OF OUTRIGGER MAT SHALL BE NO CLOSER THAN 1200mm FROM NEAREST TOP OF SLOPE, TYP. ALL CRANE MATS.